STORMWATER FACILITIES MANAGEMENT REPORT FOR



Store # 10075

NEC Central and Oliver

Wichita, KS

Owner:

Missouri CVS Pharmacy, L.L.C.

A Limited Liability Company

Contact Person: Richard Smart

1165 North Clark Street

Chicago, IL 60610

Continuing Authority:

Missouri CVS Pharmacy, L.L.C.

A Missouri Limited Liability Company

Prepared By:



308 TCW Court

Lake St. Louis, MO 63367





City of Wichita/Sedgwick County Subdivision Drainage Plan Checklist



Submit completed forms to City of Wichita Public Works & Utilities, 455 N. Main 8th Floor, Wichita KS 67202; or Sedgwick County Stormwater Management, 1144 S. Seneca, Wichita KS 67213

Project Name:	CVS PHARMACY	- CENTRAL & (DLÍVER				
Total Area of Project:	19	acres					
Development Type:	Commercial	Bir Hi	Other:				
Developer Name:	VELMEIR COMPA	NIES	_Contact:	JIM COLLIER	Phone.	248-794-9768	_
Email:	jcollier@velmeir.co	om		*			
Engineer Name:	PREMIER CIVIL E	NGINEERING	Contact:	MATT FOGARTY	Phone:	314-925-7452	
Email.	mfogarty@pcestl.c	om					

Directions.

- (1) Fill-out this checklist completely and include it with the Dramage Plan submittal. This checklist should be included in the bound copy, behind the cover sheet for the submittal Incomplete Oralinage Plans and checklists will not be accepted.
- (2) Indicate whether a plan element is included or not included in the submittal by choosing "Yes" or "No" from the dropdown list in the "Element Included?" column. The quastion must be answered for every plan element for this checklist to be considered complete. An explanation must be provided for all "No" answers.

Drain	age Plan Checklist		
Diaii	age Fian Greekiist		
#	Plan Element Description	Element Included?	Explanation/Notes
1.0	General		
1.1	Digital copy of drainage plan, including preliminary Master Grading Plan, preliminary plat and proposed plat, in PDF format and one half size, bound, paper copy.	Yes	
1.2	Professional Engineer's seal, signature and date on plan cover.	Yes	
1.3	Site location map, using color ortho-imagery and showing the project boundaries, a north arrow and an accurate scale.	Yes	
1.4	Narrative of the development type, existing conditions and proposed impacts on stormwater runoff, wetlands, riparian zones and floodplains/floodways.	Yes	
1.5	Discussion of off-site conditions surrounding the proposed development.	Yes	
1.6	Summary table of runoff calculations (pre/post development).	Yes	
1.7	Narrative description of the type and function of the permanent structural stormwater management facilities.	Yes	
	Existing Conditions Information		
2.1	Existing Conditions Drainage Map		
211	On-site and off-site topography: NAVD 88 datum, one-foot contours with spot elevations	Yes	SHOWN ON PLAN
21.2	On-site and off-site drainage features, including perennial and intermittent streams (with names labeled), conveyance systems such as open channels, ditches, swales and areas of overland flow. Flow direction must be indicated by arrows.	Yes	SHOWN ON PLAN
213	Storm sewer system components, including storm drains, inlets, catch basins, guiters, manholes, headwalls, pipes and culverts. Material and size must be noted for all pipes and culverts.	Yes	SHOWN ON PLAN
214	Location and boundaries of natural features such as wellands, lakes, ponds with the normal water elevation noted, rock outcroppings, weeded areas and tree rows	Yes	SHOWN ON PLAN
215	Location, dimensions and elevations of existing bridges and culvert crossings.	Yes	SHOWN ON FLAN
216	Location of existing utilities (e.g., water, sewer, gas, electric, cable, etc.) with labels and easement boundaries.	Yes	SHOWN ON PLAN
2.1.7	Groundwater elevations, if applicable.	Yes	SHOWN ON PLAN
218	Delineation of predominant soil based on USDA soil surveys and/or on-site soil borings, indicate NRCS soil name and Hydrologic Soil Group for undisturbed surface soils	Yes	SHOWN ON PLAN
2.1.9	Land use types per NRCS nomenclature	Yes	SHOWN ON PLAN
2.1.10	Footprint of existing impervious areas (labeled, area given in acres).	Yes	SHOWN ON PLAN
2.1.11	internal drainage subbasin boundaries used for hydrologic calculations (labeled with ID, total area in acres, impervious area in acres and curve number).	Yes	
2.1.12	Time of concentration flow paths. Indicate and label each segment separately (i.e., overland flow, shallow concentrated, channel1, channel2, etc.). For each segment, provide the appropriate data to calculate Tc (e.g., length, slope, cover type, paved/unpaved, roughness parameters, geometric properties, etc.).	Yes	SHOWN ON PLAN

Drain	age Plan Checklist	10	
		Element	
#	Plan Element Description	Included?	Explanation/Notes
2.2	Existing Conditions Hydrology and Hydraulics Analysis Narrative of the hydrologic analysis methodology used (e.g.,		SEE COORESPONDING SECTION IN REPORT
2.2 1	unit hydrograph or other approved methods).	Yes	
11130415-3525	A summary table of drainage subbasin hydrologic parameters	Yes	
2.2.2	(subbasin ID, area in acres, curve number, Tc, etc.)	Y 98	
2.2 3	Table of existing condition runoff curve numbers with	Yes	SEE COORESPONDING SECTION IN REPORT
2.2.3	supporting data and calculations. Table of existing condition times of concentration with		SEE COORESPONDING SECTION IN REPORT
2.2.4	supporting data and calculations.	Yes	SEE COORESPONDING SECTION IN REPORT
2.2.5	A summary table of rainfall data used in the hydrologic	Yes	SEE COORESPONDING SECTION IN REPORT
	analysis, and a reference for the source of the data. Cross-sections and other diagrams of existing open channels,		Not applicable to this repert
2.2 6	bridge and culvert sections and other hydraulic features as	No	Company of the same of the sam
2.2 4	required to illustrate the basis for hydraulic analysis	140	
	-ydrologic and hydraulic analyses for runoff rates, volumes.		SEE COORESPONDING SECTION IN REPORT
	velocities and elevations. Provide supporting data not		
2.2 7	specified above and identify assumptions include detailed calculations for the 2, 5, 10, 25 & 100-year, 24-hour storm	Yes	
	events. Provide results in a labular form. Provide digital		
	copies of any computer files and models used		
3.0	postdevelopment Conditions Information		
3.1	postdevelopment Conditions Drainage Map Proposed project boundary	1/-	SHOWN ON PLAN
3 1.1		Yes	
3.1.2	on-site and off-site topography: NAVD 88 datum, one-foot contours with soot elevations.	Yeş	SHOWN ON PLAN
	Existing on-site and off-site drainage features that are to		SHOWN ON PLAN
	remain after development, including perennial and intermittent streams (with names [abeled), conveyance systems such as		
3.1.3	open channels, ditches, swales and areas of overland flow.	Yes	
	Flow direction must be indicated by arrows		
	Location and description of off-site through-drainage	~~~	SHOWN ON PLAN
3.1.4	conveyances which are confined to an easement, dedication	Yes	Constitution of the Consti
	and/or reserve Footprint of proposed impervious areas, including roads,		SHOWN ON PLAN
3.1.5	parking lots, buildings and other structures.	Yes	L CONTROL CONT
3.1.6	Lipication of proposed utilities (e.g., water, sewer, gas, electric,	Yes	SHOWN ON PLAN
	caple, etc.) with labels and easement boundaries. [Delineation of predominant solls, based on antidipated soll		SHOWN ON PLAN
3.1.7	textures and NRCS guidelines if different from predevelopment	Yes	
J	soil conditions; indicate NRCS soil name and Hydrologic Soil Group for surface soils.	1.00	
318	Land use cover per NRCS nomendature	Yes	SHOWN ON PLAN
	Internal drainage subbasin boundaries used for hydrologic		SHOWN ON PLAN
3.1.9	calculations (labeled with ID, total area in acres, impervious	Yes	
	area in acres and curve number).		SHOWN ON PLAN
3 1.10		Yes	
	Time of concentration flow paths indicate and label each segment separately (i.e., overland flow, shallow concentrated.		SHOWN ON PLAN
3 1.11	channel1, channel2, etc.). For each segment, provide the	Yes	
3 1.11	appropriate data to calculate 1c (e.g., length, siche, cover	162	
	type, paved/unplayed, roughness parameters, geometric properties, etc.).		
3.2	Proposed Conveyances Map	NE I	COLUMN TO A STATE OF THE PARTY
	on-site and off-site drainage features, including perantial and jinterminent streams (with names labeled), proposed		SEE POSTDEVELOPMENT CONDITIONS DRAINAGE MAP
3.2.1	conveyance systems (such as open channels, ditches, swales	Yes	
	and areas of overland flow, including backyard drainage) Flow direction must be indicated by arrows		
	Storm sewer system components, including storm drains,	*********	SHOWN ON PLAN
3.2 2	inless, natichbasins, gutters, manholes, headvalls, pipes and culverts. Material and size must be noted for all pices and	Yes	
	culveris.		
	For any subbasin or drainage area > 40 ocros, show that the	1	SHOWN ON PLAN
3.2.3	stornwater flow is confined to an open channel with required with benches and freeboard, or conformance to applicable	Yes	
	policy and design requirements if partially enclosed.		
	Location(s) of stormwater management facilities and any		Consideration of the Constant
3:2.4	associated drainage easements	Yes	
3.2.5	িনহাতজ্ঞৰ energy dissipaters লাব other channel protection devices.	Yes	SHOWN ON PLAN
324	proving stand dimension(s) of programmed channel bridge and	Yes	SHOWN ON PLAN
321	oulwert crossings	162	
3.2.7	Normal pixol and 100-year pool elevations for pands and lakes.	Yes	
328	Descriptions and rate outfall control structure is for poors	Yes	SHOWN ON PLAN
	Emergency overflow spillways and top of permiclevations for	<u> </u>	SHOWN ON PLAN
3.23	ponds and other valume/peak discharge control facilities	Yes	
	IF loodplains, gends, and stormwater management facilities		SHOWN ON PLAN
321	located in reserves.	Yes	- Control of the cont
3.3	postdavelopment Conditions Hydrology & Hydraulics		

100		Element	
#	Plan Element Description		Explanation/Notes
31	Narrative of the hydrologic analysis methodology used (e o .	Yes	
	unit hydrograph or other approved methods).	162	***************************************
3 2	A summary table of drainage subbasin hydrologic parameters (subbasin ID, area in acres, curve number, Tc, etc.).	Yes	
.3 2	(Sobbasii) io, area iii acies, corve number, 10, etc./.	162	
	Table of postdevelopment condition runoff curve numbers with	A) a	Does not apply to this project due to limited Green Area CN values based on 100% impervious
.33	supporting data and calculations.	No	This would enhance the conservative factor for the site detention,
.3 4	Table of postdevelopment condition times of concentration	Yes	
	with supporting data and calculations. Cross-sections and other diagrams of existing open channels.		Does not apply to this project
.3 5	bridge and culven sections and other hydraulic features as	No	Boes not apply to this project
	Hydrologic and hydraulic analyses for runoff rates, volumes,		Hydroflow Report as part of Project Analysis
	velocities and elevations. Provide supporting data not		
20	specified above and identify assumptions. Include detailed calculations for the 2, 5, 10, 25 & 100-year, 24-hour storm	Yes	
,, 3 0	events. Provide results in a tabular form. Provide digital	163	
	copies of any computer files and models used.		
*****	Deupara am park désaharan panananas /100/ Pulai rasulta		100/ pila dese nat perlu far this preises table distributed again 7.10 press
127	Downstream peak discharge assessment (10% Rule) results and supporting data and calculations. Provide digital copies of	No	10% rule does not apply for this project, total disturbed area is 2.19 acres
	any computer files and models used	110	
3.8	Stage-storage-discharge or other outlet rating curves and	Yes	
	intlow/outflow hydrographs for all ponds.	168	
.3.9	Demonstrate that the pond contours on the master grading plan and the stage-storage-discharge data are consistent for	No	Does not apply for this project based on providing underground detention and porous pavement
J.9	all ponds.	140	
3.10	Demonstrate that all ponds have one foot of freeboard above	Yes	Underground Detention/Porous Pavement
.0.10	the 100-year, 24-hour high water level	168	
	Demonstrate that runoff from the proposed project site is		Refer to section 3.3.6, pursuant to conversations with Tim Davidson, Watershed area A has bee
3.11	discharged in the same manner as prior to development, using level spreaders, energy dissipaters, other devices or grading	Yes	reduced to remove runoff to residential area to the Southwest of the project.
	as required, or identify an appropriate flowage essement.	. 03	
3.4	Stormwater Quantity Control Sizing Hydraulic sizing calculations for all stormwater management		
14,1	controls.	Yes	
	Table(s) listing all stormwater management controls. Present		**************************************
3.4.2	the types, sizes, elevations, flows, velocities and depths for	Yes	
	each control, as applicable. Verify that vefecities are self- cleaning and non-erosive.		
	Typical details (including cross-sections where applicable) for	500	Included details from Wichita Public Works for SB 303 and SB 108. Additional Details will be
3 4 3	outlet structures, embankments, spillways, grade contirol	Yes	provided at the Construction Document Phase
	structures, conveyance channels, etc.		
3,5	Stormwater Quality Management Facilities Table(s) listing all stormwater management facilities. Present		
	the description, % TSS removal value, water quality volume	V	
3.5 1	handled, contributing drainage area in acres and contributing	Yes	
	impervious area in acres.		
7.5.3	Indicate the responsible party for maintenance, as shown in the plat text (i.e., Home Owners Association, Lot Owners	Yes	
3.3.2	Association, property owner, etc.)	162	
353	Water quality volume (total and by facility), with supporting	Yes	
127			
	% TSS removal value (total and by facility) with supporting	Vaa	
3.54	data and calculation. Must be equal to orgreater than 80%.	Yes	
	Channel protection volume with supporting data and		
	TCHANNEL DIGLECTION VOIDING WITH SUPPORTING DATA AND		
355	calculations.	Yes	
	calculations. Water quality volume and channel protection volume orlice	Yes No	Water Cluality being provided by flow. No channel protection is proposed.
3.5.6	calculations. Water quality volume and channel protection volume orlice size calculations.		
3.5.6	calculations. Water quality volume and channel projection volume orlice size calculations. Other calculations required for each stomwater management facility as specified in the Wichita/Sedgwick County.	No	Water Cluslity being provided by flow. No channel protection is proposed. This section is not applicable to this project.
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3.5.6	calculations. Water quality volume and channel protection volume orlice size calculations. Other calculations required for each stormwater management facility as specified in the Wichita/Sedgwick County Stormwater Manual. Typical details (including cross-sections where applicable) for	No	
3.5.6 3.5 7 3.5.8	calculations: Water quality volume and channel profession volume orifice size calculations, Other calculations required for each stomwater management facility as specified in the Wichha/Sedgwick County Stomwater Manual Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stifution prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc.	No No	
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3.5.6 3.5.7 3.5.8 4.0 4.1	calculations: Water quality volume and channel protection volume orifice size calculations, required for each stommoter management facility as specified in the Wichita/Sedgwick County Stommother Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other sittation prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains	No No Yes	
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3.5.6 3.5.7 4.0 4.1	calculations: Water quality volume and channel protection volume orifice size calculations, required for each stammater management facility as specified in the Wichha/Sedgwick County Stammater Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stitution prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location.	No No Yes Yes	
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3.5.6 3.5.7 4.0 4.1 4.2	calculations. Water quality volume and channel projection volume onlice size calculations, required for each stammater management facility as specified in the Wichha/Sedgwick County Stammather Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stifuction prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of postdevelopment regulatory floodplain/floodway limits; limits to be per elevation and scaled location.	No No Yes Yes Yes	This section is not applicable to this project.
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3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3	calculations: Water quality volume and channel protection volume orifice size calculations required for each stomwater management facility as specified in the Wichita/Sedgwick County Stomwater Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other sitiation prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of nearest base flood elevations. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of postatevelopment regulatory floodplain/floodway limits, limits to be per elevation and scaled location, with project limits snown. Floodway data table and discharges	No No Yes Yes Yes Yes Yes	This section is not applicable to this project. Project is not within floodway.
3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3	calculations. Water quality volume and channel projection volume onlice size calculations, required for each stammater management facility as specified in the Wichha/Sedgwick County Stammather Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stifuction prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of postdevelopment regulatory floodplain/floodway limits; limits to be per elevation and scaled location.	No No Yes Yes Yes Yes Yes No	This section is not applicable to this project.
3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3	calculations. Water quality volume and channel projection volume onlice size calculations required for each stammater management facility as specified in the Wichila/Sedgwick County Stammather Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other sitiation prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of pesidevelopment regulatory floodplain/floodway limits, limits to be per elevation and scaled location. Polineation of pesidevelopment regulatory floodplain/floodway limits, limits to be per elevation and scaled location. Floodway data table and discharges Hydrologic and frydraulic study information for local floodplain	No No Yes Yes Yes Yes Yes	This section is not applicable to this project. Project is not within floodway.
3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3	calculations: Water quality volume and channel protection volume orifice size calculations required for each stommater management facility as specified in the Wichita/Sedgwick County Stommwater Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stitation prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of postatevelopment regulatory floodplain/floodway limits, limits to be per elevation and scaled location. Pelineation of postatevelopment regulatory floodplain/floodway limits; limits to be per elevation and scaled location. Floodway data table and discharges Hydrologic and hydraulic study information for local floodplain analysis, unnumbered Zone A elevation determinations and floodplain map revisions or required permits.	No No Yes Yes Yes Yes Yes No	Project is not within floodway. Floodplaim Study submitted to Scott Lindeback previously
3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3 4.4	calculations: Water quality volume and channel projection volume onlice size calculations required for each stammater management facility as specified in the Wichha/Sedgwick County Stammahar Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stifuction prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of nearest base flood elevations. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of posturevelopment regulatory floodplain/floodway limits; limits to be per elevation and scaled location. Floodway data table and discharges Hydrologic and hydraulic study information for local floodplain analysis, unnumbered Zorce A elevation determinations and floodplain map revisions or required permits. Regulatory floodway and floor inequal profile models (10, 20,	No No No Yes Yes Yes Yes Yes Yes Yes No Yes	This section is not applicable to this project. Project is not within floodway.
3.5.6 3.5.7 3.5.8 4.0 4.1 4.2 4.3	calculations: Water quality volume and channel protection volume orifice size calculations required for each stommater management facility as specified in the Wichita/Sedgwick County Stommwater Manual. Typical details (including cross-sections where applicable) for outlet structures, embankments, internal grading, forebays and other stitation prefilters, filtration/infiltration media, vegetation, check dams, operational controls, etc. Floodplains Reference the source of flood profile, floodplain, floodway and stream discharge information. Delineation of predevelopment regulatory floodplain/floodway limits using FEMA's current GIS database; limits to be per elevation and scaled location. Delineation of postatevelopment regulatory floodplain/floodway limits, limits to be per elevation and scaled location. Pelineation of postatevelopment regulatory floodplain/floodway limits; limits to be per elevation and scaled location. Floodway data table and discharges Hydrologic and hydraulic study information for local floodplain analysis, unnumbered Zone A elevation determinations and floodplain map revisions or required permits.	No No Yes Yes Yes Yes Yes No	Project is not within floodway. Floodplaim Study submitted to Scott Lindeback previously

	age Plan Checklist	Element	
#	Plan Element Description	Included?	Explanation/Notes
4.9	Floodplain cut and fill calculations for volume sensitive basins.	No	This section is not applicable to this project.
	Demonstrate that floodway elevations and velocities do not		This section is not applicable to this project.
	increase due to construction in the floodway ("No Rise Certification").	No	
	Federal, State and Local Permits		
5.1	US Army Corps of Engineers regulatory program permits	No	All permits will be obtained throughout the construction document review and approval process.
	(Section 404 permit) Kansas Department of Agriculture - Division of Water		
5,2	Resources Permits (Stream Obstruction, Channel Change, Floodplain Fill, Levee, Water Appropriations, Dam Safety permit, etc.).	No	
5 3	FEMA letters of map change/revision - LOMA, LOMR, LOMR-f. CLOMR, etc.; shall be included and approved when project modifies the limits of the floodplain/floodway.	No	All permits will be obtained throughout the construction document review and approval process
6.0	Half Scale Preliminary Master Grading Plan		
6.1	One set of plans and associated PDF of plans	Yes	
	Professional Engineer's seal, signature and date	Yes	
6.2		·	
83	Title block including subdivision name and phase and dated revision documentation.	Yes	
6.4	Future phases shown but cross-hatched as information only,	Yes	
6.5	Scale, not greater than 1-inch = 60 feet.	Yes	
	North arrow.		
8.8		Yes	
A 7	Index or legend key.	Yes	
6.8	Benchmarks (minimum of 2) used for site control (NAVD 88	Yes	
	vertical datum). Existing contours of entire site with contour interval of one		
R 9	foot.	Yes	
6.10	Proposed contours for channels, ponds, and other permanent stormwater management facilities, with contour interval of one foot.	Yes	
8.11	Spot elevations shown to the nearest tenth of a foot for critical locations, including lot and property boundaries.	Yes	
6.12	Proposed lot and street layout.	Yes	
	Locations of underground storm drains.		11
8 13		Yes	***************************************
6 14	Overflow locations for storms exceeding storm drain capacity, with elevations.	Yes	EA31469-1974-1979-1979
6 15	Top elevations of storm drains at all inlets, mentioles, and flow	Yes	
6.46	line elevations for all outfalls. Locations of open ditches and lakes.	No	Not applicable to project
6 18	Flow direction arrows.	L	
6.17		Yes	
6.18	Proposed flow line elevations of all open ditches at maximum 100 foot intervals, and 100-year flood elevations thereon.	No	Not applicable to project
6.19	Ponds: Location, bottom elevation, normal pool elevation, 100- year flood elevation, emergency overflow elevation.	No	
6.20	Proposed top of curb elevations at points where drainage will	No	No drainage over curb proposed for site
4,20	be required to flow over the surb. Platted minimum building opening elevation for each lot, in	140	
6.21	table form for all lots (excluding basement floor elevations)	Yes	
6.22	Standard foundation and elevation detail for slab on grade, full basement, view-cut, partial view-out and/or vvalk-out construction.	No	
6.23	Top of foundation elevation for each lot.	Yes	
6.24	Notation for builders for each lot as to the type of structure that may be constructed and the view-out, walk-out or pad	No	Does not apply to project
	elevation, as applicable.		
6.25		Yes	
6.26	Indicate that grading around structures conforms to perimeter drainage requirements	Yes	
6.27	Indicate that backyard drainage grading conforms to backyard	No.	Site is a commercial lot
	drainage requirements Adjacent subdivision lot lines, with tot lisbels and subdivision		
6.28	names.	Yes	
6.29	Boundaries and labels for all easements, rights-of-way and reserves.	Yes	
	Statement on proposed final plat: "A drainage plan has been	************	
6 30	developed for the subdivision and all drainage easements, rights-of-way, or reserves shall remain at the established grades and remain unobstructed to allow for the conveyance of starnwater."	Yes	
	pur cour distribute	-	
	1		



May 25, 2012 Scott Lindeback P E. City of Wichita

455 N. Main Wichita, KS 67202

RE. CVS Store#10075-NEC Central and Oliver-Drainage Plan Submittal

Mr Lindeback,

Pursuant to the Subdivision Drainage Plan Checklist for the City of Wichita/Sedgwick County, please let this letter serve notice for the Project Narrative and Discussion of Off-site conditions that are required per the Drainage Plan Checklist. Located below this paragraph is a summary that has been placed into sections per the Drainage Plan Checklist.



Section 1.3 Site Location Map



Site is located on the NEC Central and Oliver



Section 1.4
Project Narrative

The Proposed CVS/Pharmacy will consist of a 13,225 sq. ft. building located on 1.39 acres. The disturbed area for the site will be approximately 1.93 acres. The site is currently undeveloped ground with an existing in-ground pool. The site is Soil Class "B" Urban land-Farnum, 0 to 3 percent slopes pursuant to the USDA soil survey.

Section 1.5
Offsite Conditions

A CVS Pharmacy is proposed at the Northeast corner of the intersection of East Central Avenue and North Oliver Street. Much of the tract of land located at the Northeast corner of East Central Avenue and North Oliver is shown on the Flood Insurance Rate Map Panel 359 of 700 for Sedgwick County Kansas in the Zone AE flood hazard area of the West Branch of Gypsum Creek with a base flood elevation of 1367. A consultant, AMEC, has been hired to complete a new study of the creek for the purpose of preparing new Flood Insurance Rate Maps for Sedowick County. The AMEC study represents the newest and best information on the West Branch of Gypsum Creek. The City of Wichita ordinance requires that the floor elevation of new buildings be set a minimum of two feet above the base flood elevation shown on the map or two feet above the newest and best information if a newer study is available. The purpose of this study is to determine the effects of the proposed grading on the revised base flood elevation which is approximately three feet lower than the base flood elevation shown on the map. A copy of the SWMM hydrologic and hydraulic model for the West Branch Dry Creek of Gypsum Creek in Sedgwick County, KS was obtained from AMEC. This model included the triple box culvert under East Central Avenue (Culvert C5848-0068), the double box culvert under East Ninth Street (Culvert C5848-0112) and an open channel between the culverts. A single cross section defines the channel from Culvert C5848-0068 upstream to East Murdoch Street. To more accurately model the effects of the proposed CVS pharmacy located on the Northeast corner of the intersection of East Central Avenue and North Oliver Street, two cross sections were added to the AMEC model at the location of the upstream and downstream ends of the proposed building. These cross sections show existing topography. The Lidar topographic data obtained from the City of Wichita was used to cut the cross sections in the existing conditions model. Both cross sections were modified to reflect the proposed grading and building in the proposed conditions model. The rest of the AMEC model was left unchanged. A 100-year storm was run through each of the three models to determine the effect of adding the two cross sections and modifying the cross sections to reflect the proposed construction.

Section 1.6
Summary Table of Runoff Calculations (pre/post development)

THE RESERVE OF THE PARTY OF THE	DESIGN STORM FLOWS (cfs)								
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR		
PRE DEVELOPED	11.08	13.92	17.96	20.79	24.42	27.64	31.26		
POST DEVELOPED	7.975	10.02	12.93	14.96	17.57	19.89	22.50		

Section 1.7
Description, Type and Function of Stormwater Management Facilities

The proposed impacts on stormwater runoff will be designed to be minimal per the Wichita Stormwater Manual. This site will consist of three separate watersheds. All three



watersheds will be impacted, however the watershed that is served by the residential properties to the Northeast will be reduced

The second and third watershed will drain to inlets located on the CVS Site and will connect to the existing storm sewers located on Central and Oliver. The watershed to Oliver will contain a Vortsentry HS unit. The Vortsentry HS will allow the collection of 80% of TSS. Pursuant to the Stormwater manual, this site is a redevelopment and 30% of the site must be treated.

Section 2.2.1

Narrative of Hydrologic analysis methodology used

The runoff method used for modeling the site was based on the 24 Hour SCS Hydrograph Method in Hydraflow Hydrographs Due to the limited Time of Concentration, User Input was needed vs. the TR-55 method to determine the peak Q at one minute intervals

Section 2.2.2 Summary Table of Drainage Sub basin Hydrologic Parameters

	AREA 1	AREA 2	AREA 3	AREA 4	AREA 5	AREA 6	AREA 7	AREA 8
AREA IN ACRES	1.78	0.08	0.04	0.25	0.11	0.09	0.16	.13
COEFFICIENT	.011	.011	.011	.011	.011	.011	.011	.011
CURVE NUMBER	98	98	98	98	98	98	98	98
TIME OF CONCENTRATION	3	2*	2*	2*	2*	2*	2*	2*

^{***} Areas are user defined inputs due to Q less than the time of concentration 1 minute interval. ***

Section 2.2.3

Table of Existing Condition Runoff Curve Numbers

Located in Section 2.2.2

Section 2.2.4

Table of Existing Conditions Time of Concentration

Located in Section 2.2.2

Section 2.2.5

Table of Rainfall data used in the Hydrologic Analysis

Located in Hydrographs Section 2.2.7

Section 2.2.7

Hydrologic and Hydraulic analyses for runoff Rates, Volumes Velocities and Elevations.



Section 3.3

Section 3.3.1

The hydrologic analysis methodology used is the SCS Hydrograph based on TR-55 using Autodesk Storm and Sanitary Sewer Analysis 2012.

Section 3.3.2.

Summary Table of Drainage Sub-Basin Hydrologic Parameters

	AREA	AREA	AREA	AREA	AREA	AREA	AREA G	AREA H	AREA I
AREA IN ACRES	0.12	B 0.37	0.12	0.35	0.08	0.25	0.25	0.19	17
CURVE NUMBER	98	80	98	98	98	98	98	98	98
COEFFICIENT	0:011	0.011	0.011	0.011	0.011	0.011	0.011	0.011	0.011
TIME OF CONCENTRATION	2	2	2	2	2	2	2	2	2

Section 3.3.3

Does not apply for project. Due to limited Green Area CN values based on 100% impervious. This would enhance the conservative factor for the site detention.

Section 3.3.4

Table of Post Development Conditions Time of Concentration

See Section 3.3.2

Section 3.3.5

Section does not apply for this project.

Section 3.3.6

Hydrologic and Hydraulic Analyses for Runoff Rates, Volumes, Velocities and Elevations Provided at the Construction Document Phase

Section 3.3 7

10% Rule does not apply for project. Total disturbed area for site is 1.92Acres

Section 3.38

Stage-Storage-Discharge or other Outlet Rating Curves and Inflow/Outflow hydrographs for all ponds.

No detention is provided for this site.

Section 3.3.9

Section does not apply to project.

Section 3.3.10

Refer to Section 3.36

Section 3.3.11

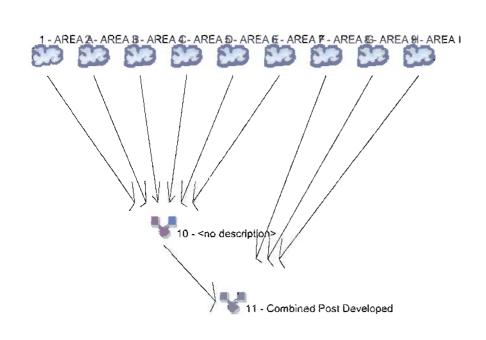


Does not apply to this project.

Section 3.4
Stormwater Quantity Control Sizing
Hydraulic Sizing calculations for all Stormwater Management controls.
Will be provided at the construction drawing phase.

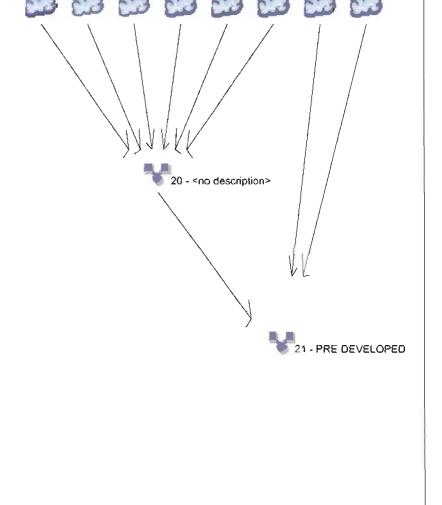
Section 3.4.2 Table listing all stormwater Management Controls.

Watershed Model Schematic



Legend

12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	Hyd.	<u>Origin</u>	Description
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4 SCS Runoff AREA D 5 SCS Runoff AREA E 6 SCS Runoff AREA F 7 SCS Runoff AREA G 8 SCS Runoff AREA H 9 SCS Runoff AREA I 10 Combine <no description=""> 11 Combine Combined Post Developed 12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8</no>	2	SCS Runoff	AREA 8
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6 SCS Runoff AREA F 7 SCS Runoff AREA G 8 SCS Runoff AREA H 9 SCS Runoff AREA I 10 Combine	4	SCS Runoff	AREA D
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8 SCS Runoff AREA H 9 SCS Runoff AREA I 10 Combine <no description=""> 11 Combine Combined Post Developed 12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8</no>	6	SCS Runoff	AREA F
9 SCS Runoff AREA I 10 Combine <no description=""> 11 Combine Combined Post Developed 12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8</no>	7	SCS Runoff	AREA G
10 Combine <no description=""> 11 Combine Combined Post Developed 12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8</no>	8	SCS Runoff	AREA H
11 Combine Combined Post Developed 12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	9	SCS Runoff	AREAI
12 SCS Runoff AREA 1 13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	10	Combine	<no description=""></no>
13 SCS Runoff AREA 2 14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	11	Combine	Combined Post Developed
14 SCS Runoff AREA 3 15 SCS Runoff AREA 4 16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	12	SCS Runoff	AREA 1
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16 SCS Runoff AREA 5 17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	14	SCS Runoff	AREA 3
17 SCS Runoff AREA 6 18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	15	SCS Runoff	AREA 4
18 SCS Runoff AREA 7 19 SCS Runoff AREA 8	16	SCS Runoff	AREA 5
19 SCS Runoff AREA 8	17	SCS Runoff	AREA 6
70 000 71017017 71112710	18	SCS Runoff	AREA 7
20 Cambina and descriptions	19	SCS Runoff	AREA 8
20 Combine 400 description/	20	Combine	<no description=""></no>
21 Combine PRE DEVELOPED	21	Combine	PRE DEVELOPED



12 - AREA3 - AREA2 - AREA3 - AREA3 - AREA3 - AREA3 - AREA3 - AREA3 - AREA3

Project: Hydraflow Central and Oliver 5.24.12.gpw

Tuesday, 00 29, 2012

Hydrograph Return Period Recap

-	Hydrograph	Inflow				Peak Out	tflow (cfs))			Hydrograph
No.	type (orlgin)	hyd(s)	1-yr	2-yr	3-yr	5-уг	10-yr	25 - yr	50-уг	100-yr	Description
1	SCS Runoff		0.504	0 633		0 817	0 945	1.110	1.256	1.421	AREA A
2	SCS Runoff		1.553	1.951		2.518	2.913	3 422	3.873	4.381	AREA B
3	SCS Runoff		0.504	0.633		0.817	0.945	1.110	1.256	1.421	AREA C
4	SCS Runoff		1.469	1.846		2.381	2.756	3.237	3.664	4.144	AREA D
5	SCS Runoff		0.336	0 422		0 544	0.630	0.740	0.837	0.947	AREA E
6	SCS Runoff		1.049	1 318		1.701	1.969	2.312	2.617	2.960	AREA F
7	SCS Runoff		1.049	1 318		1.701	1.969	2.312	2.617	2.960	AREA G
8	SCS Runoff		0 798	1 002		1 293	1.496	1 757	1 989	2.250	AREA H
9	SCS Runoff		0 714	0 896		1 157	1.339	1 572	1 780	2.013	AREAI
10	Combine	1. 2, 3,	5.415	6.802		8 777	10.16	11.93	13.50	15.27	<no description=""></no>
11	Combine	4, 5, 6, 7, 8, 9.	7.975	10.02		12.93	14.96	17.57	19.89	22 50	Combined Post Developed
12	SCS Runoff	10	7.472	9.386		12.11	14 02	16,46	18.63	21.08	AREA 1
13	SCS Runoff		0.336	0.422		0 544	0.630	0.740	0.837	0.947	AREA 2
14	SCS Runoff		0.168	0.211		0 272	0.315	0 370	0 4 1 9	0.474	AREA 3
15	SCS Runoff		1.049	1 318		1 701	1.969	2 312	2.617	2.960	AREA 4
16	SCS Runoff		0.462	0 580		0.748	0.866	1 017	1.152	1.302	AREA 5
17	SCS Runoff		0.378	0.475		0 612	0.709	0.832	0 942	1.066	AREA 6
18	SCS Runoff		0.672	0.844		1 089	1.260	1.480	1.675	1.895	AREA 7
19	SCS Runoff		0.546	0 685		0 885	1 024	1 202	1.361	1.539	AREA 8
20	Cambine	12, 13, 14,	9.864	12.39		15.99	18.50	21 73	24.60	27.83	<no description=""></no>
21	Combine	15, 16, 17, 18, 19, 20	11.08	13.92		17.96	20 79	24 42	27.64	31.26	PRE DEVELOPED

Proj. file: Hydraflow Central and Oliver 5.24.12.gpw

Tuesday, 00 29, 2012

Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

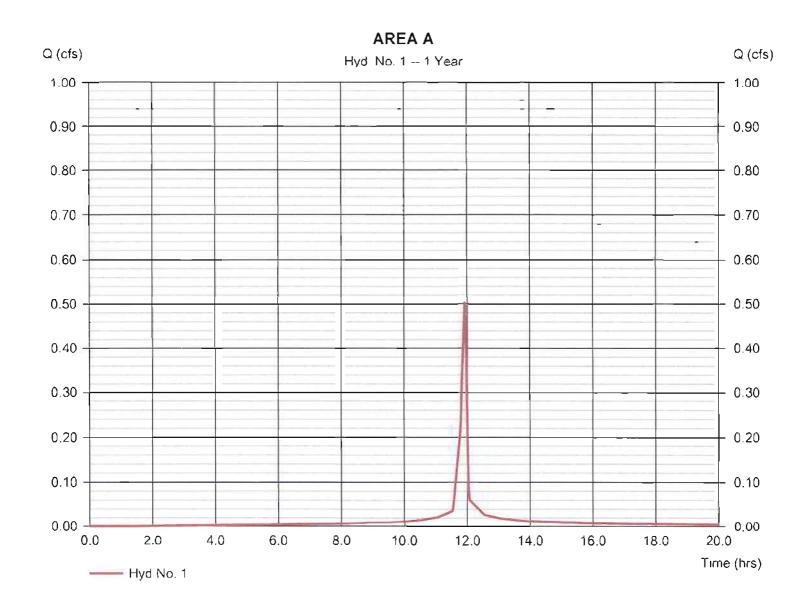
Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	0.504	1	715	0.024				AREA A
2	SCS Runoff	1.553	1	715	0.074				AREA B
3	SÇŞ Runoff	0.504	1	715	0.024				AREA C
4	SCS Runoff	1.469	1	715	0.070				AREA D
5	SCS Runoff	0.336	1	715	0.016				AREA E
6	SCS Runoff	1.049	1	715	0.050				AREA F
7	SCS Runoff	1.049	1	715	0.050			*****	AREA Ç
8	SCS Runoff	0.798	1	715	0 038				AREA H
Ð	SCS Runoff	0 714	1	715	0 034				AREA I
10	Combine	5 415	1	715	0 259	1, 2, 3, 4, 5, 6,			<no description=""></no>
11	Combine	7.975	1	715	0 381	7, 8, 9,			Combined Post Developed
12	SCS Runoff	7.472	1	715	0.357				AREA 1
13	SCS Runoff	0 336	1	715	0.016				AREA 2
14	SCS Runoff	0.168	1	715	0.008				AREA 3
15	SCS Runoff	1.049	1	715	0.050				AREA 4
16	SCS Runoff	0.462	1	715	0 022				AREA 5
17	SCS Runoff	0.378	1	715	0.018				AREA 6
18	SCS Runoff	0 672	1	715	0 032				AREA 7
19	SCS Runoff	0.546	1	715	0.026				AREA 8
20	Combine	9.864	1	715	0.472	12, 13, 14, 15, 16, 17,			<no description=""></no>
21	Combine	11.08	1	715	0.530	18, 19, 20			PRE DEVELOPED
Hva	 draflow Centr	⊥_ al and Oli	ver 5.24	.12.gpw	Return F	Period 1 Ye	ear	Tuesday (00 29, 2012

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge = 0.504 cfsTime to peak Storm frequency = 1 yrs= 11.92 hrsTime interval = 1 minHyd. volume = 0.024 acft= 0.120 acCurve number Drainage area = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 2.00 \, \text{min}$ = User Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

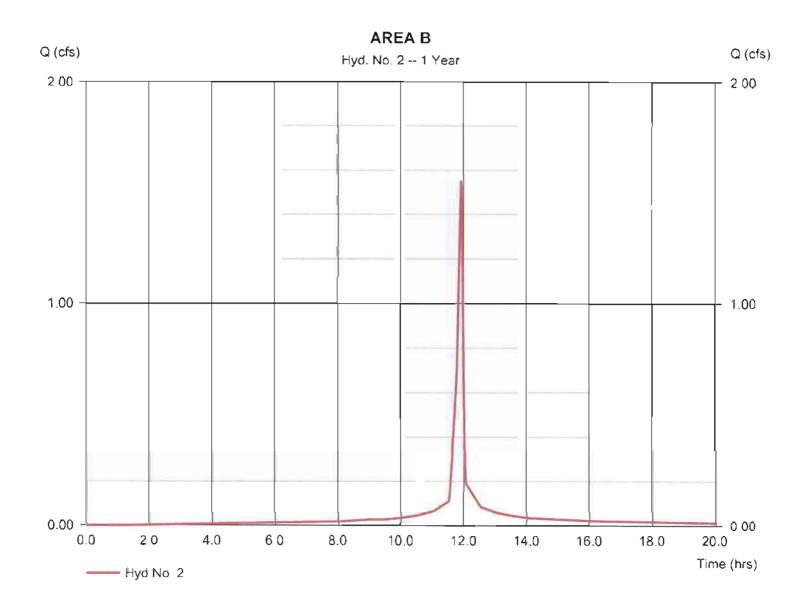


Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = SCS Runoff Peak discharge = 1.553 cfsStorm frequency Time to peak = 1 yrs= 11.92 hrsTime interval = 1 minHyd. volume = 0.074 acftDrainage area = 0.370 acCurve number = 98 Basin Slope = 0.0 %= 0 ftHydraulic length Tc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

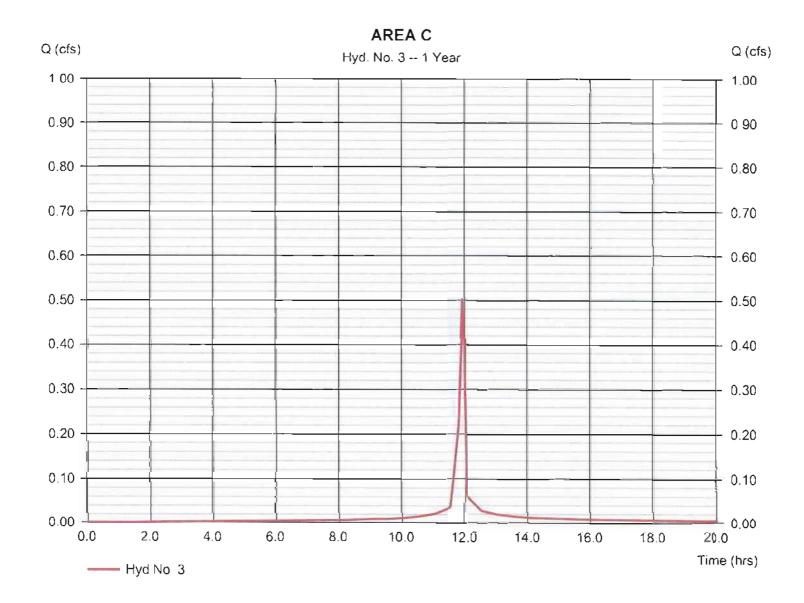


Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 0.504 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.024 acft Drainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



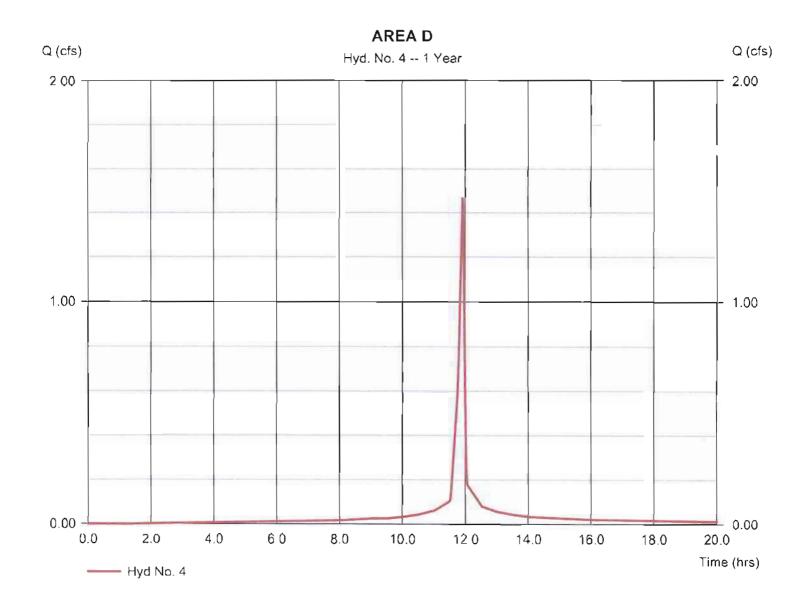
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

Peak discharge = 1.469 cfs= SCS Runoff Hydrograph type Time to peak Storm frequency = 11.92 hrs= 1 yrsTime interval = 1 min Hyd. volume = 0.070 acftDrainage area = 0.350 acCurve number = 98 Hydraulic length = 0.0 %= 0 ftBasin Slope Time of conc. (Tc) Tc method = TR55 $= 1.70 \, \text{min}$ Distribution = Type II Total precip. = 2.80 in= 24 hrs Shape factor = 484 Storm duration

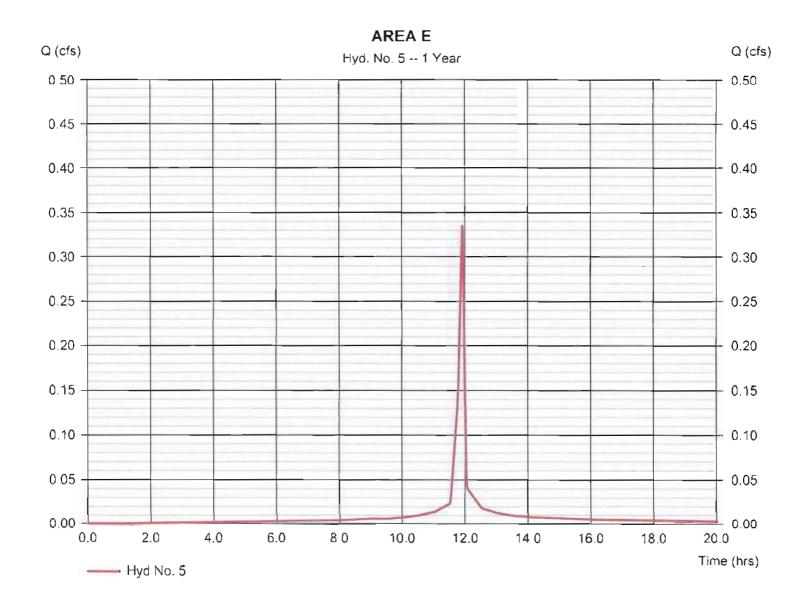


Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.336 cfsStorm frequency = 1 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.016 acft Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



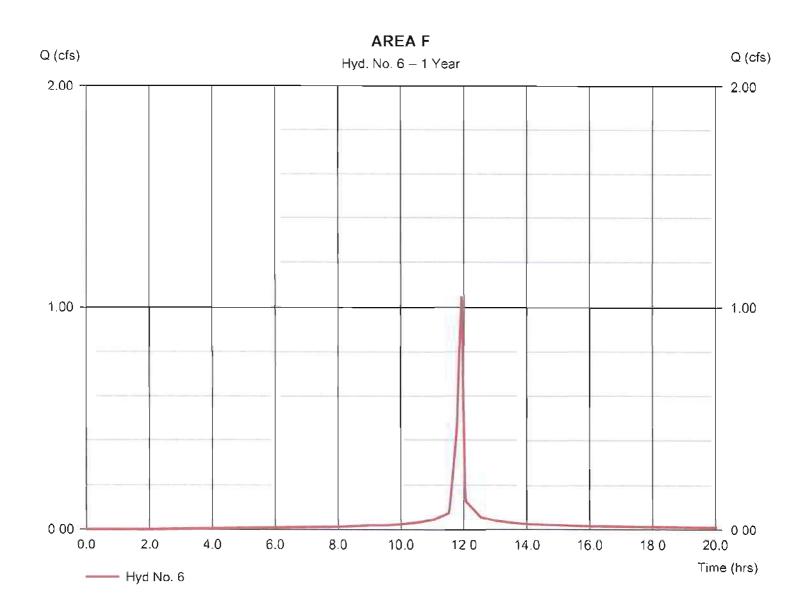
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 1.049 cfsStorm frequency = 1 yrsTime to peak $= 11.92 \, hrs$ Time interval $= 1 \min$ Hyd. volume = 0.050 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



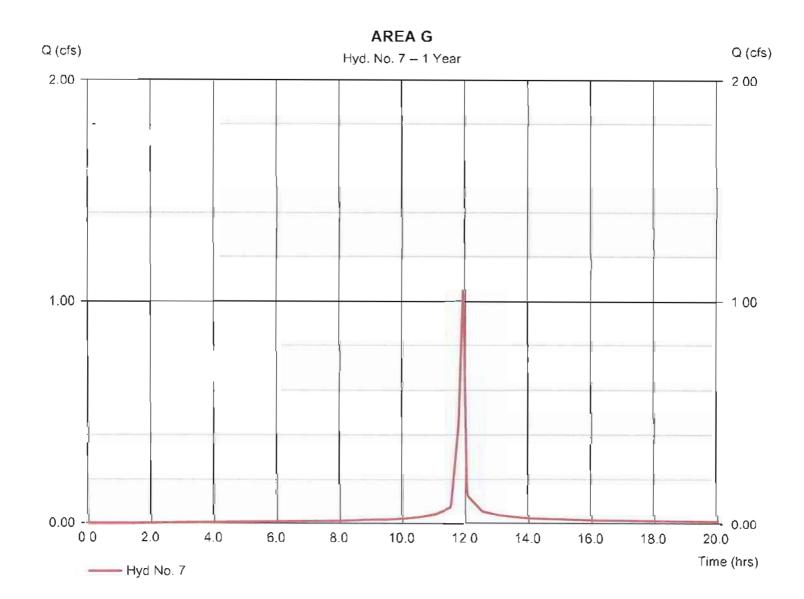
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff = 1.049 cfsPeak discharge Storm frequency = 1 yrs Time to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.050 acft= 0.250 acDrainage area Curve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



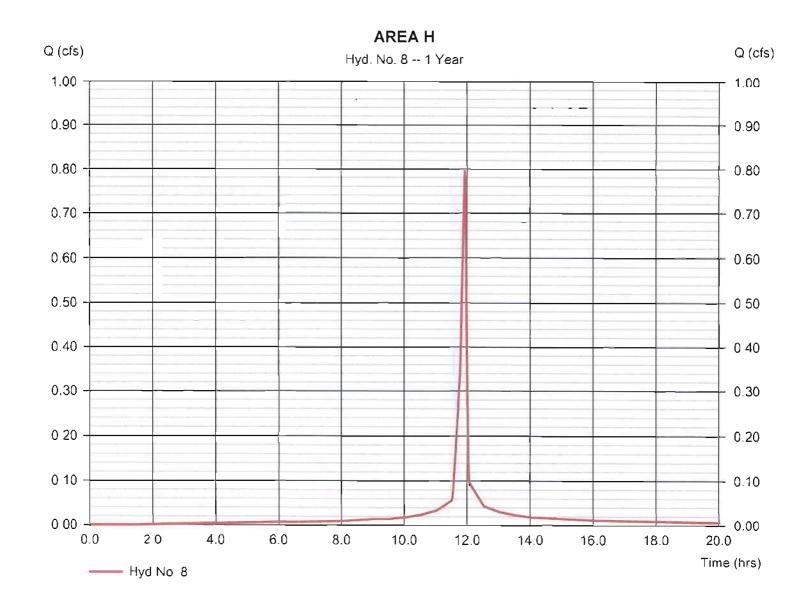
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

= SCS Runoff Hydrograph type Peak discharge = 0.798 cfs= 1 yrs Storm frequency Time to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.038 acft = 0.190 acDrainage area Curve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



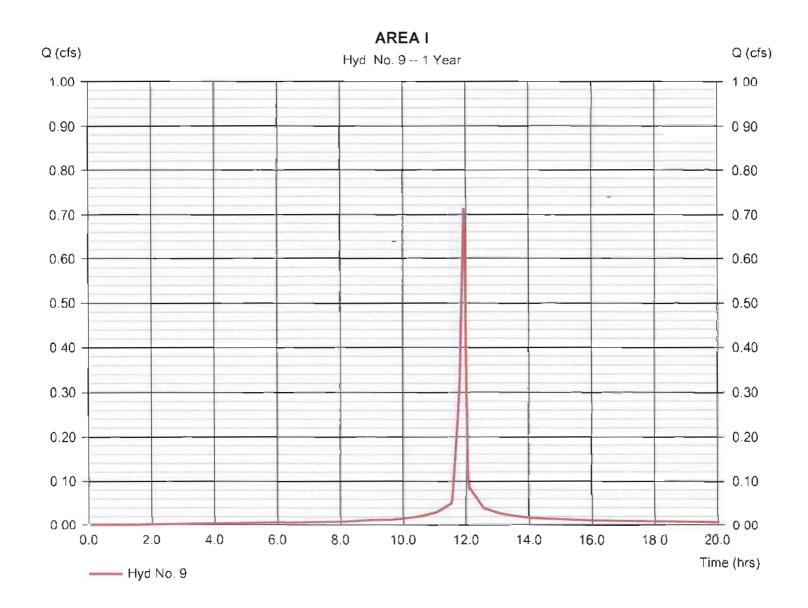
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday. 00 29. 2012

Hyd. No. 9

AREA I

Hydrograph type = SCS Runoff Peak discharge = 0.714 cfsStorm frequency Time to peak $= 11.92 \, hrs$ = 1 yrsTime interval = 1 min Hyd. volume = 0.034 acftDrainage area = 0.170 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 2.00 \, \text{min}$ = User Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

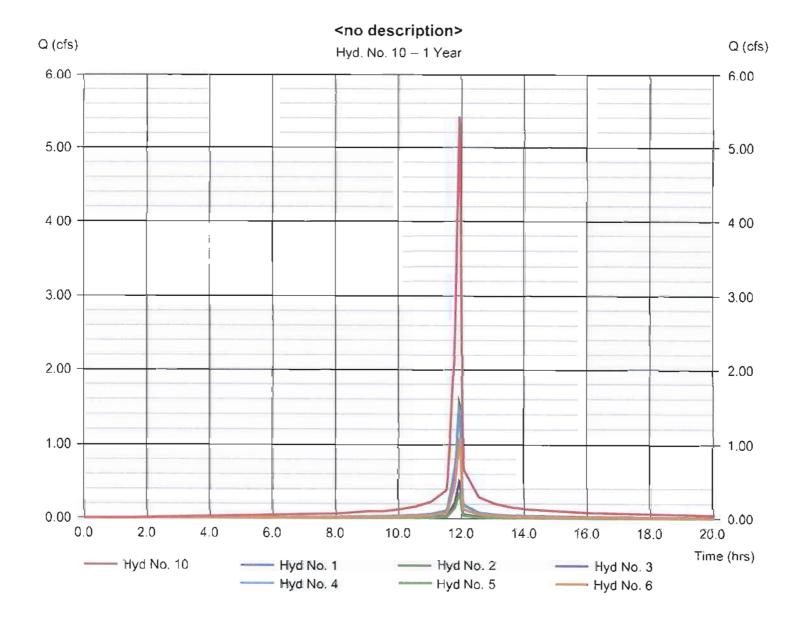
Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

Hydrograph type = Combine
Storm frequency = 1 yrs
Time interval = 1 min
Inflow hyds. = 1, 2, 3, 4, 5, 6

Peak discharge = 5.415 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.259 acft
Contrib. drain. area = 1.290 ac



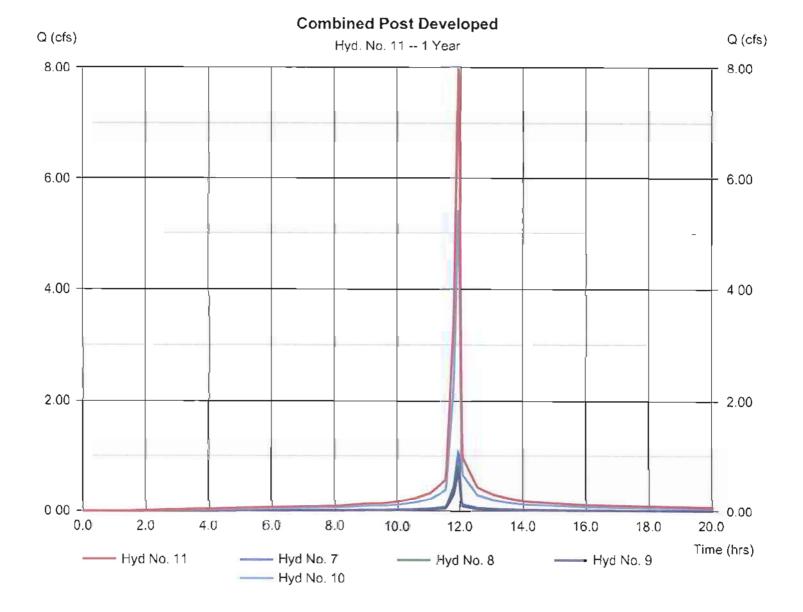
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine Storm frequency = 1 yrs Time interval = 1 min Inflow hyds. = 7, 8, 9, 10 Peak discharge = 7.975 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.381 acft
Contrib. drain. area = 0.610 ac



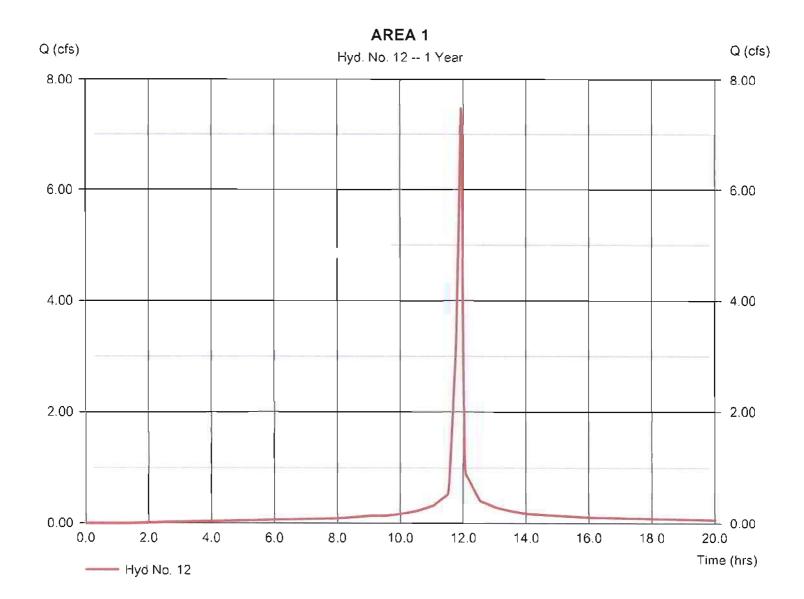
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 7.472 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.357 acft = 1.780 acDrainage area Curve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = TR55 $= 3.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration $= 24 \, hrs$ Shape factor = 484



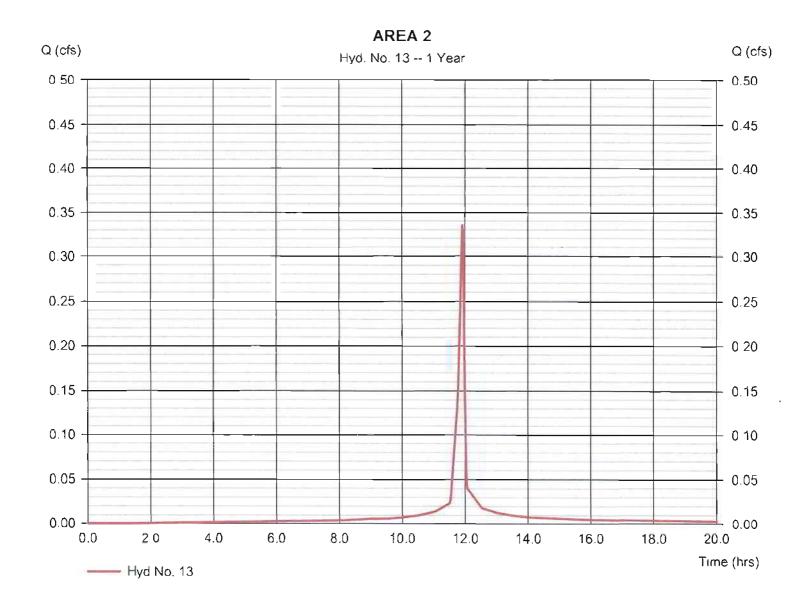
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge = 0.336 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval Hyd. volume $= 1 \, \text{min}$ = 0.016 acft Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



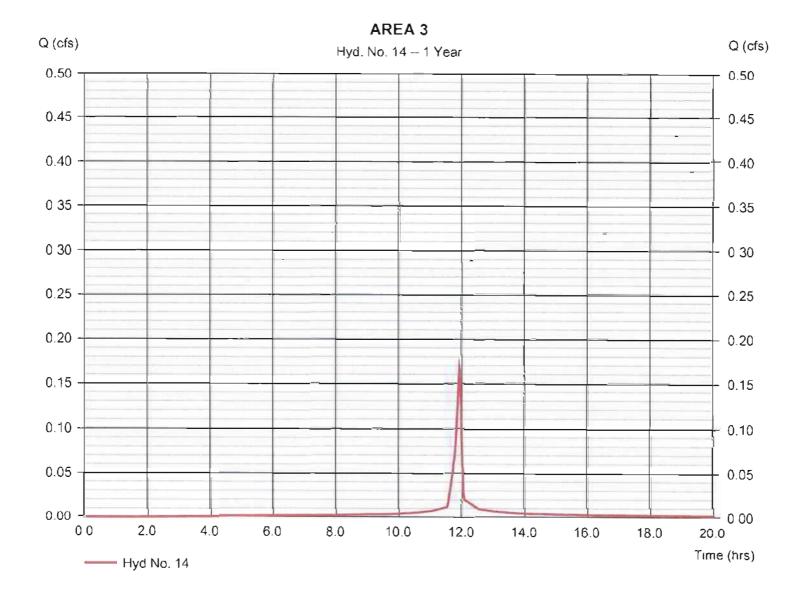
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Hydrograph type = SCS Runoff Peak discharge = 0.168 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.008 acft Drainage area = 0.040 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



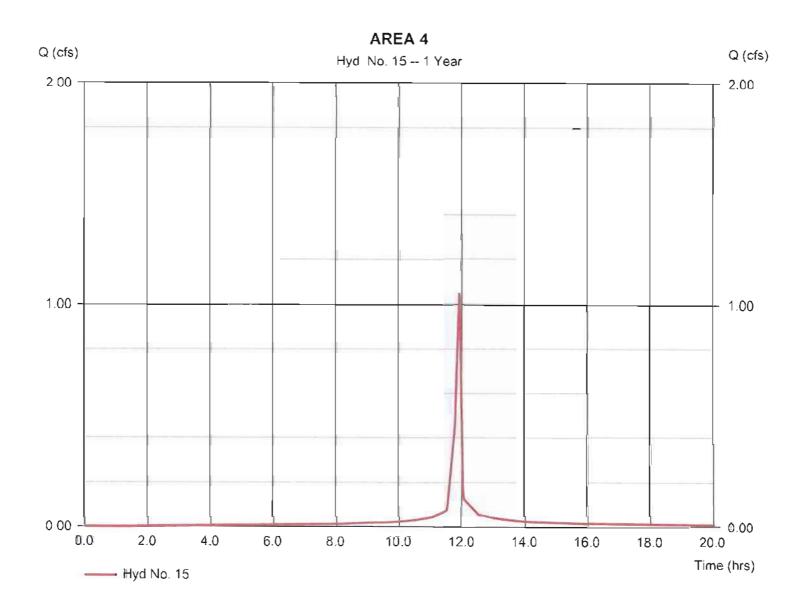
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Peak discharge Hydrograph type = SCS Runoff = 1.049 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.050 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

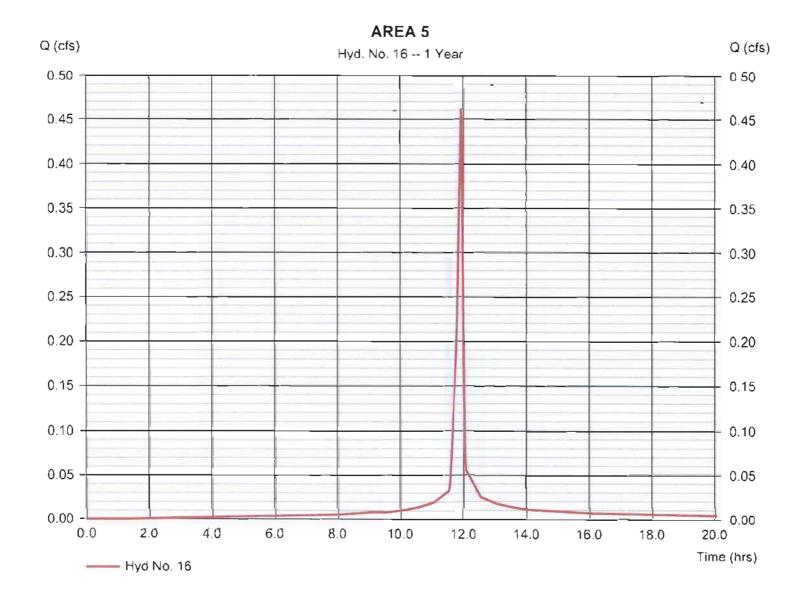


Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

Hydrograph type = SCS Runoff Peak discharge = 0.462 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.022 acft Drainage area Curve number = 0.110 ac= 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

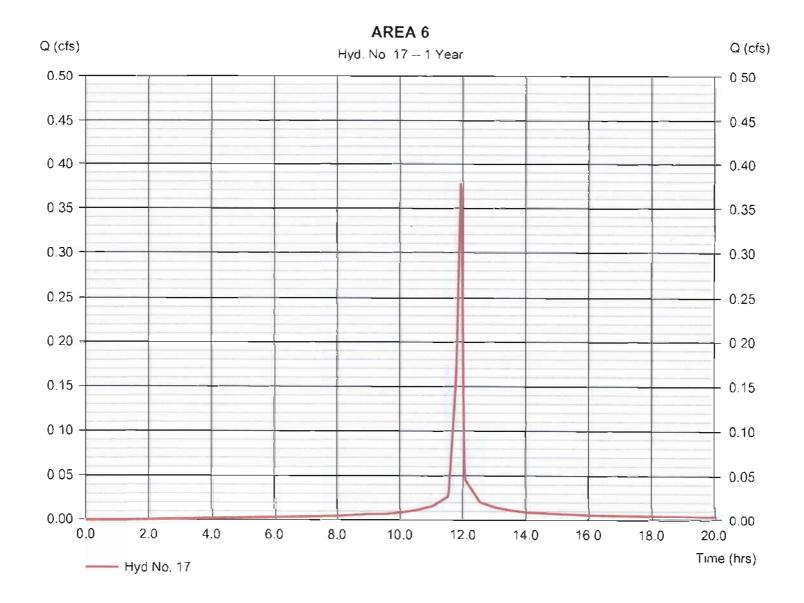


Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 0.378 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrs Time interval = 1 min Hyd. volume = 0.018 acftDrainage area = 0.090 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration Shape factor = 24 hrs = 484



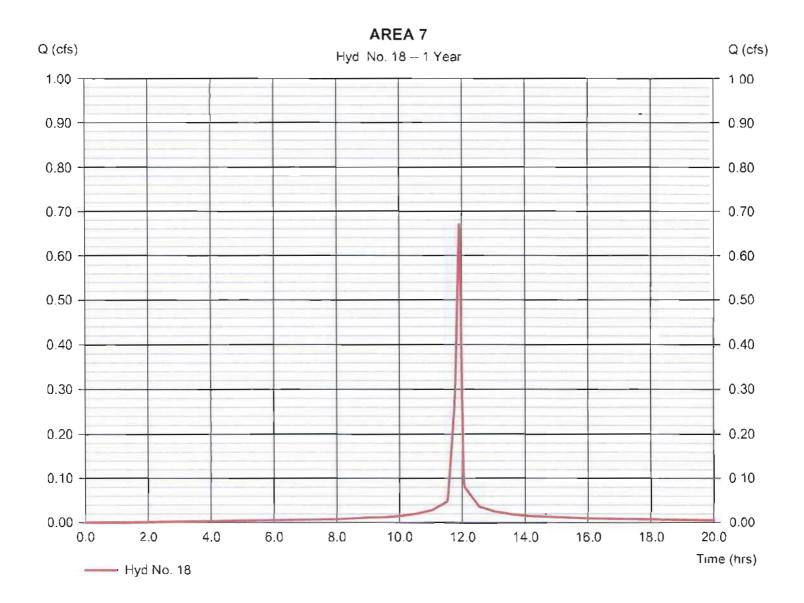
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff Peak discharge = 0.672 cfsStorm frequency Time to peak = 1 yrs $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.032 acft Curve number Drainage area = 0.160 ac= 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



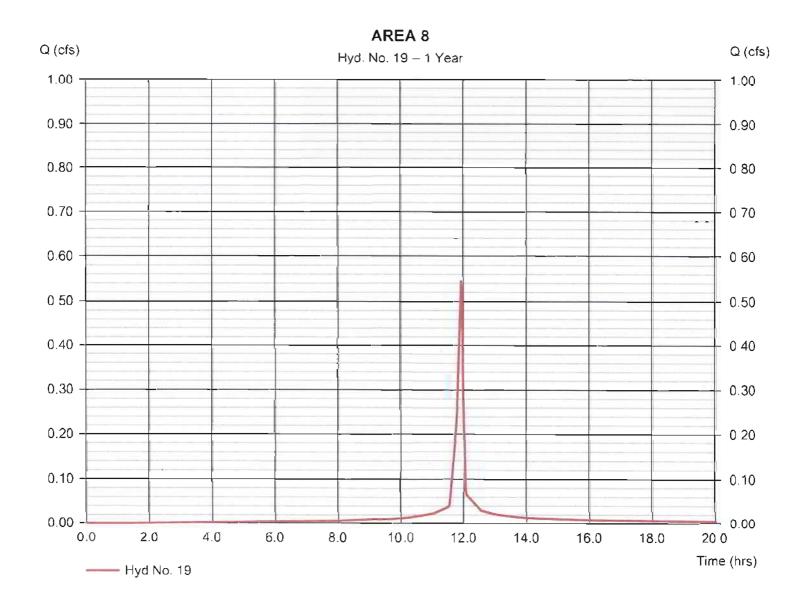
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SCS Runoff Peak discharge = 0.546 cfsStorm frequency = 1 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.026 acft Drainage area = 0.130 acCurve number = 98Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 2.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 20

<no description>

Hydrograph type Storm frequency Time interval = Combine = 1 yrs

= 1 min

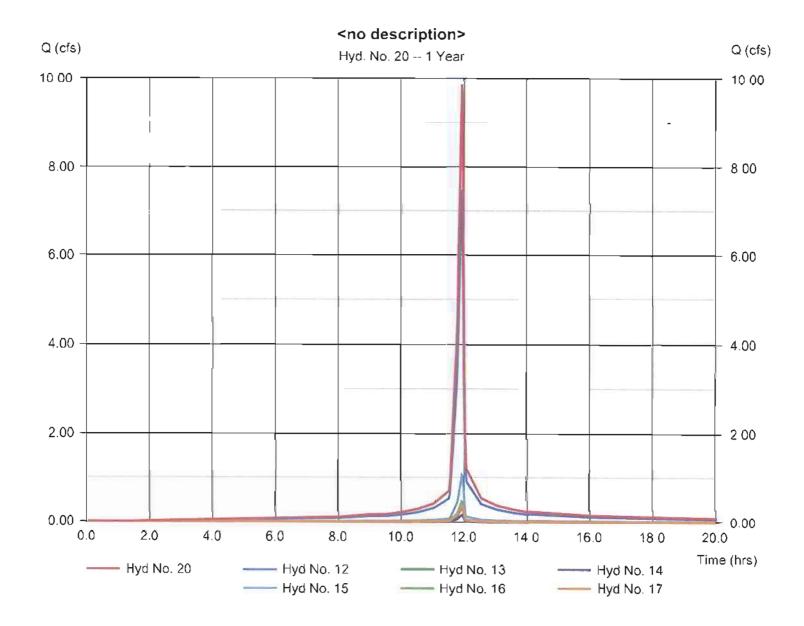
Inflow hyds. = 12, 13, 14, 15, 16, 17

Peak discharge

= 9.864 cfs

Time to peak = 11.92 hrs Hyd. volume = 0.472 acft

Contrib. drain. area = 2.350 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type Storm frequency Time interval Inflow hyds.

= Combine

= 1 yrs

= 1 min = 18, 19, 20 Peak discharge

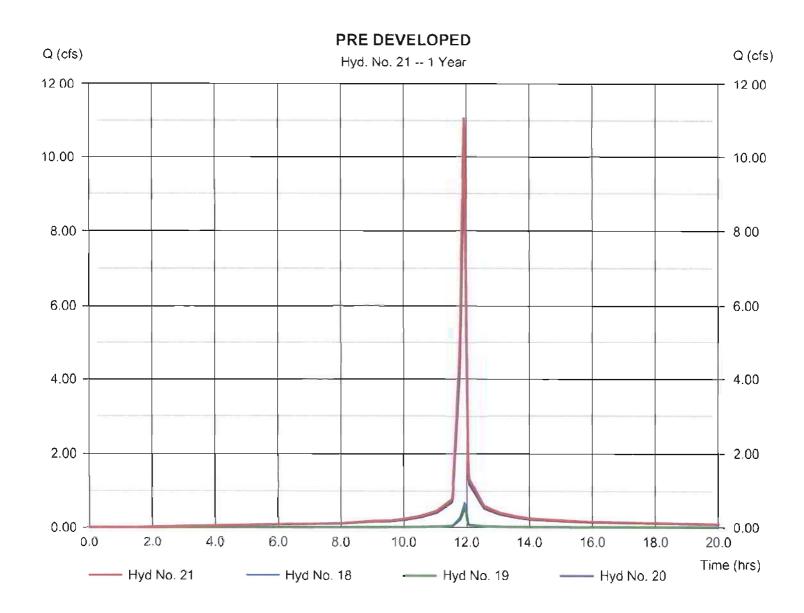
= 11.08 cfs

Time to peak Hyd. volume

= 11.92 hrs

Contrib. drain. area

= 0.530 acft = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9 $\,$

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	0 633	1	715	0.031				AREA A
2	SCS Runoff	1.951	1	715	0.094				AREA 8
3	SCS Runoff	0.633	1	715	0.031				AREA C
4	SCS Runoff	1.846	1	715	0.089				AREA D
5	SCS Runoff	0 422	1	715	0.020				AREA E
6	SCS Runoff	1 318	1	715	0.064				AREA F
7	SCS Runoff	1.318	1	715	0.064				AREA G
8	SCS Runoff	1 002	1	715	0 048				AREA H
9	SCS Runoff	0.896	1	715	0.043				AREA I
10	Combine	6.802	1	715	0 329	1, 2, 3,			<no description=""></no>
11	Combine	10.02	1	715	0.485	4, 5, 6, 7, 8, 9.			Combined Post Developed
12	SCS Runoff	9.386	1	715	0.454	10			AREA 1
13	SCS Runoff	0 422	1	715	0.020				AREA 2
14	SCS Runoff	0 211	1	715	0.010				AREA 3
15	SCS Runoff	1,318	1	715	0.064			*****	AREA 4
16	SCS Runoff	0 580	1	715	0.028				AREA 5
17	SCS Runoff	0 475	1	715	0 023				AREA 6
18	SCS Runoff	0 844	1	715	0 041				AREA 7
19	SCS Runoff	0 685	1	715	0.033				AREA 8
20	Combine	12.39	1	715	0.600	12, 13, 14,			<no description=""></no>
21	Combine	13.92	1	715	0.674	15, 16, 17, 18, 19, 20			PRE DEVELOPED
Ну	draflow Centr	al and Ol	ver 5.24	.12.gpw	Return	Period: 2 Ye	ear	Tuesday, 0	0 29, 2012

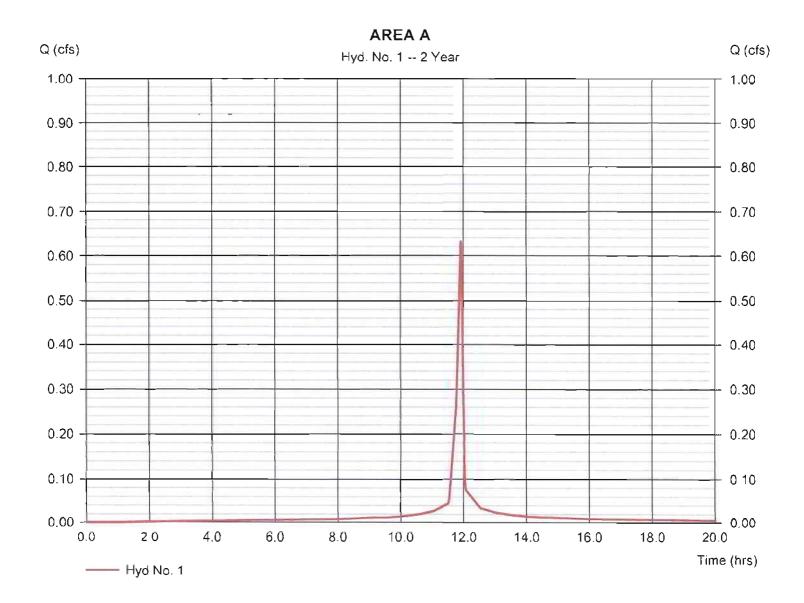
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge $= 0.633 \, \text{cfs}$ Storm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 min Hvd. volume = 0.031 acftCurve number Drainage area = 0.120 ac= 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



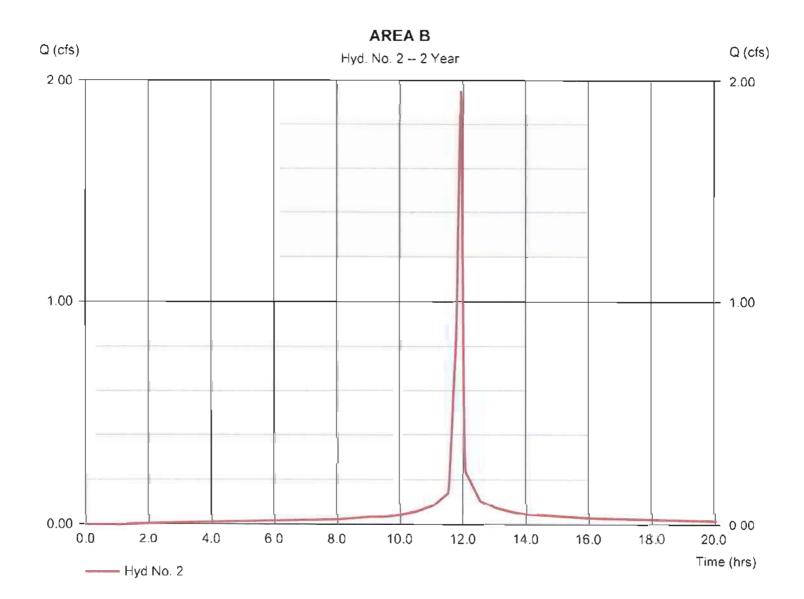
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = SCS Runoff Peak discharge = 1.951 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.094 acftDrainage area = 0.370 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



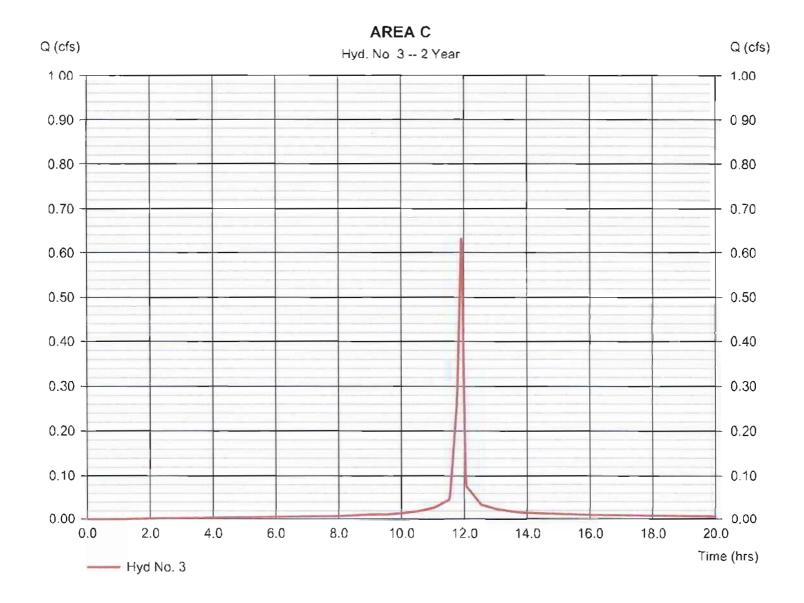
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc v9

Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

= SCS Runoff Hydrograph type Peak discharge = 0.633 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.031 acftDrainage area = 0.120 acCurve number ~ 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



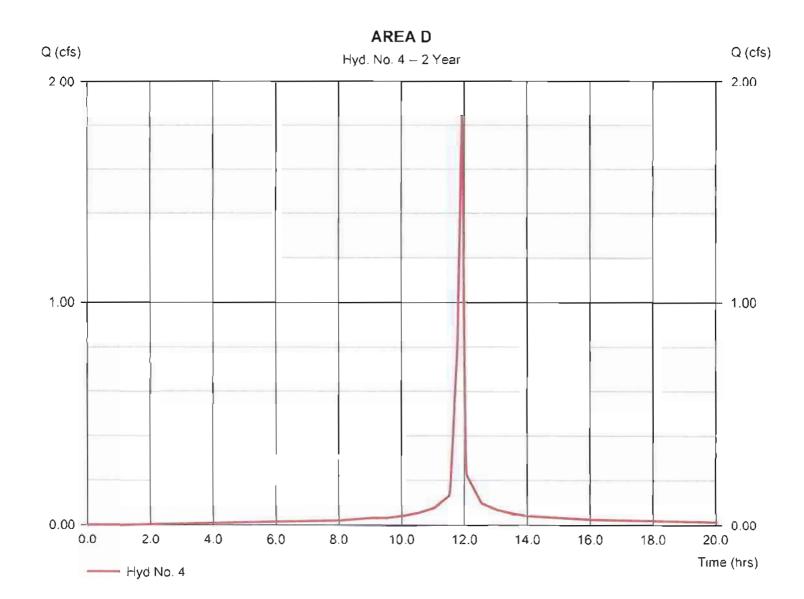
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

= 1.846 cfs= SCS Runoff Peak discharge Hydrograph type Storm frequency Time to peak = 11.92 hrs= 2 yrsTime interval = 1 min Hyd. volume = 0.089 acftCurve number Drainage area = 0.350 ac= 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method = TR55 Time of conc. (Tc) $= 1.70 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



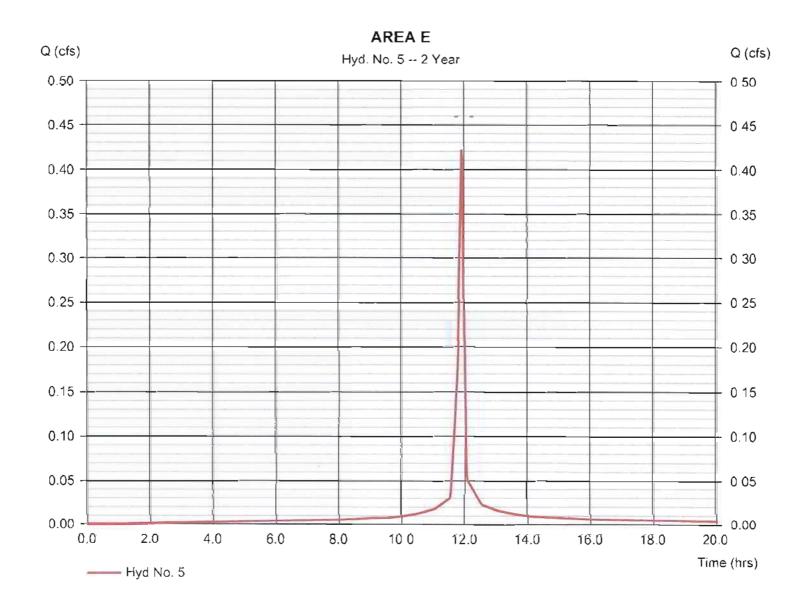
Hydraflow Hydrographs Extension for AutoCAO® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.422 cfsStorm frequency = 2 yrs Time to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.020 acftDrainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



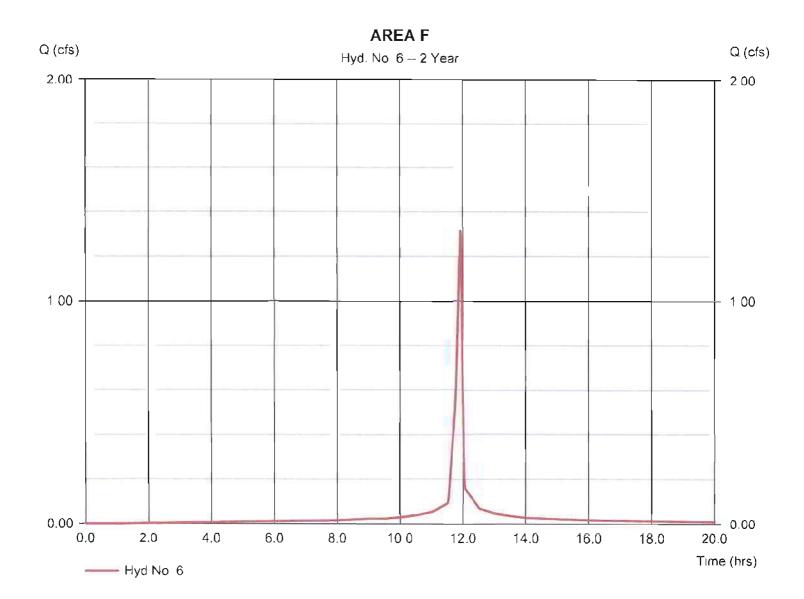
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 1.318 cfsStorm frequency Time to peak = 2 yrs= 11.92 hrsTime interval = 1 min Hyd. volume = 0.064 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



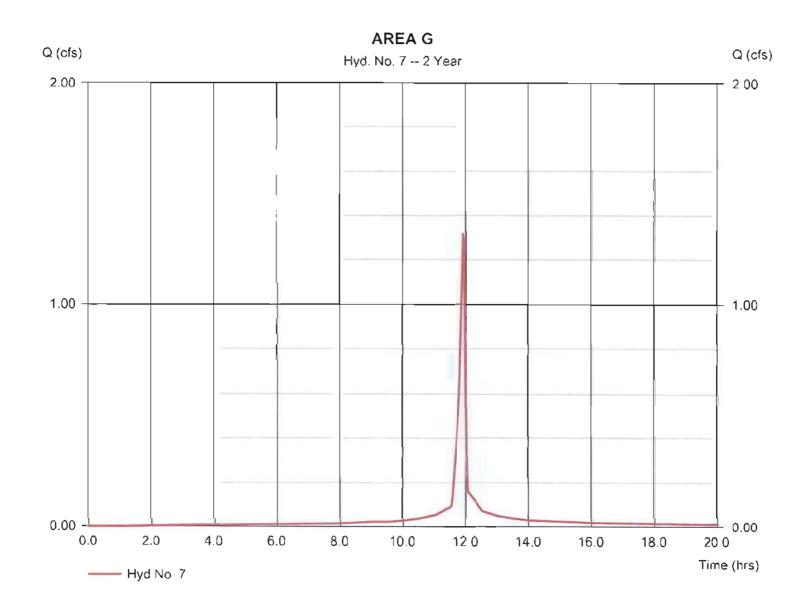
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff Peak discharge = 1.318 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval $= 1 \min$ Hyd. volume = 0.064 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



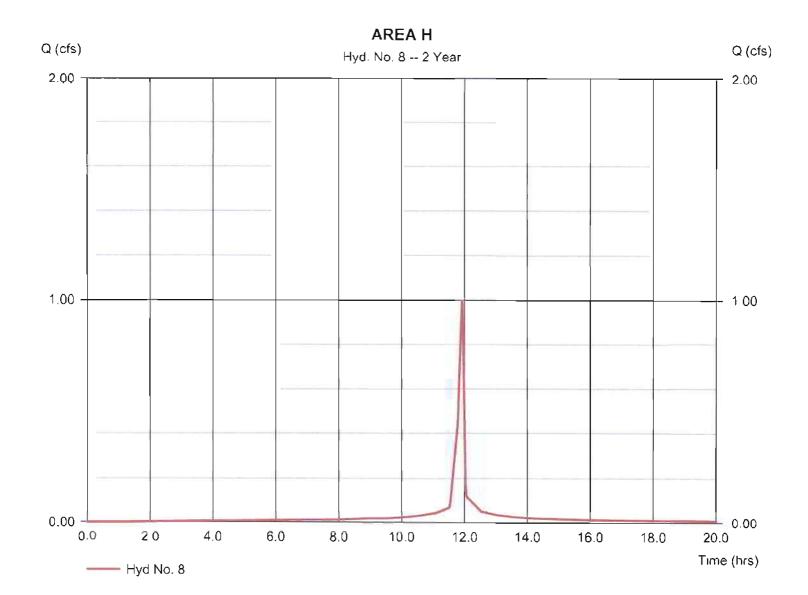
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

= SCS Runoff Peak discharge Hydrograph type = 1.002 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.048 acft Drainage area = 0.190 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



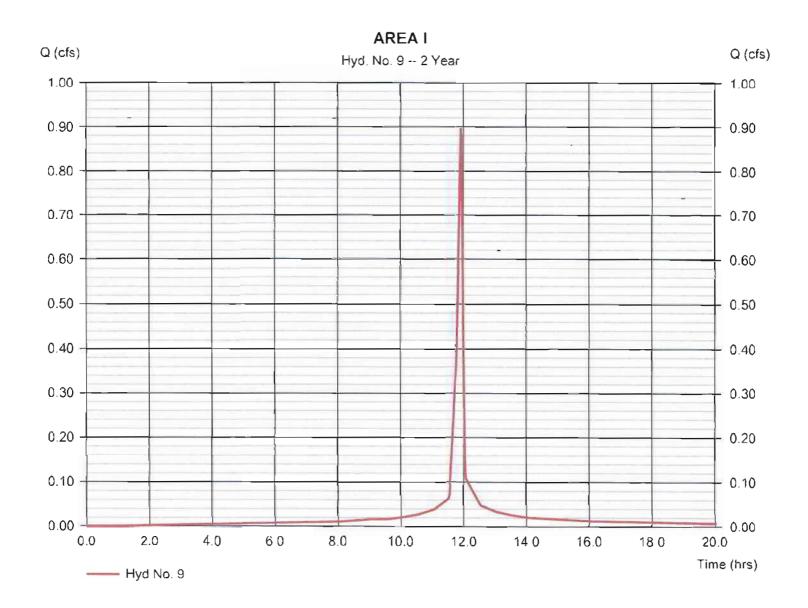
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 9

AREA I

Hydrograph type = SCS Runoff Peak discharge = 0.896 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.043 acft Drainage area = 0.170 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

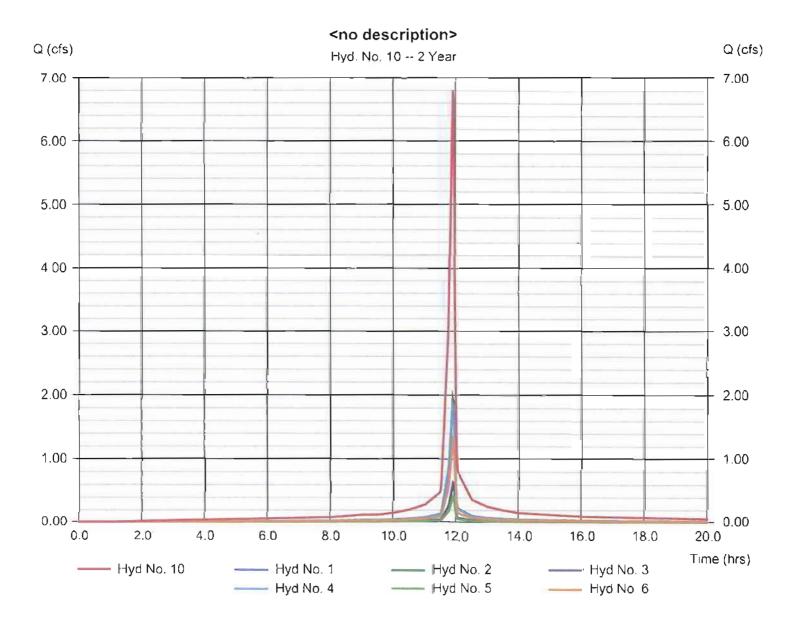
Hydrograph type Storm frequency Time interval Inflow hyds.

= Combine = 2 yrs = 1 min

= 1, 2, 3, 4, 5, 6

Peak discharge = 6.802 cfs Time to peak = 11.92 hrs

Hyd. volume = 0.329 acft Contrib. drain. area = 1.290 ac



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Tuesday. 00 29. 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type Storm frequency Time interval Inflow hyds.

= Combine = 2 yrs

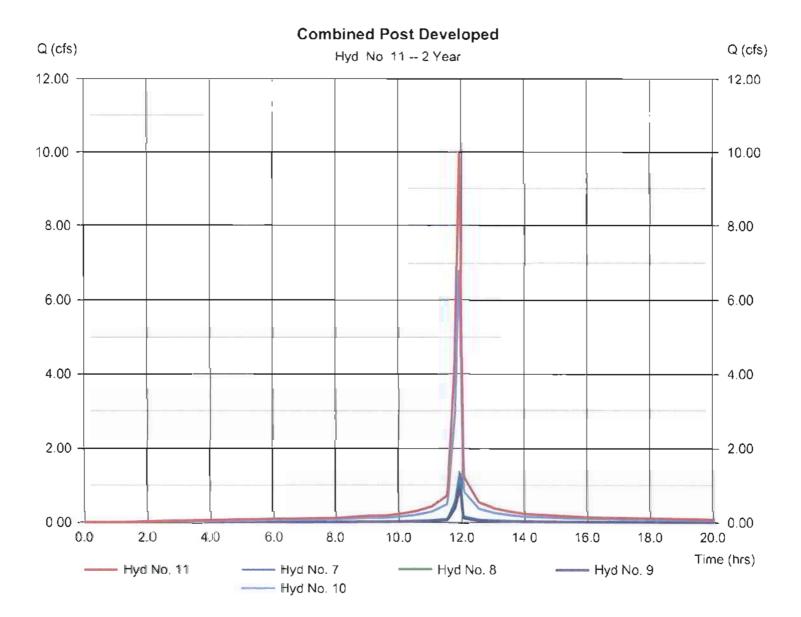
= 1 min = 7, 8, 9, 10 Peak discharge

= 10.02 cfs

Time to peak Hyd. volume

= 11.92 hrs = 0.485 acft

Contrib. drain. area = 0.610 ac



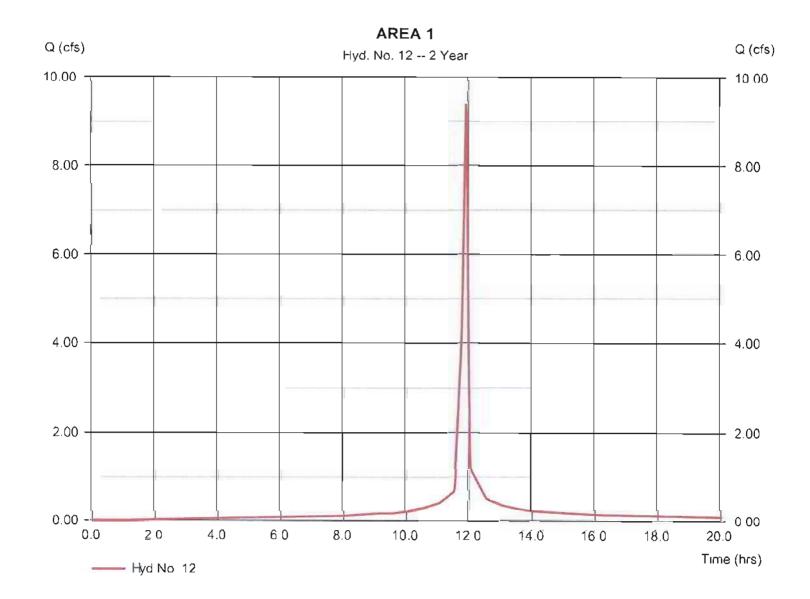
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 9.386 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.454 acftDrainage area = 1.780 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 3.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



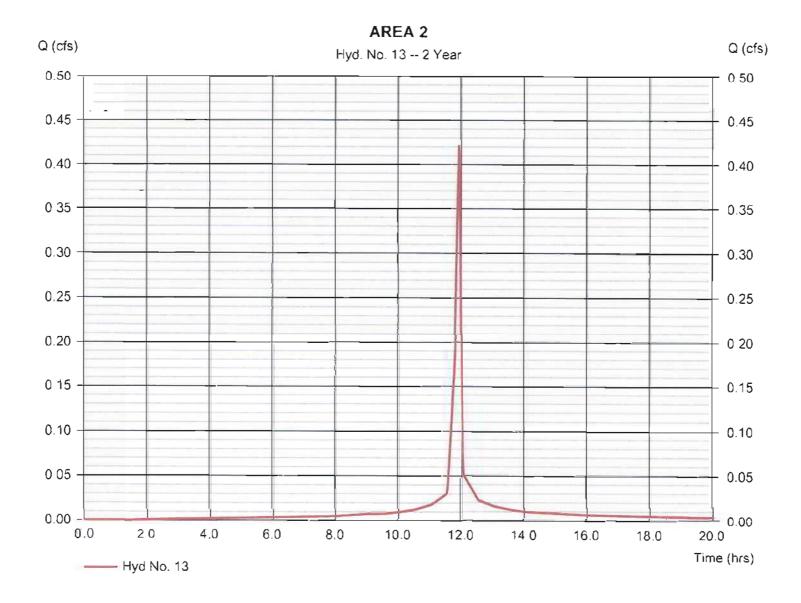
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

= SCS Runoff Hydrograph type Peak discharge = 0.422 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.020 acft Drainage area = 0.080 acCurve number = 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



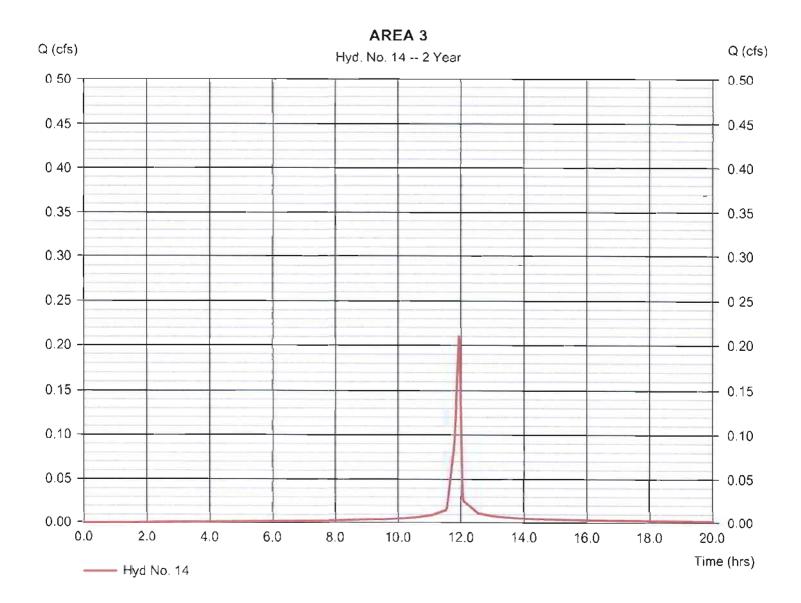
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Hydrograph type = SCS Runoff Peak discharge = 0.211 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.010 acftDrainage area = 0.040 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



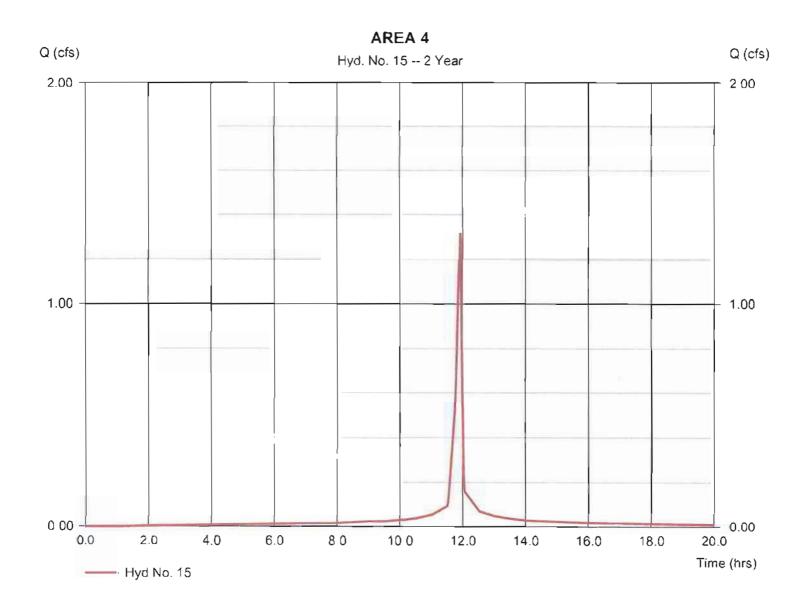
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Hydrograph type = SCS Runoff Peak discharge = 1.318 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.064 acftDrainage area = 0.250 acCurve number = 98 Hydraulic length Basin Slope = 0.0 % = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration $= 24 \, hrs$ Shape factor = 484



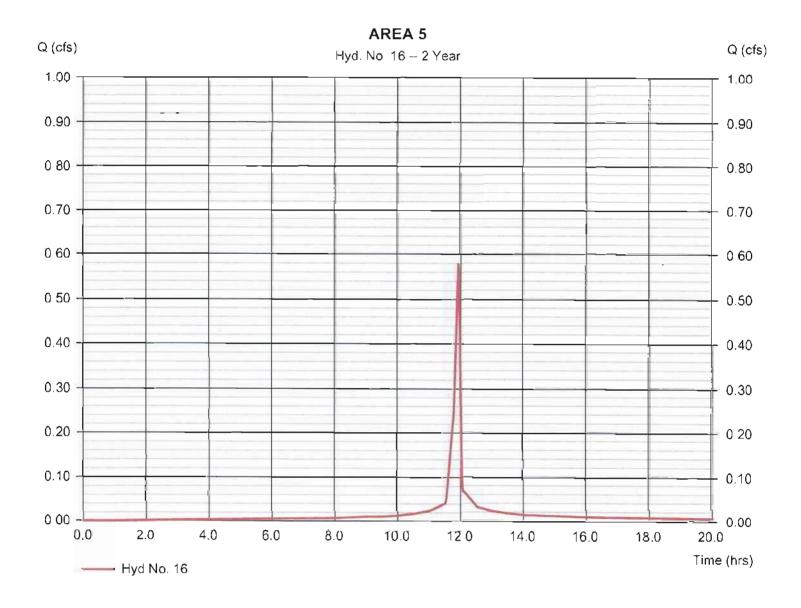
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

Hydrograph type = SCS Runoff Peak discharge = 0.580 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.028 acftDrainage area = 0.110 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



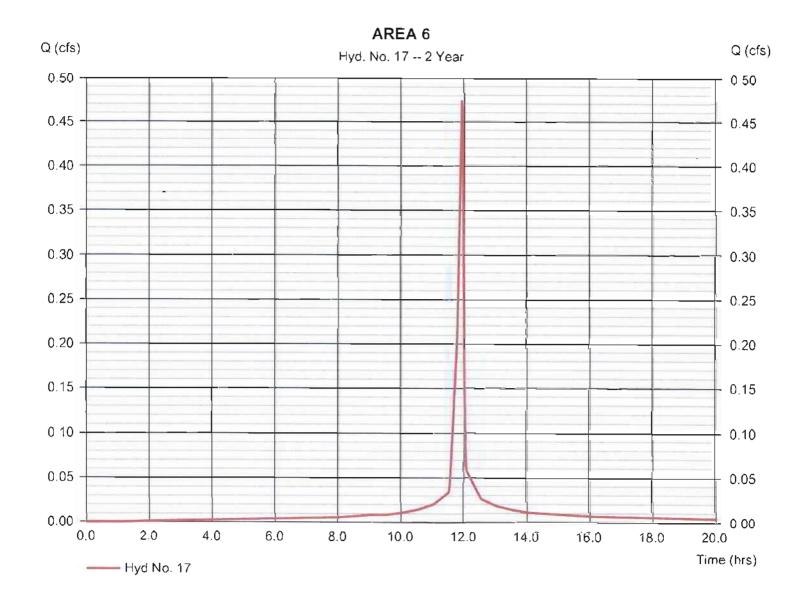
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 0.475 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.023 acft Drainage area = 0.090 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



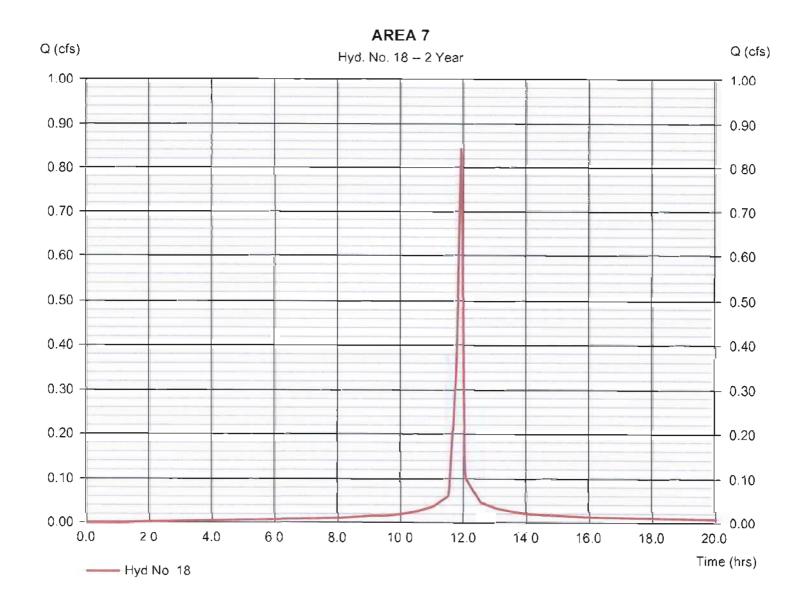
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff Peak discharge = 0.844 cfsStorm frequency = 2 yrsTime to peak = 11.92 hrs Time interval $= 1 \min$ Hyd. volume = 0.041 acftDrainage area = 0.160 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



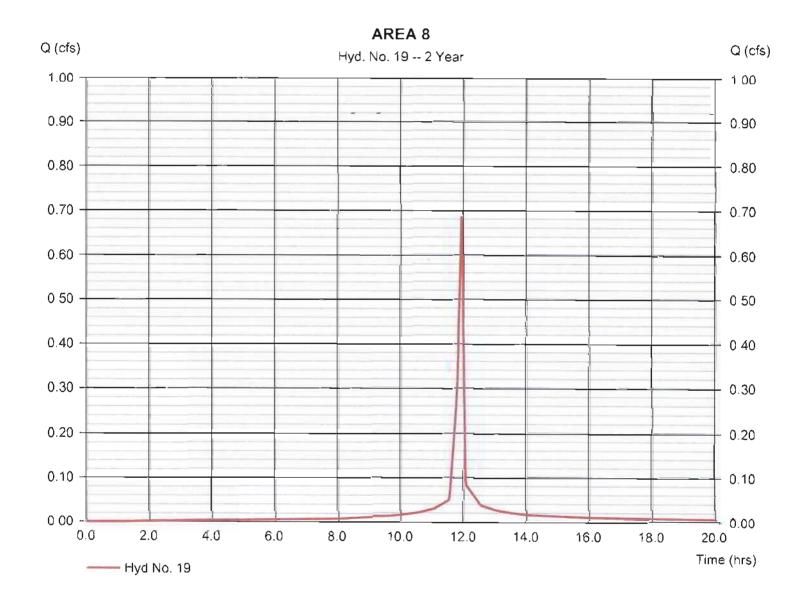
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SC\$ Runoff Peak discharge = 0.685 cfsStorm frequency = 2 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.033 acft Drainage area = 0.130 acCurve number = 98 = 0.0 % Basin Slope Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 3.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 20

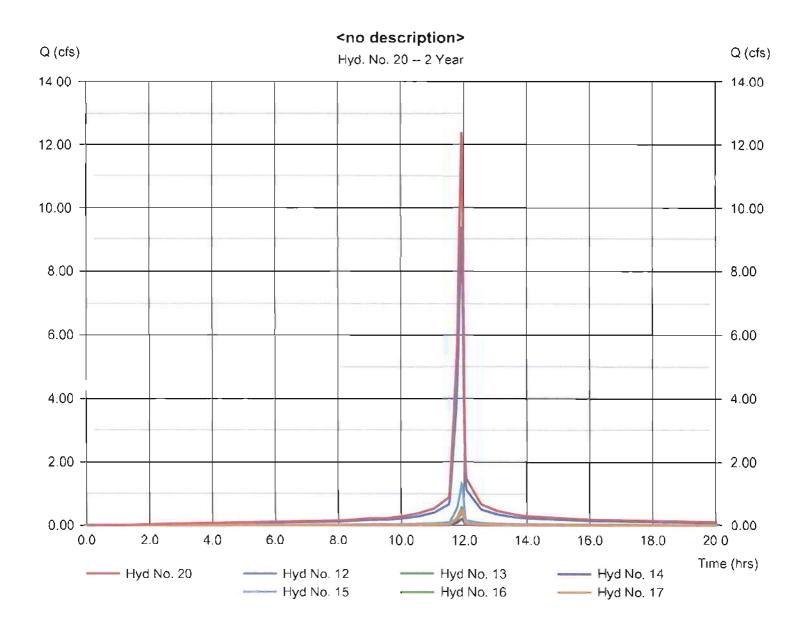
<no description>

Hydrograph type = Combine Storm frequency = 2 yrs Time interval = 1 min

Inflow hyds. = 12, 13, 14, 15, 16, 17

Peak discharge = 12.39 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.600 acft

Contrib. drain. area = 2.350 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc v9

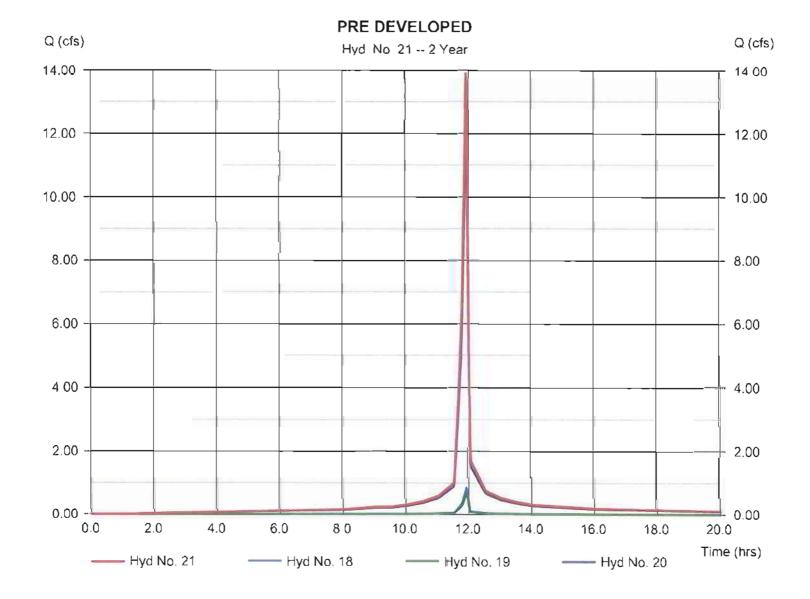
Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine
Storm frequency = 2 yrs
Time interval = 1 min
Inflow hyds. = 18, 19, 20

Peak discharge = 13.92 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.674 acft
Contrib. drain, area = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (mln)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	0.817	1	715	0.040				AREA A
2	SCS Runoff	2.518	1	715	0.123				AREA B
3	SCS Runoff	0.817	1	715	0 040				AREA C
4	SCS Runoff	2.381	1	715	0 117				AREA D
5	SCS Runoff	0 544	1	715	0.027				AREA E
6	SCS Runoff	1 701	1	715	0.083				AREA F
7	SCS Runoff	1.701	1	715	0.083				AREA G
8	SCS Runoff	1.293	1	715	0 063		******		AREA H
9	SCS Runoff	1.157	1	715	0 057				AREA I
10	Combine	8.777	1	715	0.430	1. 2, 3, 4, 5, 6,			<no description=""></no>
11	Combine	12.93	1	715	0 633	7, 8, 9,			Combined Post Developed
12	SCS Runoff	12.11	1	715	0 593				AREA 1
13	SCS Runoff	0.544	1	715	0.027				AREA 2
14	SCS Runoff	0.272	1	715	0.013				AREA 3
15	SCS Runoff	1.701	1	715	0 083				AREA 4
16	SCS Runoff	0.748	1	715	0 037				AREA 5
17	SCS Runoff	0.612	1	715	0.030				AREA 6
18	SCS Runoff	1.089	1	715	0.053				AREA 7
19	SCS Runoff	0.885	1	715	0.043				AREA 8
20	Combine	15 99	1	715	0.783	12, 13, 14, 15, 16, 17,			<no description=""></no>
21	Combine	17 96	1	715	0.879	18, 19, 20			PRE DEVELOPED
Hydraflow Central and Oliver 5.24.12.gpw					Return Period: 5 Year			Tuesday, 00 29, 2012	

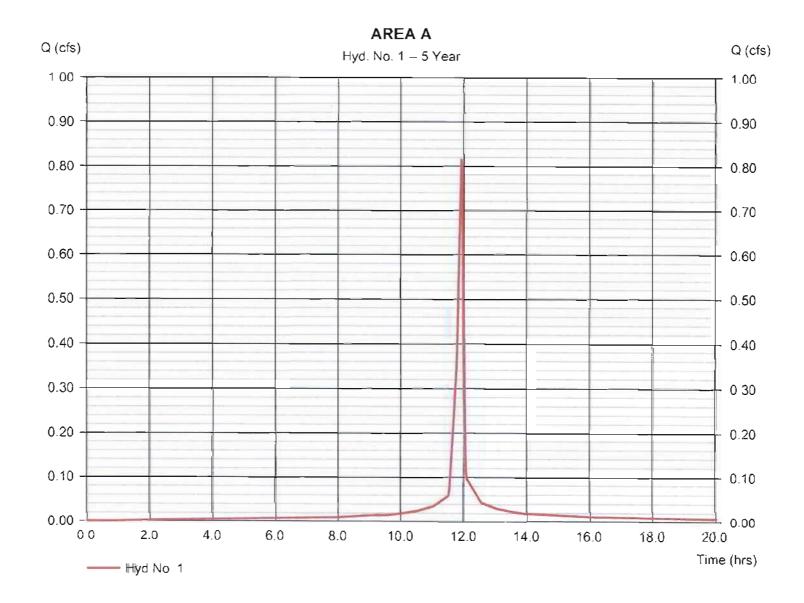
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge = 0.817 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.040 acft Drainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



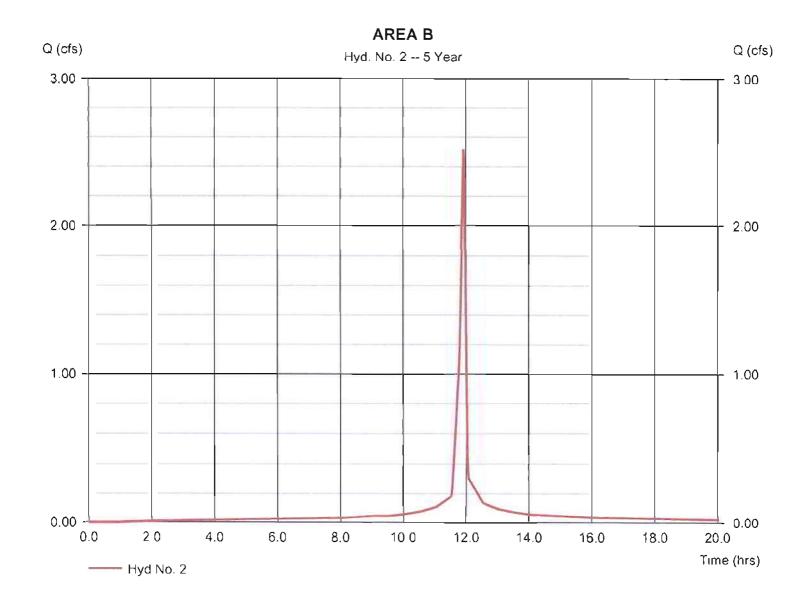
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = \$C\$ Runoff Peak discharge = 2.518 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.123 acftDrainage area = 0.370 acCurve number = 98 = 0.0 % Hydraulic length Basin Slope = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



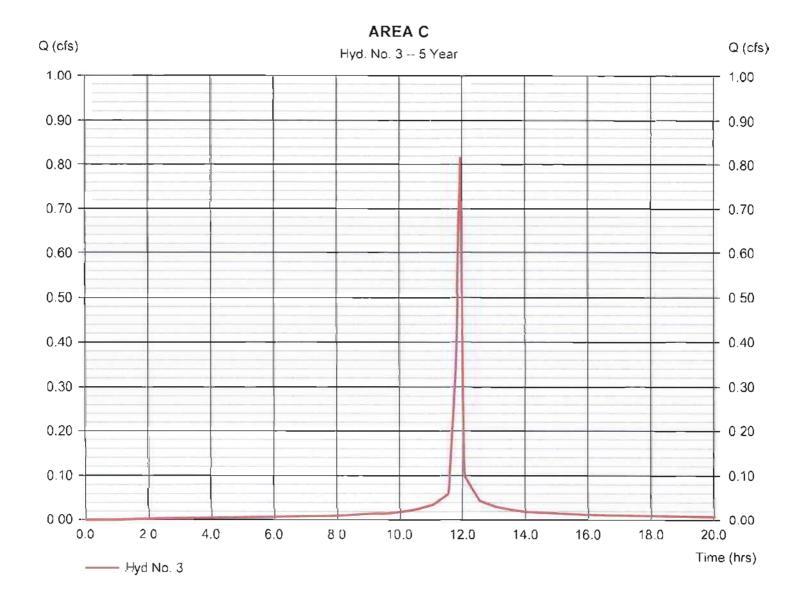
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 0.817 cfsStorm frequency = 5 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.040 acftDrainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



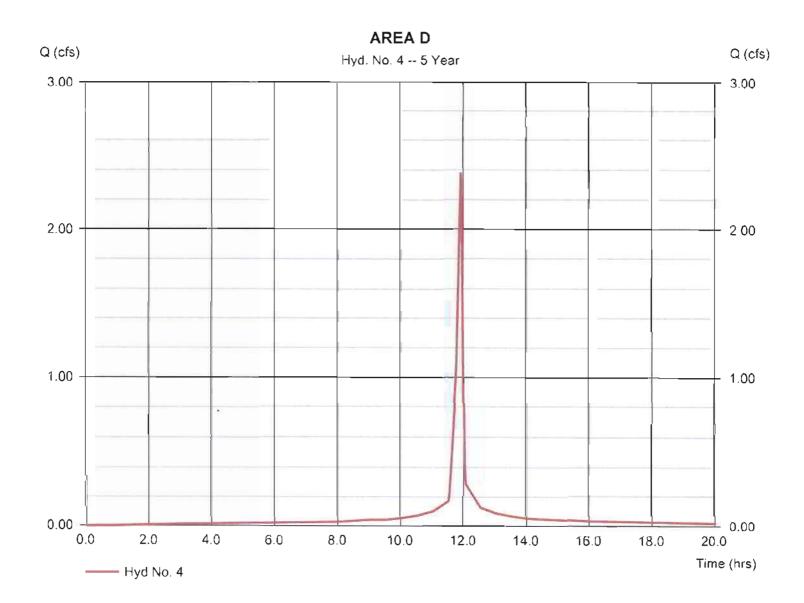
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

Hydrograph type = SCS Runoff Peak discharge = 2.381 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.117 acft = 0.350 acCurve number Drainage area = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTime of conc. (Tc) Tc method = TR55 $= 1.70 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



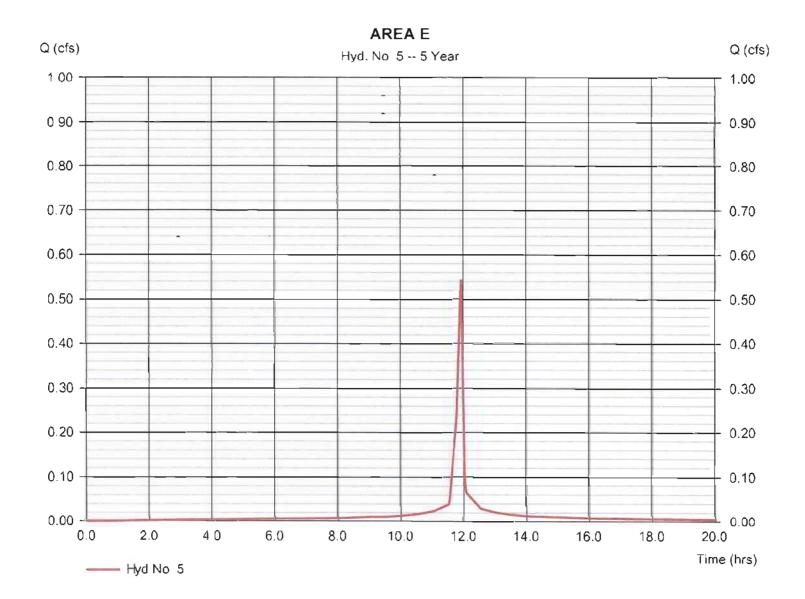
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.544 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.027 acftDrainage area Curve number = 0.080 ac= 98 Basin Slope = 0.0 % Hydraulic length = 0 ft= User Tc method Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



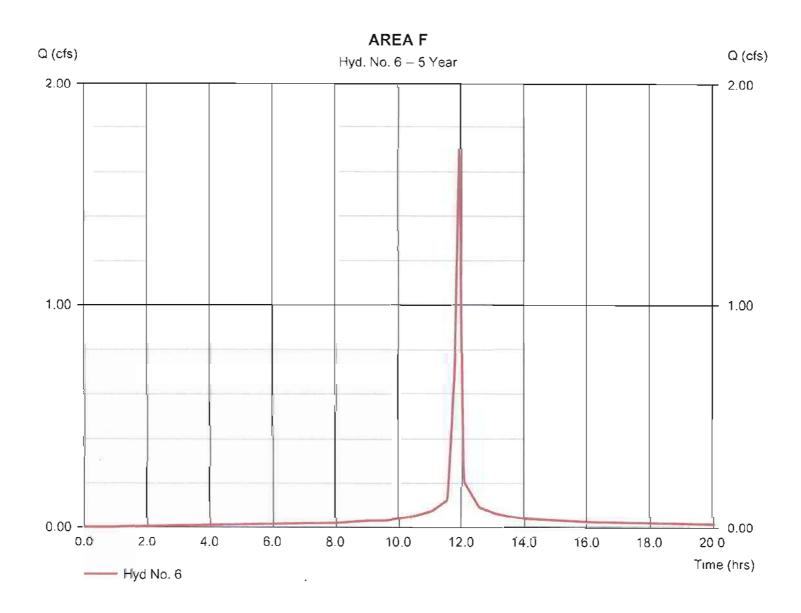
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 1.701 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.083 acft = 0.250 acDrainage area Curve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



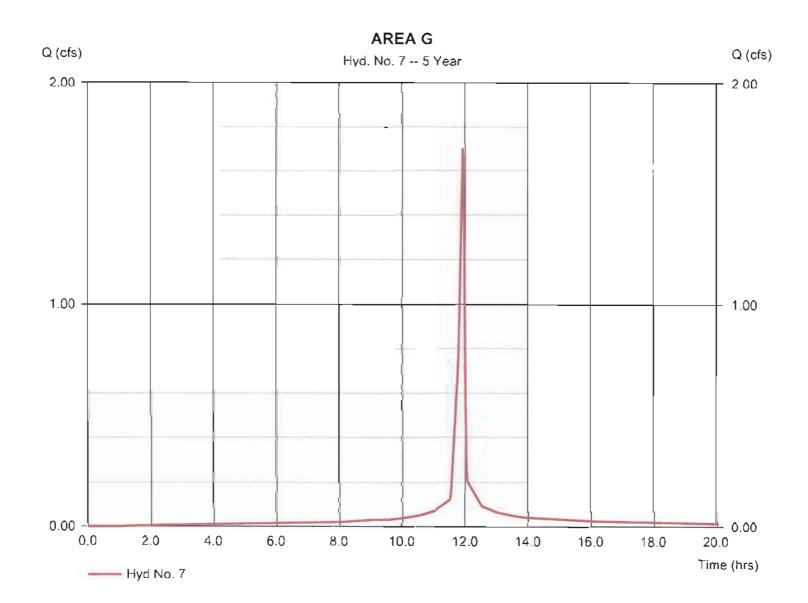
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff Peak discharge = 1.701 cfsStorm frequency = 5 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.083 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 in= Type II Distribution Storm duration = 24 hrsShape factor = 484



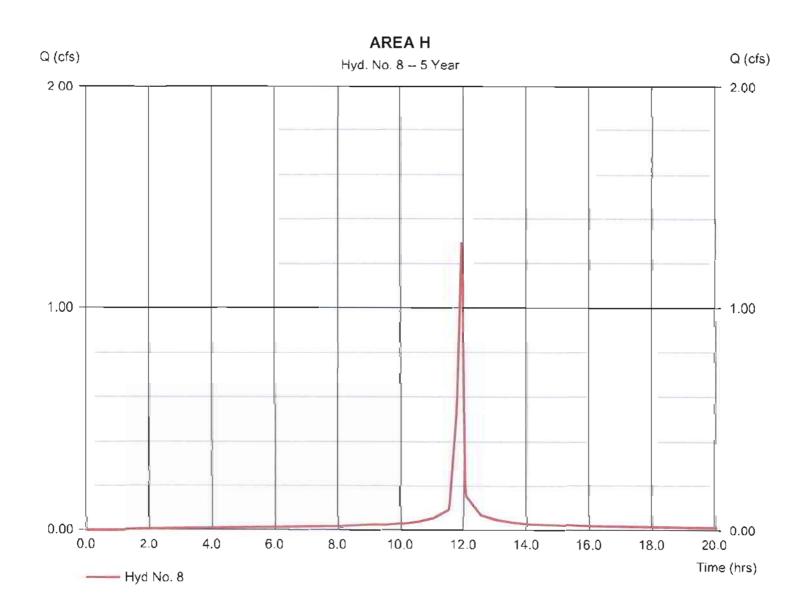
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

Hydrograph type = SCS Runoff Peak discharge = 1.293 cfsStorm frequency = 5 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.063 acft Drainage area = 0.190 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



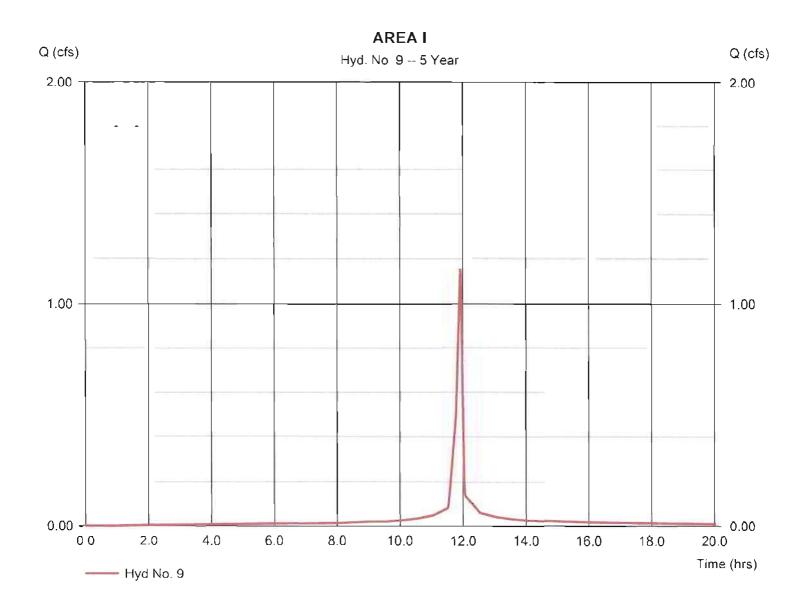
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 9

AREA I

Hydrograph type = SCS Runoff Peak discharge = 1.157 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval $= 1 \, \text{min}$ Hyd. volume = 0.057 acftDrainage area = 0.170 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

Hydrograph type Storm frequency Time interval Inflow hyds. = Combine = 5 yrs

= 1 min

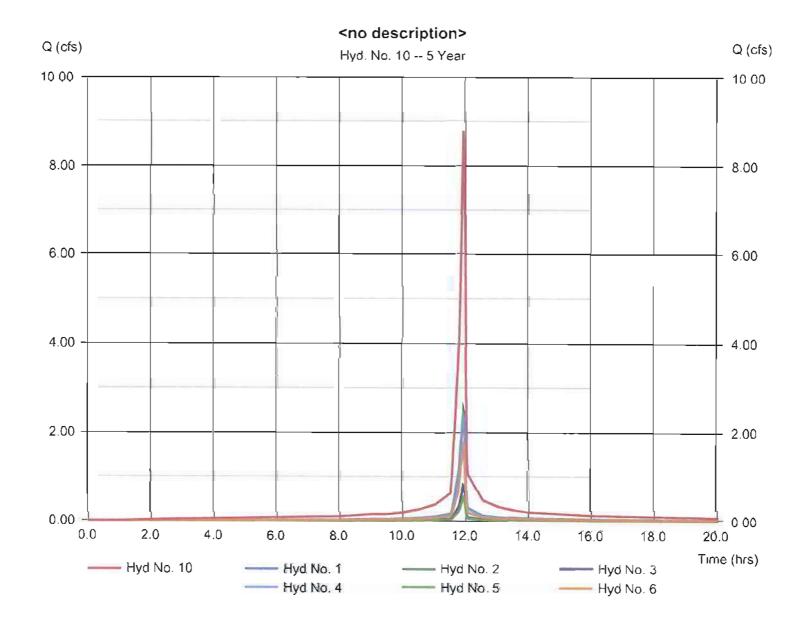
= 1, 2, 3, 4, 5, 6

Peak discharge

= 8.777 cfs

Time to peak = 11.92 hrsHyd. volume = 0.430 acft

Contrib. drain. area = 1.290 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

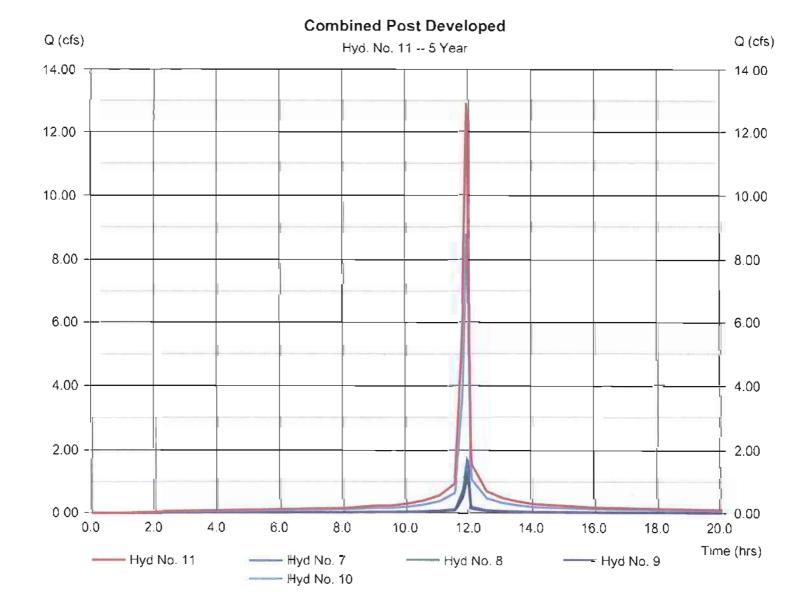
Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 7, 8, 9, 10

Peak discharge = 12.93 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.633 acft
Contrib. drain. area = 0.610 ac



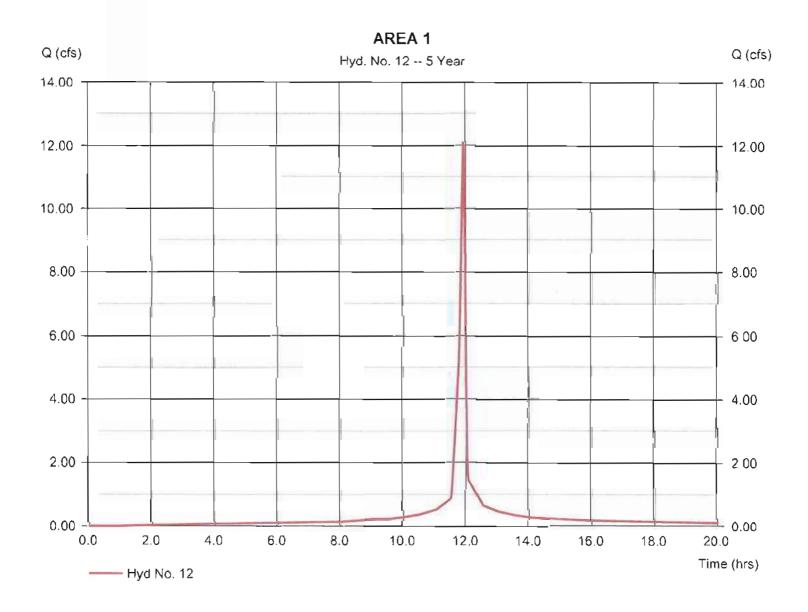
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 12.11 cfsStorm frequency = 5 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.593 acft Drainage area = 1.780 acCurve number = 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method = TR55 Time of conc. (Tc) $= 3.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



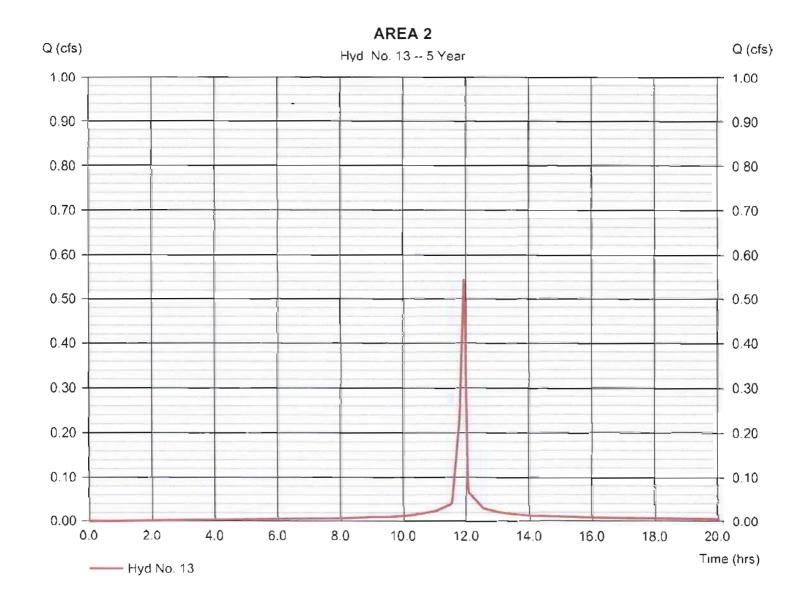
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge = 0.544 cfs= 5 yrs Storm frequency Time to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.027 acft Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



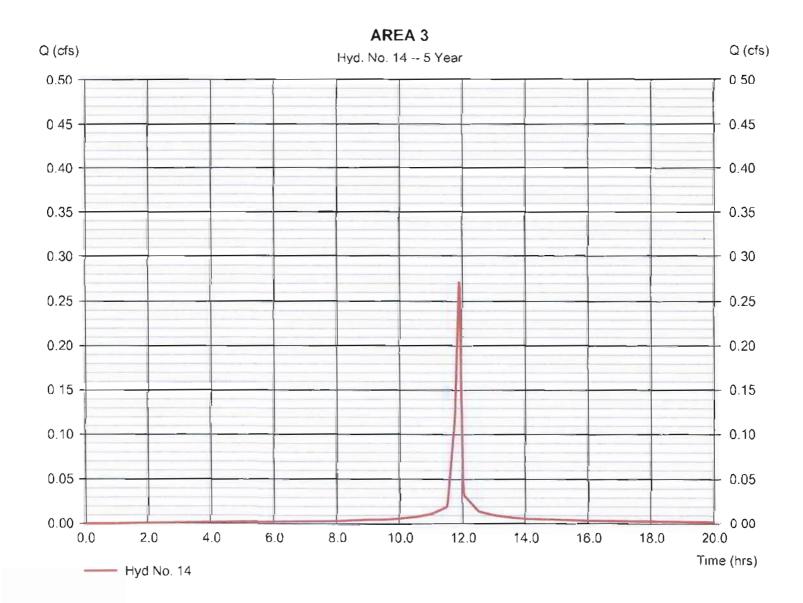
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Peak discharge = 0.272 cfs= SCS Runoff Hydrograph type Time to peak $= 11.92 \, hrs$ Storm frequency = 5 yrsTime interval = 1 min Hvd. volume = 0.013 acft Curve number Drainage area = 0.040 ac= 98 Hydraulic length = 0 ftBasin Slope = 0.0 %Time of conc. (Tc) $= 2.00 \, \text{min}$ Tc method = User Total precip. = 4.50 inDistribution = Type II Shape factor = 484 Storm duration = 24 hrs



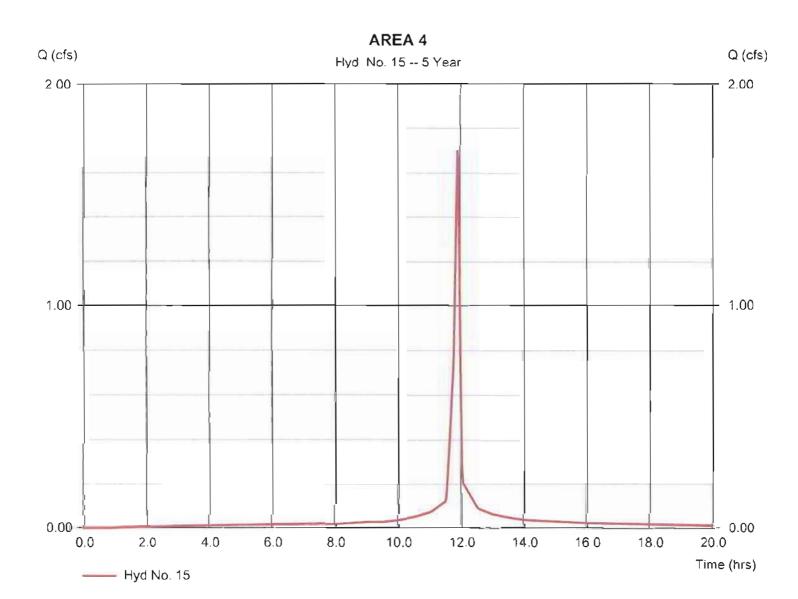
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Hydrograph type = SCS Runoff Peak discharge = 1.701 cfsStorm frequency = 5 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.083 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



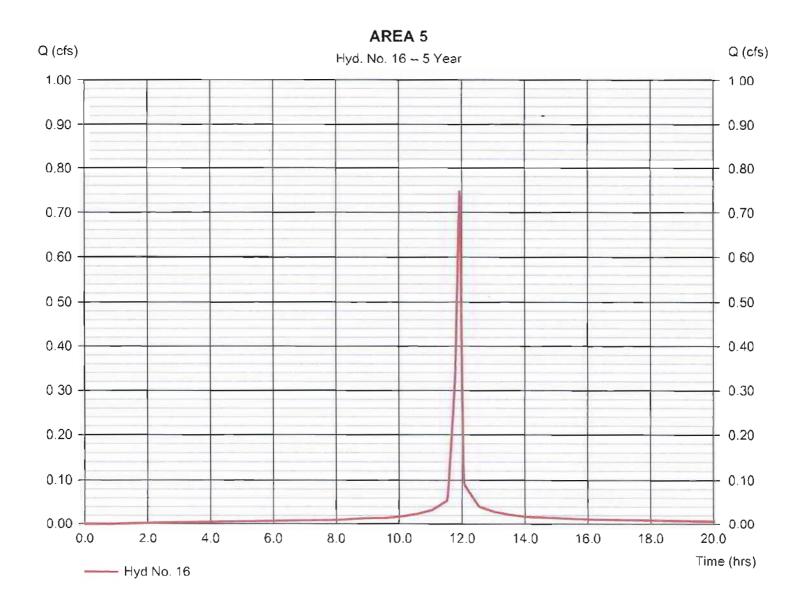
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

Hydrograph type = SCS Runoff Peak discharge = 0.748 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval $= 1 \, \text{min}$ Hyd. volume = 0.037 acftDrainage area = 0.110 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = Type II = 4.50 inDistribution Storm duration = 24 hrs Shape factor = 484



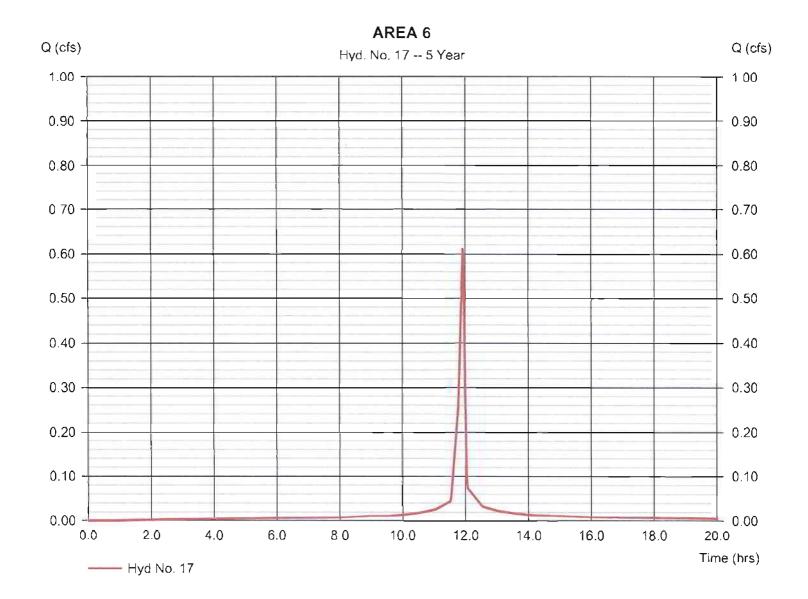
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 0.612 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval Hyd. volume = 1 min= 0.030 acft= 0.090 acCurve number Drainage area = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



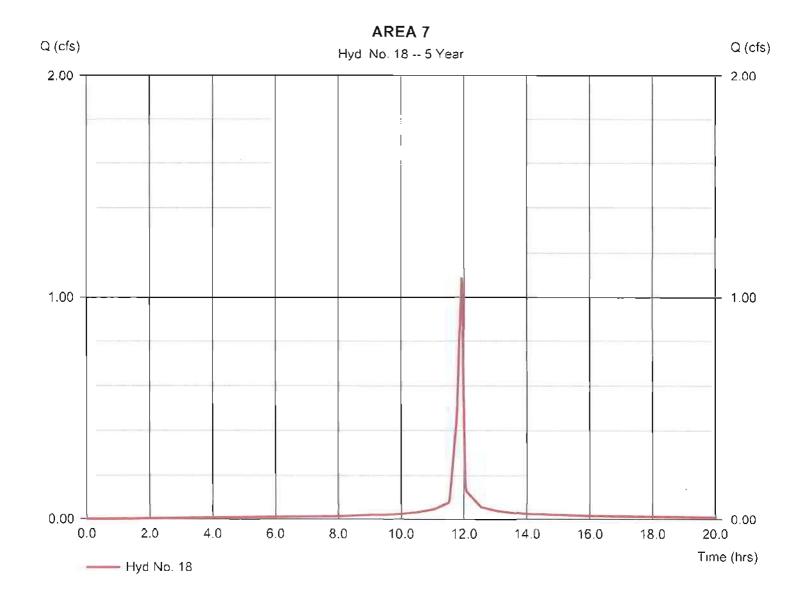
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff = 1.089 cfsPeak discharge Storm frequency = 5 yrsTime to peak = 11.92 hrsTime interval Hyd. volume $= 1 \min$ = 0.053 acft Drainage area Curve number = 0.160 ac= 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



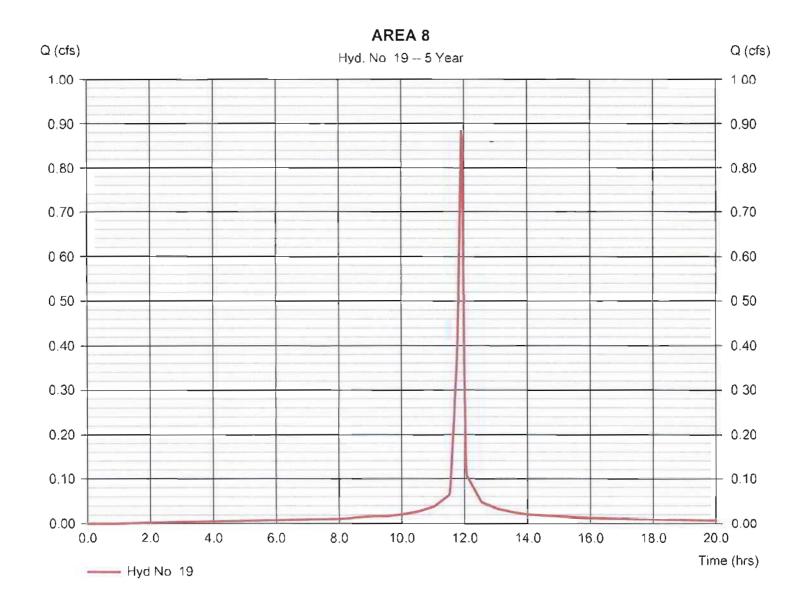
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SCS Runoff Peak discharge = 0.885 cfsStorm frequency = 5 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.043 acftDrainage area = 0.130 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 4.50 inDistribution = Type II Storm duration = 24 hrs = 484 Shape factor



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 20

<no description>

Hydrograph type Storm frequency

Time interval

= Combine

= 5 yrs

= 1 min

Inflow hyds. = 12, 13, 14, 15, 16, 17 Peak discharge Time to peak

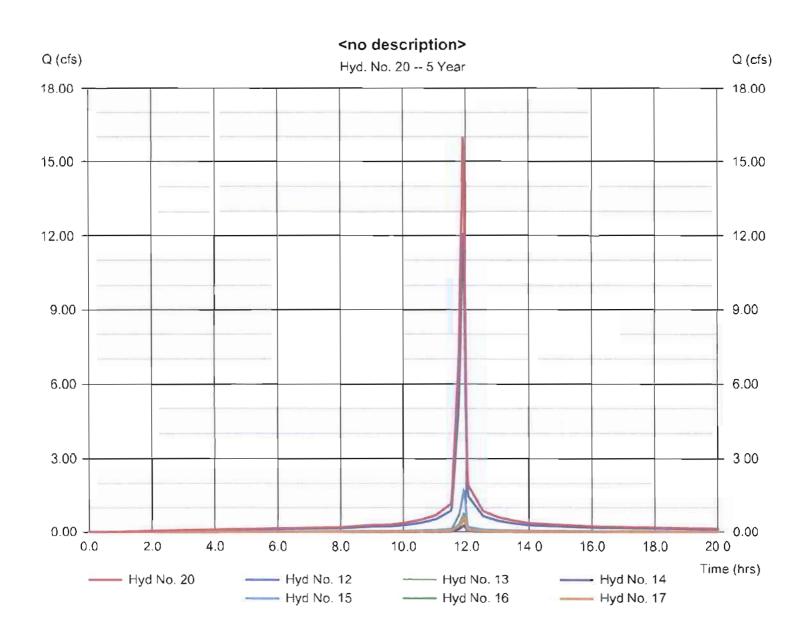
= 15.99 cfs

Hyd. volume

 $= 11.92 \, hrs$ = 0.783 acft

Contrib. drain. area

= 2.350 ac



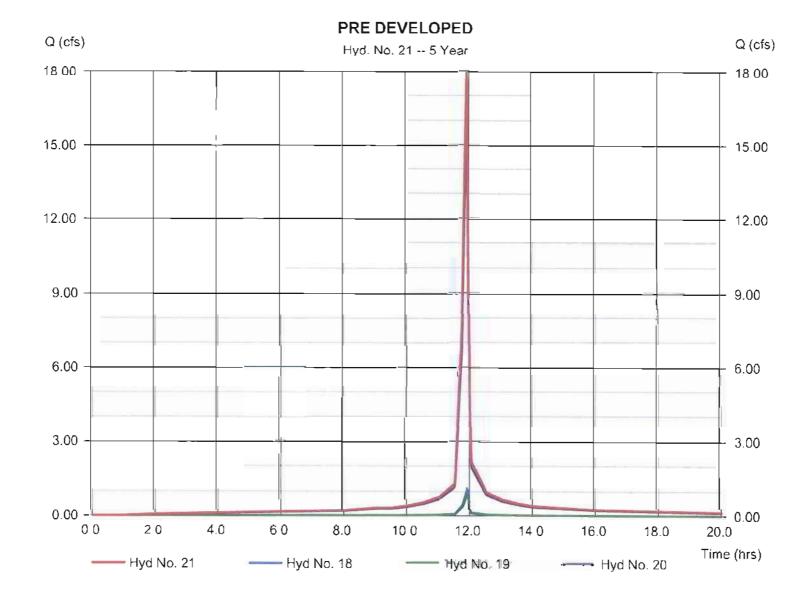
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine Storm frequency = 5 yrs Time interval = 1 min Inflow hyds. = 18, 19, 20 Peak discharge = 17.96 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.879 acft
Contrib. drain. area = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. ∨9

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (min)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	0 945	1	715	0.047				AREA A
2	SCS Runoff	2.913	1	715	0.143				AREA 8
3	SCS Runoff	0 945	1	715	0.047				AREA C
4	SCS Runoff	2 756	1	715	0.136				AREA D
5	SCS Runoff	0.630	1	715	0.031				AREA E
6	SCS Runoff	1.969	1	715	0.097				AREA F
7	SCS Runoff	1.969	1	715	0.097				AREA G
8	SCS Runoff	1.496	1	715	0.074				AREA H
9	SCS Runoff	1.339	1	715	0.066				AREAI
10	Combine	10.16	1	715	0 500	1, 2, 3,			<no description=""></no>
11	Combine	14.96	1	715	0.737	4, 5, 6, 7, 8, 9,			Combined Post Developed
12	SCS Runoff	14.02	1	715	0 690	10			AREA 1
13	SCS Runoff	0.630	1	715	0.031				AREA 2
14	SCS Runoff	0.315	1	715	0.016				AREA 3
15	SCS Runoff	1 969	1	715	0.097				AREA 4
16	SCS Runoff	0.866	1	715	0.043				AREA 5
17	SCS Runoff	0 709	1	715	0.035				AREA 6
18	SCS Runoff	1.260	1	715	0.062			****	AREA 7
19	SCS Runoff	1.024	1	715	0.050				AREA 8
20	Combine	18.50	1	715	0.911	12, 13, 14, 15, 16, 17,			<no description=""></no>
21	Combine	20.79	1	715	1.024	18, 19, 20			PRE DEVELOPED
Hydraflow Central and Oliver 5.24.12.gpw					Return	Period: 10 \	/ear	Tuesday, (00 29, 2012

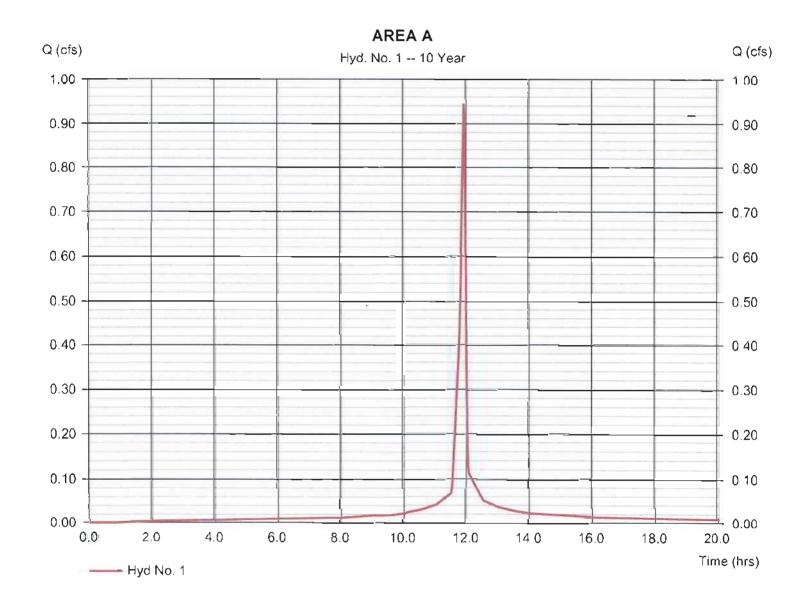
Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge = 0.945 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.047 acftDrainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



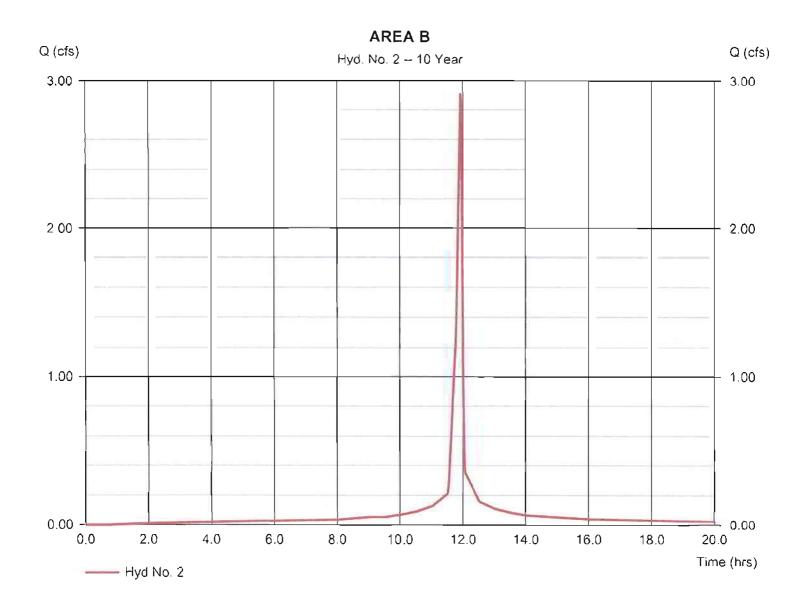
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = SCS Runoff Peak discharge = 2.913 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.143 acftCurve number Drainage area = 0.370 ac= 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



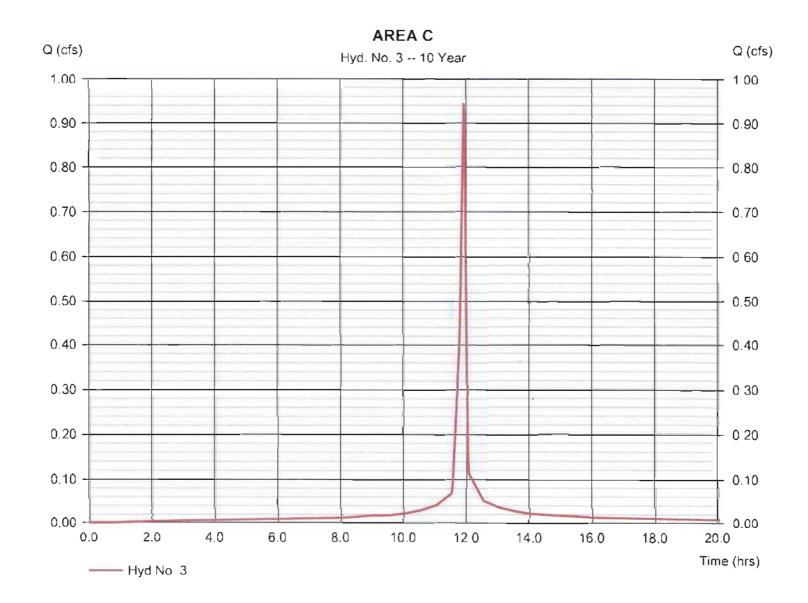
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 0.945 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.047 acft Drainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



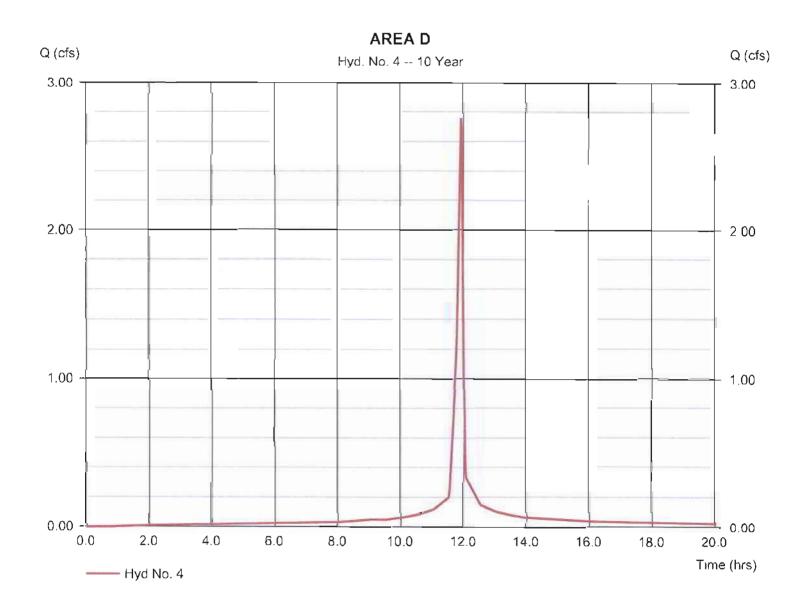
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

Hydrograph type = SCS Runoff Peak discharge = 2.756 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.136 acftDrainage area = 0.350 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 1.70 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



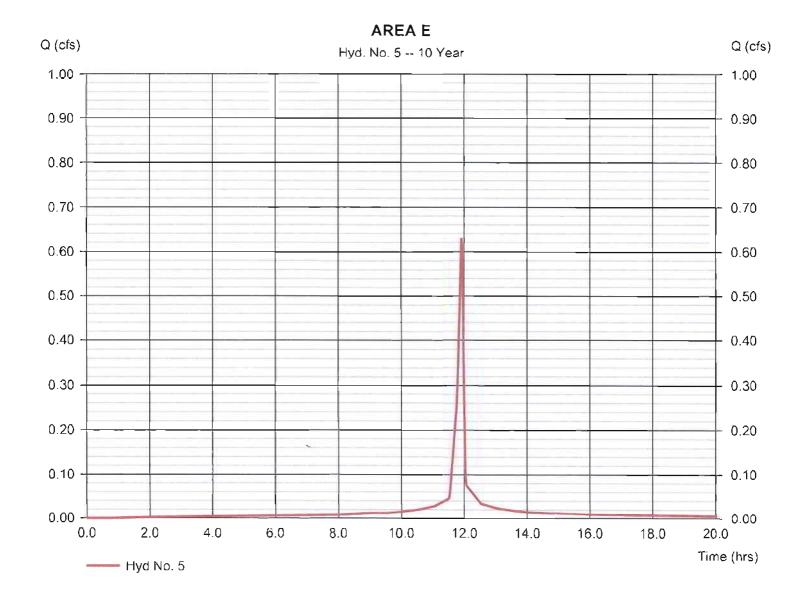
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.630 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.031 acft Drainage area = 0.080 acCurve number = 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



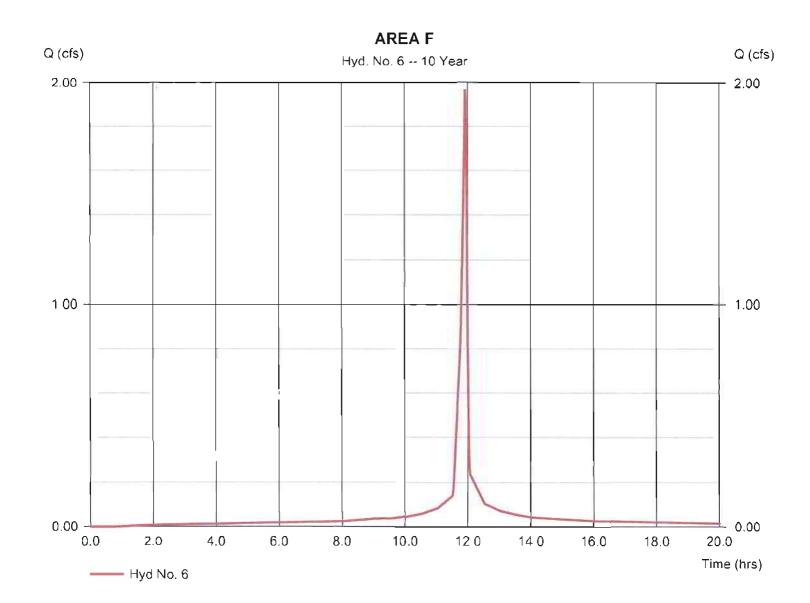
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 1.969 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Time interval $= 1 \min$ Hyd. volume = 0.097 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ft= User Tc method Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



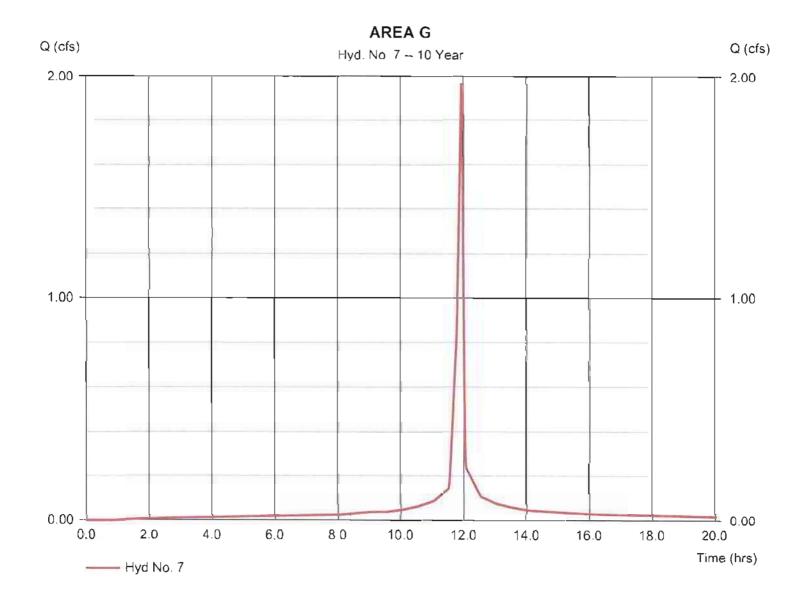
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff Peak discharge = 1.969 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.097 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



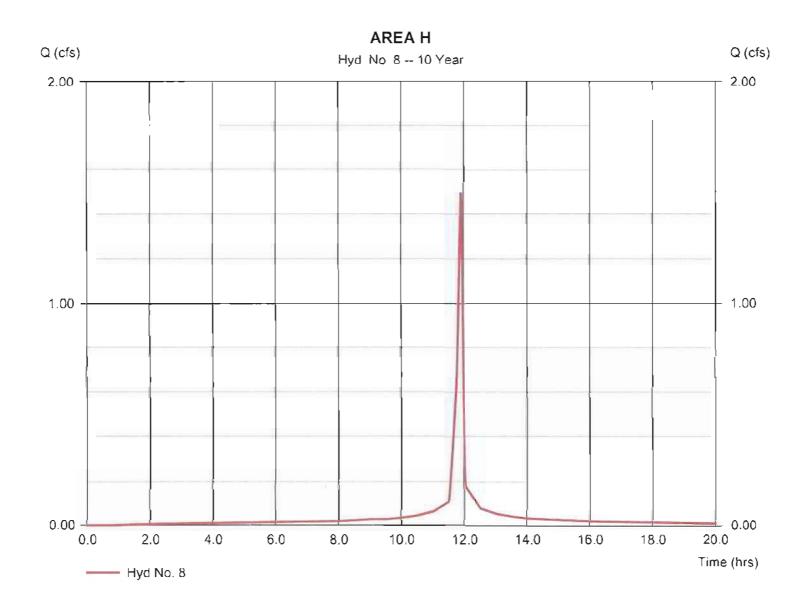
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

Hydrograph type = SCS Runoff Peak discharge = 1.496 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval Hyd. volume = 0.074 acft= 1 min Drainage area Curve number = 98 = 0.190 acBasin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ = 5.20 inTotal precip. Distribution = Type II Storm duration = 24 hrs Shape factor = 484



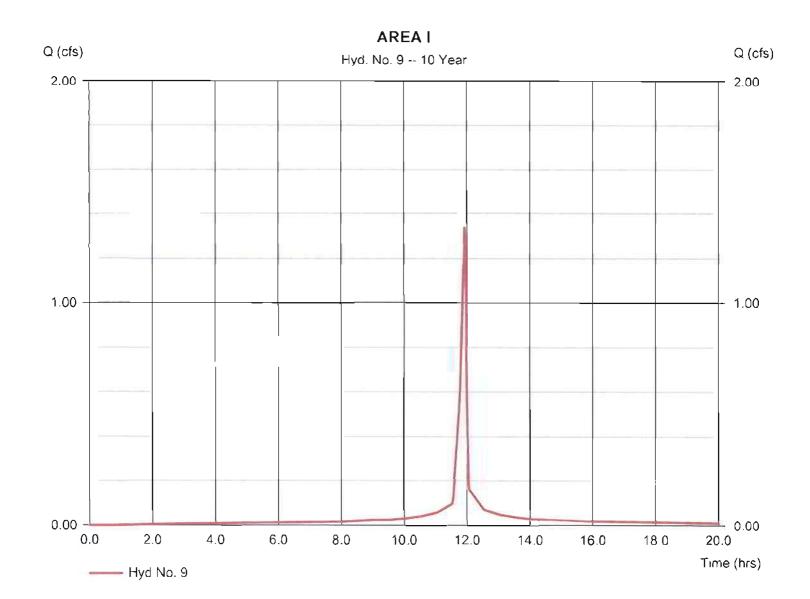
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 9

AREA I

Hydrograph type = SCS Runoff Peak discharge = 1.339 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.066 acft Drainage area = 0.170 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 in= Type II Distribution Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

Hydrograph type Storm frequency Time interval Inflow hyds. = Combine = 10 yrs

= 10 yrs = 1 min

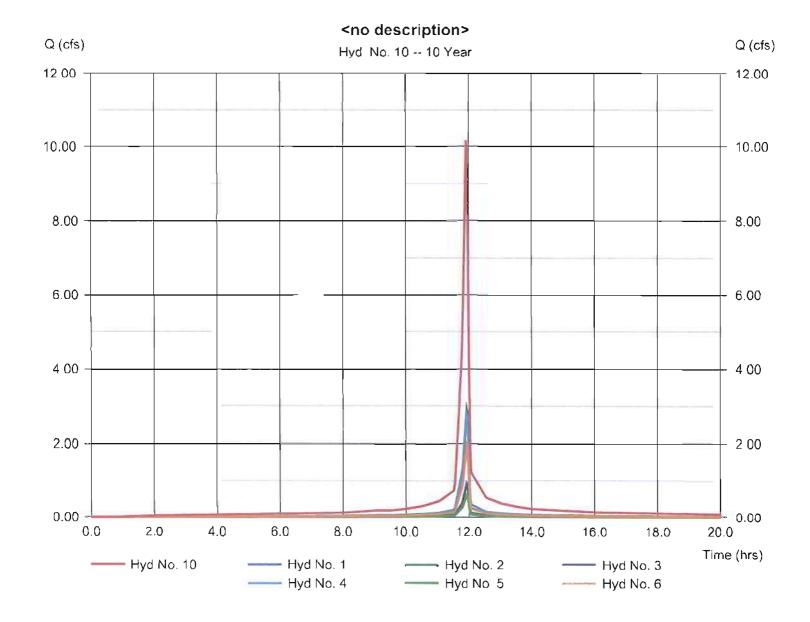
= 1, 2, 3, 4, 5, 6

Peak discharge Time to peak = 10.16 cfs = 11.92 hrs

Hyd. volume

= 0.500 acft

Contrib. drain. area = 1.290 ac



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

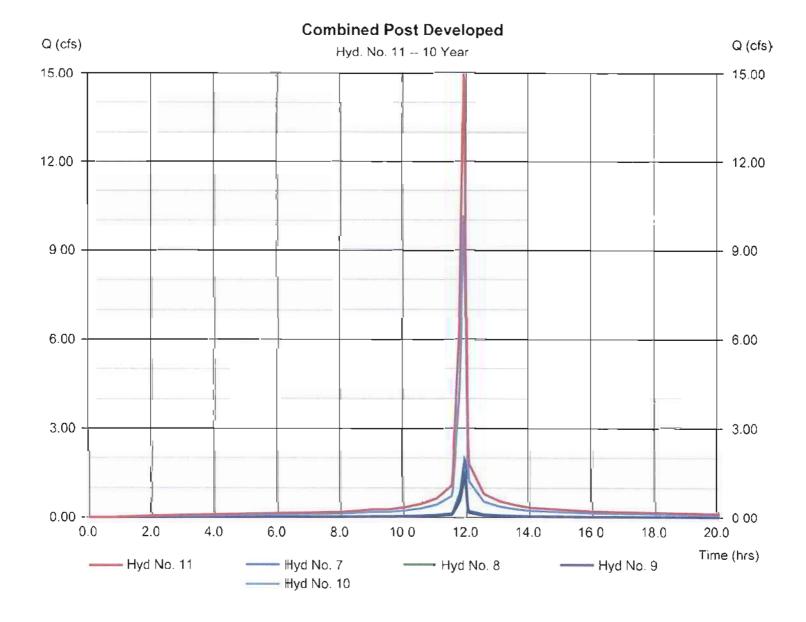
Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine
Storm frequency = 10 yrs
Time interval = 1 min
Inflow hyds. = 7, 8, 9, 10

Peak discharge = 14.96 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.737 acft
Contrib. drain. area = 0.610 ac



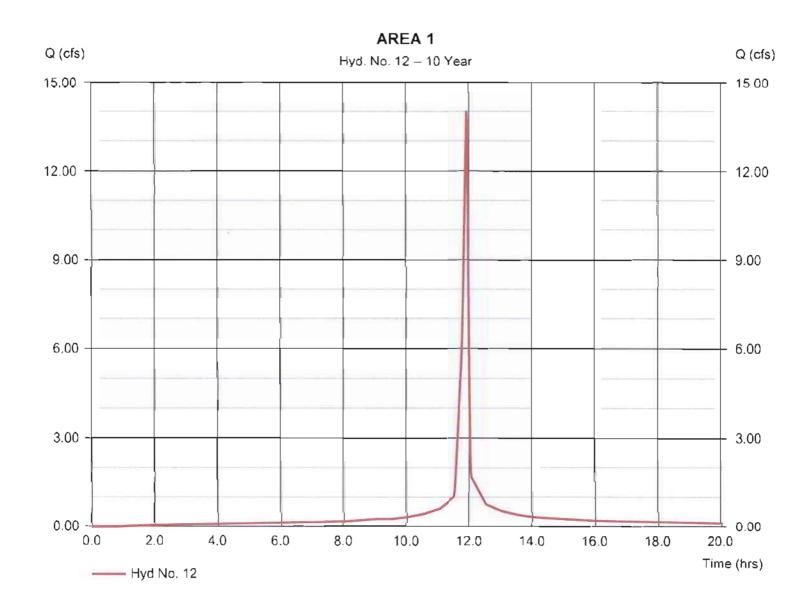
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 14.02 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.690 acftDrainage area = 1.780 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = TR55 $= 3.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



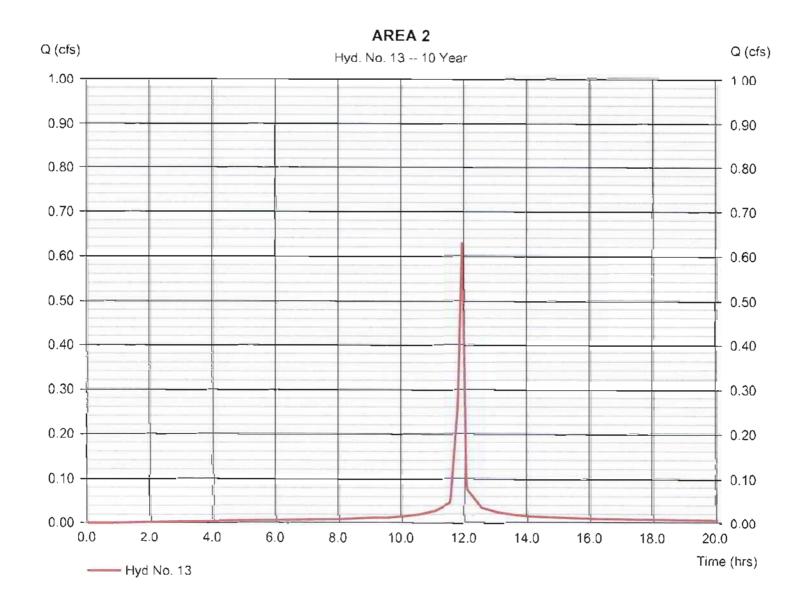
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge = 0.630 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.031 acftDrainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



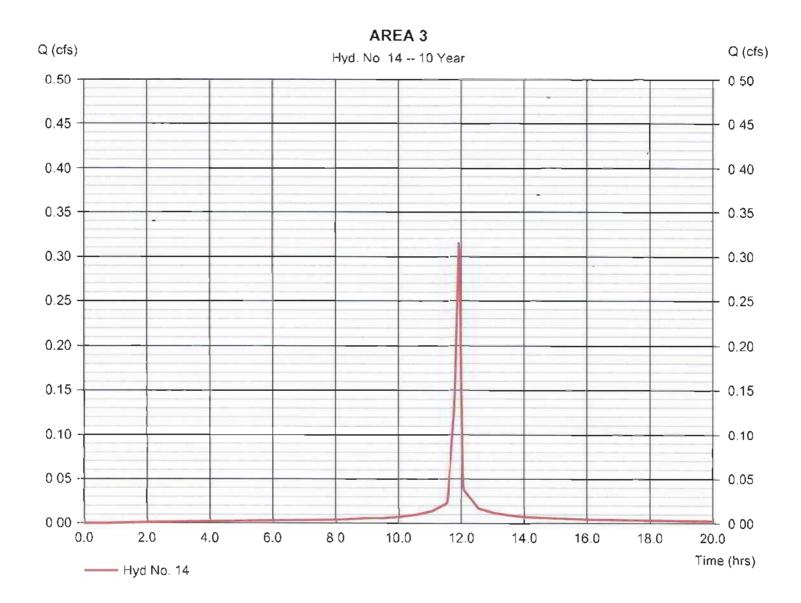
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Hydrograph type ≃ SCS Runoff Peak discharge = 0.315 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.016 acftDrainage area = 0.040 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



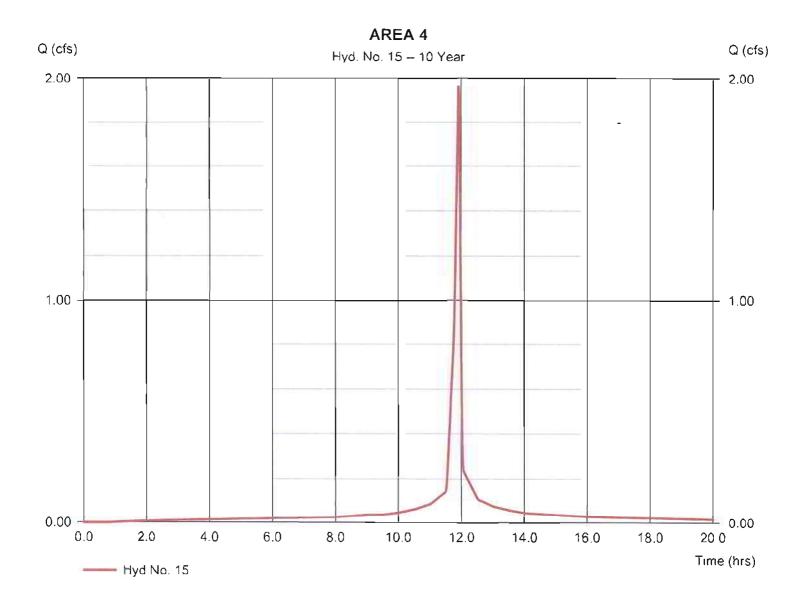
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Hydrograph type = SCS Runoff Peak discharge = 1.969 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.097 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



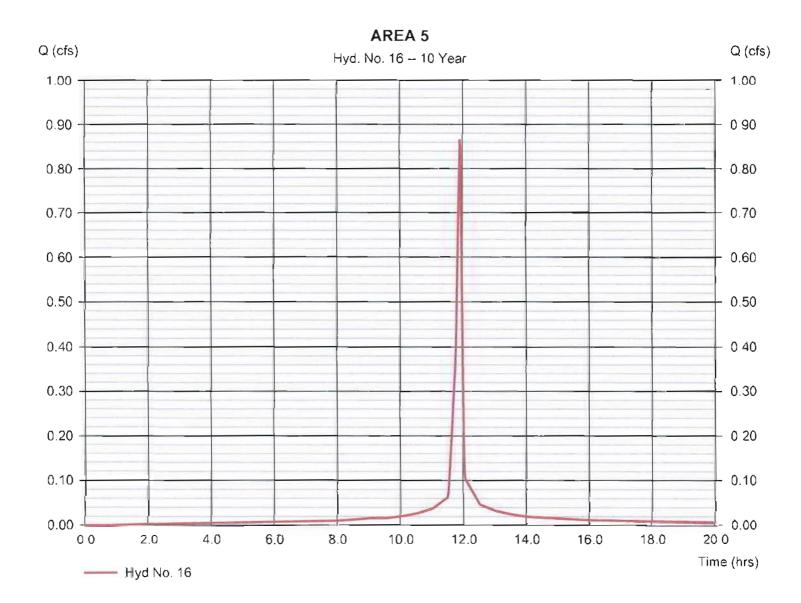
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

Hydrograph type ≃ SCS Runoff Peak discharge = 0.866 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.043 acftDrainage area = 0.110 acCurve number = 98 = 0.0 % Hydraulic length Basin Slope = 0 ftTime of conc. (Tc) $= 2.00 \, \text{min}$ Tc method = User Total precip. = 5.20 inDistribution = Type !! Storm duration = 24 hrsShape factor = 484



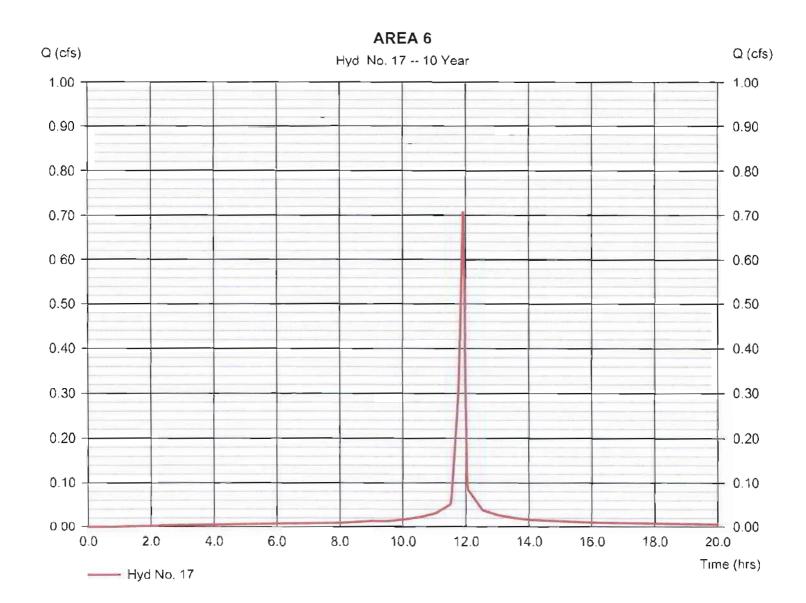
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 0.709 cfsStorm frequency = 10 yrsTime to peak $= 11.92 \, hrs$ Hyd. volume Time interval = 1 min = 0.035 acftDrainage area = 0.090 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



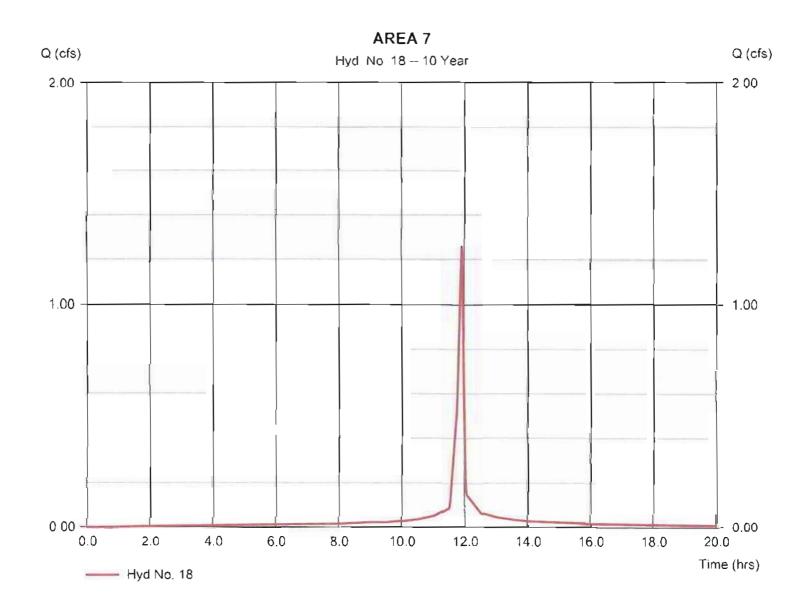
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff Peak discharge = 1.260 cfsStorm frequency Time to peak $= 11.92 \, hrs$ = 10 yrsTime interval = 1 minHyd. volume = 0.062 acftDrainage area = 0.160 acCurve number = 98 Hydraulic length Basin Slope = 0.0 % = 0 ftTime of conc. (Tc) Tc method $= 2.00 \, \text{min}$ = User Total precip. = 5.20 inDistribution = Type II Storm duration Shape factor = 484 = 24 hrs



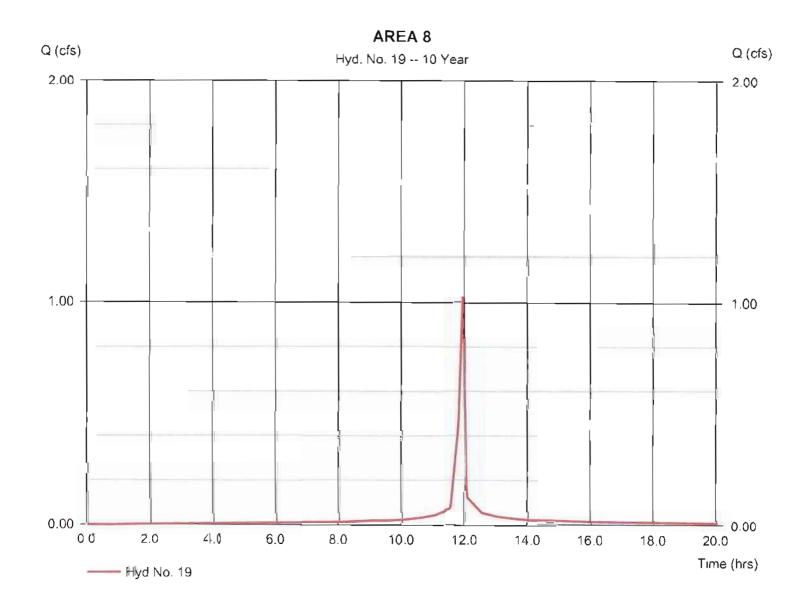
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SCS Runoff Peak discharge = 1.024 cfsStorm frequency = 10 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.050 acftDrainage area = 0.130 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 5.20 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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Tuesday, 00 29, 2012

Hyd. No. 20

<no description>

Hydrograph type Storm frequency Time interval

Inflow hyds.

= Combine

= 10 yrs = 1 min

= 12, 13, 14, 15, 16, 17

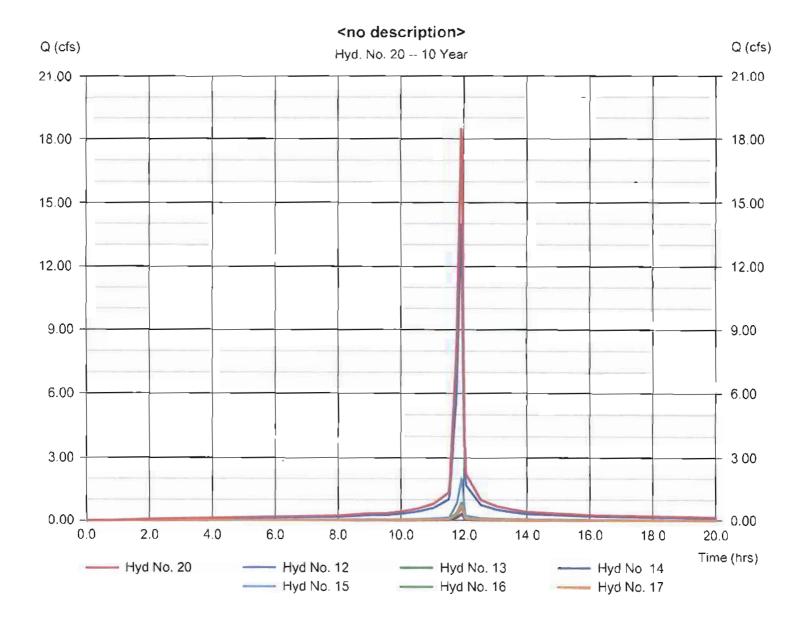
Peak discharge

= 18.50 cfs = 11.92 hrs

Time to peak Hyd. volume

= 0.911 acft

Contrib. drain, area = 2.350 ac



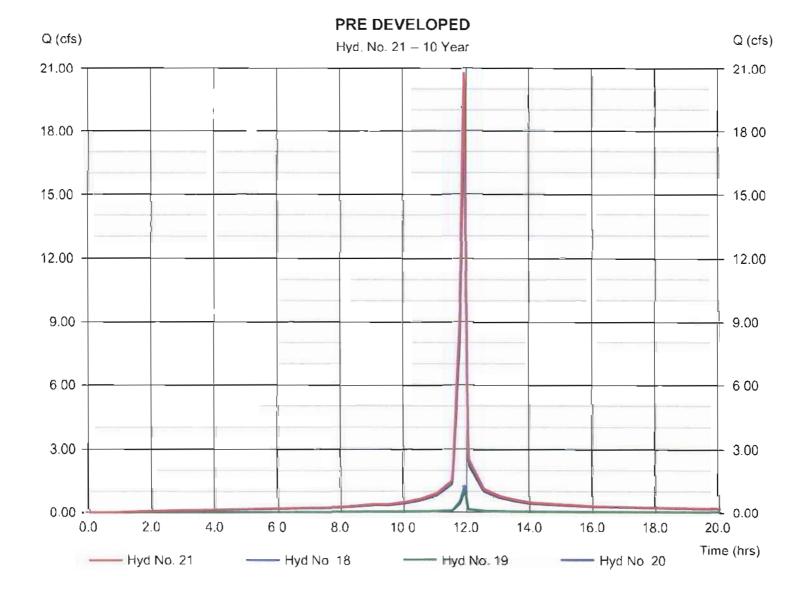
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine Storm frequency = 10 yrs Time interval = 1 min Inflow hyds. = 18, 19, 20 Peak discharge = 20.79 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.024 acft
Contrib. drain. area = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (mln)	Hyd. volume (acft)	inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	1.110	1	715	0.055				AREA A
2	SCS Runoff	3.422	1	715	0.169				AREA B
3	SCS Runoff	1.110	1	715	0 055				AREA C
4	SCS Runoff	3.237	1	715	0.160				AREA D
5	SCS Runoff	0 740	1	715	0.037				AREA E
6	SCS Runoff	2.312	1	715	0.114				AREA F
7	SCS Runoff	2 312	1	715	0.114				AREA G
8	SCS Runoff	1.757	1	715	0.087				AREA H
9	SCS Runoff	1.572	1	715	0 078		******		AREAI
10	Combine	11.93	1	715	0.591	1, 2, 3,			<no description=""></no>
11	Combine	17.57	1	715	0.870	4. 5, 6. 7, 8, 9,			Combined Post Developed
12	SCS Runoff	16.46	1	715	0.815	10		-	AREA 1
13	SCS Runoff	0.740	1	715	0 037				AREA 2
14	SCS Runoff	0 370	1	715	0.018				AREA 3
15	SCS Runoff	2.312	1	715	0.114				AREA 4
16	SCS Runoff	1.017	1	715	0.050				AREA 5
17	SCS Runoff	0.832	1	715	0.041				AREA 6
18	SCS Runoff	1.480	1	715	0 073				AREA 7
19	SCS Runoff	1.202	1	715	0.060				AREA 8
20	Combine	21.73	1	715	1 076	12, 13, 14,			<no description=""></no>
21	Combine	24.42	1	715	1.209	15, 16, 17, 18, 19, 20			PRE DEVELOPED
Hydraflow Central and Oliver 5.24 12.gpw					Return Period, 25 Year			Tuesday, 0	00 29, 2012

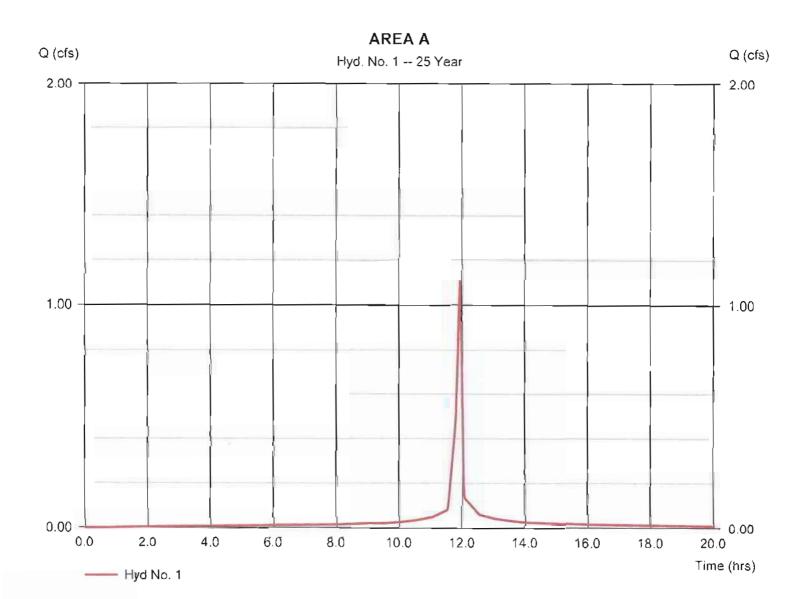
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge = 1.110 cfsTime to peak Storm frequency = 25 yrs $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.055 acft Drainage area = 0.120 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



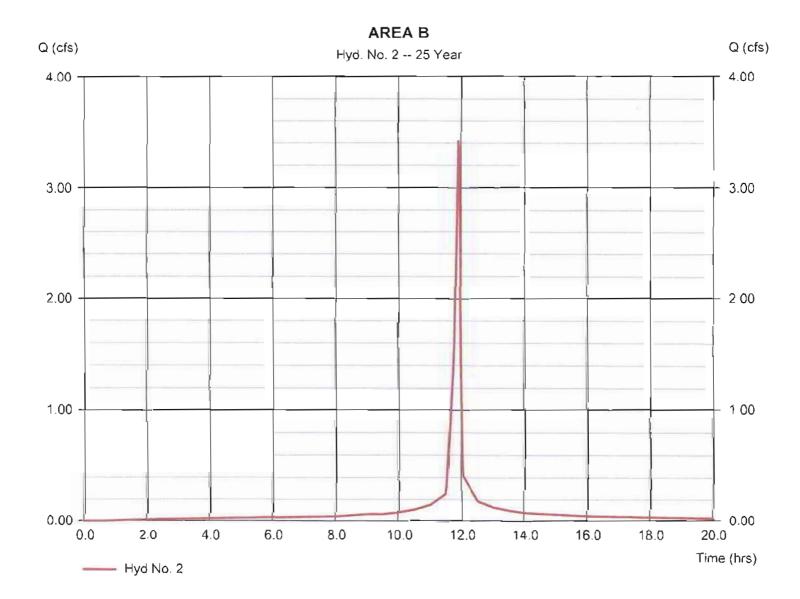
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

= SCS Runoff Peak discharge = 3.422 cfsHydrograph type Storm frequency Time to peak = 11.92 hrs= 25 yrsTime interval = 1 minHyd. volume = 0.169 acftCurve number = 98 Drainage area = 0.370 acHydraulic length Basin Slope = 0 ft= 0.0 %Time of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



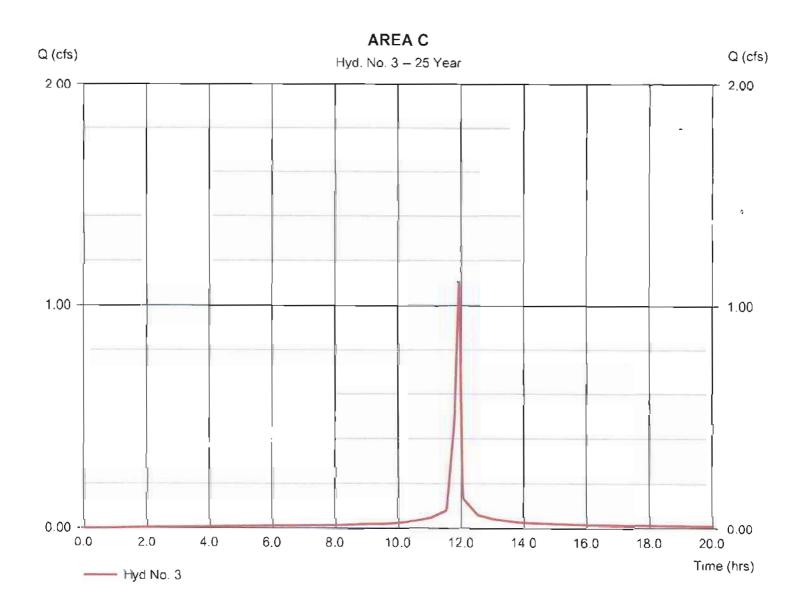
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 1.110 cfsStorm frequency = 25 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.055 acft Drainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



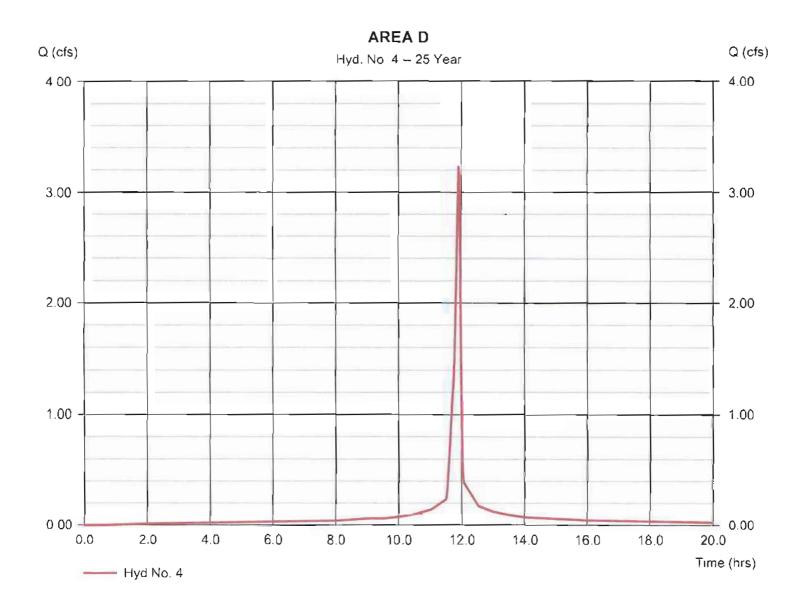
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday. 00 29, 2012

Hyd. No. 4

AREA D

= SCS Runoff Peak discharge = 3.237 cfsHydrograph type Time to peak $= 11.92 \, hrs$ Storm frequency = 25 yrsHyd. volume = 0.160 acft Time interval = 1 min = 0.350 acCurve number = 98 Drainage area Hydraulic length Basin Slope = 0.0 % = 0 ftTime of conc. (Tc) $= 1.70 \, \text{min}$ = TR55 Tc method Distribution = Type II Total precip. = 6.10 inStorm duration = 24 hrs Shape factor = 484



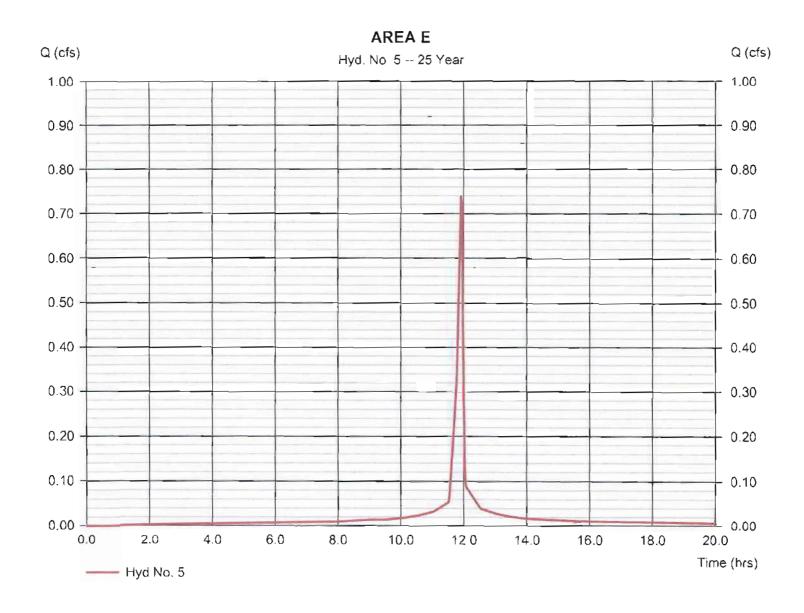
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.740 cfsTime to peak $= 11.92 \, hrs$ Storm frequency = 25 yrsHyd. volume = 0.037 acft Time interval $= 1 \min$ Drainage area = 0.080 acCurve number = 98= 0.0 %Hydraulic length = 0 ftBasin Slope Time of conc. (Tc) Tc method $= 2.00 \, \text{min}$ = User Total precip. = 6.10 inDistribution = Type II Shape factor = 484Storm duration = 24 hrs



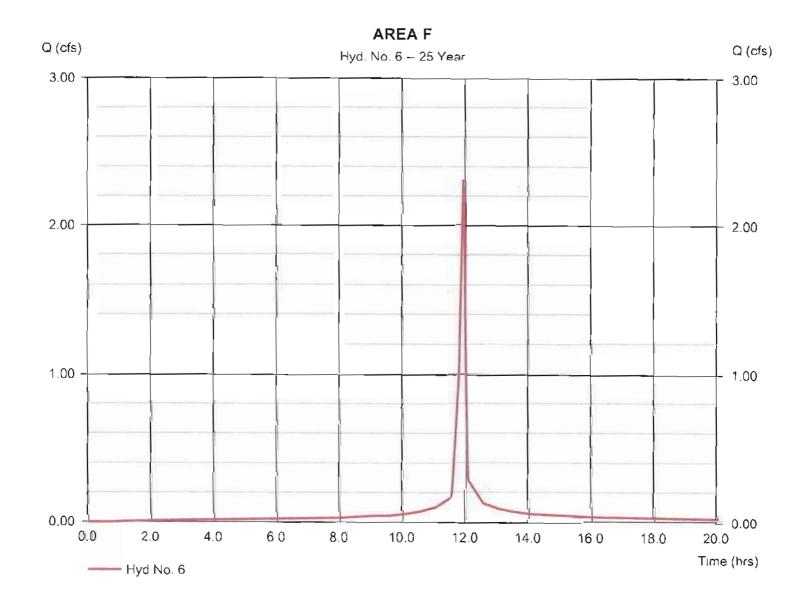
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 2.312 cfsStorm frequency = 25 yrs Time to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.114 acft Drainage area = 0.250 acCurve number = 98Hydraulic length Basin Slope = 0.0 % = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type !! Storm duration = 24 hrs Shape factor = 484



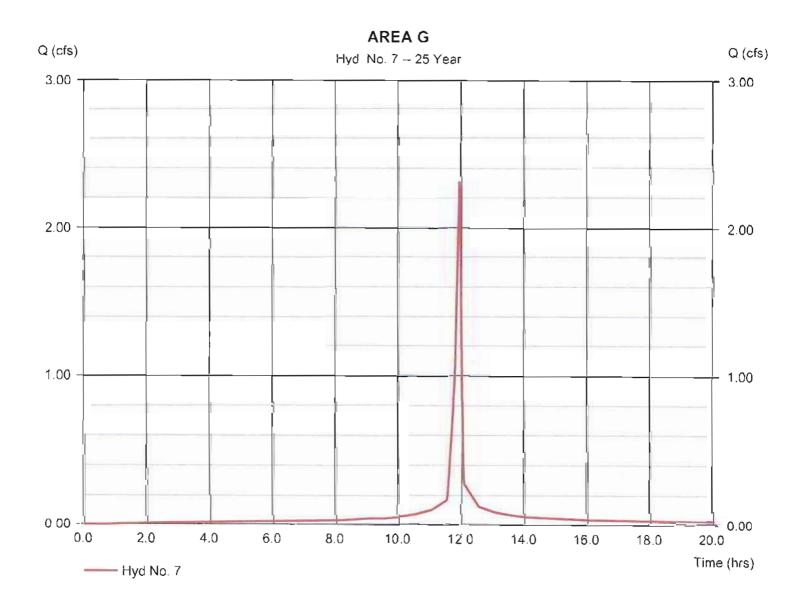
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff Peak discharge = 2.312 cfsStorm frequency = 25 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.114 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



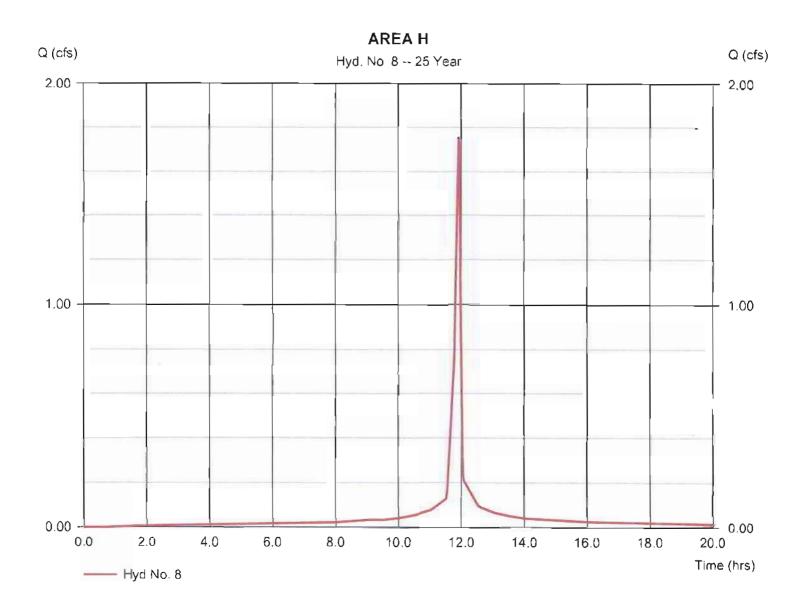
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

Hydrograph type = SCS Runoff Peak discharge = 1.757 cfsStorm frequency = 25 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.087 acft Curve number Drainage area = 0.190 ac= 98 = 0 ftBasin Slope = 0.0 % Hydraulic length Tc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



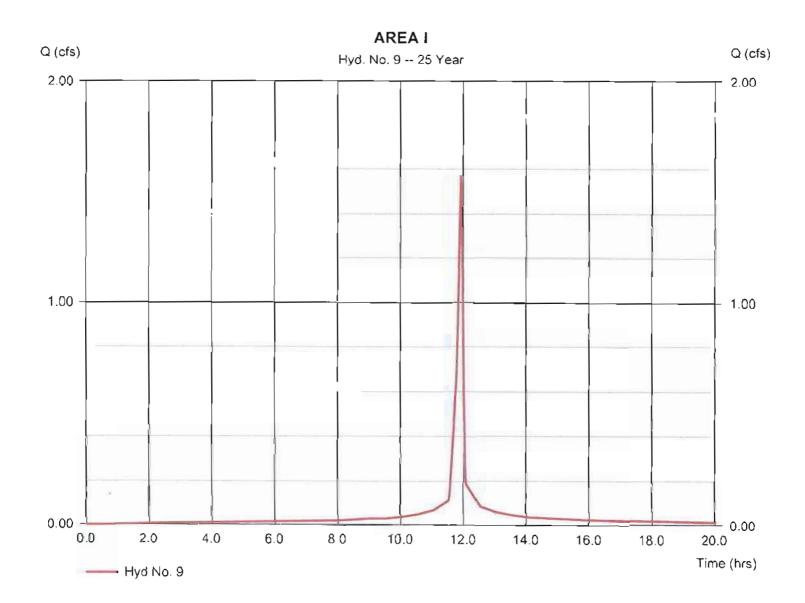
Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 9

AREA I

Peak discharge Hydrograph type = SCS Runoff = 1.572 cfsStorm frequency = 25 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.078 acft Drainage area = 0.170 acCurve number = 98Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

Hydrograph type Storm frequency Time interval

Inflow hyds.

= Combine

= 25 yrs

= 1 min = 1, 2, 3, 4, 5, 6 Peak discharge Time to peak

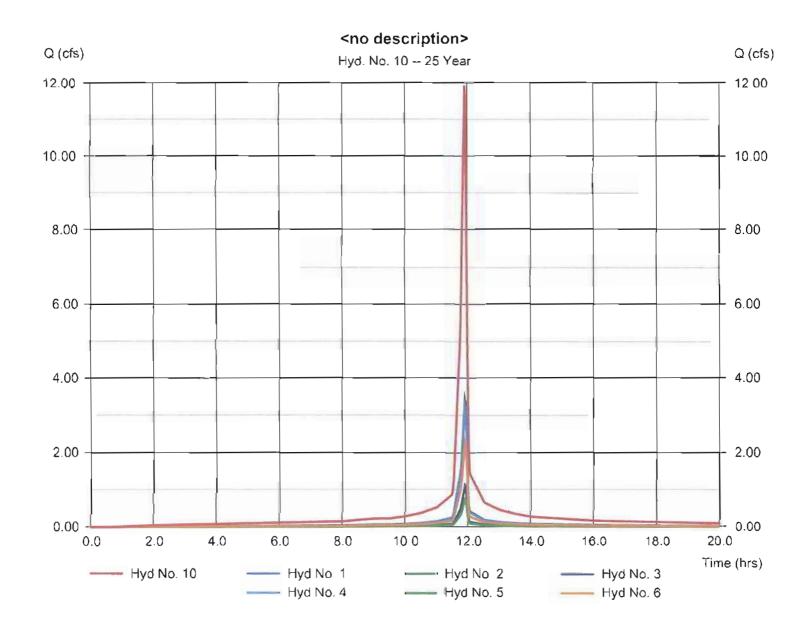
= 11.93 cfs = 11.92 hrs

Hyd. volume

= 0.591 acft

Contrib. drain, area =

= 1.290 ac



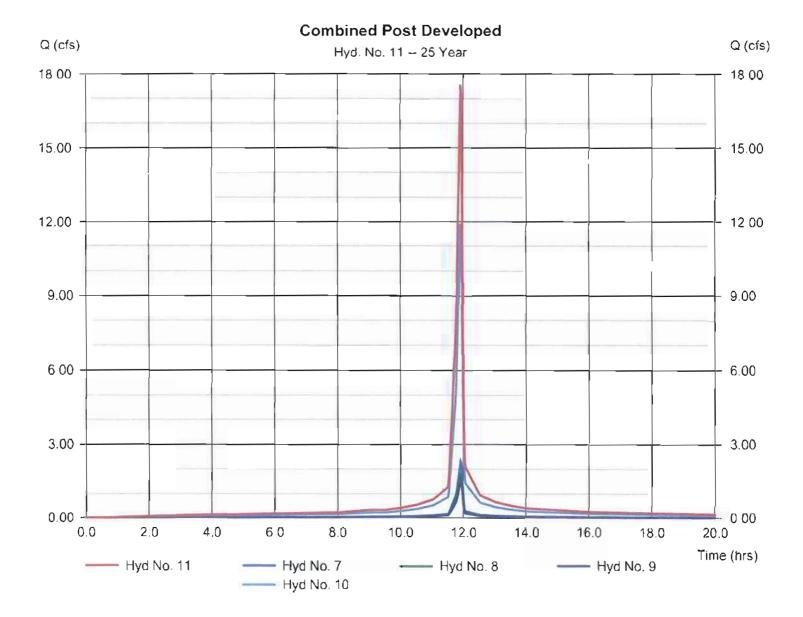
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 7, 8, 9, 10 Peak discharge = 17.57 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.870 acft
Contrib. drain. area = 0.610 ac



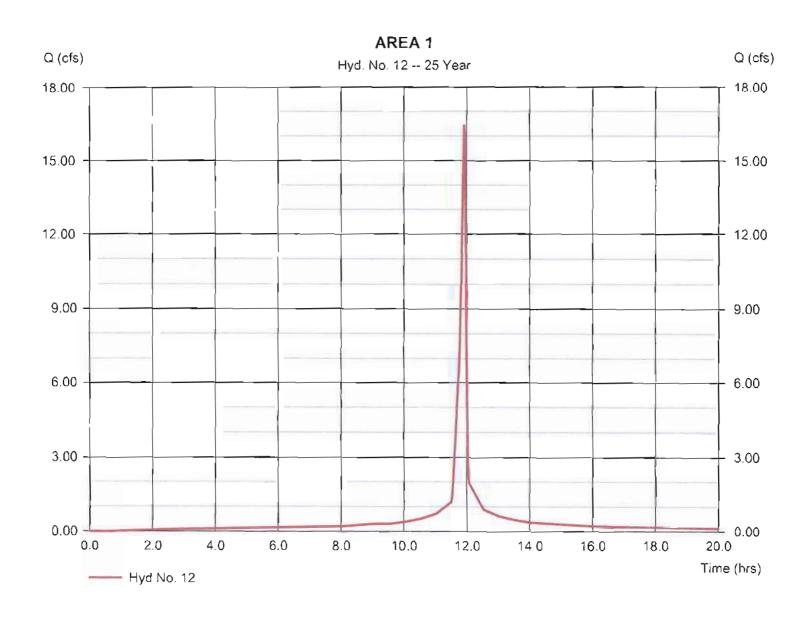
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 16.46 cfsTime to peak = 11.92 hrsStorm frequency = 25 yrsHyd. volume = 0.815 acft Time interval = 1 minDrainage area = 1.780 acCurve number = 98 Hydraulic length = 0.0 % = 0 ftBasin Slope Time of conc. (Tc) $= 3.00 \, \text{min}$ Tc method = TR55 Total precip. = 6.10 inDistribution = Type II Shape factor = 484 Storm duration = 24 hrs

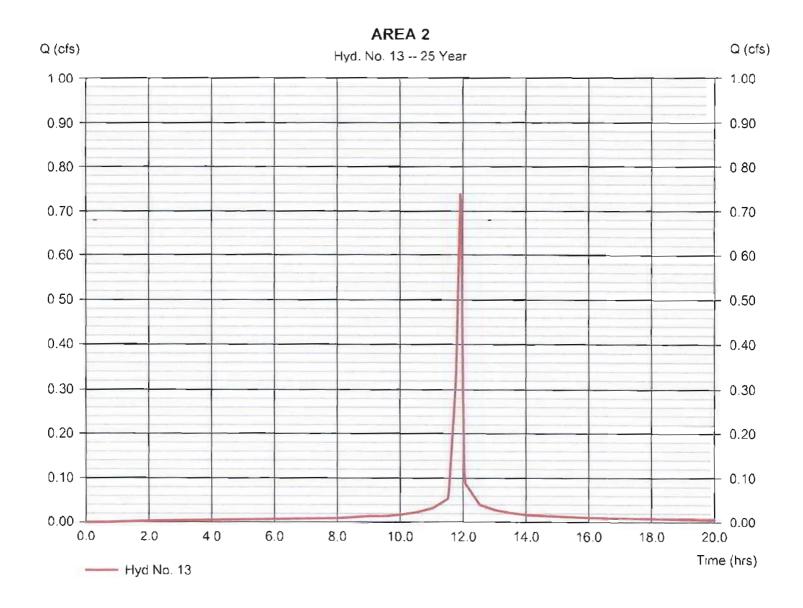


Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge = 0.740 cfsTime to peak Storm frequency = 25 yrs $= 11.92 \, hrs$ = 1 min Hyd. volume = 0.037 acft Time interval Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type !! Shape factor = 484 Storm duration = 24 hrs



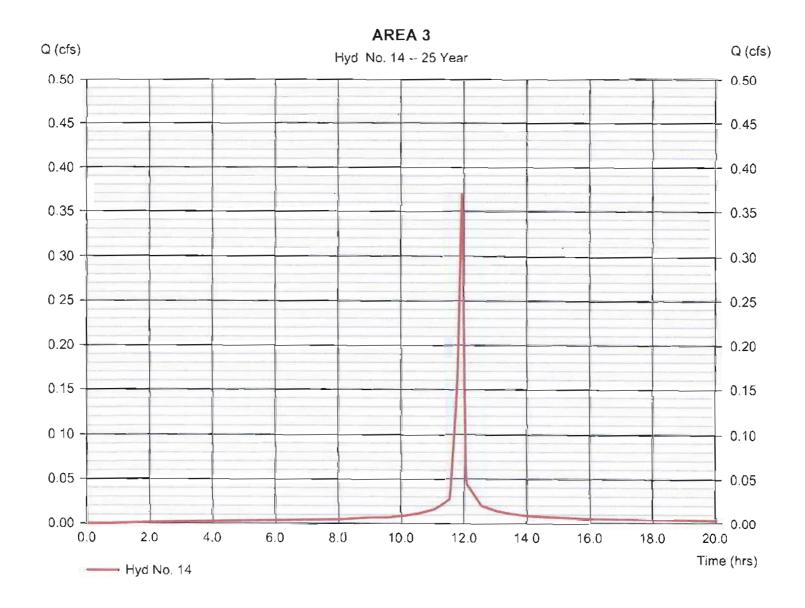
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Hydrograph type = SCS Runoff Peak discharge = 0.370 cfsTime to peak Storm frequency = 25 yrs $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.018 acft Drainage area = 0.040 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type !! Storm duration = 24 hrs Shape factor = 484



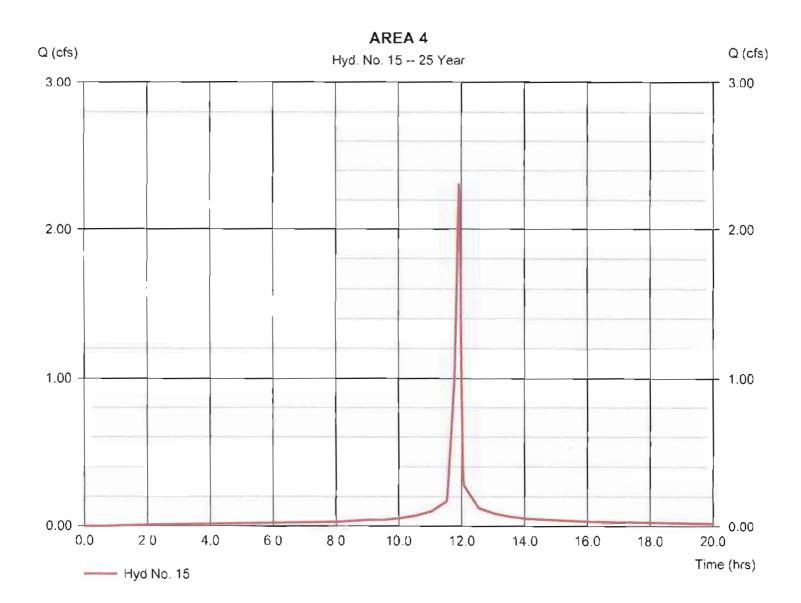
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

= SCS Runoff Hydrograph type Peak discharge = 2.312 cfsStorm frequency = 25 yrs Time to peak = 11.92 hrsTime interval $= 1 \, \text{min}$ Hyd. volume = 0.114 acftDrainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ = 6.10 inTotal precip. Distribution = Type II Storm duration = 24 hrs Shape factor = 484



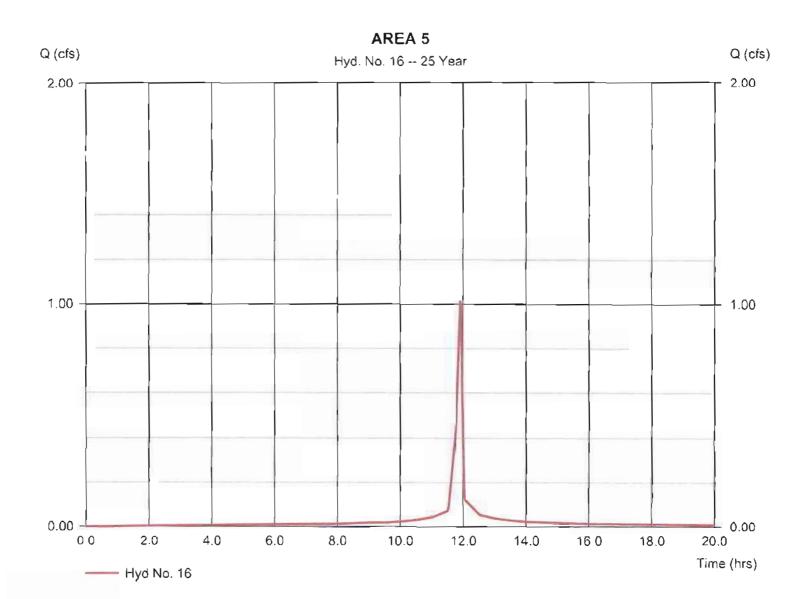
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

Hydrograph type = SCS Runoff Peak discharge \simeq 1.017 cfs Storm frequency Time to peak $= 11.92 \, hrs$ = 25 yrsHyd. volume = 0.050 acftTime interval = 1 minDrainage area = 0.110 acCurve number = 98 Hydraulic length = 0.0 % = 0 ftBasin Slope Time of conc. (Tc) $= 2.00 \, \text{min}$ Tc method = User Total precip. = 6.10 inDistribution = Type (I Storm duration Shape factor = 484 = 24 hrs



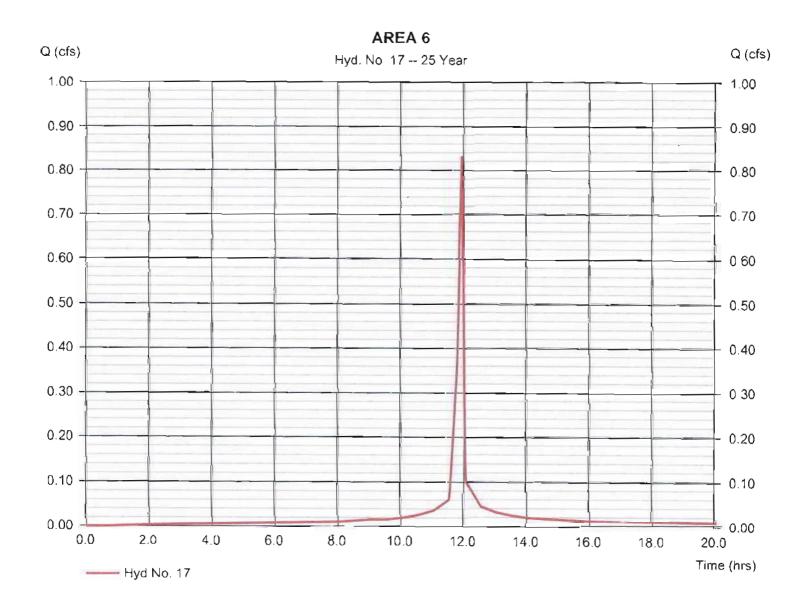
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 0.832 cfsStorm frequency = 25 yrs Time to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.041 acftDrainage area = 0.090 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.10 in= Type II Distribution Storm duration = 24 hrs Shape factor = 484



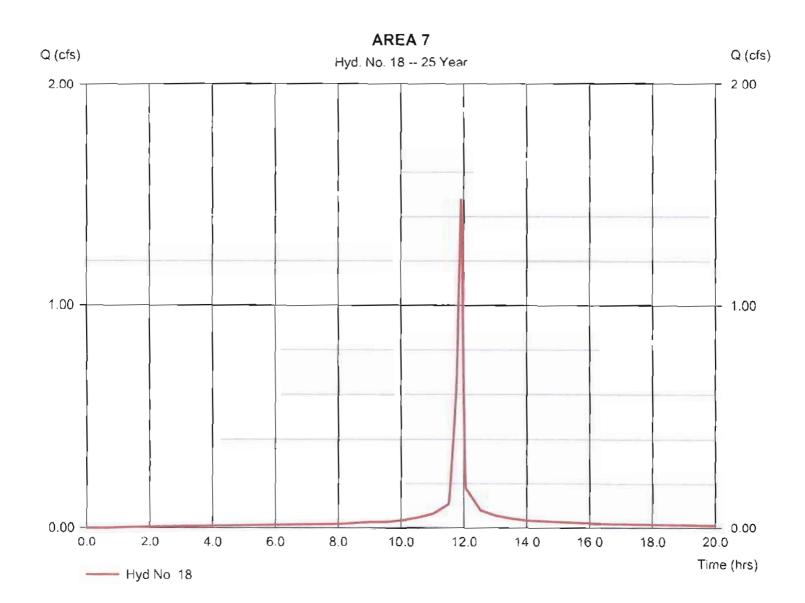
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff Peak discharge = 1.480 cfsTime to peak Storm frequency = 25 yrs $= 11.92 \, hrs$ Time interval Hyd. volume = 0.073 acft $= 1 \min$ Drainage area = 0.160 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method Time of conc. (Tc) $= 2.00 \, \text{min}$ = User Total precip. = 6.10 inDistribution ≈ Type II Storm duration Shape factor = 484= 24 hrs



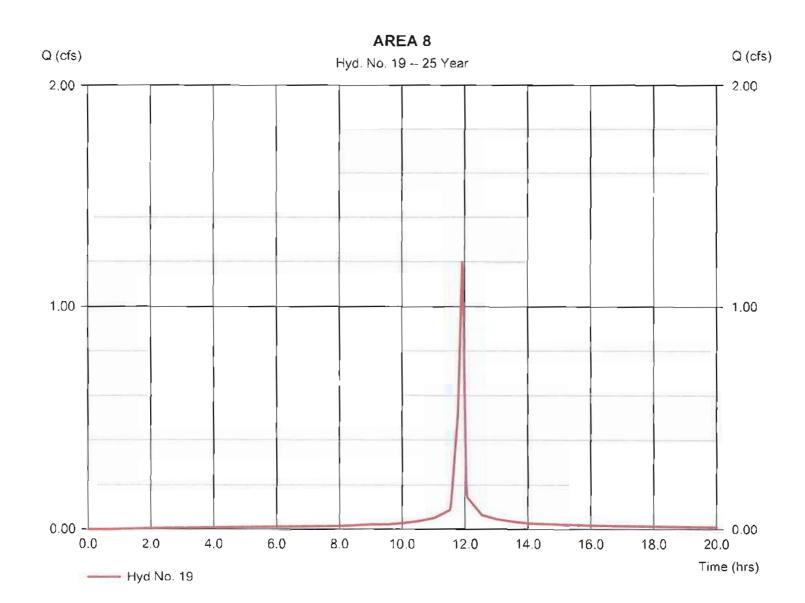
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SCS Runoff Peak discharge = 1.202 cfsStorm frequency = 25 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.060 acft Drainage area = 0.130 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.10 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday. 00 29, 2012

Hyd. No. 20

<no description>

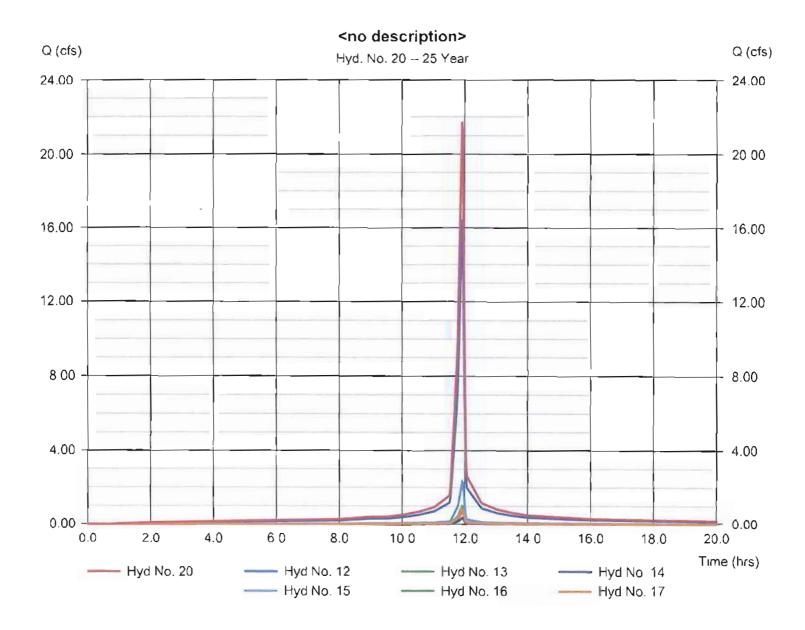
Hydrograph type Storm frequency Time interval = Combine = 25 yrs

Time interval = 1 min Inflow hyds. = 12, 13, 14, 15, 16, 17 Peak discharge

= 21.73 cfs

Time to peak = 11.92 hrs Hyd. volume = 1.076 acft

Contrib. drain. area = 2.350 ac



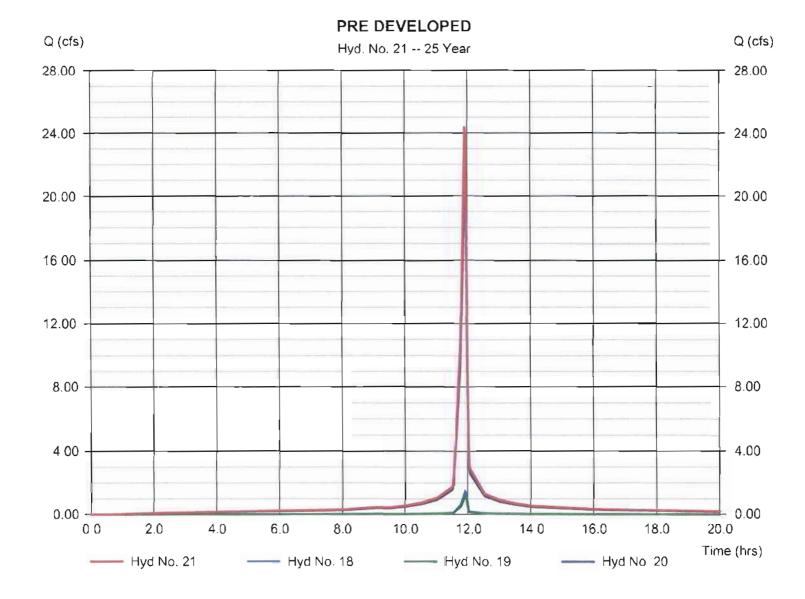
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine Storm frequency = 25 yrs Time interval = 1 min Inflow hyds. = 18, 19, 20 Peak discharge = 24.42 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.209 acft
Contrib. drain. area = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (mln)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	1.256	1	715	0 062			•	AREA A
2	SCS Runoff	3.873	1	715	0.193				AREA B
3	SCS Runoff	1 256	1	715	0.062				AREA C
4	SCS Runoff	3.664	1	715	0 182				AREA D
5	SCS Runoff	0.837	1	715	0 042				AREA E
6	SCS Runoff	2.617	1	715	0.130				AREA F
7	SCS Runoff	2.617	1	715	0.130				AREA G
8	SCS Runoff	1.989	1	715	0.099				AREA H
9	SCS Runoff	1.780	1	715	880.0				AREA I
10	Combine	13.50	1	715	0.671	1, 2, 3, 4, 5, 6.			<no description=""></no>
11	Combine	19.89	1	715	0 989	7, 8, 9,			Combined Post Developed
12	SCS Runoff	18.63	1	715	0 926				AREA 1
13	SCS Runoff	0.837	1	715	0.042				AREA 2
14	SCS Runoff	0.419	1	715	0.021				AREA 3
15	SCS Runoff	2 617	1	715	0 130				AREA 4
16	SCS Runoff	1 152	1	715	0.057			-	AREA 5
17	SCS Runoff	0.942	1	715	0.047				AREA 6
18	SCS Runoff	1 675	1	715	0.083				AREA 7
19	SCS Runoff	1.361	1	715	0.068				AREA 8
20	Combine	24.60	1	715	1 223	12, 13, 14, 15, 16, 17,			<no description=""></no>
21	Combine	27.64	1	715	1.374	18, 19, 20			PRE DEVELOPED
Hydraflow Central and Oliver 5.24.12.gpw					Return Period: 50 Year			Tuesday, 0	00 29, 2012

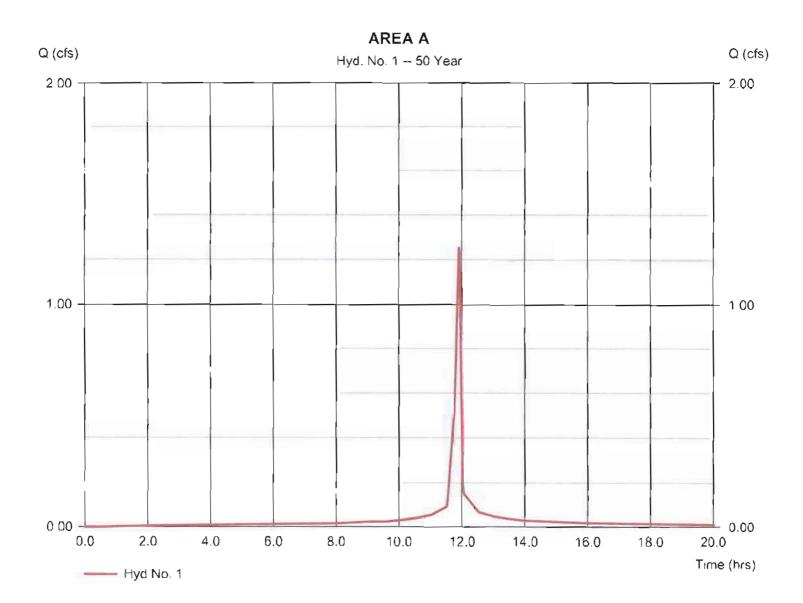
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

= SCS Runoff Peak discharge Hydrograph type = 1.256 cfsTime to peak Storm frequency = 50 yrs $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.062 acftCurve number = 98 Drainage area = 0.120 acHydraulic length Basin Slope = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Distribution Total precip. = 6.90 in= Type !! Storm duration = 24 hrsShape factor = 484



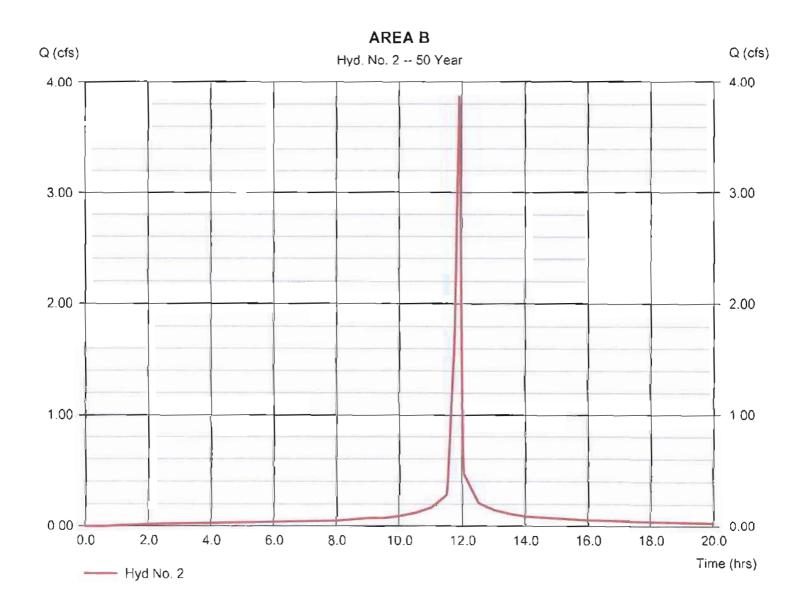
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = SCS Runoff Peak discharge = 3.873 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.193 acft Drainage area Curve number = 0.370 ac= 98 Basin Slope = 0.0 % Hydraulic length f = 0Time of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration Shape factor = 484= 24 hrs

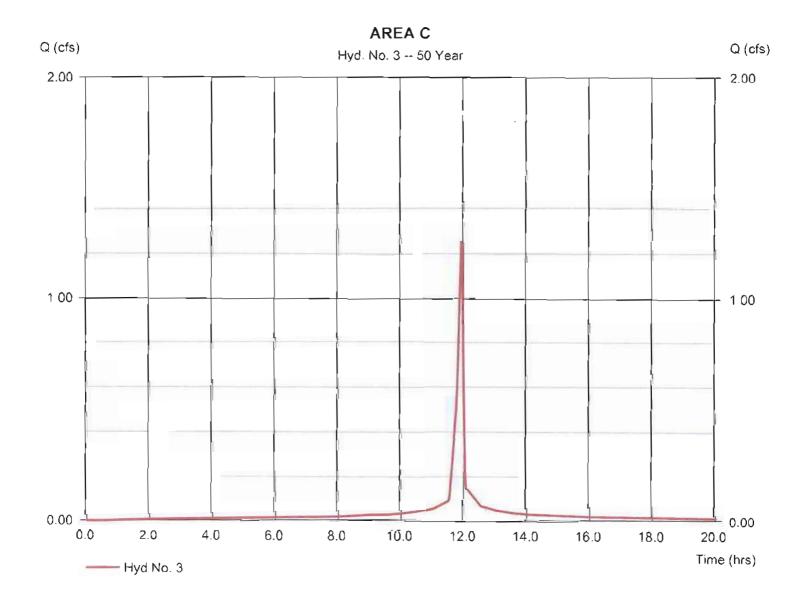


Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 1.256 cfsStorm frequency Time to peak = 50 yrs= 11.92 hrsTime interval = 1 minHyd. volume = 0.062 acft Drainage area = 0.120 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTc method $= 2.00 \, min$ = User Time of conc. (Tc) Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



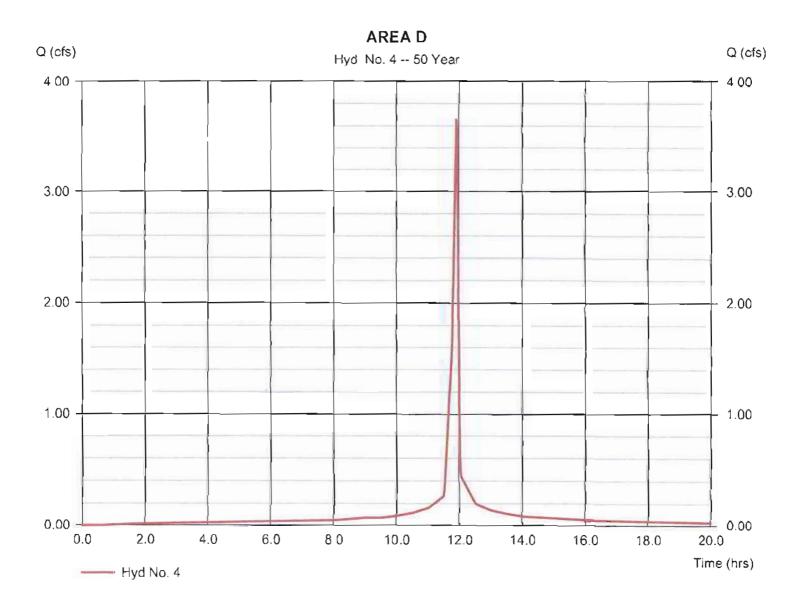
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

Hydrograph type = SCS Runoff Peak discharge = 3.664 cfsStorm frequency Time to peak = 50 yrs= 11.92 hrsTime interval Hyd. volume = 1 min = 0.182 acftDrainage area = 0.350 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTime of conc. (Tc) Tc method = TR55 $= 1.70 \, \text{min}$ Total precip. Distribution = 6.90 in= Type II Storm duration = 24 hrs Shape factor = 484



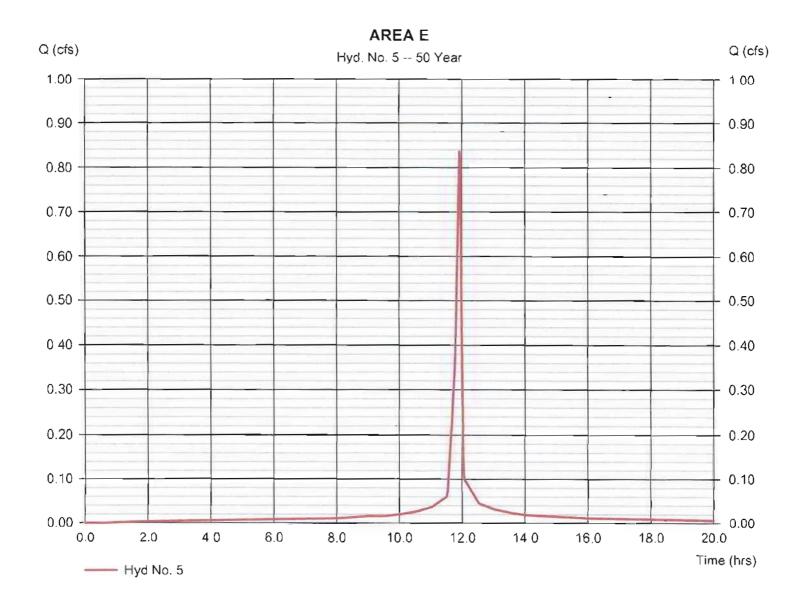
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

ARËA E

Hydrograph type = SCS Runoff Peak discharge = 0.837 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.042 acft Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



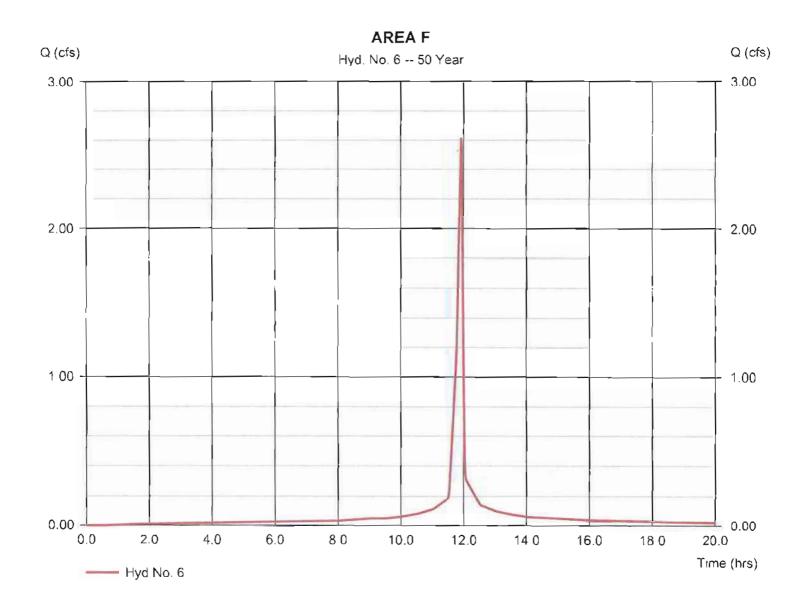
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 2.617 cfsTime to peak = 11.92 hrsStorm frequency = 50 yrsHyd. volume = 0.130 acftTime interval = 1 min = 0.250 acCurve number = 98 Drainage area Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) $= 2.00 \, \text{min}$ Tc method = User Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



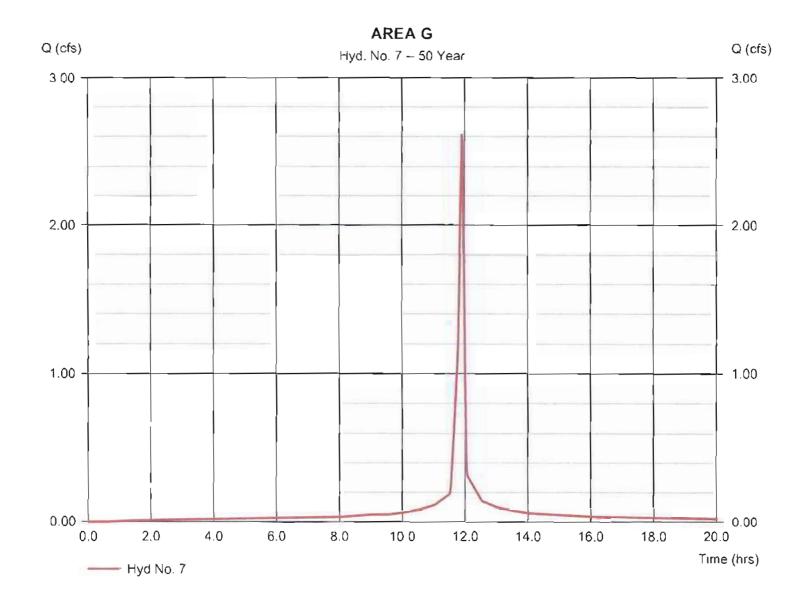
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

= SCS Runoff Hydrograph type Peak discharge = 2.617 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.130 acftDrainage area Curve number = 0.250 ac= 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484

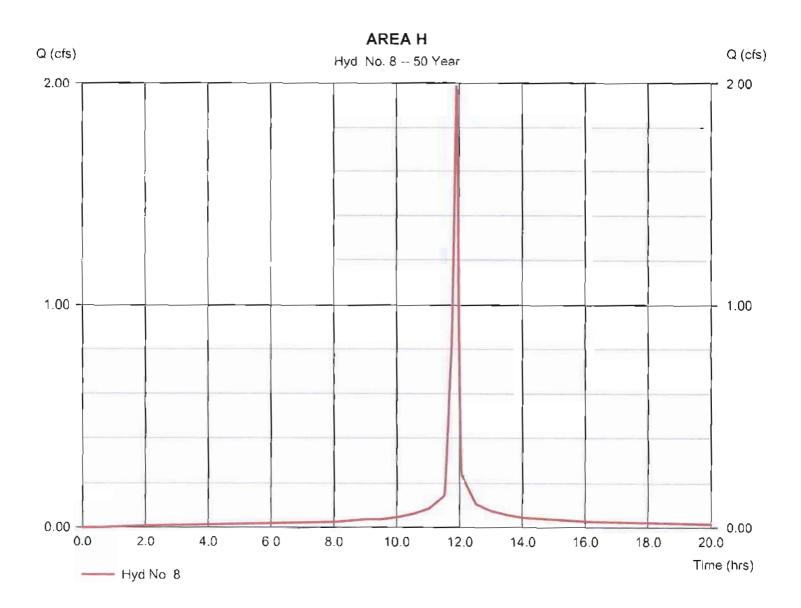


Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

Hydrograph type = SCS Runoff Peak discharge = 1.989 cfsStorm frequency Time to peak = 50 yrs= 11.92 hrsTime interval Hyd. volume = 1 min= 0.099 acftDrainage area = 0.190 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. \approx 6.90 in Distribution = Type II Storm duration = 24 hrs Shape factor = 484

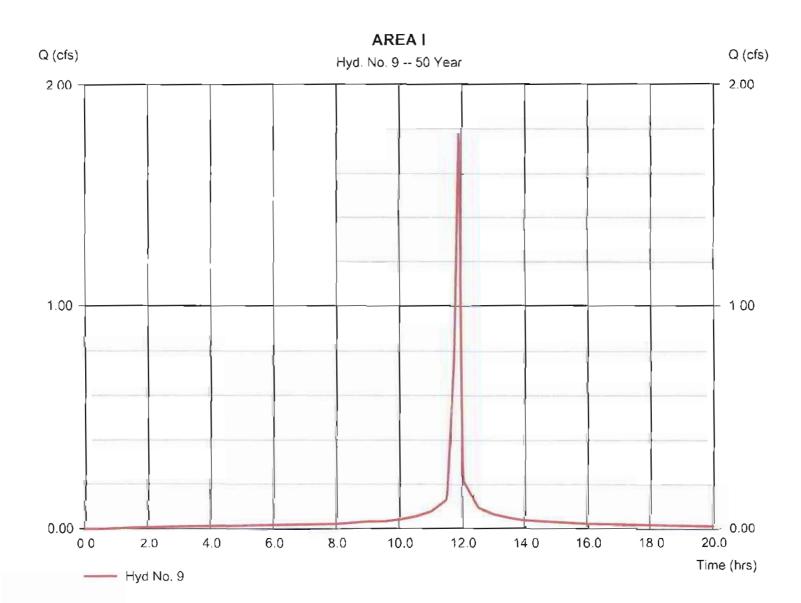


Tuesday, 00 29. 2012

Hyd. No. 9

AREA I

Hydrograph type = SCS Runoff Peak discharge = 1.780 cfsTime to peak = 11.92 hrsStorm frequency = 50 yrsHyd. volume = 0.088 acft Time interval = 1 minDrainage area = 0.170 acCurve number = 98 = 0.0 % Hydraulic length = 0 ftBasin Slope Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Tc method = 6.90 inDistribution = Type II Total precip. = 484 Storm duration = 24 hrs Shape factor



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29. 2012

Hyd. No. 10

<no description>

Hydrograph type Storm frequency Time interval

Inflow hyds.

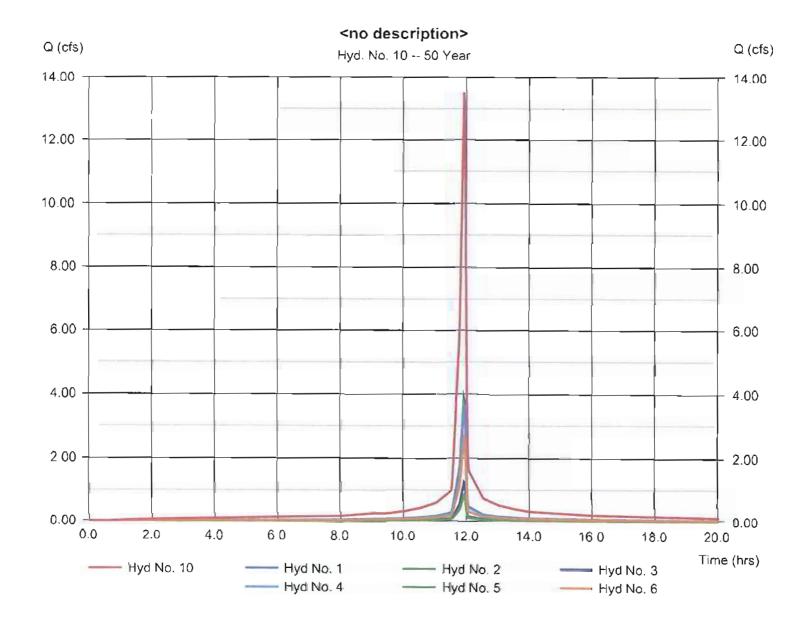
= Combine = 50 yrs

= 1 min = 1, 2, 3, 4, 5, 6 Peak discharge Time to peak = 13.50 cfs = 11.92 hrs

Hyd. volume

= 0.671 acft

Contrib. drain. area = 1.290 aç

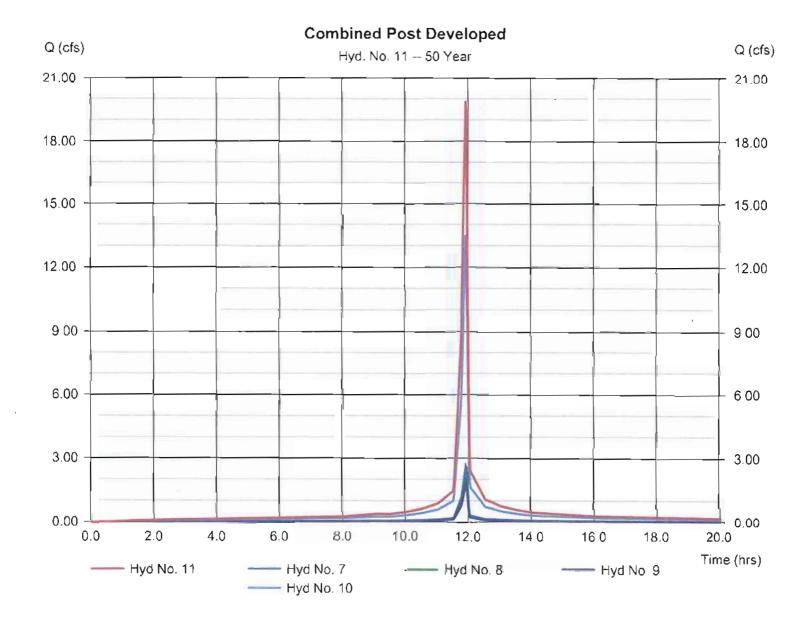


Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 7, 8, 9, 10 Peak discharge = 19.89 cfs
Time to peak = 11.92 hrs
Hyd. volume = 0.989 acft
Contrib. drain. area = 0.610 ac



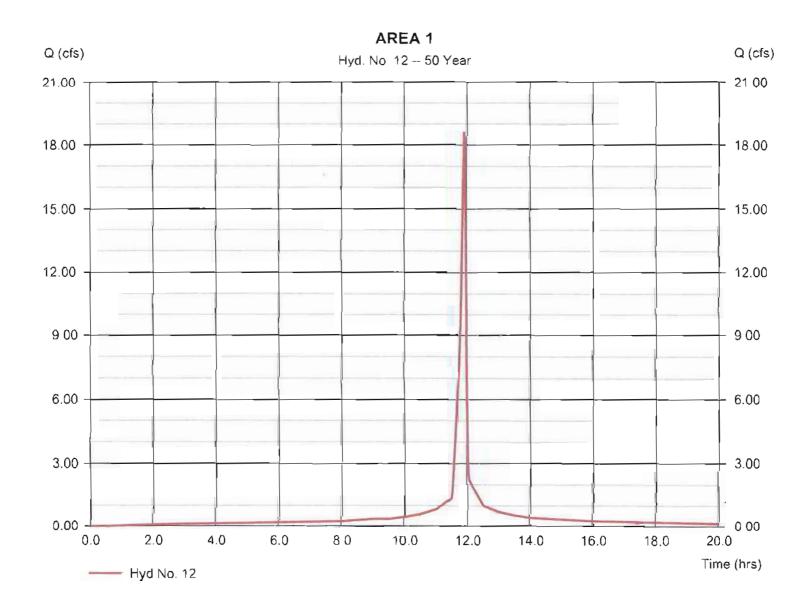
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

Hydrograph type = SCS Runoff Peak discharge = 18.63 cfsTime to peak = 11.92 hrsStorm frequency = 50 yrsHyd. volume = 0.926 acft Time interval = 1 min Drainage area = 1.780 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTime of conc. (Tc) $= 3.00 \, \text{min}$ Tc method = TR55 Total precip. = 6.90 inDistribution = Type II Shape factor = 484 Storm duration = 24 hrs



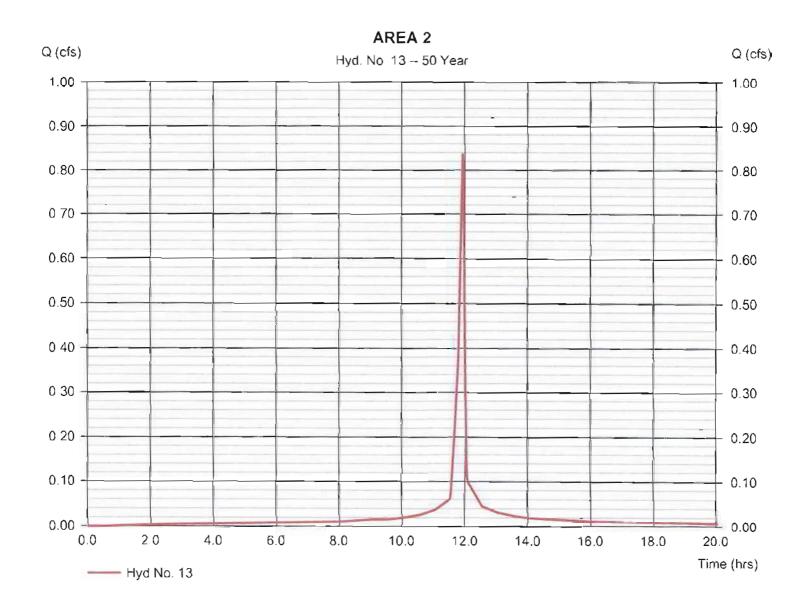
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge = 0.837 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.042 acftDrainage area Curve number = 0.080 ac= 98Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



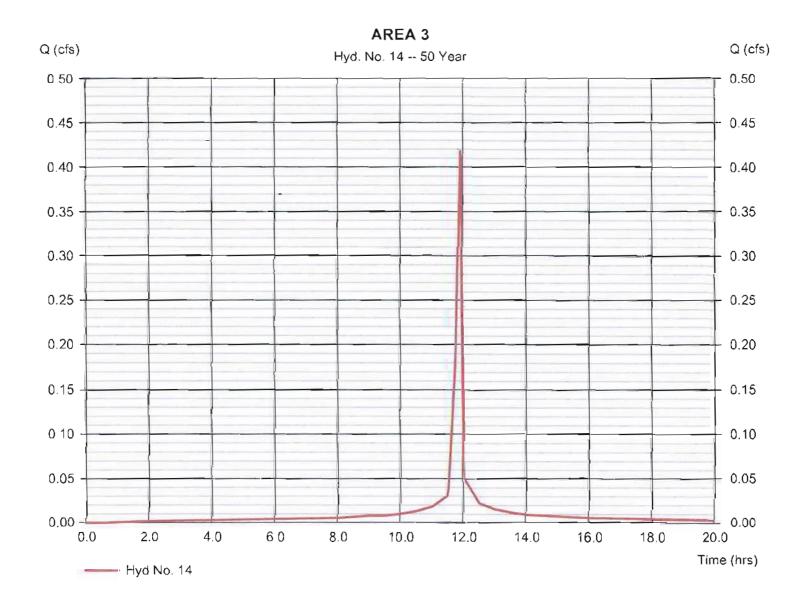
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

= SCS Runoff Peak discharge = 0.419 cfsHydrograph type Time to peak Storm frequency = 11.92 hrs = 50 yrsTime interval = 1 min Hyd. volume = 0.021 acftCurve number = 0.040 ac= 98 Drainage area Hydraulic length = 0.0 %= 0 ftBasin Slope Time of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Distribution Total precip. = 6.90 in= Type (i Storm duration = 24 hrs Shape factor = 484

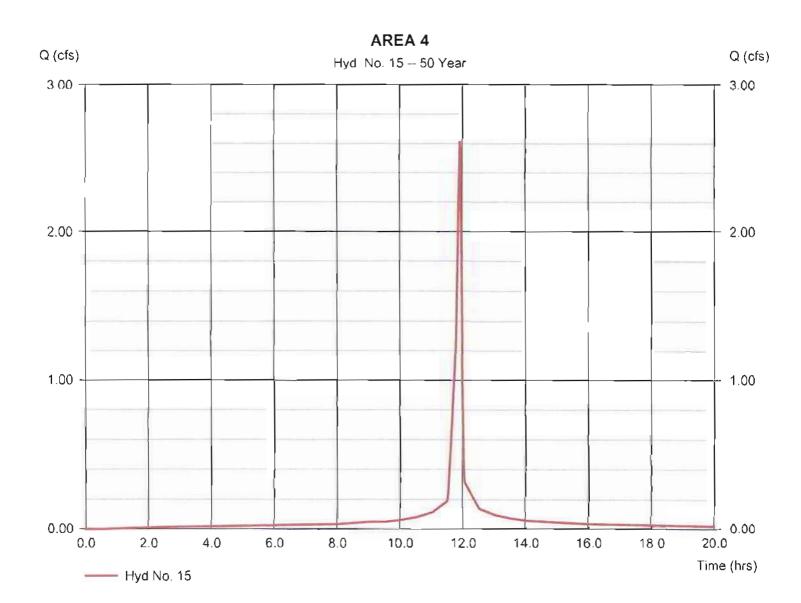


Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Hydrograph type = SCS Runoff Peak discharge = 2.617 cfsStorm frequency Time to peak = 50 yrs = 11.92 hrsTime interval = 1 min Hyd. volume = 0.130 acftDrainage area = 0.250 acCurve number = 98 = 0.0 %Basin Slope Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrsShape factor = 484



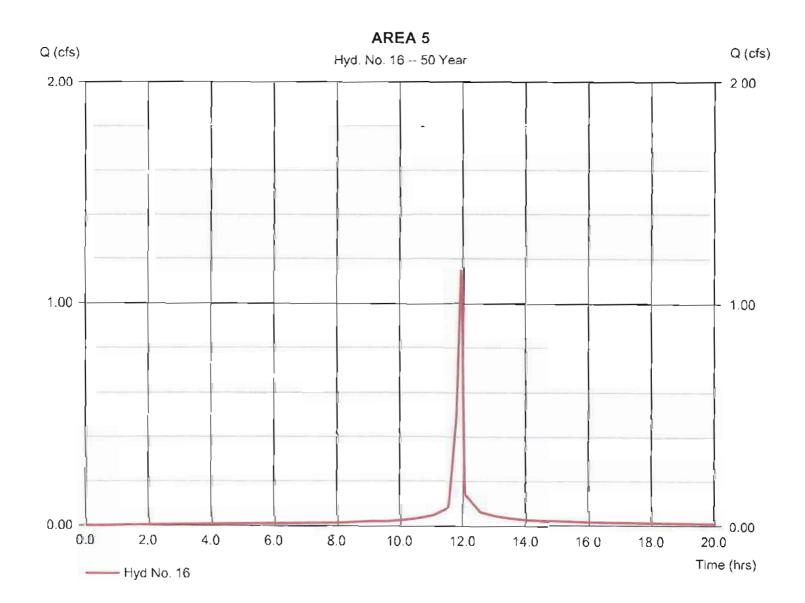
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 16

AREA 5

= SCS Runoff Hydrograph type Peak discharge = 1.152 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.057 acftCurve number Drainage area = 0.110 ac= 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.90 in= Type II Distribution Storm duration = 24 hrs Shape factor = 484



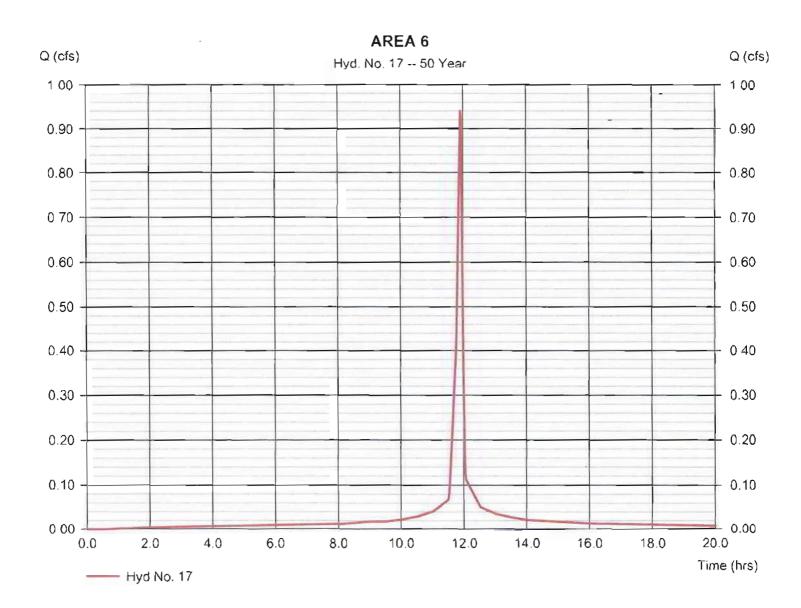
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

= SCS Runoff Hydrograph type Peak discharge = 0.942 cfsStorm frequency = 50 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.047 acft= 0.090 acCurve number Drainage area = 98= 0.0 % Basin Slope Hydraulic length = 0 ftTc method Time of conc. (Tc) $= 2.00 \, \text{min}$ = User = Type II Total precip. = 6.90 inDistribution Storm duration Shape factor = 484 = 24 hrs

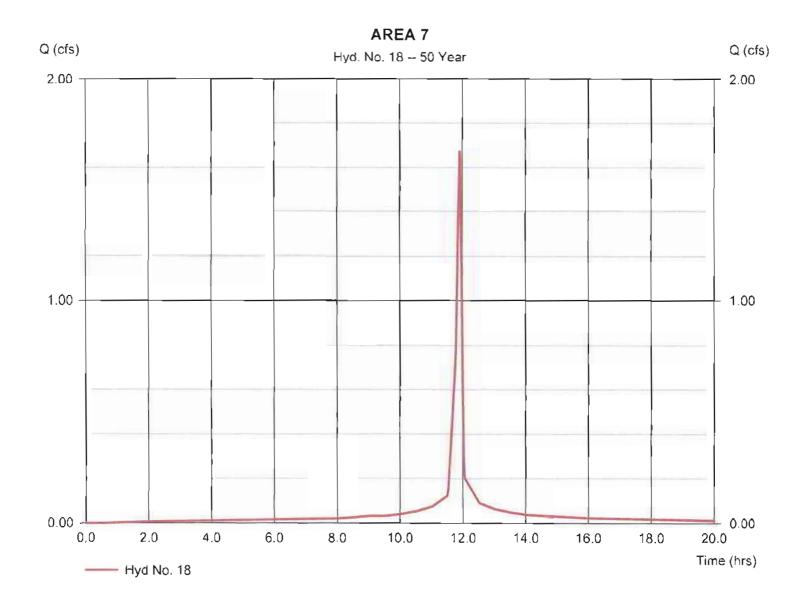


Tuesday, 00 29. 2012

Hyd. No. 18

AREA 7

= SCS Runoff Hydrograph type Peak discharge = 1.675 cfsStorm frequency = 50 vrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.083 acft Drainage area = 0.160 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484

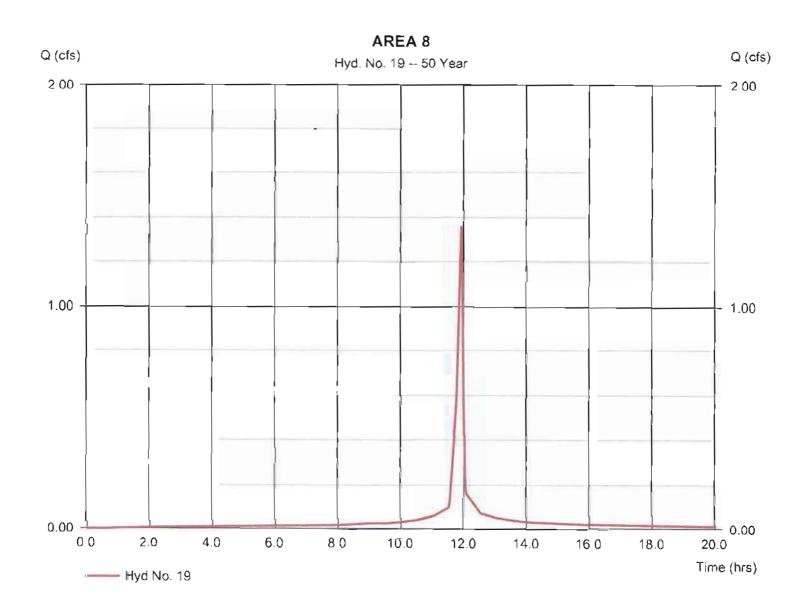


Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

Hydrograph type = SCS Runoff Peak discharge = 1.361 cfsStorm frequency Time to peak $= 11.92 \, hrs$ = 50 yrsTime interval Hyd. volume $= 1 \min$ = 0.068 acft Drainage area = 0.130 acCurve number = 98 Hydraulic length Basin Slope = 0.0 %= 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 6.90 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9

Tuesday. 00 29. 2012

Hyd. No. 20

<no description>

Hydrograph type Storm frequency Time interval

Inflow hyds.

= Combine = 50 yrs

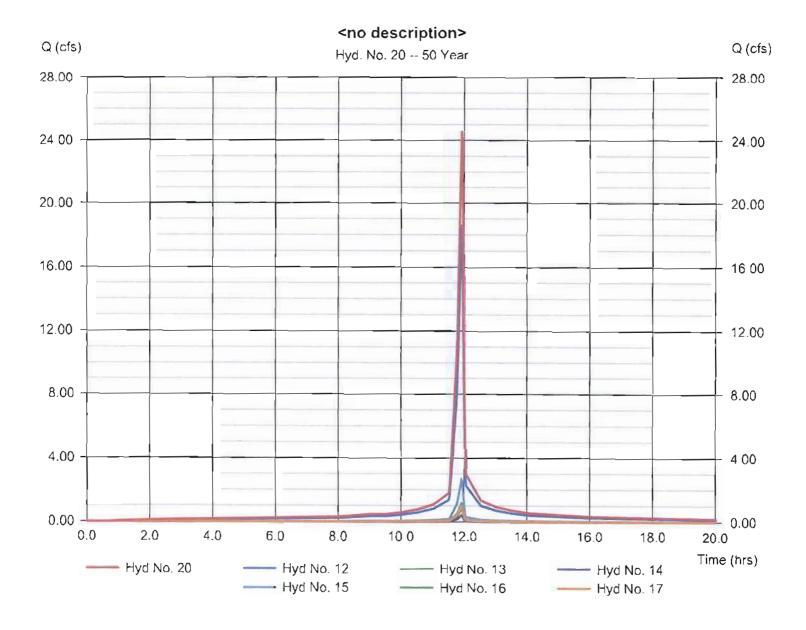
= 1 min

= 12, 13, 14, 15, 16, 17

Peak discharge Time to peak = 24.60 cfs = 11.92 hrs

Hyd. volume = 1.223 acft

Contrib. drain. area = 2.350 ac



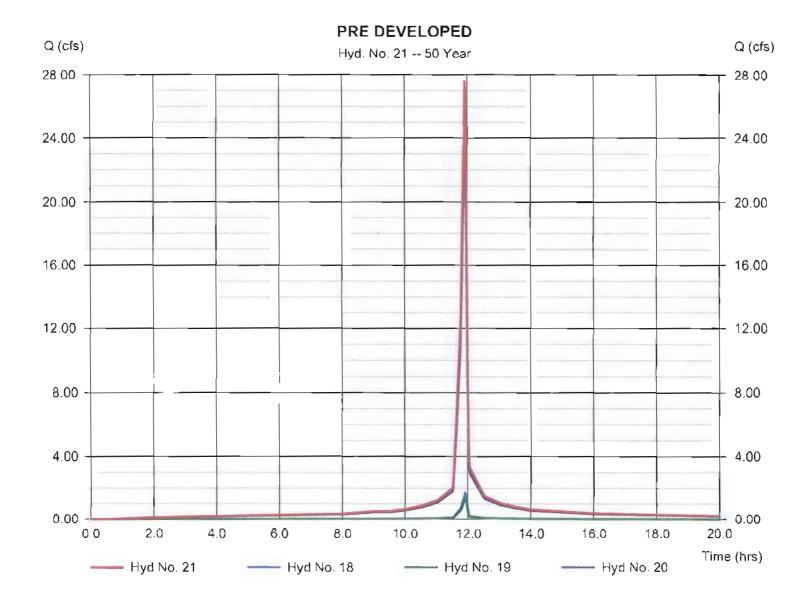
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine Storm frequency = 50 yrs Time interval = 1 min Inflow hyds. = 18, 19, 20 Peak discharge = 27.64 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.374 acft
Contrib. drain. area = 0.290 ac



Hydrograph Summary Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9 $\,$

Hyd. No.	Hydrograph type (origin)	Peak flow (cfs)	Time interval (min)	Time to Peak (mln)	Hyd. volume (acft)	Inflow hyd(s)	Maximum elevation (ft)	Total strge used (acft)	Hydrograph Description
1	SCS Runoff	1 421	1	715	0.071		AAARdii		AREA A
2	SCS Runoff	4.381	1	715	0.219				AREA 8
3	SCS Runoff	1.421	1	715	0.071				AREA C
4	SCS Runoff	4,144	1	715	0 207				AREA D
5	SCS Runoff	0.947	1	715	0.047				AREA E
6	SCS Runoff	2.960	1	715	0.148				AREA F
7	SCS Runoff	2 960	1	715	0.148				AREA G
8	SCS Runoff	2.250	1	715	0 112				AREA H
9	SCS Runoff	2.013	1	715	0 100				AREA I
10	Combine	15.27	1	715	0.762	1, 2, 3,			<no description=""></no>
11	Сотыпа	22 50	1	715	1.122	4, 5, 6, 7, 8, 9,			Combined Post Developed
12	SCS Runoff	21.08	1	715	1.051	10			AREA 1
13	SCS Runoff	0.947	1	715	0.047				AREA 2
14	SCS Runoff	0.474	1	715	0.024				AREA 3
15	SCS Runoff	2 960	1	715	0.148				AREA 4
16	SCS Runoff	1.302	1	715	0 065				AREA 5
17	SCS Runoff	1.066	1	715	0.053				AREA 6
18	SCS Runoff	1 895	1	715	0.095				AREA 7
19	SCS Runoff	1.539	1	715	0.077				AREA 8
20	Combine	27 83	1	715	1.388	12, 13, 14,			<no description=""></no>
21	Combine	31.26	1	715	1.559	15, 16, 17, 18, 19, 20			PRE DEVELOPED
Нус	draflow Centr	ral and OI	 iver 5.24	.12 gpw	Return	Period: 100	Year	Tuesday, (00 29, 2012

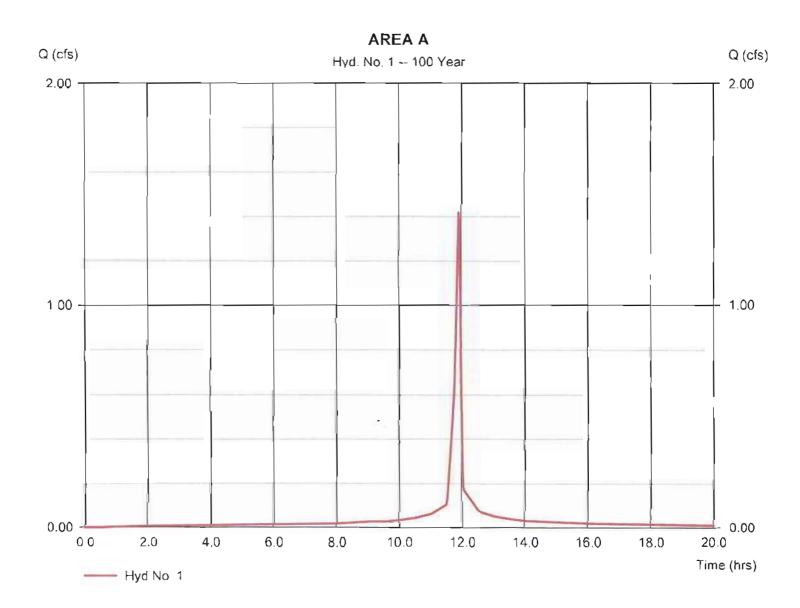
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 1

AREA A

Hydrograph type = SCS Runoff Peak discharge = 1.421 cfsStorm frequency = 100 yrs Time to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.071 acft Drainage area = 0.120 acCurve number = 98 = 0.0 % Basin Slope Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs= 484 Shape factor



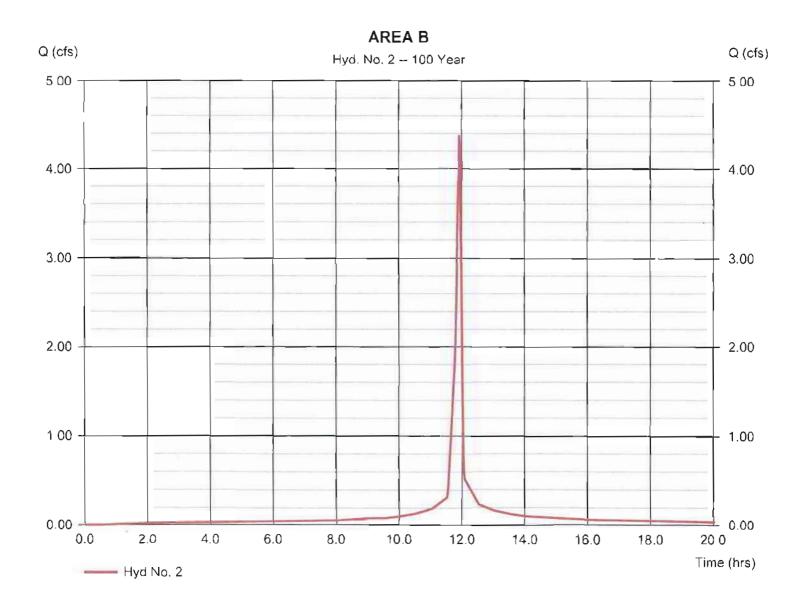
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 2

AREA B

Hydrograph type = SCS Runoff Peak discharge = 4.381 cfsStorm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.219 acftDrainage area = 0.370 acCurve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



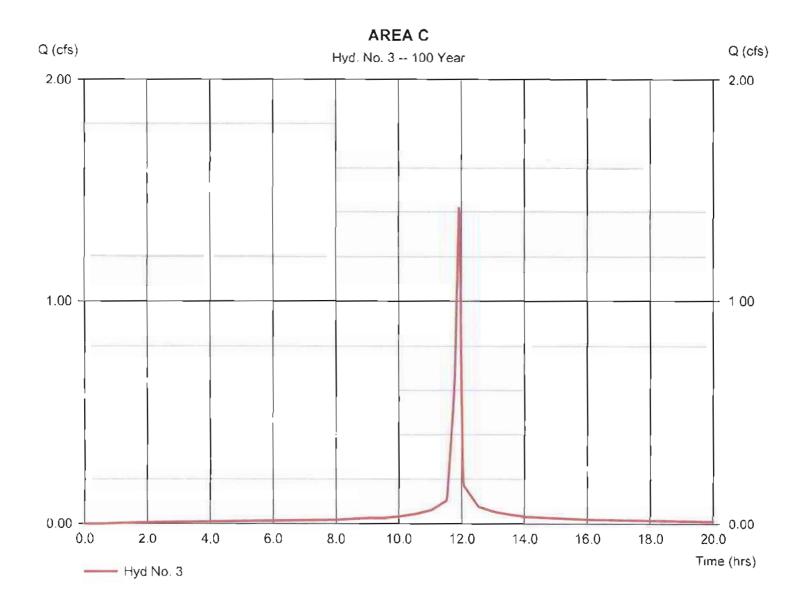
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 3

AREA C

Hydrograph type = SCS Runoff Peak discharge = 1.421 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrs= 1 min Time interval Hyd. volume = 0.071 acftDrainage area = 0.120 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



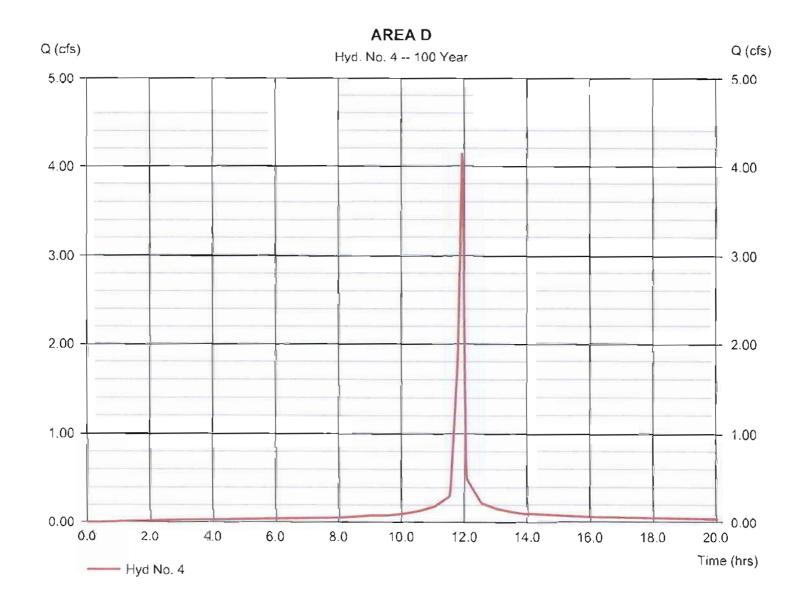
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Tuesday, 00 29, 2012

Hyd. No. 4

AREA D

= SCS Runoff Hydrograph type Peak discharge = 4.144 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.207 acftDrainage area = 0.350 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = TR55 Time of conc. (Tc) $= 1.70 \, \text{min}$ Total precip. = 7.80 inDistribution = Type !! Storm duration = 24 hrsShape factor = 484



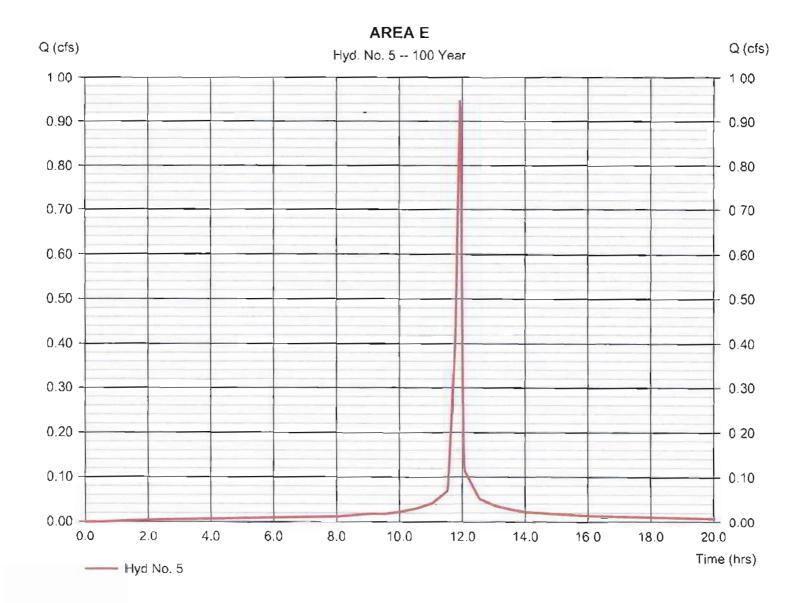
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 5

AREA E

Hydrograph type = SCS Runoff Peak discharge = 0.947 cfsStorm frequency Time to peak $= 11.92 \, hrs$ = 100 yrs= 0.047 acftTime interval Hyd. volume $= 1 \, \text{min}$ Drainage area = 0.080 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ = 7.80 inTotal precip. Distribution = Type II Storm duration = 24 hrs Shape factor = 484



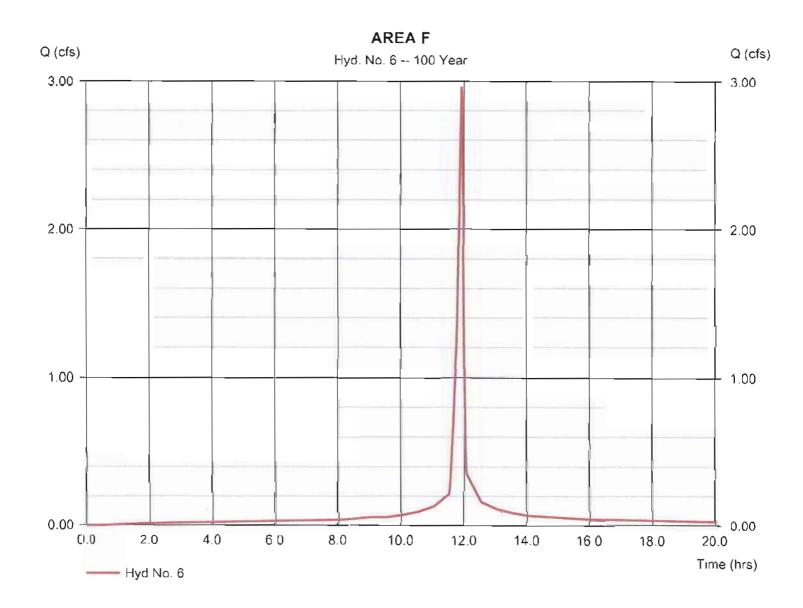
Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 6

AREA F

Hydrograph type = SCS Runoff Peak discharge = 2.960 cfs= 100 yrs Storm frequency Time to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.148 acft Drainage area = 0.250 acCurve number = 98Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



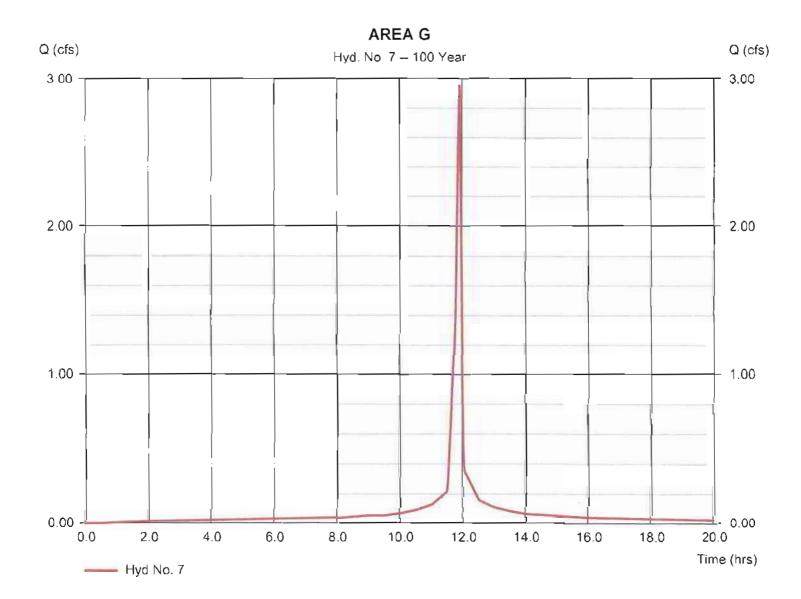
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 7

AREA G

Hydrograph type = SCS Runoff Peak discharge = 2.960 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.148 acftDrainage area Curve number = 0.250 ac= 98Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 in= Type II Distribution = 484 Storm duration = 24 hrs Shape factor



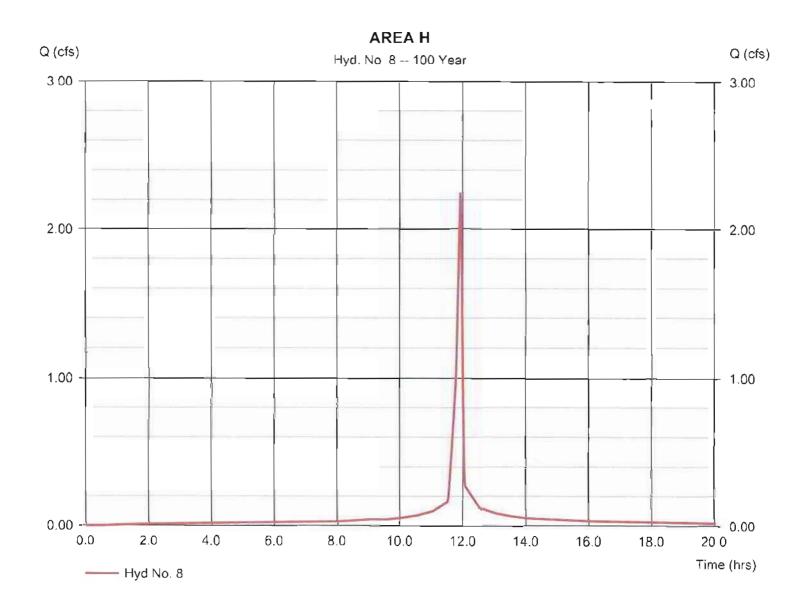
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 8

AREA H

Hydrograph type = SCS Runoff Peak discharge = 2.250 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.112 acftDrainage area = 0.190 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



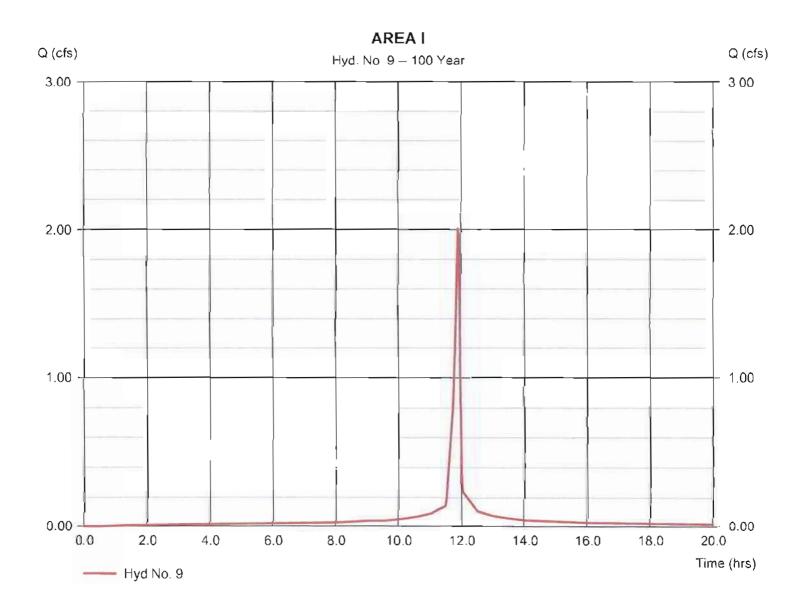
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 9

AREA I

Peak discharge Hydrograph type = SCS Runoff = 2.013 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrsTime interval = 1 minHyd. volume = 0.100 acftDrainage area = 0.170 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



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Tuesday, 00 29, 2012

Hyd. No. 10

<no description>

Hydrograph type Storm frequency Time interval

Inflow hyds.

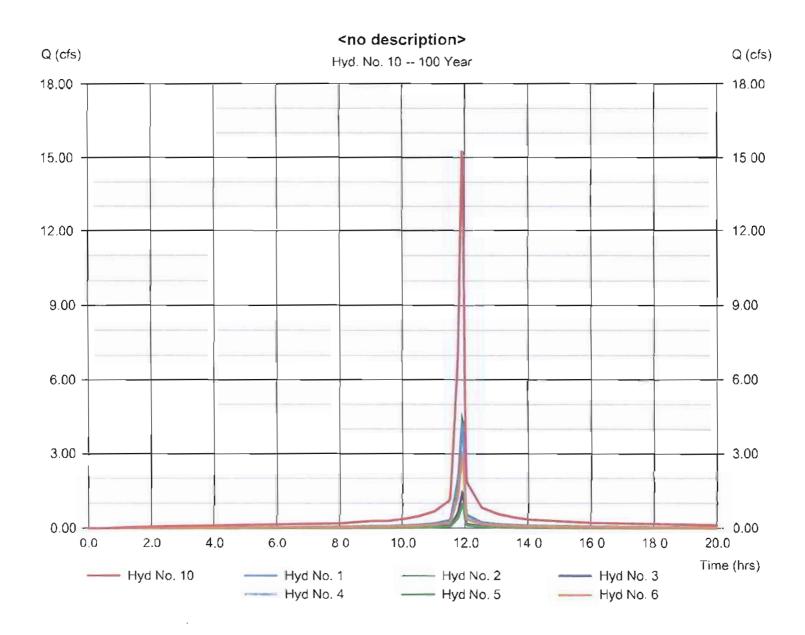
= Combine = 100 yrs

= 1 min = 1, 2, 3, 4, 5, 6 Peak discharge Time to peak = 15.27 cfs = 11.92 hrs

Hyd. volume

= 0.762 acft

Contrib. drain. area = 1.290 ac



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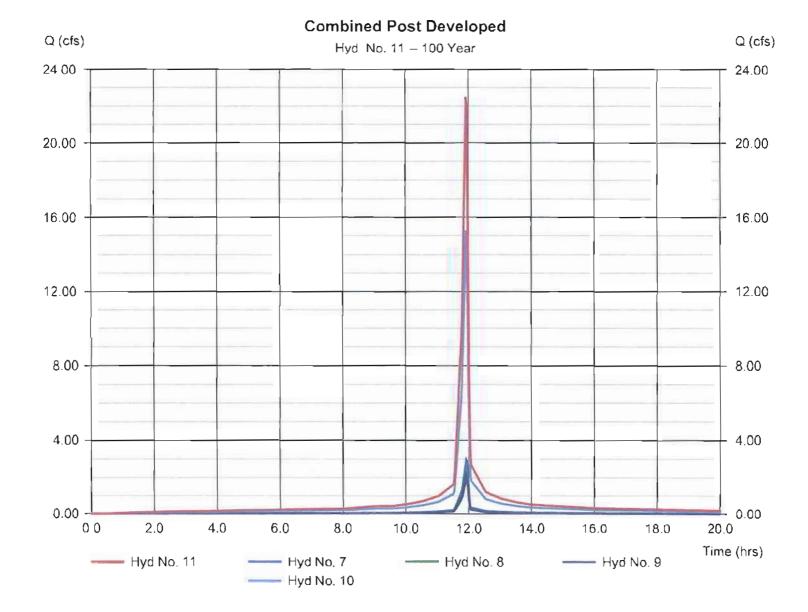
Tuesday, 00 29, 2012

Hyd. No. 11

Combined Post Developed

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 1 min
Inflow hyds. = 7, 8, 9, 10

Peak discharge = 22.50 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.122 acft
Contrib. drain. area = 0.610 ac



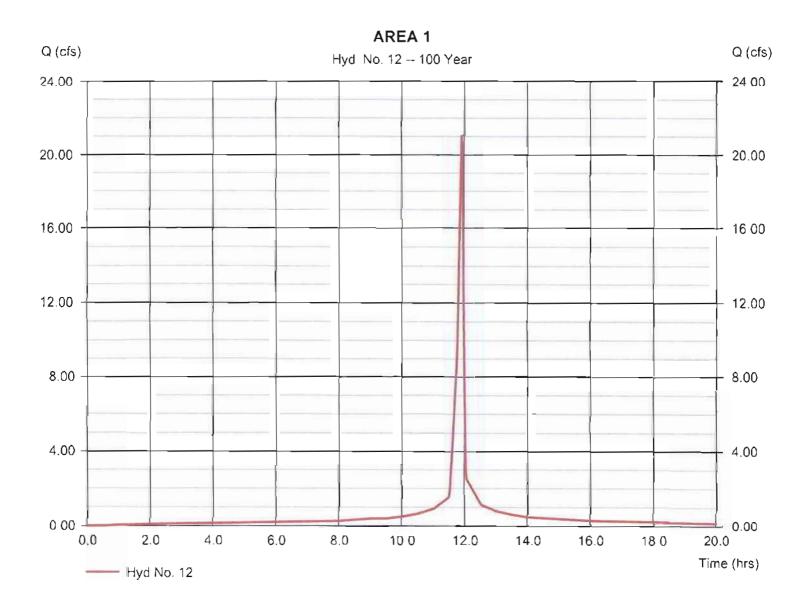
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Tuesday, 00 29, 2012

Hyd. No. 12

AREA 1

= SCS Runoff Peak discharge = 21.08 cfsHydrograph type = 100 yrs Time to peak Storm frequency $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 1.051 acftDrainage area Curve number = 98= 1.780 acHydraulic length Basin Slope = 0.0 %= 0 ftTc method = TR55 Time of conc. (Tc) $= 3.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type ((Storm duration = 24 hrsShape factor = 484



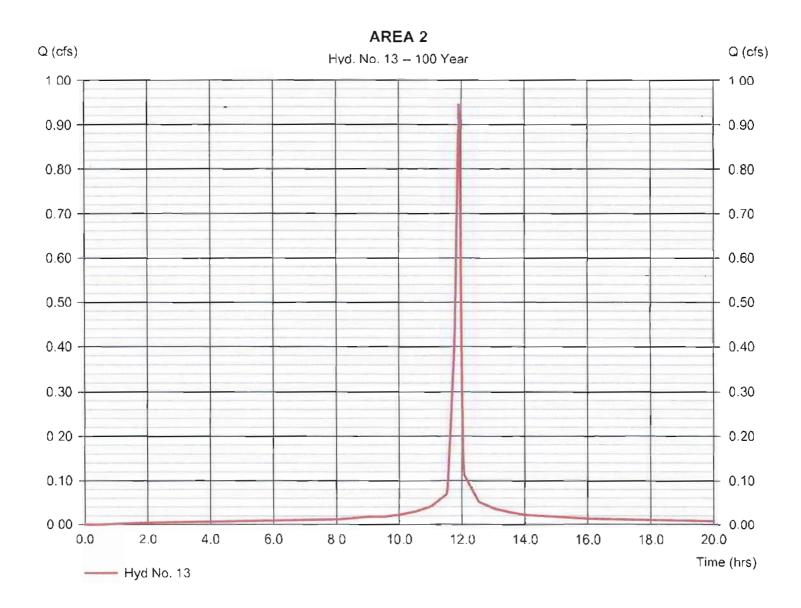
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Tuesday, 00 29, 2012

Hyd. No. 13

AREA 2

Hydrograph type = SCS Runoff Peak discharge $= 0.947 \, \text{cfs}$ Storm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.047 acftDrainage area = 0.080 acCurve number = 98 = 0.0 %Basin Slope Hydraulic length = 0 ftTime of conc. (Tc) Tc method = User $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Shape factor Storm duration = 24 hrs = 484



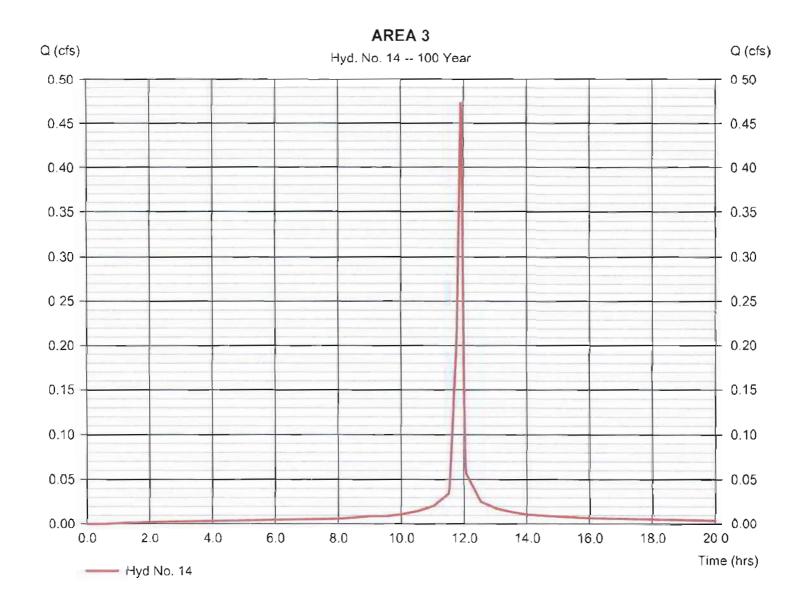
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 14

AREA 3

Hydrograph type = SCS Runoff Peak discharge $= 0.474 \, \text{cfs}$ Storm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHvd. volume = 0.024 acft= 0.040 acDrainage area Curve number = 98 Basin Slope = 0.0 % Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs= 484 Shape factor



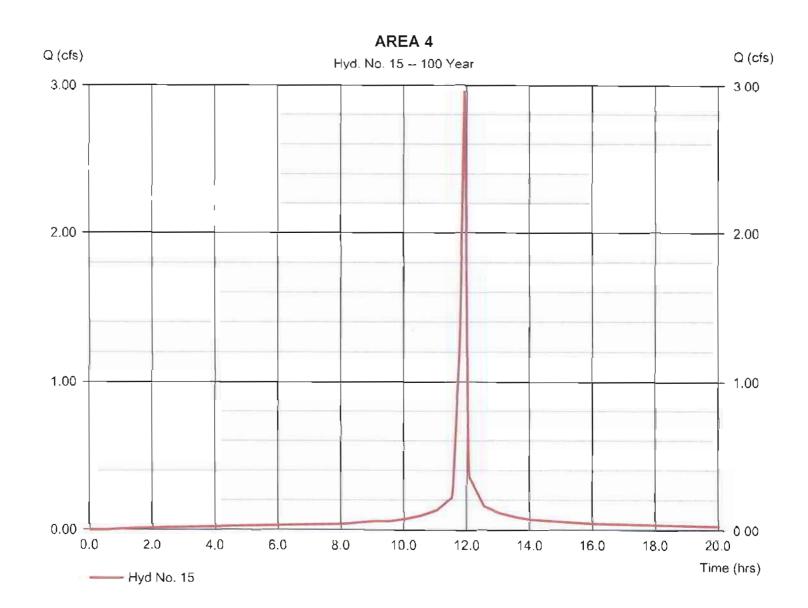
Hydraflow Hydrographs Extension for AutoCAD® Civil 30® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 15

AREA 4

Hydrograph type = SCS Runoff Peak discharge = 2.960 cfsStorm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.148 acft Drainage area = 0.250 acCurve number = 98 Basin Slope = 0.0 %Hydraulic length = 0 ftTc method Time of conc. (Tc) = User $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



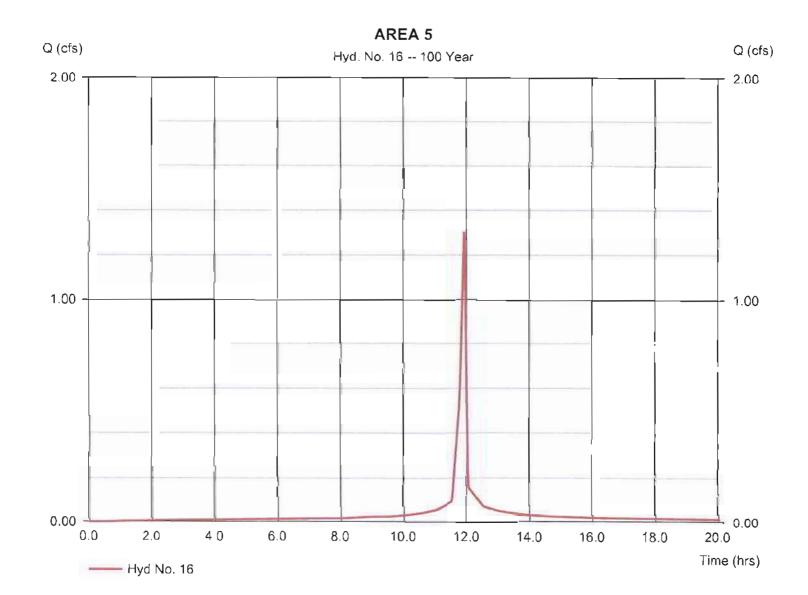
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Tuesday. 00 29, 2012

Hyd. No. 16

AREA 5

Peak discharge Hydrograph type = SCS Runoff = 1.302 cfsStorm frequency = 100 vrsTime to peak $= 11.92 \, hrs$ Time interval = 1 min Hyd. volume = 0.065 acft Drainage area = 0.110 acCurve number = 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



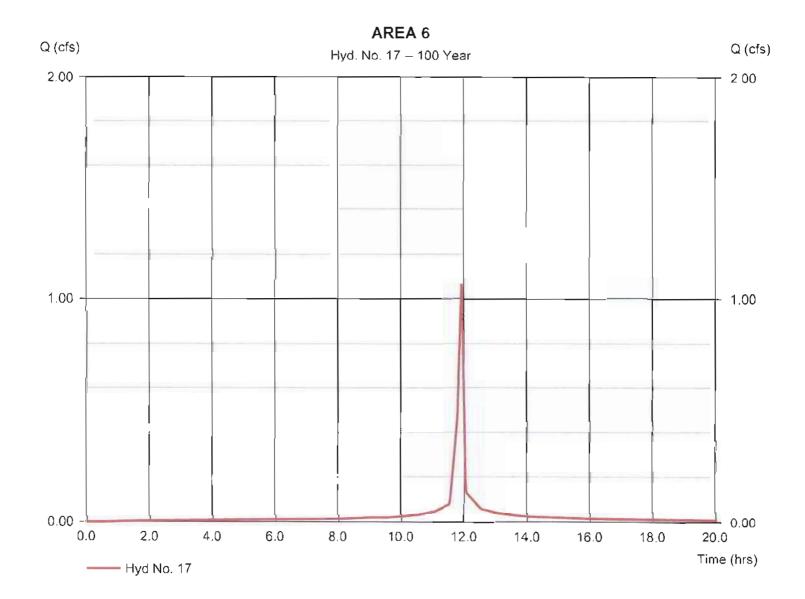
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Tuesday, 00 29, 2012

Hyd. No. 17

AREA 6

Hydrograph type = SCS Runoff Peak discharge = 1.066 cfsStorm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 1 minHyd. volume = 0.053 acft Drainage area = 0.090 acCurve number = 98 Basin Slope Hydraulic length = 0.0 %= 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



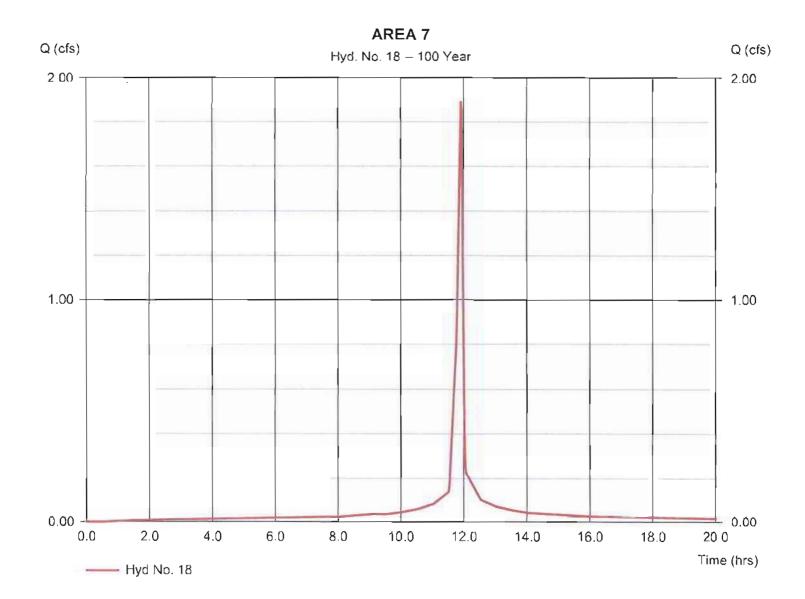
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Tuesday. 00 29, 2012

Hyd. No. 18

AREA 7

Hydrograph type = SCS Runoff Peak discharge = 1.895 cfsStorm frequency = 100 yrsTime to peak $= 11.92 \, hrs$ Time interval = 0.095 acft = 1 min Hyd. volume Drainage area = 0.160 acCurve number = 98Basin Slope = 0.0 %Hydraulic length = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = Type II = 7.80 inDistribution Storm duration = 24 hrs Shape factor = 484



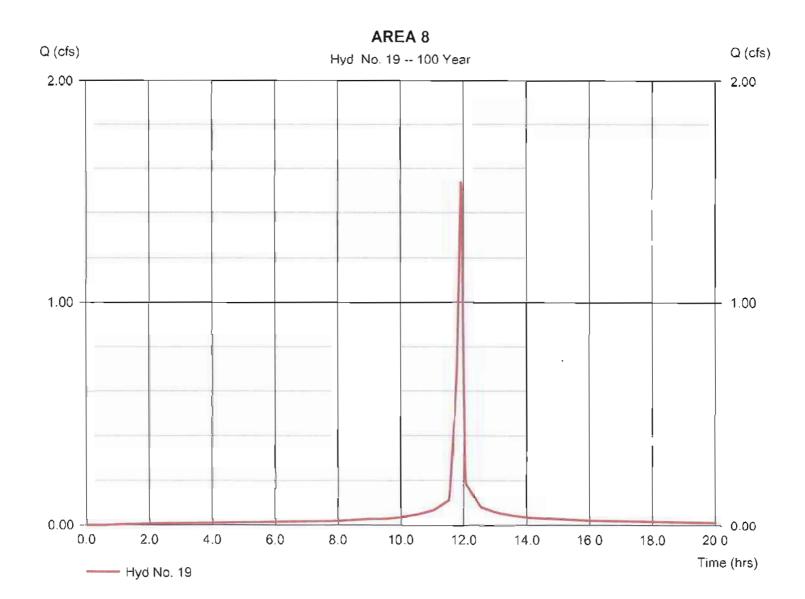
Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 19

AREA 8

= SCS Runoff Hydrograph type Peak discharge = 1.539 cfsStorm frequency = 100 yrsTime to peak = 11.92 hrsTime interval = 1 min Hyd. volume = 0.077 acft Drainage area = 0.130 acCurve number = 98 Hydraulic length Basin Slope = 0.0 % = 0 ftTc method = User Time of conc. (Tc) $= 2.00 \, \text{min}$ Total precip. = 7.80 inDistribution = Type II Storm duration = 24 hrs Shape factor = 484



Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Hyd. No. 20

<no description>

Hydrograph type = Combine Storm frequency = 100 yrs Time interval = 1 min

Inflow hyds. = 12, 13, 14, 15, 16, 17

Peak discharge = 27.83 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.388 acft
Contrib. drain. area = 2.350 ac

<no description> Q (cfs) Q (cfs) Hyd No. 20 -- 100 Year 28.00 28 00 24.00 24.00 20 00 20.00 16.00 - 16.00 12.00 - 12.00 8.00 8.00 4.00 4.00 0.00 0 00 0.0 20 4.0 6.0 8.0 10.0 12.0 14.0 16.0 18.0 20.0 Time (hrs) - Hyd No. 20 - Hyd No 12 Hyd No. 13 - Hyd No. 14 - Hyd No. 15 Hyd No. 16 - Hyd No. 17

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

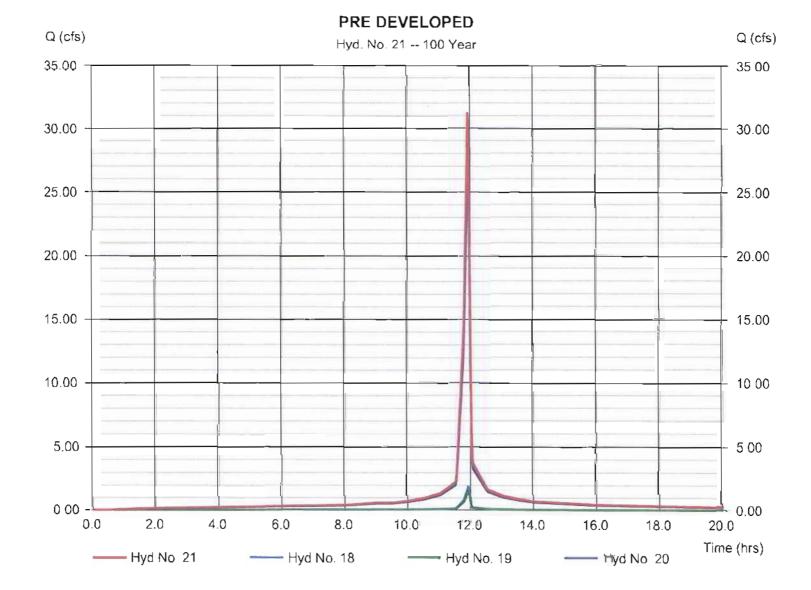
Tuesday, 00 29, 2012

Hyd. No. 21

PRE DEVELOPED

Hydrograph type = Combine
Storm frequency = 100 yrs
Time interval = 1 min
Inflow hyds. = 18, 19, 20

Peak discharge = 31.26 cfs
Time to peak = 11.92 hrs
Hyd. volume = 1.559 acft
Contrib. drain. area = 0.290 ac



Hydraflow Rainfall Report

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk, Inc. v9

Tuesday, 00 29, 2012

Return Period	Intensity-Duration-Frequency Equation Coefficients (FHA)								
(Yrs)	В	D	E	(N/A)					
1	27 8967	9.8000	0.7047						
2	76.3137	14 3000	0.8844						
3	0.0000	0.0000	0.0000						
5	52.6224	11.2000	0.7497						
10	55.1841	11.1000	0 7229						
25	60.7012	11 1000	0.7068						
50	66.9222	11.3000	0.7004	*******					
100	62.2794	10 1000	0.6624	********					

File name. WICHITA IDF IDF

Intensity = $B / (Tc + D)^E$

Return					Intens	ity Values	(in/hr)					
Period (Yrs)	5 min	10	15	20	25	30	35	40	45	50	55	60
1	4.18	3.40	2.90	2.55	2 29	2.08	1.91	1 78	1 66	1.56	1 48	1.40
2	5.57	4.54	3.85	3.35	2.97	2.67	2.43	2 23	2.06	1.92	1.80	1 69
3	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
5	6.52	5.33	4.55	3.99	3 57	3.24	2.97	2.75	2 57	2.41	2 27	2.15
10	7.40	6.09	5.22	4.60	4 13	3.76	3 46	3.21	3.00	2 82	2 67	2.53
25	8.51	7.03	6.05	5.35	4.81	4.39	4 05	3.76	3.52	3.32	3.14	2 98
50	9.47	7.86	6.78	6 00	5.41	4 94	4.56	4 24	3.98	3 75	3 55	3.37
100	10.31	8.53	7.37	6.53	5.90	5.40	5 00	4.66	4 37	4.13	3.92	3 73

Tc = time in minutes. Values may exceed 60.

E name: C:\Users\user\Desktop\WICHITA STORMWATER\Wichita Point Rainfall Depths 24 Hour Design Storm pop

		F	Rainfall	Precipita	ation Tab	ole (in)		
Storm Distribution	1-yr	2-yr	3-yr	5-yr	10-yr	25-yr	50-yr	100-yr
SCS 24-hour	2.80	3.50	0.00	4.50	5.20	6 10	6.90	7,80
SCS 6-Hr	0.00	0.00	0.00	0.00	0.00	0 00	0.00	0.00
Huff-1st	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-2nd	0.00	0 00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-3rd	0.00	0.00	0 00	0.00	0 00	0.00	0.00	0.00
Huff-4th	0.00	0 00	0.00	0.00	0.00	0.00	0.00	0.00
Huff-Indy	0.00	0 00	0.00	0.00	0.00	0.00	0.00	0.00
Custom	0.00	0.00	0.00	0 00	0.00	0.00	0.00	0.00

Hydraflow Hydrographs Extension for AutoCAD® Civil 3D® 2012 by Autodesk. Inc. v9

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VortSentry® HS Maintenance

The VortSentry HS system should be inspected at regular intervals and maintained when necessary to ensure optimum performance. The rate at which the system collects pollutants will depend more heavily on site activities than the size of the unit, i.e., unstable soils or heavy winter sanding will cause the treatment chamber to fill more quickly, but regular sweeping will slow accumulation.

Inspection

Inspection is the key to effective maintenance and is easily performed. Pollutant deposition and transport may vary from year to year and regular inspections will help ensure that the system is cleaned out at the appropriate time. At a minimum, inspections should be performed twice per year (i.e. spring and fall) however more frequent inspections may be necessary in equipment washdown areas and in climates where winter sanding operations may lead to rapid accumulations of a large volume of sediment. It is useful and often required as part of a permit to keep a record of each inspection. A simple inspection and maintenance log form for doing so is available for download at www.contechstormwater.com.

The VortSentry HS should be cleaned when the sediment has accumulated to a depth of two feet in the treatment chamber. This determination can be made by taking two measurements with a stadia rod or similar measuring device; one measurement from the manhole opening to the top of the sediment pile and the other from the manhole opening to the water surface. If the difference between these measurements is less than the distance given in Table 1, the VortSentry HS should be maintained to ensure effective treatment.

Cleaning

Cleaning of the VortSentry HS should be done during dry weather conditions when no flow is entering the system Cleanout of the VortSentry HS with a vacuum truck is generally the most effective and convenient method of excavating pollutants from the system. Simply remove the manhole cover and insert the vacuum hose into the sump. All pollutants can be removed from this one access point from the surface with no requirements for Confined Space Entry

In installations where the risk of petroleum spills is small, liquid contaminants may not accumulate as quickly as sediment. However, an oil or gasoline spill should be cleaned out immediately. Motor oil and other hydrocarbons that accumulate on a more routine basis should be removed when an appreciable layer has been captured. To remove these pollutants, it may be preferable to use adsorbent pads, which solidify the oils. These are usually much easier to remove from the unit individually, and less expensive to dispose than the oil/water emulsion that may be created by vacuuming the oily layer. Floating trash can be netted out if you wish to separate it from the other pollutants.

Manhole covers should be securely seated following cleaning activities to prevent leakage of runoff into the system from above and also to ensure proper safety precautions. If anyone physically enters the unit, Confined Space Entry procedures need to be followed

Disposal of all material removed from the VortSentry HS should be done is accordance with local regulations. In many locations, disposal of evacuated sediments may be handled in the same manner as disposal of sediments removed from eatch basins or deep sump marktoles. Check

your local regulations for specific requirements on disposal.

VortSentry HS Model	Dian	neter	Dista Betweer Surface of Storag	n Water and Top	1500	ment rage		Spill rage
	in.	m	ft.	m	yd³	m³	gal.	liter
HS36	36	0.9	3.6	1.1	0.5	0.4	83	314
HS48	48	1.2	4.7	1.4	0.9	0.7	158	598
HS60	60	1.5	6.0	1.8	1.5	1.1	258	978
HS72	12	1.8	7.1	2.2	2.1	1.6	372	1409
HS84	84	2.1	8.4	2.6	2.9	2.2	649	2458
HS96	96	2.4	9.5	2.9	3.7	2.8	845	3199

Table 1. VortSentry HS Maintenance Indicators and Sediment Storage Capacities.

VortSentry HS Inspection & Maintenance Log

VertEaster HC Madel	
VortSentry HS Model:Location:	

Date	Water depth to shipment ⁱ	Floatable Layer Thickness?	Describe Maintenance Performed	Maintenance Personnel	Comments

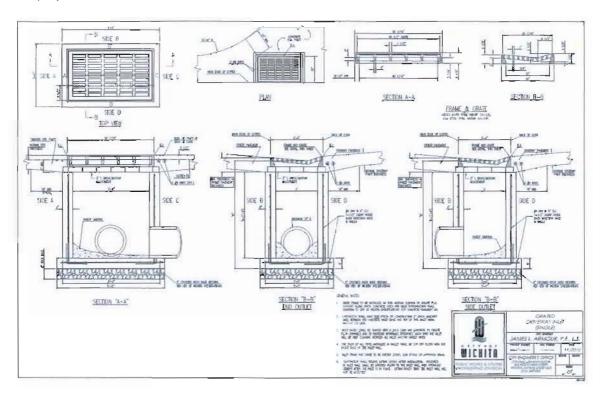
^{1.} The water depth to sediment is determined by taking two measurements with a stadia rod, one measurement from the manhole opening to the top of the sediment pile and the other from the manhole opening to the water surface. If the difference between these measurements is less than the distance given in Table 1, the system should be cleaned out. Note: To avoid underestimating the volume of sediment in the chamber, the measuring device must be carefully lowered to the top of the sediment pile.

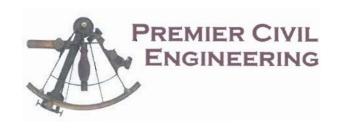
For optimum performance, the system should be cleaned out when the floating hydrocarbon layer accumulates to an appreciable thickness. In the event of an oil spill, the system should be deaned immediately.

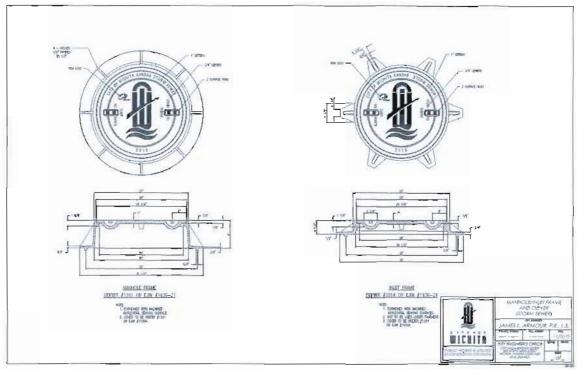


Section 3.4.3 Typical details

Below are City of Wichita details that will be further analyzed based upon the approval of the Plat and preparation of the construction documents.







Section 3.5

Section 3.5.1
Table listing all Stormwater Management Facilities

	Stormwater Management Facilities			
	% TSS	WATER QUALITY FLOW		
VORTSENTRY HS	80%	0.0523 ac-ft		

Section 3.5.2 Responsible Party for Maintenance

The proposed Plat will indicate that it will be the responsibility of each individual property to owner maintain their Stormwater Management Controls and Facilities.

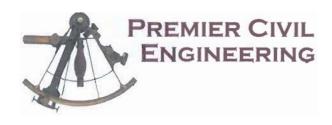
Section 3.5.3 Water Quality Volume for Facilities

Land Use	Hydrologic Soil Group					
	A	В	С	D		
Undisturbed Woods, Meadows or Ag Land (Rvu)	0.02	0.03	0.04	0.05		
Turf or Disturbed Soils (Rvo)	0.15	0.20	0.22	0.25		
Impervious Cover (R _{vi})	0.95	0.95	0.95	0.95		

Calculation equation:



Section 3.5.4 %TSS removal value for Facilities



Section 3.5.5 Channel Protection Volume

Not required for redevelopment site.

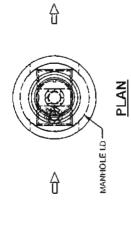
Section 3.5.6
Water quality volume and Channel Protection volume orifice Calculations
Volume calculations not provided for site. Flow Calculation have been provided in the report.
See Section 3.3.6

Section 3.5.7 Not Applicable for this project

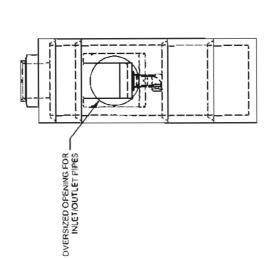
Section 3 5.8 Typical Details for Vortsentry HS

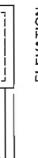
This CADD file is for the europeae of specifying stormwaler Ireetinent equipment the furnished kin COVITECH Stormwaler Solutions and may only be frankered to other deciminate exectly as provided by CONTECH Stormwaler Solutions and file block information, excluding the CONTECH Stormwaler Solutions logo and the VortSentry AS Stormwaler Treatment System designation and patent numbers, may be defined if heressign; Reviewns to any part of this CAUD file without prior contribution with CONTECH Stormwaler Schulions shall be considered unautherized use of proprietary information.

THE VORTSENTRY HS CCATROL SECTION SHALL BE STENCILED WITH THE CONTECH STORMWATER SOLUTIONS NAME AND LOGO. PIPE OPENINGS SHALL BE STENCILED "NILET" AS APPROPRIATE









NLET/OUTLET	
<u> </u>	0

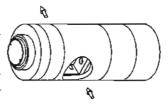
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LEFT SIDE

	L	_						
Maximum Pipe Diameter (ID)		۳u	450	900	750	900	1050	1200
		п	18	24	30	36	42	48
Approximate Minimum Distance Rim to Invert	(See Nate 7)	я	0.91	1.22	1 47	1.70	1.45	2 11
		H	3.00	4 00	4 82	5 59	4 77	6.91
Typical Depth Below Invert (Inside)	O	TTI LLU	1702	2057	2807	2788	3156	3518
Typical Depth Below Invert (Inside)		н	1.24 5.5833	6.75	9.21	9.15	10.35	2,59 11,54 3518
Typical Distance Rim to Invert	۵	m (1.83	1.98	2 06	2.36	2.59
		ίτ	4.08	009	6 50	6.75	7.75	8.50
Typical Total Distance Rim to Outside Bottom	۷	ű	3 10	4.04	4.99	5 05	575	6.36
		ŧι	10.16	34 0 13.25	16 38	104 8 16.56	1586 18.85	20.87
Total Treatment Flow Rate		s/I	156		62.3	1048	1586	229.4 20.87
		sJo	SS 0	1.26	2 20	3.76	29.5	8.10
Manhole Dlameter (ID)		шш	006	120C	1500	1800	2100	2400
		¥	၈	4	ß	9	2	8
VortSentry Model			HS36	HS48	09SH	HS72	HS84	96SH
		_	_	_	_	_		=

FOR INFORMATIONAL PURPOSES ONLY - NOT INTENDED FOR CONSTRUCTION

- 1 STORMWATER TREATMENT SYSTEM (SWTS) SHALL REMOVE 80% OF A SEDIMENT GRADATION WITH AN AVERAGE PARTICLE SIZE OF 240 MICRONS AT THE DESIGNATED TREATMENT FLOW RATE LISTED IN THE TABLE FOR EACH CORRESPONDING MODEL.
- 2. SWTS REMOVAL EFFICIENCY CLAIM SHALL BE CORROBORATED BY FULL SCALE LABORATORY TEST PERFORMANCE DATA
- SWTS MAINTENANCE RECOMMENDATION SHALL BE SUPPORTED BY FULL SCALE WASH-OUT TESTING.
- 4 SWTS SHALL PROVIDE INTERNAL BYPASS OF FLOWS THAT EXCEED THE TREATMENT FLOW RATE.
- 5 SWTS MAXIMUM HYDRAULIC CAPACITY MAY VARY DEPENDING UPON THE INLET PIPE DIAMETER, MATERIAL AND SLOPE.
- 8. SWTS INVERTS IN AND OUT SHALL BE AT THE SAME ELEVATION INLET AND OUTLET PIPES MUST BE 180° FROM EACH OTHER
- 7. MINIMUM RIM TO INVERT DISTANCE MAY BE REDUCED DEPENDING UPON ACTUAL PIPE DIAMETER. CONTACT CONTECH STORMINATER SOLUTIONS FOR SITE SPECIFIC INFORMATION
- 8. PIPE SIZE MAY BE SMALLER THAN THE MAXIMUM PIPE SHOWN ON THE TABLE; SEE SITE PLAN FOR PIPE SIZE
- PURCHASER SHALL NOT BE RESPONSIBLE FOR ASSEMBLY OF INTERNAL COMPONENTS.
- 10 ACCESS FRAME AND COVER SUPPLIED WITH SYSTEM, NOT INSTALLED SWTS MAY ALSO HAVE A GRATED NILET COVER (NOT SHOWN)
- 11 PURCHASER TO PREPARE EXCAVATION AND PROVIDE LIFTING EQUIPMENT
- 12. VÜRTÜSENTRY HS BY CONTECH STORMWATER SOLUTIONS, PORTLAND, OR (800) 548-4687, SCARIBOROUGH, ME (877) 907-8676, LINTHICUM, MD (866) 740-3318.



ASSEMBLED VIEW

SP

STORMWATER TREATMENT SYSTEM TYPICAL DETAIL WITH SIZING TABLE VORTSENTRY® HS US PATENT PENDING



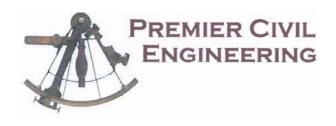
FILE NAME, VSHS TYPTBL CHECKED GWB NDG DRAWN

8/17/07

DAYE

NONE

SCALE



Section 4.0

Floodplain (Does not apply to the project)

The floodplain application has been attached on the following pages for review.

Section 5.0

Federal, State and Local Permits

All permits will be obtained throughout the construction document review and approval process.

Sincerely,

Matt Fogarty