

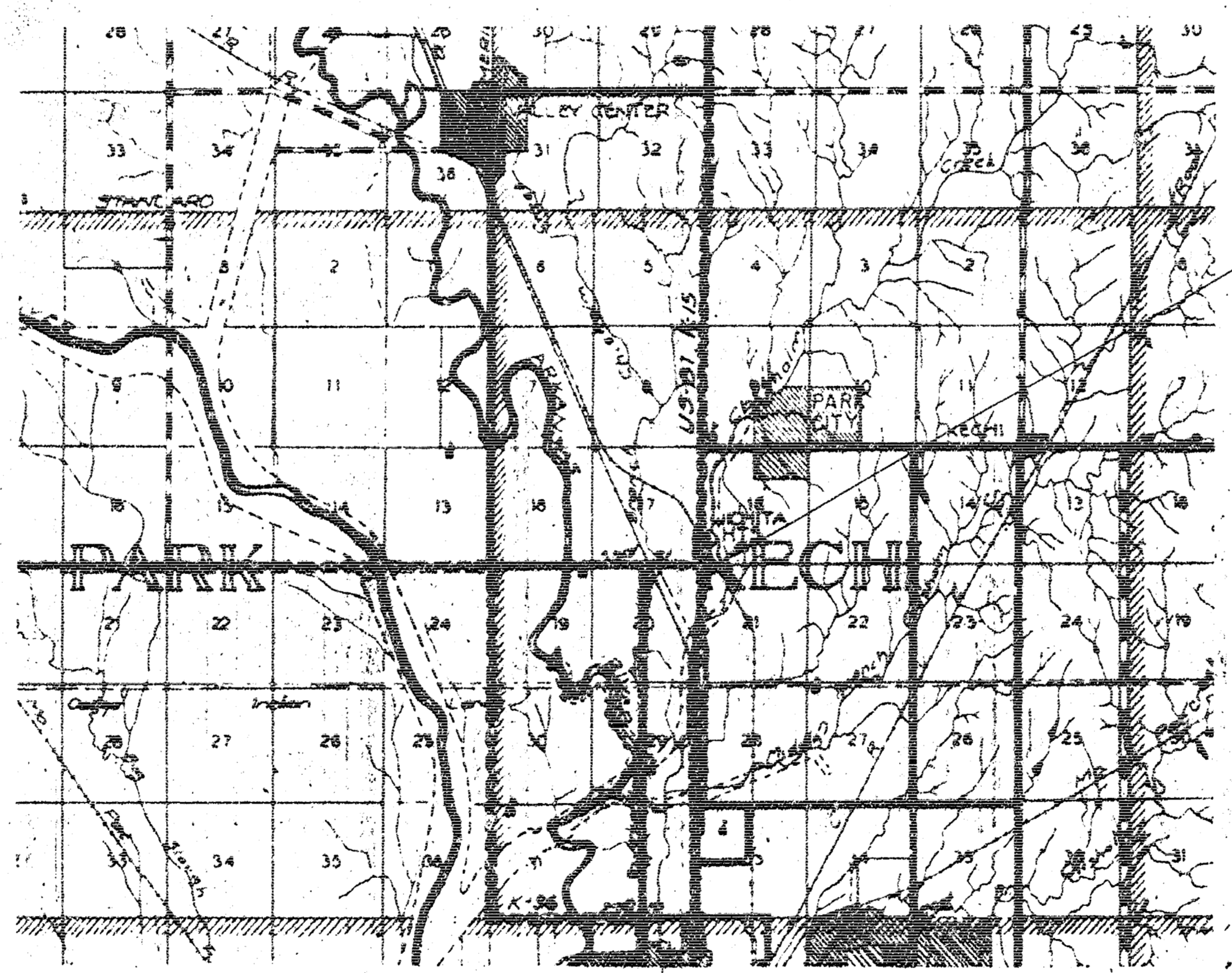
STATE	PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
KANSAS				



BRIDGE GRADING  
& RIP-RAP

**INDEX OF SHEETS**

SHEET NO.	TITLE
SHEET NO. 1	TITLE SHEET
SHEET NO. 2	PLAN - PROFILE AND RIP-RAP PLAN
SHEET NO. 3	CONSTRUCTION LAYOUT
SHEET NO. 4	CONCRETE FLOOR DETAILS
SHEET NO. 5	STEEL LAYOUT & CONCRETE DETAILS
SHEET NO. 6	ABUTMENT, PIER, AND AUXILIARY DETAILS
SHEET NO. 7	BAR DIAGRAMS & REINFORCING STEEL QUANTITIES
SHEET NO. 8	STANDARD FILE DETAILS
SHEET NO. 9	CROSS SECTIONS



CONSTRUCT BRIDGE NO. 606-27-983 (2°12'-20" LT. SKEW)  
CONTINUOUS STEEL-BEAM WITH  
CONCRETE DECK - 98'-0" RDWY.  
WITH ALUMINUM HANDRAIL.  
SPANS: 30'-4 @ 42'-6" - 30'

TRAFFIC COUNT - 600 CARS PER DAY

INCLOSURE WITH LETTER DATED 19 Dec 1957  
FROM J. L. B. [Signature]

REC'D DEC 19 1957

**CONVENTIONAL SIGNS**

COUNTY LINE	---
SECTION LINE	---
WIRE FENCE	-----
HEDGE ROW	~~~~~
RAILROADS	-----
SURVEY LINE	-----
RIGHT OF WAY	-----
TELEPHONE POLE	⊕
POWER POLE	⊕
TRAVELED WAY	-----

NET LENGTH OF PROJECT	750	FT.	.142	MILES
NET LENGTH OF BRIDGES	232'-8"	FT.	.044	MILES
NET LENGTH OF ROAD	517.33	FT.	.098	MILES
EXCEPTIONS	NONE	FT.	---	MILES
ADDITONS	NONE	FT.	---	MILES
GROSS LENGTH OF PROJECT	750	FT.	.142	MILES

PLANS PREPARED BY  
SEDGWICK CO. ENGR. DEPT.  
DATE NOVEMBER 1957

APPROVED  
COUNTY ENGINEER  
DATE NOVEMBER 1957

APPROVED  
CHAIRMAN  
BOARD OF COUNTY COMMISSIONERS

APPROVED  
DATE

REC'D JAN 16 1958  
John T. [Signature]

COUNTY CROSSINGS  
53 RD. MAIN BEACH

PLAN  
 DRAWN BY  
 CHECKED BY  
 DATE  
 SHEET NO.

PROFILE  
 DRAWN BY  
 CHECKED BY  
 DATE  
 SHEET NO.

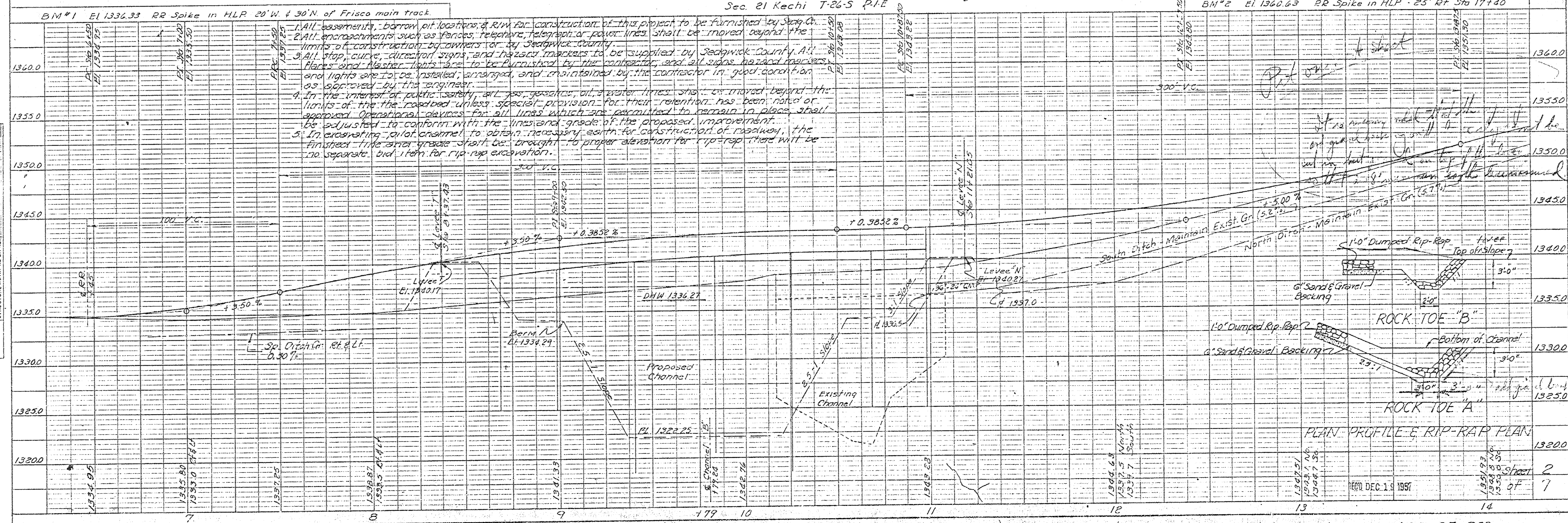
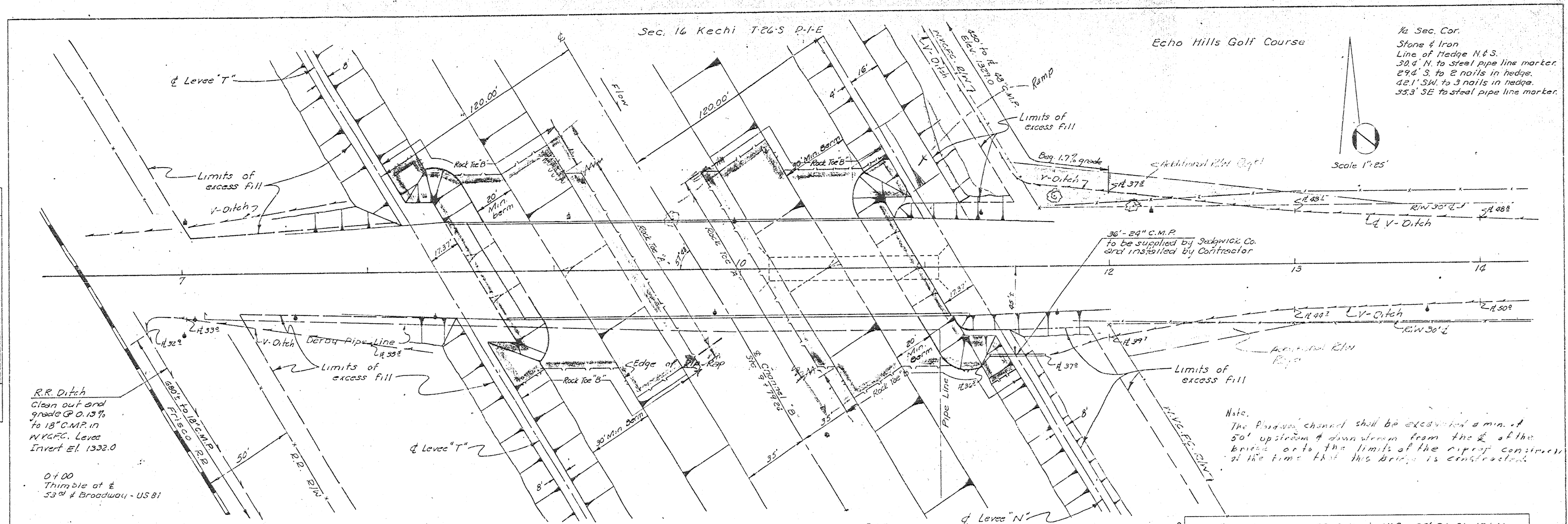


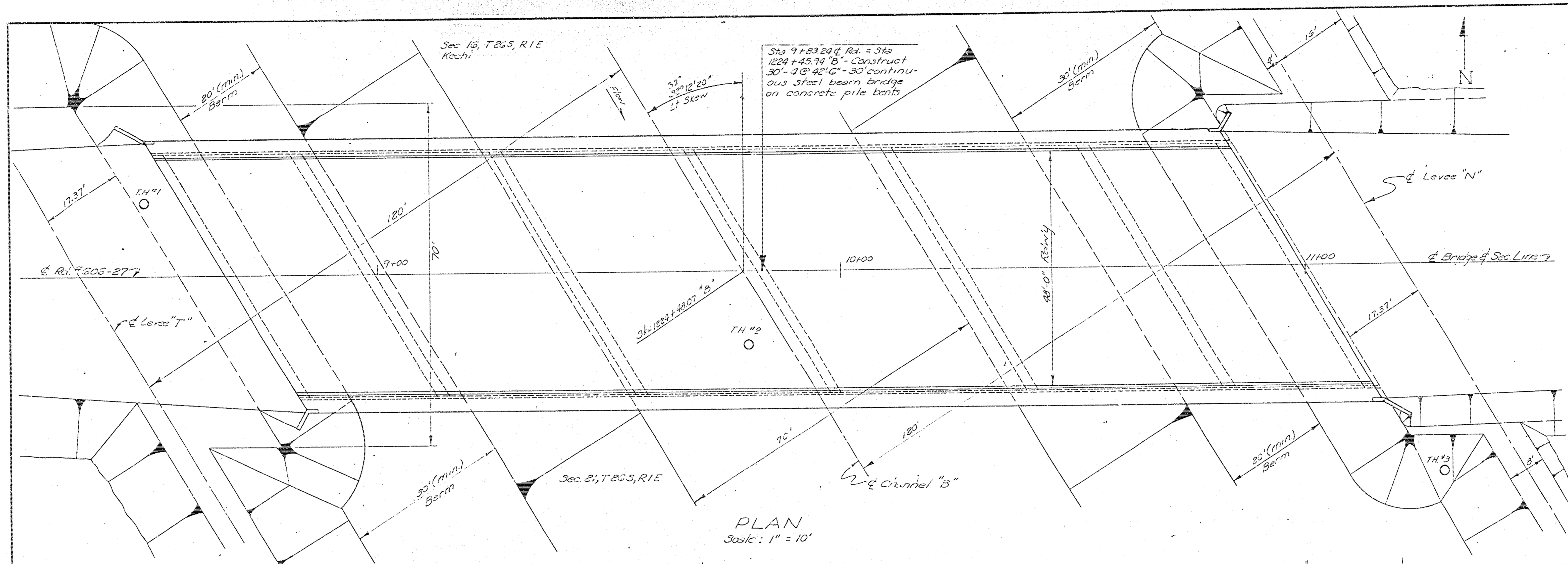
PLATE 11 - PLAN-PROFILE OF R.R. BRIDGE

53rd St & Main Br. Chisholm Creek

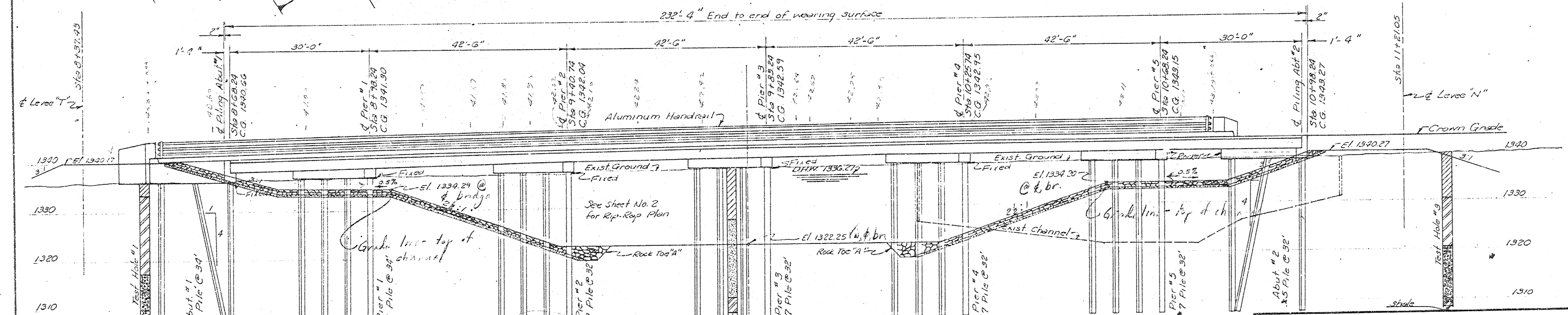
606-27-983

DEC 1 1991

SHEET 2 OF 7



PLAN  
Scale: 1" = 10'



SECTION THRU RD.  
Scale: 1" = 10'

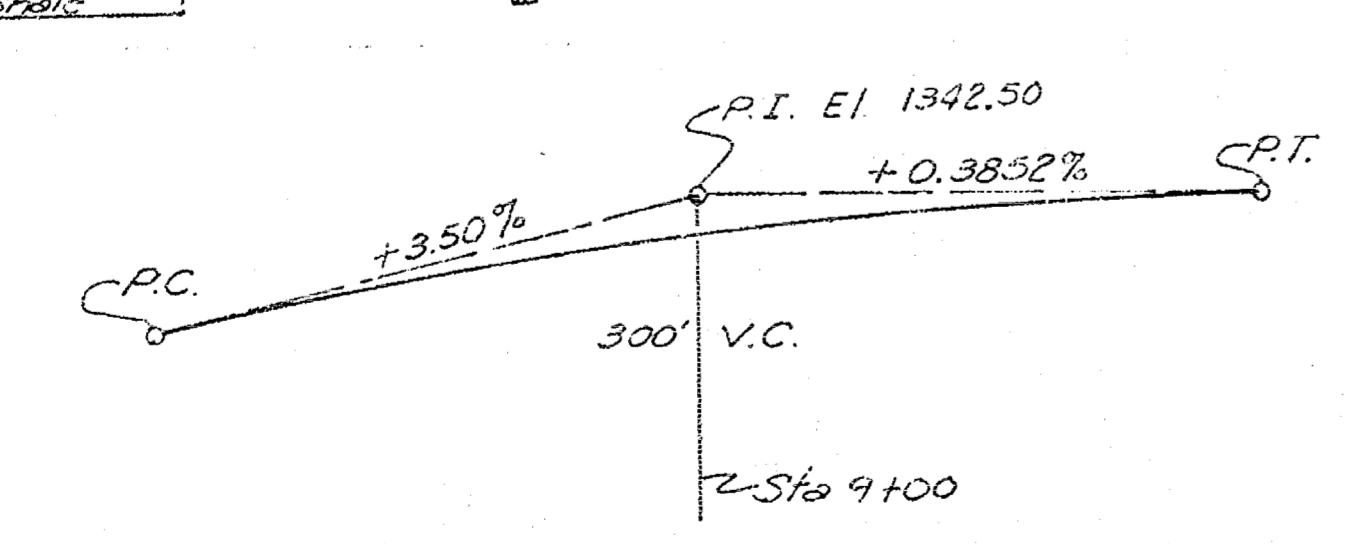
CONSTRUCTION LAYOUT  
BRIDGE NO. GOG-27-983  
53rd St. & Main Br. Chisholm Creek

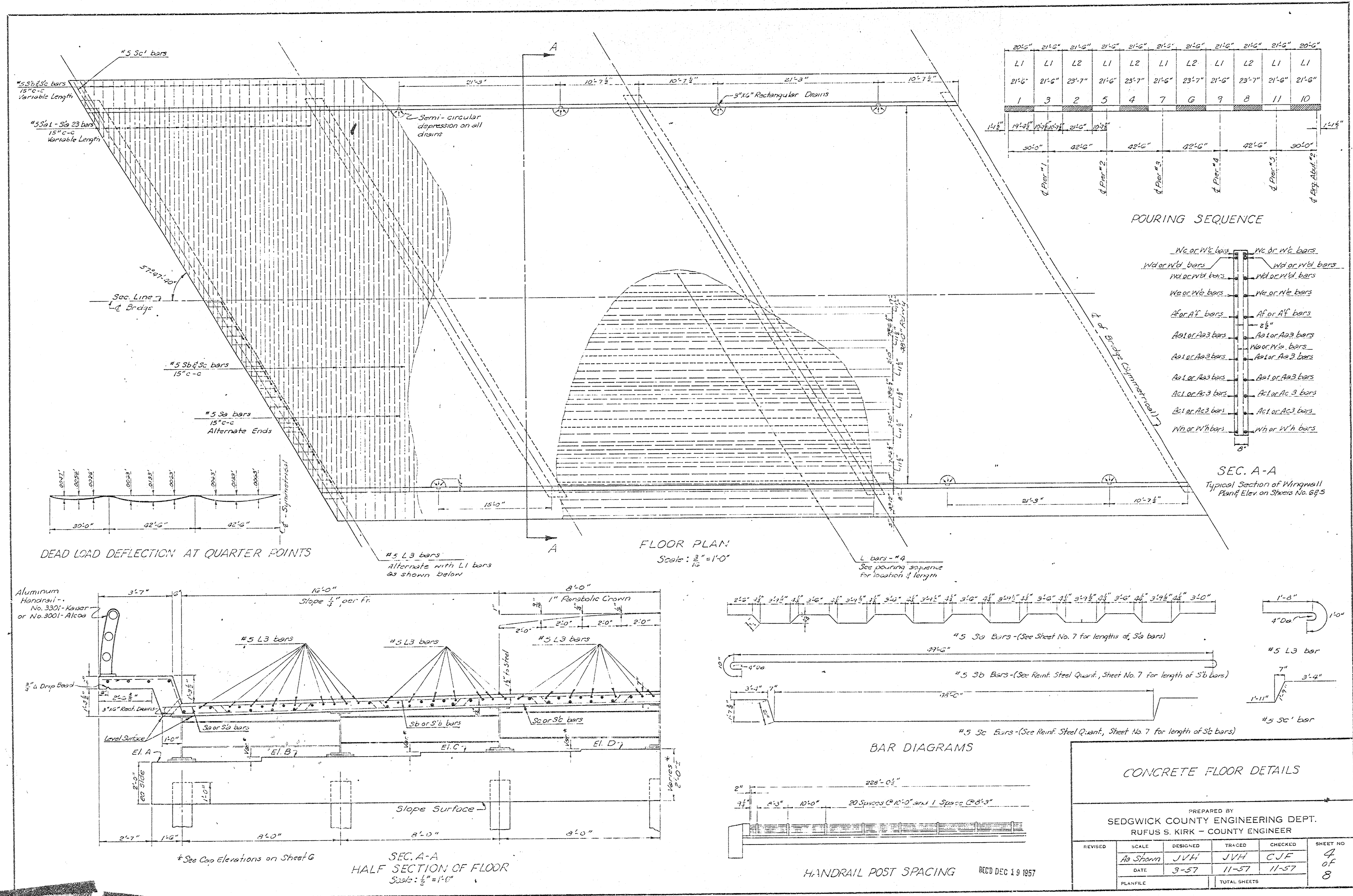
PREPARED BY  
SEDGWICK COUNTY ENGINEERING DEPT.  
RUFUS S. KIRK - COUNTY ENGINEER

REVISED	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	1" = 10'	J.V.H.	J.V.H.	C.J.F.	3
		9-57	9-57	9-57	8
PLANFILE		TOTAL SHEETS			

Note:  
Type of piling to be of the types shown on Sheet No. 7. (Pile may be 14" dia. cast or 12" dia. pre-stressed concrete or 10" dia. pipe pile as designated.) Drive second pile from each end on abutments, as shown.

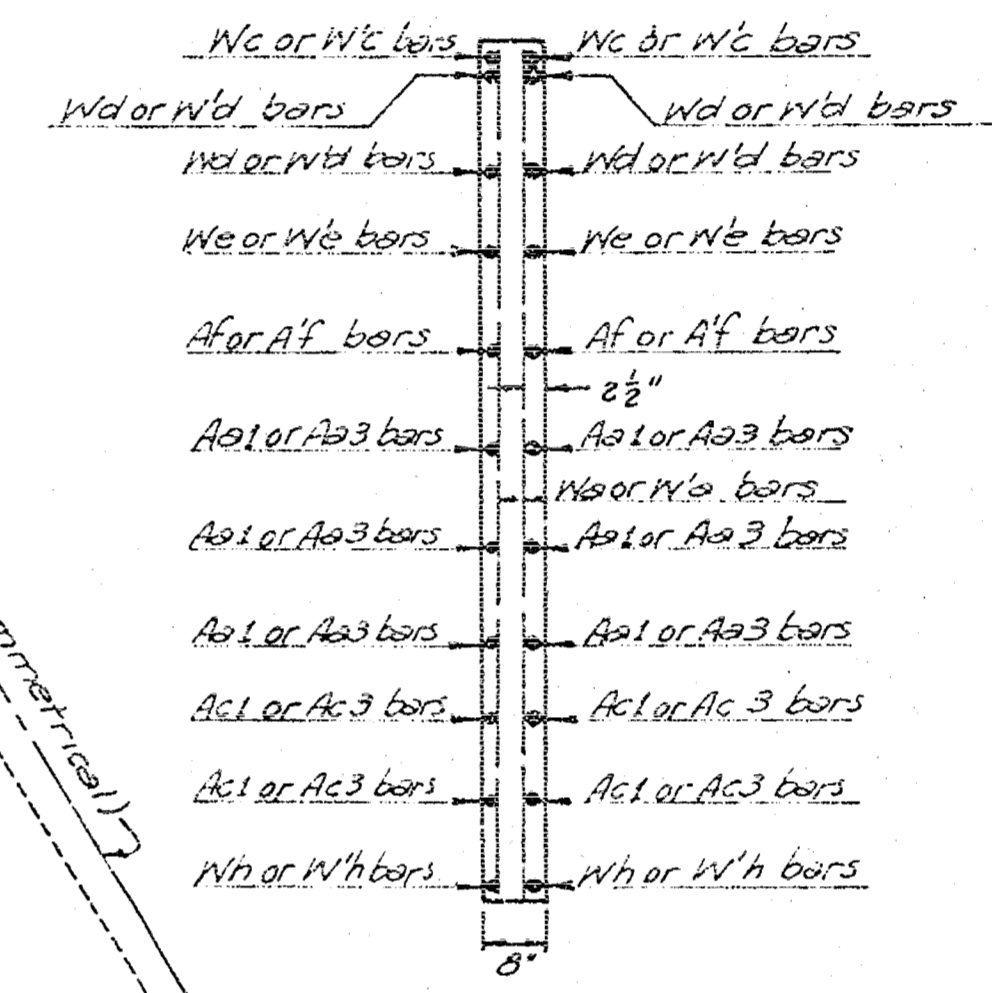
REC'D DEC 19 1957





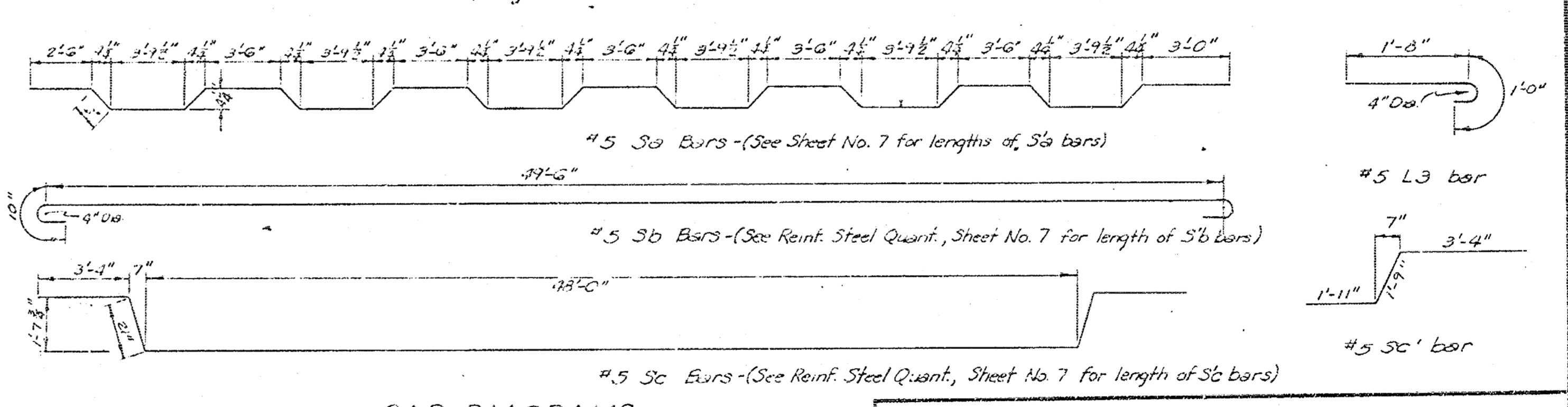
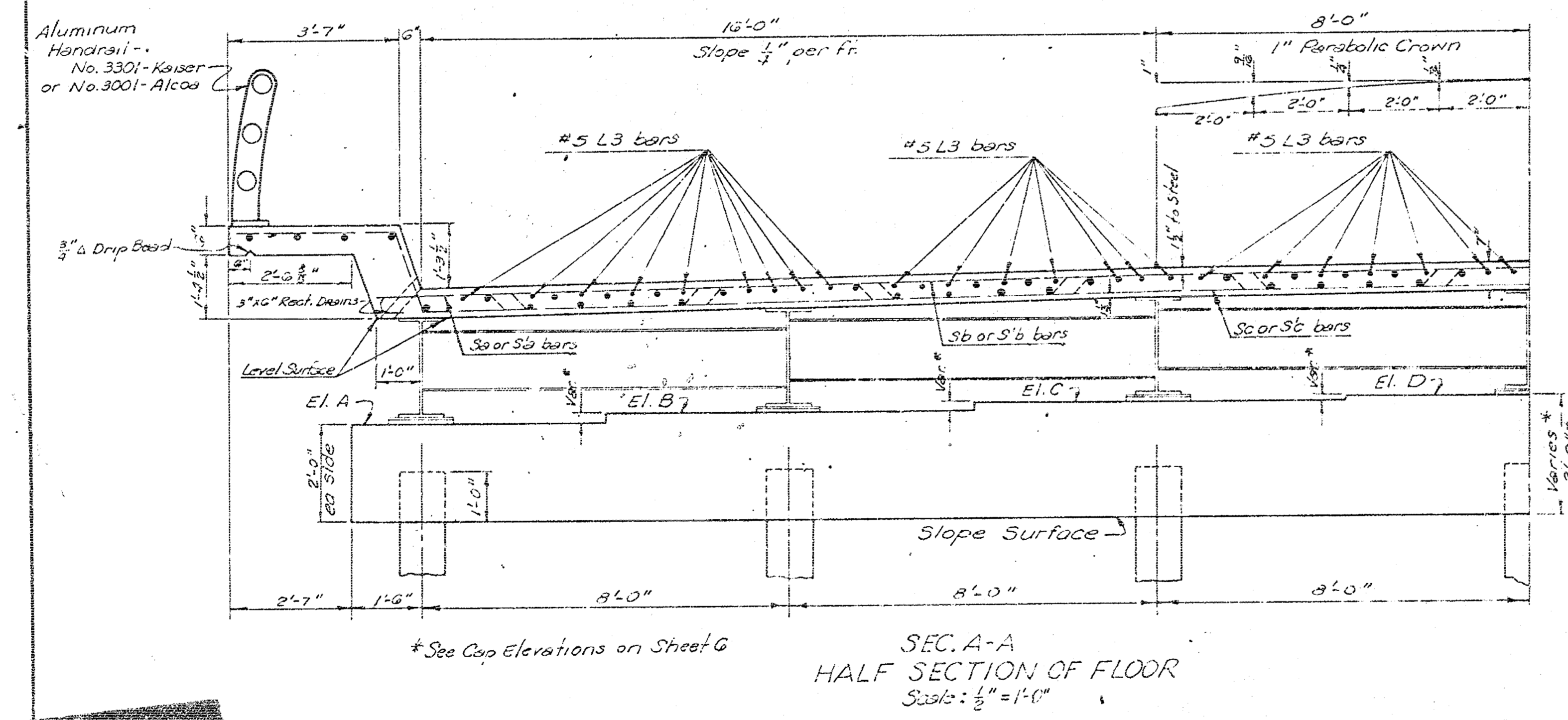
20'-0"	21'-0"	21'-0"	21'-0"	21'-0"	21'-0"	21'-0"	21'-0"	21'-0"	21'-0"	20'-0"
L1	L1	L2	L1	L2	L1	L2	L1	L2	L1	L1
21'-0"	21'-0"	23'-7"	21'-0"	23'-7"	21'-0"	23'-7"	21'-0"	23'-7"	21'-0"	21'-0"
1	3	2	5	4	7	6	9	8	11	10
14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"	14'-0"
30'-0"	42'-0"	42'-0"	42'-0"	42'-0"	42'-0"	42'-0"	42'-0"	42'-0"	42'-0"	30'-0"
of Pier #1	of Pier #2	of Pier #3	of Pier #4	of Pier #5	of Pier #6	of Pier #7	of Pier #8	of Pier #9	of Pier #10	of Pier #11

POURING SEQUENCE



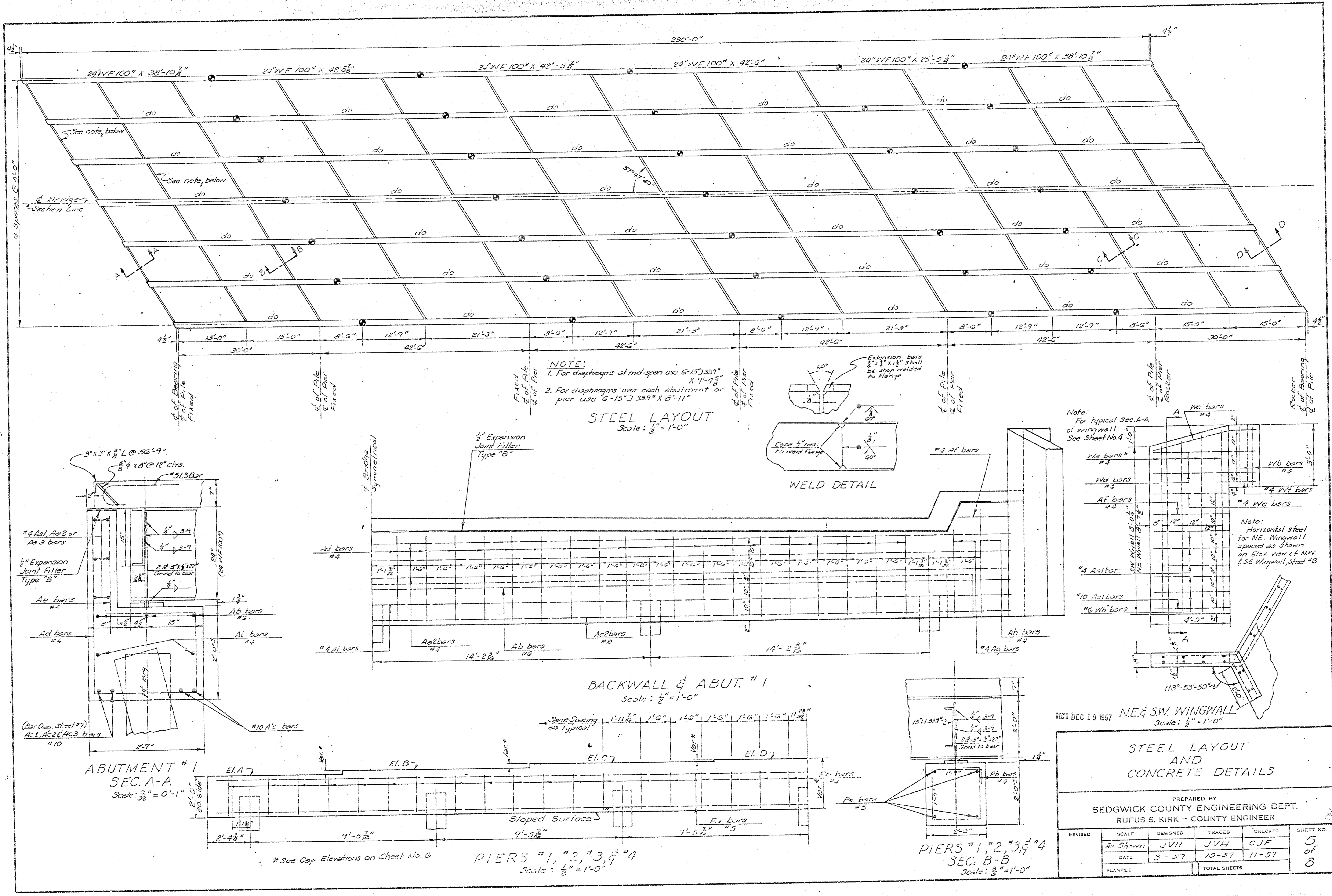
DEAD LOAD DEFLECTION AT QUARTER POINTS

FLOOR PLAN  
Scale: 3/8" = 1'-0"



HANDRAIL POST SPACING  
RECD DEC 19 1957

CONCRETE FLOOR DETAILS				
PREPARED BY SEDGWICK COUNTY ENGINEERING DEPT. RUFUS S. KIRK - COUNTY ENGINEER				
REVISED	SCALE	DESIGNED	TRACED	CHECKED
	As Shown	JVH	JVH	CJF
	DATE	3-57	11-57	11-57
	PLANFILE	TOTAL SHEETS		SHEET NO.
				4 of 8



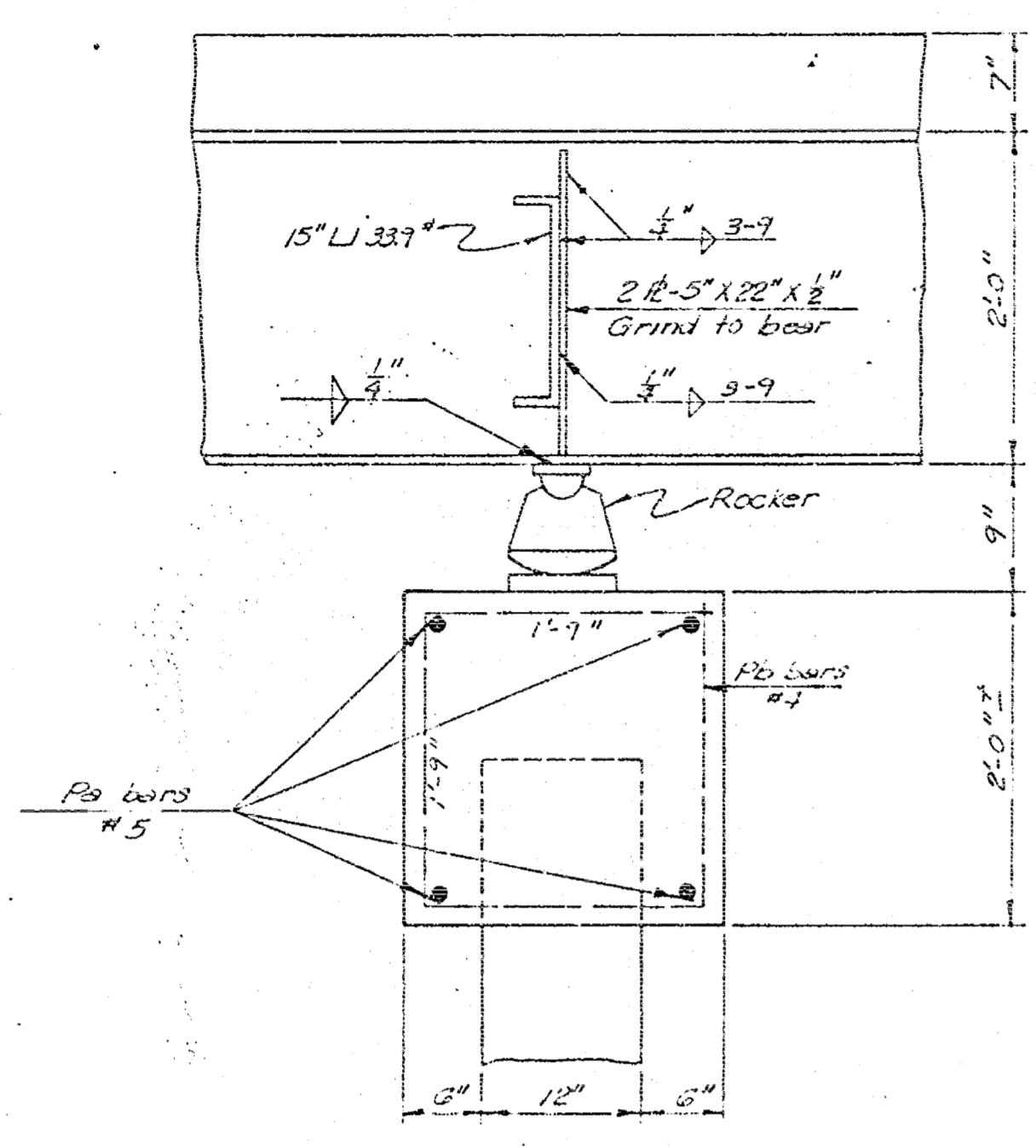
RECD DEC 19 1957

NE & SW WINGWALL  
Scale:  $\frac{3}{8}'' = 1'-0''$

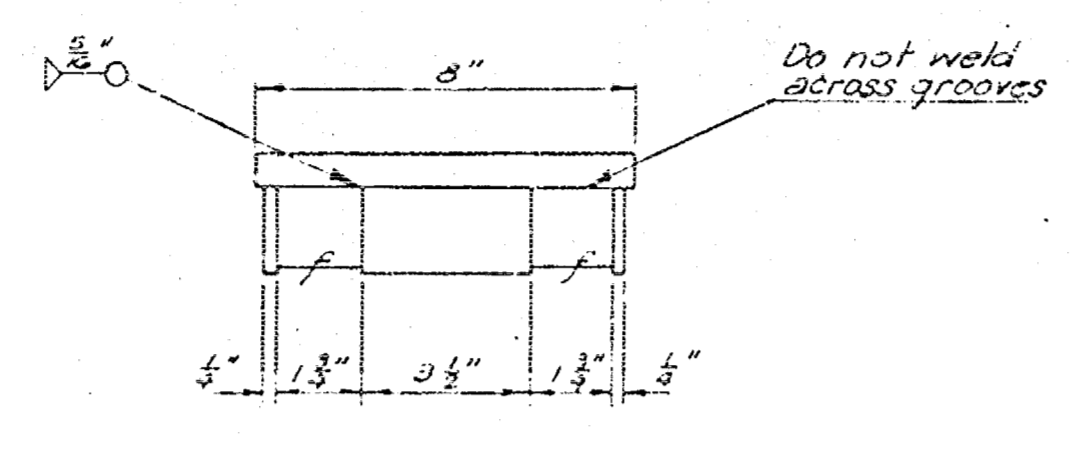
**STEEL LAYOUT AND CONCRETE DETAILS**

PREPARED BY  
SEDGWICK COUNTY ENGINEERING DEPT.  
RUFUS S. KIRK - COUNTY ENGINEER

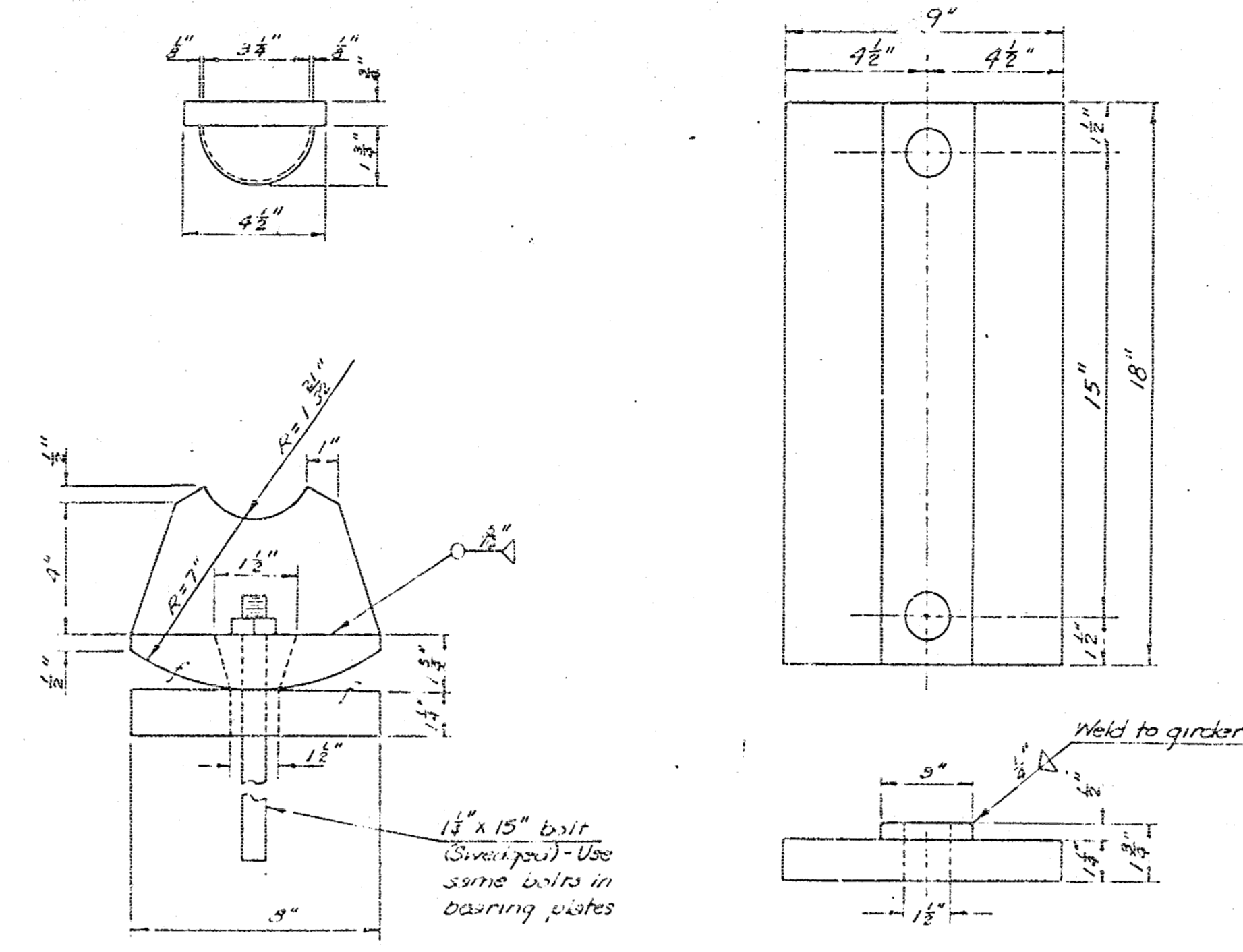
REVISED	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	As Shown	JVH	JVH	CJF	5 of 8
		DATE 3-57	10-57	11-57	
	PLANFILE	TOTAL SHEETS			



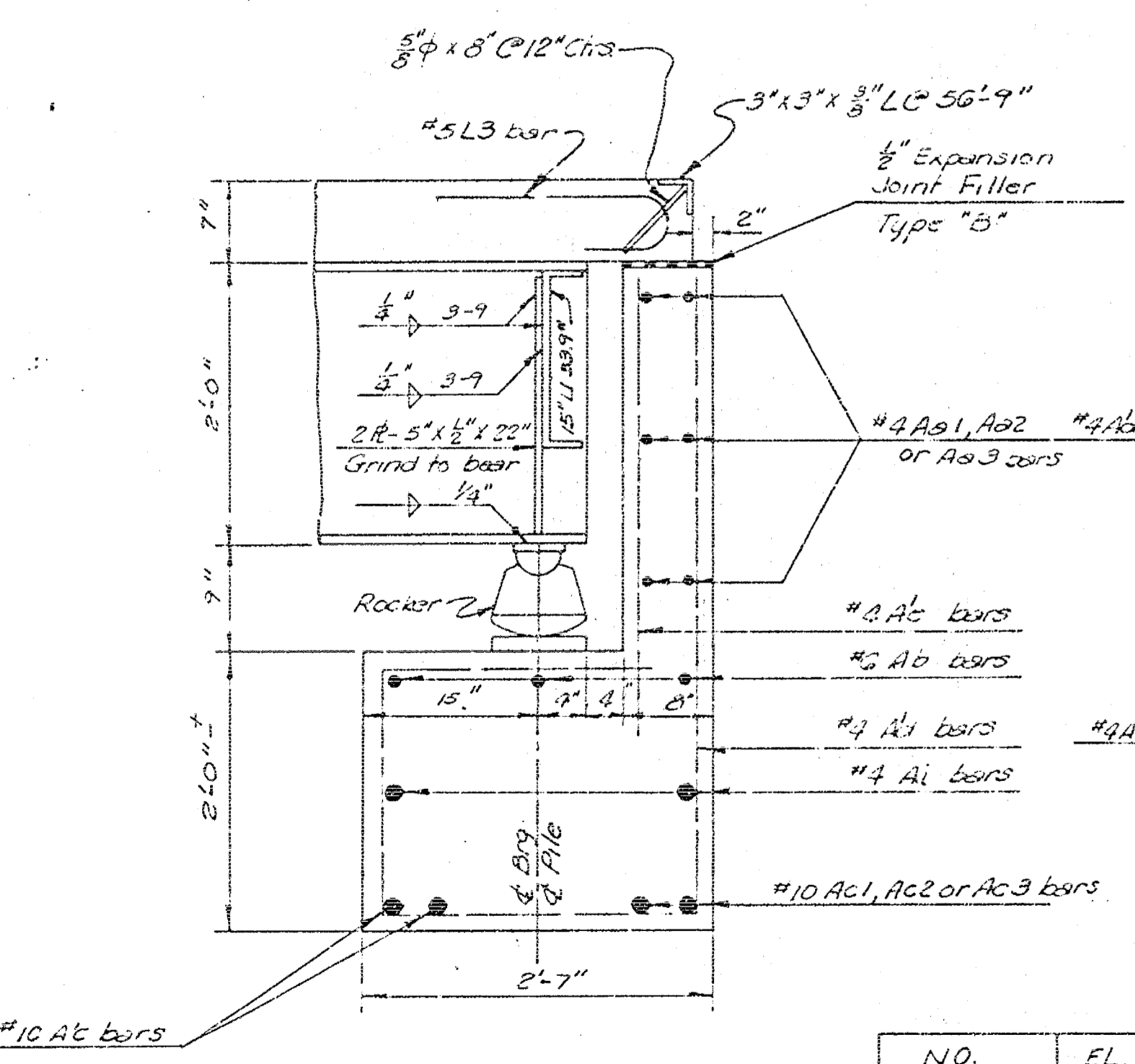
PIER #5  
SEC. C-C (Sheet No. 5)  
Scale: 1" = 1'-0"



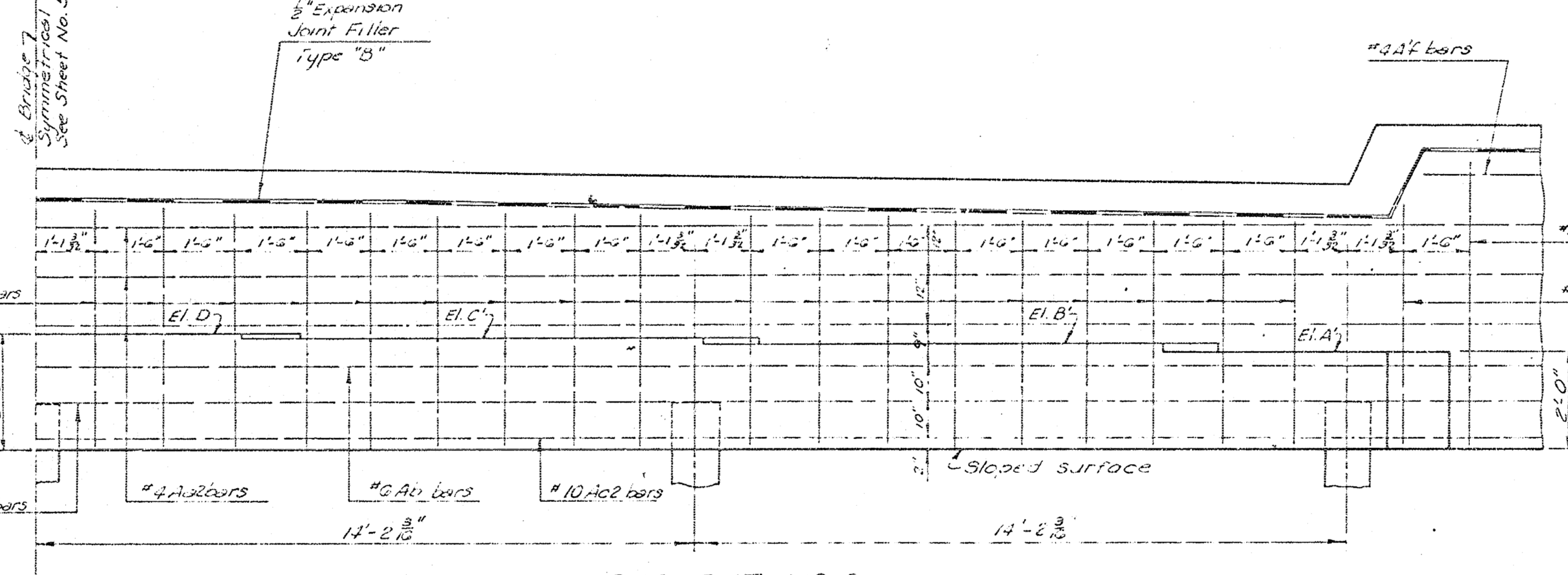
ROCKER DETAIL  
(To be used at Pier #5 & Abut. #2)  
Scale: 1/2" = 1'-0"



BEARING PLATES  
(To be used at Piers #1, #2, #3, #4 & Abut. #1)  
Scale: 1/2" = 1'-0"



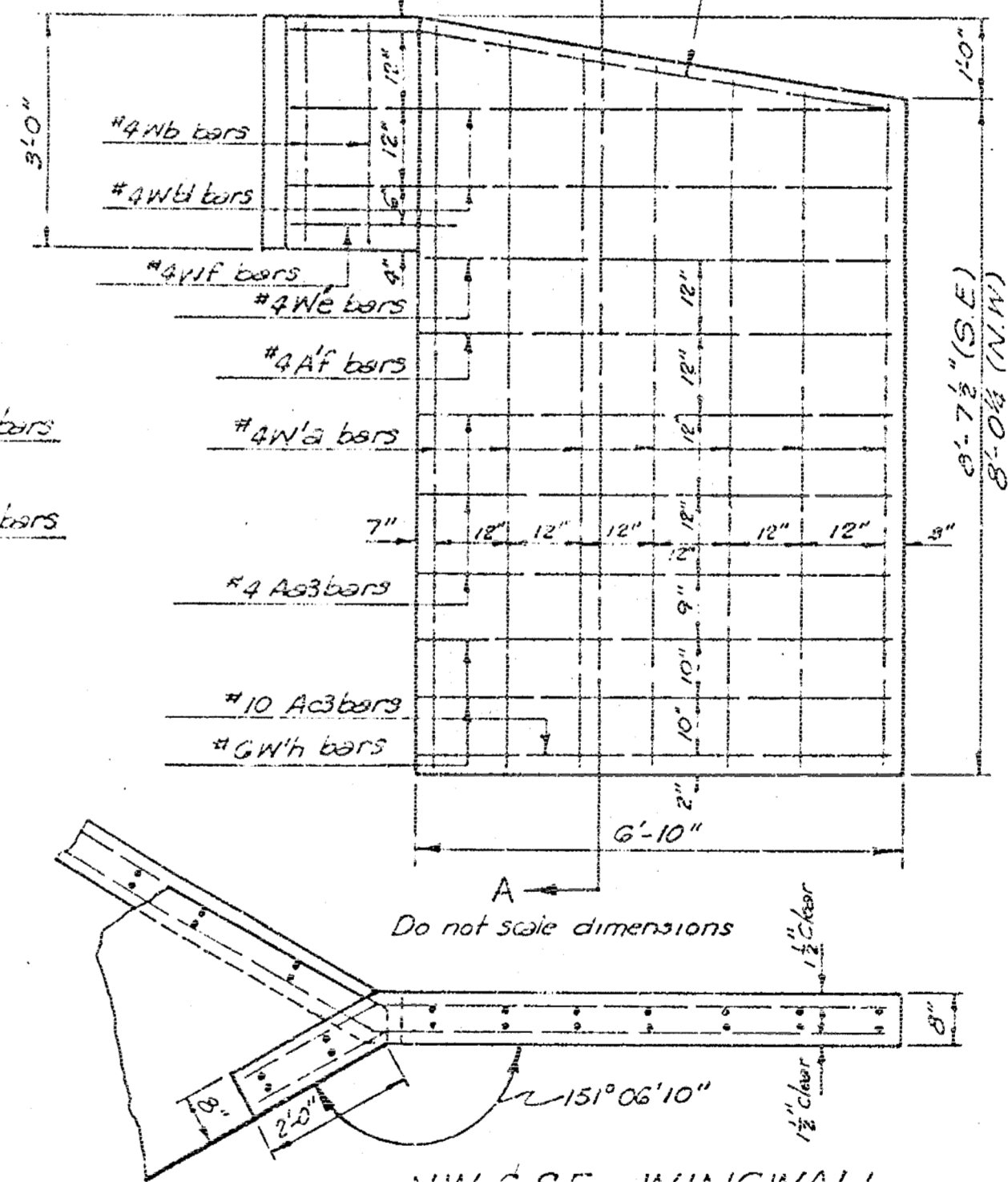
ABUT. #2  
SEC. D-D (Sheet No. 5)  
Scale: 1" = 1'-0"



ABUTMENT NO. 2  
Scale: 1/2" = 1'-0"

DESIGN - AASHTO Spec. H-20-44 loading.  
CONCRETE - Class "A" concrete,  $f_c = 1200$  psi. Bevel all exposed edges with a  $3/8$ " chamfer, unless otherwise noted.  
REINFORCING STEEL - All dimensions shown relative to placing reinforcing steel are to center of bars unless otherwise noted.  $f_y = 20,000$  psi.  
CONSTRUCTION JOINTS - Construction joints shown are optional with the contractor, but if used shall be made only at locations shown or as approved by the Engineer. See pouring sequence.  
PILING - Piling to be driven to refusal or to a computed minimum bearing value of 30 Tons per pile.  
FOUNDATIONS - Test holes No. 1, 2, & 3 drilled with a  $3/8$ " jet by Sedgwick County.  
PLAQUES - Contractor to provide and install two 12" x 16" cast bronze alloy plaques on wall adjacent to end paraspall post. Thickness and finish of plaques shall conform to Section 118.70 of "Kansas Standard Specifications" Sedgwick County to furnish full size detail drawing. Plaques shall not be paid for directly, but shall be considered as subsidiary to other items in the bridge contract.  
HANDRAIL - Aluminum handrail, Kaiser # 3301, Alcoa # 3001, or equal to be used. Anchor bolts for handrail posts shall be of aluminum alloy and conform to manufacturers specifications. These bolts shall not be used for directly but shall be considered subsidiary to Aluminum Handrail.  
OLD STRUCTURE - Bridge contractor to remove existing structure prior to construction of new bridge. Existing bridge consists of 3 simple spans, 28' 28" and 12' 34" - timber pile bents - 6 lines of 18" I-beams - laminated wood deck and timber abutments and wingwalls. All material from the existing bridge deemed salvageable by the engineer shall be handled or disposed of on the site as directed by the engineer. All material not deemed as salvage shall be wasted in a location as designated by the engineer.  
TEST PILES - Two 24" test pile shall be driven in place and in a location designated by the engineer prior to approval of the necessary pile lengths for order.

Note: Horizontal steel for NW Wingwall spaced as shown on Elev. view of N.E. & S.W. wingwall, Sh. #5  
Note: For typical Section A-A of wingwall see Sheet No. 4  
Note: 2# W6 bars on Elev. view of N.E. & S.W. wingwall, Sh. #5



NW & SE WINGWALL  
Scale: 1/2" = 1'-0"

CAP ELEVATIONS

NO.	EL. A (North)	EL. B	EL. C	EL. D (E)	EL. C'	EL. B'	EL. A' (S)
Abut. #1	1337.16	1337.95	1337.74	1337.15	1337.95	1337.71	1337.85
Pier #1	1337.34	1338.12	1338.39	1338.57	1339.58	1338.51	1338.47
Pier #2	1338.65	1338.90	1339.15	1339.31	1339.30	1339.20	1339.11
Pier #3	1339.27	1339.50	1339.72	1339.86	1339.83	1339.71	1339.59
Pier #4	1339.70	1339.90	1340.11	1340.22	1340.17	1340.03	1339.89
Pier #5	1339.34	1339.53	1339.71	1339.82	1339.75	1339.60	1339.46
Abut. #2	1339.46	1339.64	1339.83	1339.93	1339.87	1339.72	1339.57

SUMMARY OF QUANTITIES  
1' EM

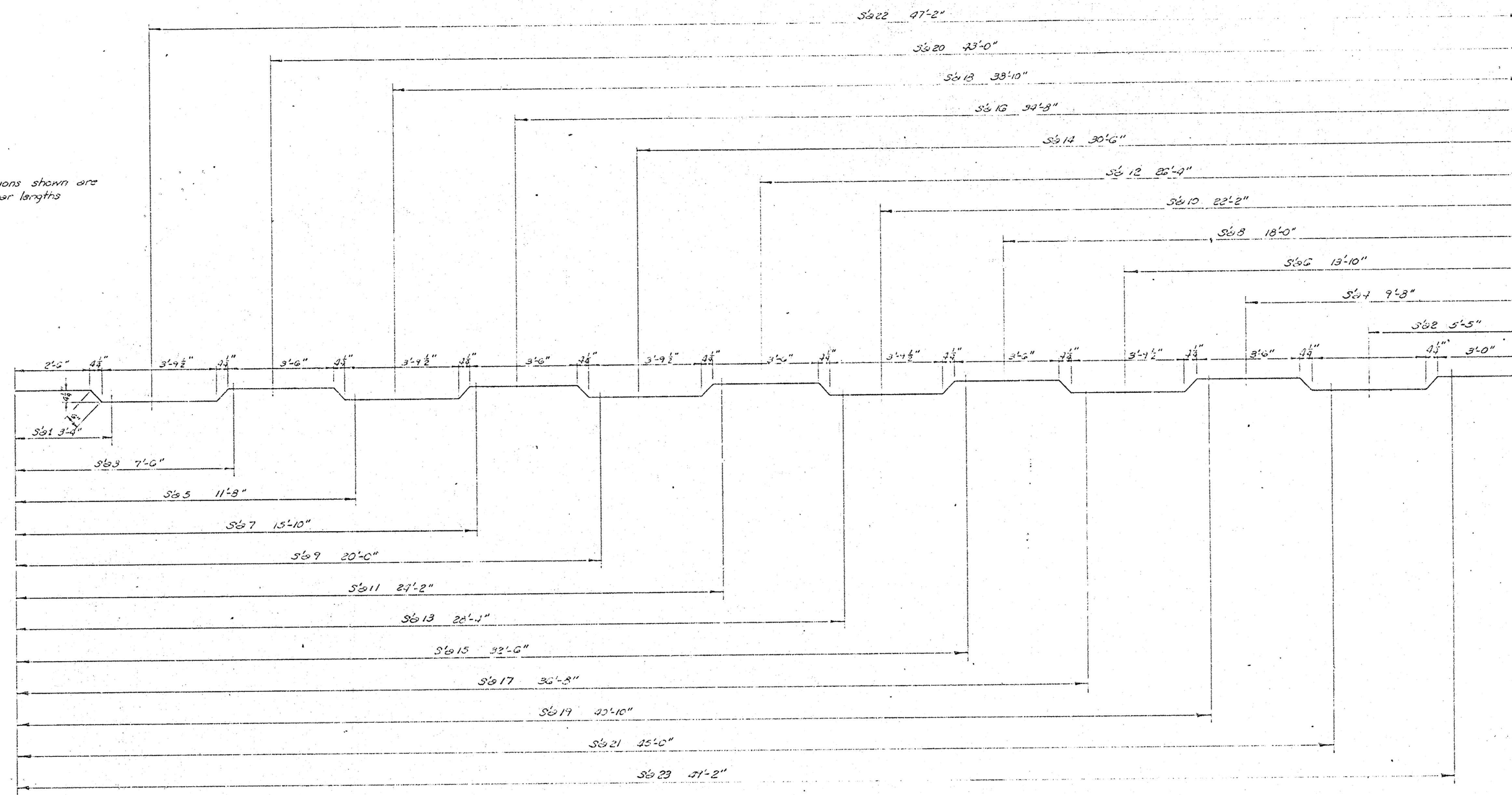
ITEM	QUANTITY	UNIT
Test Piling	72	Lin. Ft.
Concrete, Class A(AE)	3897	Cu. Yd.
Piling	1,398	Lin. Ft.
Structural Steel	128,060	Lbs.
Reinforcing Steel	44,940	Lbs.
Bearing Plates	4,595	Lbs.
Aluminum Handrail	456	Lin. Ft.
Expansion, Common	3,370	Cu. Yd.
12" Heavy Rip-Rap	1090	Cu. Yd.
6" Sand & Gravel Backing	370	Cu. Yd.

ABUTMENT, PIER, AND AUXILIARY DETAILS

PREPARED BY  
SEDGWICK COUNTY ENGINEERING DEPT.  
RUFUS S. KIRK - COUNTY ENGINEER

REVISED	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	As Shown	JVH	JVH	CJF	6 of 8
		DATE 3-57	11-57	11-57	
PLANFILE					TOTAL SHEETS

Notes:  
Dimensions shown are  
actual bar lengths

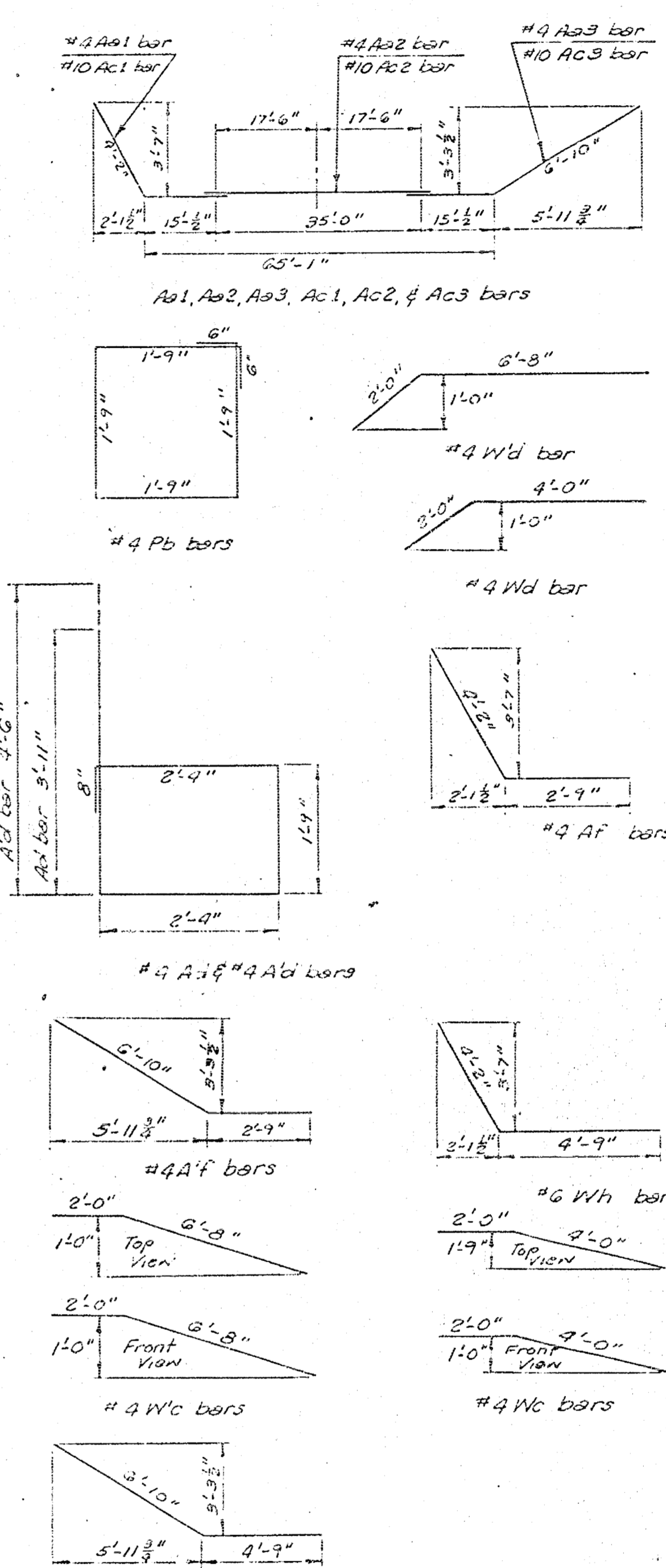


S61 through S623 Bar Diagrams (2 Sets Rebar)  
Scale: 1/8" = 1'-0"

REINFORCING STEEL QUANTITIES

Bar Desig.	Location	Size	No.	Length	Weight	Remarks	Bar Desig.	Location	Size	No.	Length	Weight	Remarks	Bar Desig.	Location	Size	No.	Length	Weight	Remarks
Ao1	Blk wall & Wingwalls	#4	12	19'-10"		Bar diag on Sh. #7	L1	Slab	#4	448	21'-6"		Straight Bar	Wd	NE & SW Wingwall	#4	8	6'-0"		Bar diag Sh. #7
Ao2	"	#4	12	36'-0"		"	L2	"	#4	256	23'-7"		"	Wt	NW & SE Wingwall	#4	8	8'-8"		"
Ao3	"	#4	12	22'-6"		"	L3	"	#4	92	2'-6"		Bar diag Sh. #7	Wt	NE & SW Wingwall	#4	4	"		Straight Bar
Ab	Abutment	#6	6	60'-0"	541	Straight bar	Po	Coop (Piers)	#5	20	60'-0"		Straight Bar	Wt	NW & SE Wingwall	#4	4	"		"
Ac1	Abut. & Wg walls	#10	4	20'-4"		Bar diag on Sh. #7	Pb	"	#4	190	8'-0"		Bar diag Sh. #7	Wt	All Wingwalls	#4	8	2'-3"		"
Ac2	"	#10	4	37'-11"		"	Sa	Slab	#5	135	51'-9"		Bar diag Sh. #7	Wt	Abut. & NE & SW Wwall	#6	8	8'-11"		Bar diag Sh. #7
Ac3	"	#10	4	23'-10"		"	Sa1	"	#5	26	7'-7"	1504	Bar diag Sh. #7	Wt	Abut. & NW & SE Wwall	#6	8	11'-7"		"
Ad	Abutment	#10	4	60'-0"	1033	Straight Bar	Sb	"	#5	134	51'-2"		Bar diag Sh. #7	Wt	NE Wingwall	#4	8	2'-0"		Same position Wt
Ae	Blk wall & Abut.	#4	38	11'-0"		Bar diag on Sh. #7	Sb	"	#5	48	58'-2"	1290	Bar diag Sh. #7	Wt	2 NW	#4	14	2'-0"		"
Af	"	#4	38	11'-7"		"	Sb	"	#5	134	58'-2"		Bar diag Sh. #7	Wt	"	#4	14	2'-0"		"
Ag	"	#4	36	3'-0"		Straight Bar	Sb	"	#5	50	50'-0"	1582	Bar diag Sh. #7	Wt	"	#4	14	2'-0"		"
Ah	"	#4	36	3'-6"		"	Sb	"	#5	1	7'-0"		Bar diag Sh. #7	Wt	"	#4	14	2'-0"		"
Af	Blk wall & NE & SW	#4	4	6'-11"		Bar diag Sh. #7	Wt	SW Wingwall	#4	8	9'-0"	90	Str bar - 2 sets req'd	Wt	NE Wingwall	#4	4	6'-0"		"
Ag	Blk wall & NW & SE Wt	#4	4	9'-7"	28	"	Wt	SE Wingwall	#4	14	16'-8"	168	"	Wt	"	#4	4	6'-0"		"
Ah	Backwall	#4	8	4'-0"		Straight Bar	Wt	All Wingwalls	#4	16	2'-9"	29	Straight Bars	Wt	"	#4	4	6'-0"		"
Ai	Abutment	#4	4	60'-0"	160	"	Wt	NE & SW Wingwalls	#4	4	8'-8"		"	Wt	"	#4	4	6'-0"		"
																	Total	44,940		

NOTE:  
Dimensions of bar lengths are indicated to  
the nearest inch.



BAR DIAGRAMS  
No Scale

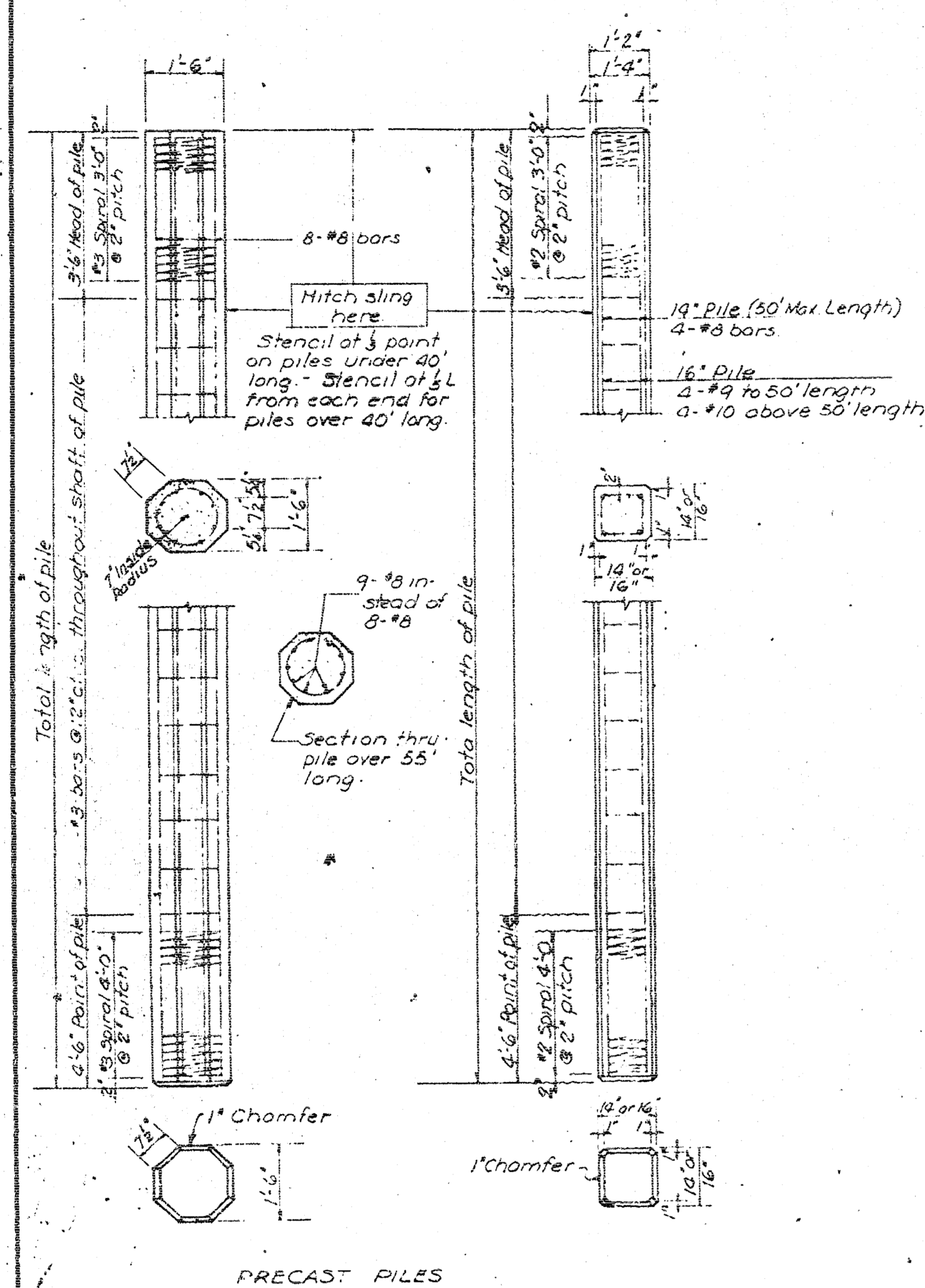
RECD DC - 10 1957

BAR DIAGRAMS  
AND  
REINFORCING STEEL QUANTITIES

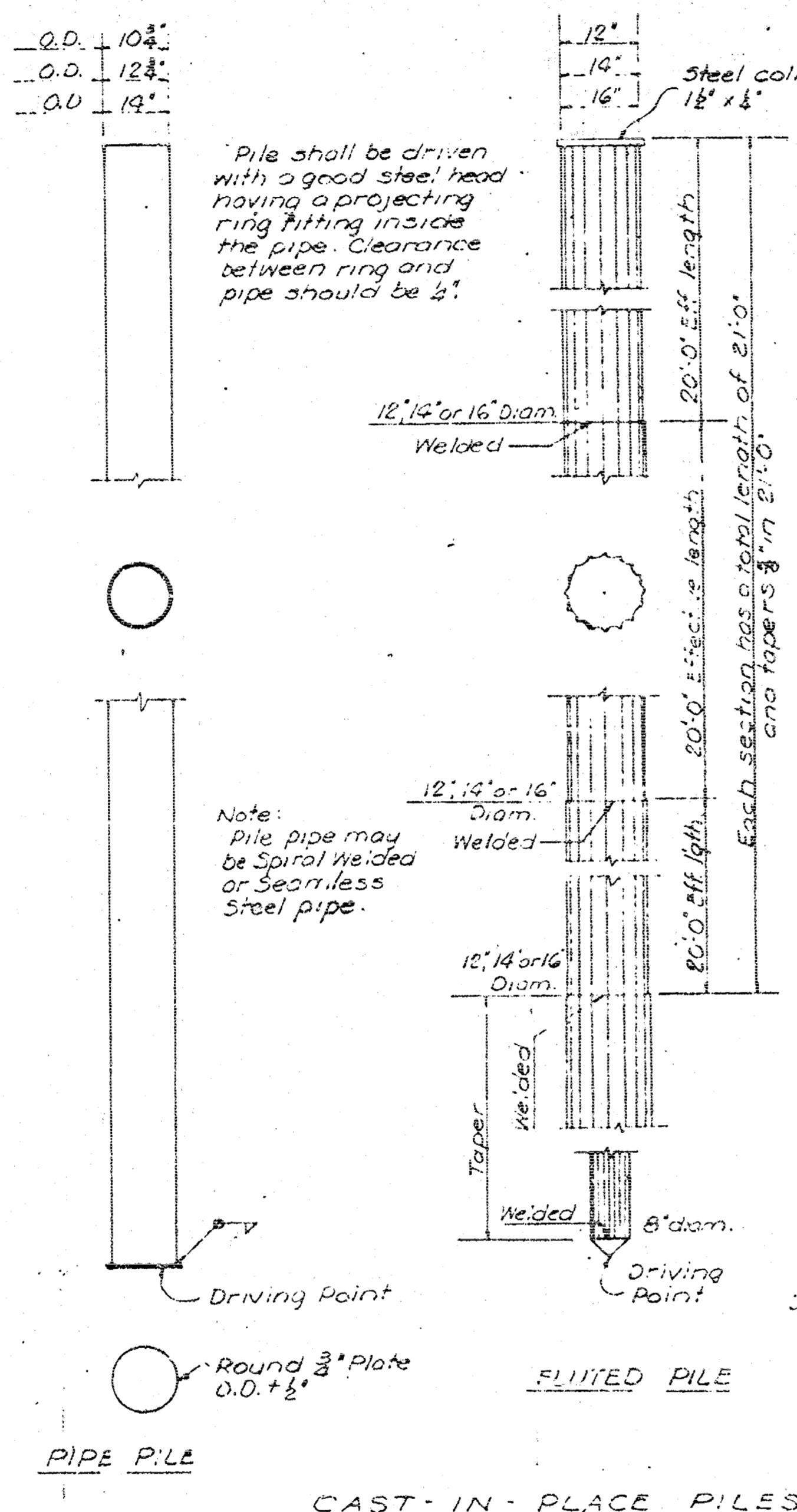
PREPARED BY  
SEDGWICK COUNTY ENGINEERING DEPT.  
RUFUS S. KIRK - COUNTY ENGINEER

REVISION	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	As Shown	J.V.H.	J.V.H.	C.J.F.	7 of 8
		3-57	11-57	11-57	

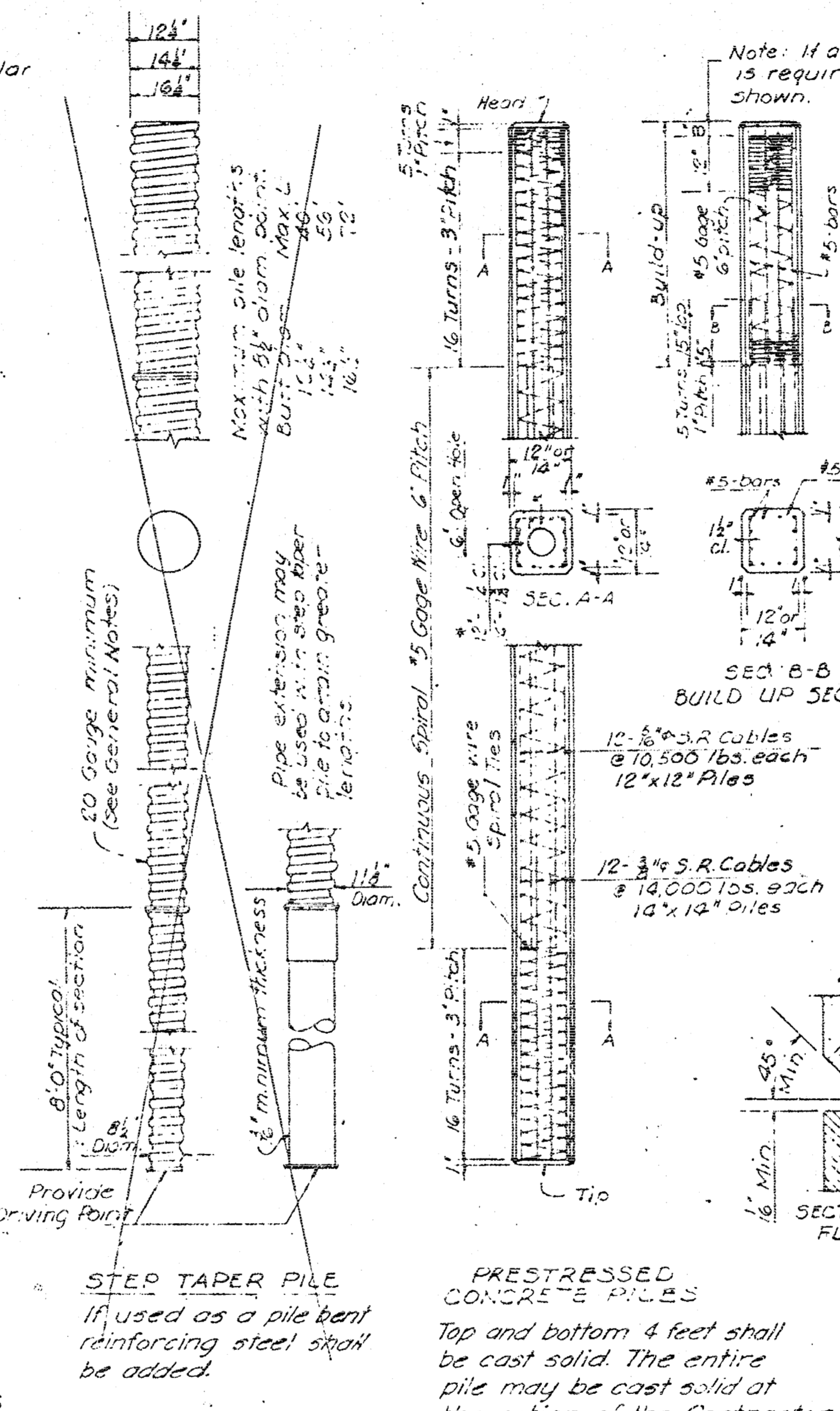
PUR. ROAD DIST. NO.	STATE	PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	KANSAS		1957		



PRECAST PILES



CAST-IN-PLACE PILES



STEPPED TAPER PILE  
 PRESTRESSED CONCRETE PILES

Note: If additional driving is required, use 1" pitch as shown.

Diagram showing single and double point pick-up details. The single point pick-up has a length of 0.7L and a pick-up point of 0.3L. The double point pick-up has a length of L and pick-up points of 0.21L, 0.58L, and 0.21L.

DOUBLE POINT PICK-UP  
 Note: Piles shall be marked at Pick-up Points to indicate proper points for attaching handling lines.

12" x 12" Piles  
 Max. length - 50' Single Point Pick-up  
 Max. length - 75' Double Point Pick-up.

14" x 14" Piles  
 Max. length - 20' Single Point Pick-up  
 Max. length - 35' Double Point Pick-up.

Note: All corners to be chamfered 1".

ALTERNATE POINT BEARING PILES		DESIGN CAPACITY BY DRIVING FORMULA	
MINIMUM BUTT		POINT BEARING	
PRECAST PIPE	EQUIVALENT CONCRETE PILES	14"	12"

SECTION THRU FLANGE PILE SPLICE DETAILS

11. Splices  
 Splice details for cast-in-place or prestressed concrete piles shall be made in accordance with the manufacturers recommendations, subject to the approval of the Engineer. Splice details for steel piles shall be in accordance with the details shown on this sheet.

12. Driving Formula  
 Piles shall be driven to the minimum bearing value as specified on the Construction Layout as determined by the driving formulas stipulated in the Specifications.

13. Mill Test Reports  
 Notarized mill test reports, in triplicate, shall be furnished by the Contractor for all steel pile and cast-in-place pile shells.

14. Payment  
 Payment for all piles will be made as set forth in the Specifications.

15. Test Piles  
 Test piles shall be driven where called for on the Bridge Plans. All test piles shall be located so that they will become part of the Bridge pile system.

General Notes

- Specifications  
 Standard specifications for State Road and Bridge Construction as currently used by State Highway Commission of Kansas.
- Choice of Piles  
 As indicated in the plans, piles will be considered as point bearing piles or as friction piles. Where point bearing piles are specified, the Contractor may elect to use either the steel pile specified on the facing plans or the equivalent precast concrete, cast-in-place concrete or prestressed concrete pile shown in the table on this sheet. Where friction piles are specified the Contractor may elect to use either the size and type concrete pile specified on the facing plans or the equivalent precast concrete, cast-in-place concrete or prestressed concrete pile shown in the table on this sheet. Steel piles are not included as an alternate where friction piles are specified. Other types of concrete piles not shown here are subject to the approval of the Engineer.
- Concrete  
 All concrete for precast and cast-in-place shall be Class 'A' f<sub>c</sub> = 3000 p.s.i.. Concrete for prestressed piles shall be Class AAA f<sub>c</sub> = 4000 p.s.i.
- Reinforcement  
 Reinforcing bars shall be new billet steel of intermediate grade without exception. Hoops and spirals may be either plain or orbanded bars.

- Precast Piles  
 Precast piles shall conform to the requirements of Section 59.310 of the Specifications.
- Cast-in-place Shells  
 A. Pile shells shall have a minimum thickness as follows:
  - Piles driven without mandrel - 8 gage except fluted pile use 9 gage minimum.
  - Piles driven with mandrel shell shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion and/or buckling due to soil pressure after being driven and the mandrel removed.
- Pile shells shall meet the following material requirements:
  - Fluted steel shells and steel collar - SAE 1020 classification for each herein steel.
  - Spiral welded steel - A.S.T.M. A252, Grade 2 Electric Fusion-Welded Spiral Seam Steel Pipe.
  - Seamless steel pipe - A.S.T.M. A252, welded and seamless steel pipe.
  - Corrugated steel shell - SAE 1020.
- The contractor shall maintain on the job at all times prior to and during the filling of the shells, a light suitable for their inspection.

- Improperly driven, broken or otherwise defective shells shall be removed and replaced or otherwise corrected to the satisfaction of the Engineer by removal and replacement, or the driving of an additional pile at no extra cost.
- Steel Piles  
 Steel pile material shall meet the requirements of A.S.T.M. A7-53T.
- Pile Points  
 All cast-in-place piles shall be equipped with a steel driving point of 1/4" minimum thickness. Driving points shall be mill welded to the pile shell. Driving points shall be either hot pressed steel meeting the requirements of SAE 1020 for forged steel, or cast steel meeting the requirements of A.S.T.M. Serial No. A-87-43, Grade 2, or structural steel meeting the requirements of A.S.T.M. A7-53T. No driving point is required. Steel piles shall have a square cut end only.
- Welding  
 All field welding shall meet the requirements of Section 51.34 of the Specifications.
- Paint  
 Shall comply with the Kansas Standard Specifications, (1955 Edition).

REC'D DEC 19 1957

NO.	DATE	BY	APP'D

STATE HIGHWAY COMMISSION OF KANSAS

STANDARD PILE DETAILS Sheet 8 of 8

SHEET NO.	OF	SCALE	APP'D	QUANTITIES	TRACED