By: Date: Page: Of: BAUGHMAN COMPANY, P.A.

Existing Flow Rates

Auburn Hills 3rd Addition

Area = 9.2 acres C = 0.4 Tc = 15 min.

Q_{100} = 27 cfs

Auburn Hills Commercial Addition

Area = 12.6 acres C = 0.4 Tc = 15 min.

Q_{100} = 37.1

Net existing flow both sites = 64 cfs

Auburn Hills 3rd Stg Allows 20 cfs to be released backyard from Auburn Hls. 3rd release 7 cfs

Total from Auburn Hills 3rd = 27 cfs

Allowable release fm Auburn Hills Commercial = 64-27 = 37cfs

Lot 3 Auburn Hills Commercial allows 5cfs to drain to Maple.

Allowable discharge from detention pond = 32 cfs

24" RCP as outlet allows the release of 26 cfs

Net change in Q_{100} = 6 cfs lower

```
*****************************************
*                                       *
*  FLOOD HYDROGRAPH PACKAGE (HEC-1)     *
*              MAY  1991                *
*            VERSION 4.0.1E             *
*  Lahey F77L-EM/32 version 5.01        *
*     Dodson & Associates, Inc.         *
* RUN DATE 05/22/97  TIME 09:39:30      *
*                                       *
*****************************************

*****************************************
*                                       *
*  U.S. ARMY CORPS OF ENGINEERS         *
*  HYDROLOGIC ENGINEERING CENTER        *
*     609 SECOND STREET                 *
*     DAVIS, CALIFORNIA 95616           *
*     (916) 551-1748                    *
*                                       *
*****************************************

X   X XXXXXXX  XXXXX        X
X   X X       X     X      XX
X   X X       X            X
XXXXXXX XXXX  X       XXXXX X
X   X X       X            X
X   X X       X     X      X
X   X XXXXXXX XXXXX        XXX

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HECIGS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -ARSXX- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL  LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1                              HEC-1 INPUT                                PAGE  1

LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
    1     ID  AUBURN HILLS COMMERCIAL DETENTION
    2     IT      5 22MAY97  00000     300
    3     IO      3     0
          *
    4     KK  AUBCOM
    5     BA    .018
    6     PH      .0     0   0.86  1.8425   3.73    4.6   5.04   5.94   6.96   8.16
    7     UD     .15
    8     LS      0    91
          *
    9     KK  POND
   10     RS      1   ELEV    39
   11     SA     .8    1.0
   12     SE     39   43.0
   13     SL   39.5   3.14     .67     .5
   14     SS   42.5     10   2.86    1.5
          *
   15     ZZ
```

```
*****************************************
*                                       *
*  FLOOD HYDROGRAPH PACKAGE (HEC-1)     *
*              MAY  1991                *
*            VERSION 4.0.1E             *
*  Lahey F77L-EM/32 version 5.01        *
*     Dodson & Associates, Inc.         *
* RUN DATE 05/22/97  TIME 09:39:36      *
*                                       *
*****************************************

*****************************************
*                                       *
*  U.S. ARMY CORPS OF ENGINEERS         *
*  HYDROLOGIC ENGINEERING CENTER        *
*     609 SECOND STREET                 *
*     DAVIS, CALIFORNIA 95616           *
*     (916) 551-1748                    *
*                                       *
*****************************************

                AUBURN HILLS COMMERCIAL DETENTION

3 IO        OUTPUT CONTROL VARIABLES
                IPRNT        3  PRINT CONTROL
                IPLOT        0  PLOT CONTROL
                QSCAL        0.  HYDROGRAPH PLOT SCALE

IT          HYDROGRAPH TIME DATA
                NMIN         5  MINUTES IN COMPUTATION INTERVAL
                IDATE    22MAY97  STARTING DATE
                ITIME        0000  STARTING TIME
                NQ            300  NUMBER OF HYDROGRAPH ORDINATES
                NDDATE   23MAY97  ENDING DATE
                NDTIME       0055  ENDING TIME
                ICENT          19  CENTURY MARK

                COMPUTATION INTERVAL     0.08 HOURS
                TOTAL TIME BASE    24.92 HOURS

ENGLISH UNITS
                DRAINAGE AREA            SQUARE MILES
                PRECIPITATION DEPTH      INCHES
                LENGTH, ELEVATION        FEET
                FLOW                     CUBIC FEET PER SECOND
                STORAGE VOLUME           ACRE-FEET
                SURFACE AREA             ACRES
                TEMPERATURE              DEGREES FAHRENHEIT

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
```

```
4 KK        ************
            *          *
            *  AUBCOM  *
            *          *
            ************

          SUBBASIN RUNOFF DATA

5 BA        SUBBASIN CHARACTERISTICS
                TAREA        0.02  SUBBASIN AREA

            PRECIPITATION DATA

6 PH                    DEPTHS FOR  0-PERCENT HYPOTHETICAL STORM
                ---- HYDRO-35 ----  ---------- TP-40 ----------  ---------- TP-49 ----------
                5-MIN  15-MIN 60-MIN   2-HR   3-HR   6-HR   12-HR  24-HR  2-DAY  4-DAY  7-DAY  10-DAY
                 0.86   1.84   3.73   4.60   5.04   5.94   6.96   8.16   0.00   0.00   0.00   0.00

                                STORM AREA =    0.02

8 LS        SCS LOSS RATE
                STRTL        0.20  INITIAL ABSTRACTION
                CRVNBR      91.00  CURVE NUMBER
                RTIMP        0.00  PERCENT IMPERVIOUS AREA

7 UD        SCS DIMENSIONLESS UNITGRAPH
                TLAG         0.15  LAG

                                ***

                                UNIT HYDROGRAPH
                                11 END-OF-PERIOD ORDINATES
                    17.     44.     39.     20.     10.      5.      2.      1.      1.      0.
                     0.

 ***        ***        ***        ***        ***

                        HYDROGRAPH AT STATION   AUBCOM

 TOTAL RAINFALL =  8.16, TOTAL LOSS =   1.08, TOTAL EXCESS =   7.08

 PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR      72-HR      24.92-HR
+  (CFS)       (HR)
                          (CFS)
<      75.    12.17          11.        3.         3.         3.
                 (INCHES)   5.561      7.082      7.082      7.082
                 (AC-FT)      5.         7.         7.         7.

                        CUMULATIVE AREA =   0.02 SQ MI

*** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** *** ***
```

```
            ************
            *          *
            *  POND    *
            *          *
            ************

          HYDROGRAPH ROUTING DATA

10 RS       STORAGE ROUTING
                NSTPS        1  NUMBER OF SUBREACHES
                ITYP       ELEV  TYPE OF INITIAL CONDITION
                RSVRIC     39.00  INITIAL CONDITION
                X             0.00  WORKING R AND D COEFFICIENT

11 SA           AREA         0.8       1.0

12 SE           ELEVATION   39.00     43.00

13 SL           LOW-LEVEL OUTLET
                ELEVL       39.50  ELEVATION AT CENTER OF OUTLET
                CAREA        5.14  CROSS-SECTIONAL AREA
                CQSL         0.67  COEFFICIENT
                EXPL         0.50  EXPONENT OF HEAD

14 SS           SPILLWAY
                CREL        42.50  SPILLWAY CREST ELEVATION
                SPWID       10.00  SPILLWAY WIDTH
                COGW         2.86  WEIR COEFFICIENT
                EXPW         1.50  EXPONENT OF HEAD

                                ***

                        COMPUTED STORAGE-ELEVATION DATA

          STORAGE     0.00     3.59
          ELEVATION  39.00    43.00

                        COMPUTED OUTFLOW-ELEVATION DATA

          OUTFLOW     0.00     0.00    16.48    17.57    18.82    20.27    21.95    23.93    26.32    29.22
          ELEVATION  39.00    39.50    40.45    40.58    40.74    40.94    41.19    41.51    41.93    42.50

          OUTFLOW    29.52    29.84    30.34    31.05    32.01    33.24    34.79    36.69    38.97    41.68
          ELEVATION  42.53    42.55    42.59    42.62    42.67    42.72    42.78    42.85    42.92    43.00

                        COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

          STORAGE     0.00     0.41     1.21     1.33     1.47     1.65     1.87     2.16     2.56     3.10
          OUTFLOW     0.00     0.00    16.48    17.57    18.82    20.27    21.95    23.93    26.32    29.22
          ELEVATION  39.00    39.50    40.45    40.58    40.74    40.94    41.19    41.51    41.93    42.50

          STORAGE     3.13     3.15     3.18     3.22     3.26     3.32     3.37     3.44     3.51     3.59
          OUTFLOW    29.52    29.84    30.34    31.05    32.01    33.24    34.79    36.69    38.97    41.68
          ELEVATION  42.53    42.55    42.59    42.62    42.67    42.72    42.78    42.85    42.92    43.00

 ***        ***        ***        ***        ***        ***

                        HYDROGRAPH AT STATION   POND

 PEAK FLOW     TIME                      MAXIMUM AVERAGE FLOW
                                    6-HR       24-HR      72-HR      24.92-HR
+  (CFS)       (HR)
                          (CFS)
+     26.    12.58          11.        3.         3.         3.
                 (INCHES)   5.519      6.645      6.645      6.645
                 (AC-FT)      5.         6.         6.         6.

 PEAK STORAGE  TIME                      MAXIMUM AVERAGE STORAGE
                                    6-HR       24-HR      72-HR      24.92-HR
+  (AC-FT)     (HR)
        3.    12.58                    1.         0.         0.         0.

 PEAK STAGE    TIME                      MAXIMUM AVERAGE STAGE
                                    6-HR       24-HR      72-HR      24.92-HR
+  (FEET)      (HR)
       41.91  12.58                   40.27      39.59      39.57      39.57

                        CUMULATIVE AREA =   0.02 SQ MI
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                RUNOFF SUMMARY
          FLOW IN CUBIC FEET PER SECOND
          TIME IN HOURS, AREA IN SQUARE MILES

+                                  AVERAGE FLOW FOR MAXIMUM PERIOD                 MAXIMUM   TIME OF
+  OPERATION   STATION  PEAK FLOW  TIME OF PEAK  6-HOUR   24-HOUR   72-HOUR  BASIN AREA  STAGE  MAX STAGE
+  HYDROGRAPH AT
+                AUBCOM      75.      12.17       11.       3.        3.        0.02
   ROUTED TO
+                POND        26.      12.58       11.       3.        3.        0.02
+                                                                                41.91    12.58

*** NORMAL END OF HEC-1 ***
```

DOT Report

Pipe	-Node- Upstream	Downstream	Inlet Area (acres)	Inlet CA (acres)	Total CA (acres)	-Ground- Upstream	Downstream (ft)	-HGL- Upstream	Downstream (ft)	-Slope- Energy Constructed (ft/ft)	-Section- Discharge Capacity (cfs)	-Section- Shape	Size	Length (ft)	Average Velocity (ft/s)	Description
P-7	I-6	I-7	0.80	0.43	0.43	143.30	143.30	143.41	143.30	0.000934	3.21	Circular	18 inch	121.00	1.82	
P-8	I-7	I-8	0.50	0.27	0.70	143.30	143.30	143.77	143.30	0.003306	6.04	Circular	18 inch	205.00	2.87	
P-9	I-8	I-9	0.50	0.27	0.97	143.30	143.30	143.77	143.30	0.002327	5.07	Circular	18 inch	87.00	3.85	
P-10	I-9	I-10	2.30	1.24	2.21	144.00	142.82	143.30	143.18	0.004205	6.81	Circular	18 inch			
P-11	I-10	I-11	0.80	0.43	2.65	144.00	142.63	144.00	142.63	0.003448	6.17	Circular	18 inch	34.00	4.90	
P-12	I-11	Outlet	2.00	1.08	3.73	144.00	142.47	144.00	142.47	0.004622	15.38	Circular	24 inch	178.00	5.98	
						144.00	142.21	144.00	142.21	0.002941	12.27	Circular	24 inch			
						144.60	141.01	144.00	141.01	0.006415	18.33	Circular	24 inch			
						144.60	140.62	144.60	140.62	0.002247	10.72	Circular	24 inch			
						142.00	139.00	144.60	140.62	0.011649	25.52	Circular	30 inch	170.00	6.14	
								142.00	139.00	0.022941	62.12					

Project Title: LARK 6TH SYSTEM 1
I:\hydro\projects\lark6\system2.stm
05/21/97 11:10:28 AM © Haestad Methods, Inc. 37 Brookside Road Waterbury CT 06708 USA (203) 755-1666
Baughman Company, P.A.
Project Engineer: Baughman Company, P.A.
StormCAD v1.0
Page 1 of 1

Pipe Report

Pipe	Upstream Node	Downstream Node	Weighted Roughness Coefficient	Section Size	Roughness	Constructed Slope (ft/ft)	Length (ft)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Upstream Ground Elevation (ft)	Inlet Area (acres)	Total CA (acres)
P-20	I-20	I-19	0.54	18 inch	0.013	0.028070	114.00	140.00	136.80	143.00	1.80	0.97
P-26	I-15	I-16	0.54	18 inch	0.013	0.002747	182.00	140.90	140.40	143.70	0.90	0.49
P-16	I-17	I-16	0.54	18 inch	0.013	0.002941	34.00	140.40	140.30	144.10	2.10	1.13
P-17	I-16	I-14	0.54	24 inch	0.013	0.001978	253.00	140.20	139.70	144.10	2.60	3.02
P-27	I-12	I-13	0.54	18 inch	0.013	0.003082	292.00	140.90	140.00	144.40	0.90	0.49
P-14	I-13	I-14	0.54	24 inch	0.013	0.001176	85.00	139.50	139.40	145.20	0.00	0.49
P-18	I-14	I-18	0.54	36 inch	0.013	0.002778	36.00	139.30	139.20	144.50	2.20	4.70
P-19	I-18	I-19	0.54	36 inch	0.013	0.010185	216.00	139.00	136.80	144.50	1.80	5.67
P-21	I-19	J-2	0.54	36 inch	0.013	0.001538	130.00	136.60	136.40	141.00	0.90	7.13
P-22	J-2	Outlet	N/A	36 inch	0.013	0.016667	60.00	136.00	135.00	141.00	N/A	7.13

Project Title: LATK 6TH SYSTEM 1
I:\hydro\projects\lark6\system3.stm
05/21/97 11:09:07 AM © Haestad Methods, Inc. 37 Brookside Road Waterbury, CT 06708 USA (203) 755-1666
Baughman Company, P.A.
Project Engineer: Baughman Company, P.A.
StormCAD v1.0
Page 1 of 1

Node Report

Node	Inlet Area (acres)	Weighted Roughness Coefficient	Ground Elevation (ft)	Inlet CA (acres)	External CA (acres)	Carryover (cfs)	Total CA (acres)	Additional Flow (cfs)	Inlet TC (min)	Known Flow (cfs)	External TC (min)	Upstream Flow Time (min)	System Flow Time (min)
I-20	1.80	0.54	143.00	0.97	0.00	0.00	0.97	0.00	15.00	0.00	0.00	0.00	15.00
I-15	0.90	0.54	143.70	0.49	0.00	0.00	0.49	0.00	15.00	0.00	0.00	0.00	15.00
I-17	2.10	0.54	144.10	1.13	0.00	0.00	1.13	0.00	15.00	0.00	0.00	0.00	15.00
I-16	2.60	0.54	144.10	1.40	0.00	0.00	3.02	0.00	15.00	0.00	0.00	17.53	17.53
I-12	0.80	0.54	144.40	0.49	0.00	0.00	0.49	0.00	15.00	0.00	0.00	0.00	15.00
I-13	0.00	0.54	145.20	0.00	0.00	0.00	0.49	0.00	15.00	2.00	0.00	17.27	17.27
I-14	2.20	0.54	144.50	1.19	0.00	0.00	4.70	0.00	15.00	2.00	0.00	18.50	18.50
I-18	1.80	0.54	144.50	0.97	0.00	0.00	5.67	0.00	15.00	2.00	0.00	18.63	18.63
I-19	0.90	0.54	141.00	0.49	0.00	0.00	7.13	0.00	15.00	2.00	0.00	19.39	19.39
J-2	N/A	N/A	141.00	N/A	N/A	N/A	7.13	N/A	N/A	N/A	0.00	19.95	19.95
Outlet	N/A	N/A	140.00	N/A	N/A	N/A	7.13	N/A	N/A	N/A	0.00	20.23	20.23

Pipe Report

Pipe	Upstream Node	Downstream Node	Weighted Roughness Coefficient	Section Size	Roughness	Constructed Slope (ft/ft)	Length (ft)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Upstream Ground Elevation (ft)	Inlet Area (acres)	Total CA (acres)
P-6	I-5	I-2	0.54	18 inch	0.013	0.002972	286.00	38.90	38.05	41.00	1.60	0.86
P-1	I-1	I-2	0.54	18 inch	0.013	0.002727	165.00	38.50	38.05	43.00	0.80	0.43
P-2	I-2	I-3	0.54	18 inch	0.013	0.003007	143.00	37.93	37.50	41.00	0.80	1.73
P-3	I-3	I-4	0.54	24 inch	0.013	0.004167	36.00	37.40	37.25	43.00	1.00	2.27
P-4	I-4	J-1	0.54	30 inch	0.013	0.002083	144.00	37.15	36.85	43.00	1.10	2.86
P-5	J-1	Outlet	N/A	30 inch	0.013	0.001923	26.00	36.75	36.70	40.00	N/A	2.86

Node Report

Node	Inlet Area (acres)	Weighted Roughness Coefficient	Ground Elevation (ft)	Inlet CA (acres)	External CA (acres)	Carryover (cfs)	Total CA (acres)	Additional Flow (cfs)	Inlet TC (min)	Known Flow (cfs)	External TC (min)	Upstream Flow Time (min)	System Flow Time (min)
I-5	1.60	0.54	41.00	0.86	0.00	0.00	0.86	0.00	15.00	0.00	0.00	0.00	15.00
I-1	0.80	0.54	43.00	0.43	0.00	0.00	0.43	0.00	15.00	0.00	0.00	0.00	15.00
I-2	0.80	0.54	41.00	0.43	0.00	0.00	1.73	0.00	15.00	0.00	0.00	16.51	16.51
I-3	1.00	0.54	43.00	0.54	0.00	0.00	2.27	0.00	15.00	0.00	0.00	16.86	16.86
I-4	1.10	0.54	43.00	0.59	0.00	0.00	2.86	0.00	15.00	0.00	0.00	16.97	16.97
J-1	N/A	N/A	40.00	N/A	N/A	N/A	2.86	N/A	N/A	N/A	0.00	17.43	17.43
Outlet	N/A	N/A	39.00	N/A	N/A	N/A	2.86	N/A	N/A	N/A	0.00	17.51	17.51

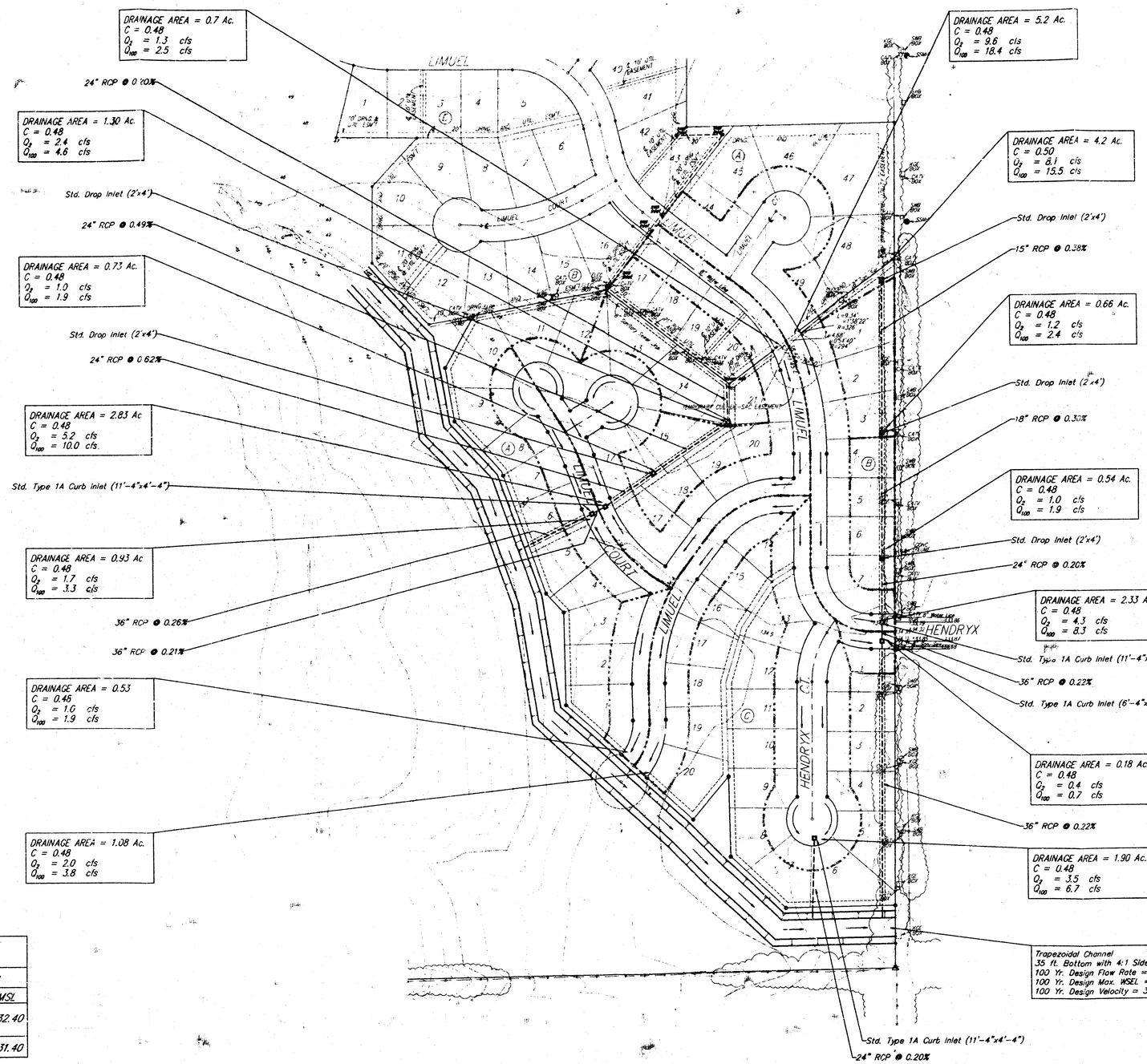
DOT Report

Pipe	-Node- Upstream Downstream	Inlet Area (acres)	Inlet CA (acres)	Total CA (acres)	-Ground- Upstream Downstream (ft)	-HGL- Upstream Downstream (ft)	-Slope- Energy Constructed (ft/ft)	-Section- Discharge Capacity (cfs)	-Section- Shape Size	Length (ft)	Average Velocity (ft/s)	Description
P-6	I-5	1.60	0.86	0.86	41.00	42.07	0.003734	6.42	Circular	286.00	3.63	
	I-2				41.00	41.00	0.002972	5.73	18 inch			
P-1	I-1	0.80	0.43	0.43	43.00	41.15	0.009834	3.21	Circular	165.00	1.82	
	I-2				41.00	41.00	0.002727	5.49	18 inch			
P-2	I-2	0.80	0.43	1.73	41.00	41.42	0.013813	12.35	Circular	143.00	6.99	
	I-3				43.00	39.44	0.003007	5.76	18 inch			
P-3	I-3	1.00	0.54	2.27	43.00	39.36	0.004495	16.06	Circular	36.00	5.14	
	I-4				43.00	39.20	0.004167	14.60	24 inch			
P-4	I-4	1.10	0.59	2.86	43.00	39.12	0.002940	20.21	Circular	144.00	5.20	
	J-1				40.00	38.59	0.002083	18.72	30 inch			
P-5	J-1	N/A	N/A	2.86	40.00	38.50	0.004034	19.99	Circular	26.00	5.93	
	Outlet				39.00	38.22	0.001923	17.99	30 inch			

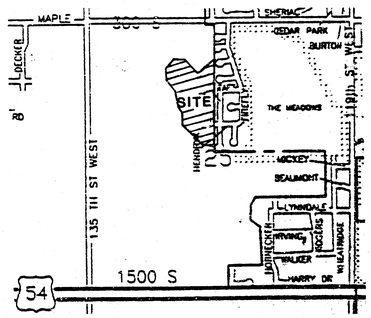
DRAINAGE PLAN

AUBURN HILLS 2nd ADDITION

SEDGWICK COUNTY, KANSAS

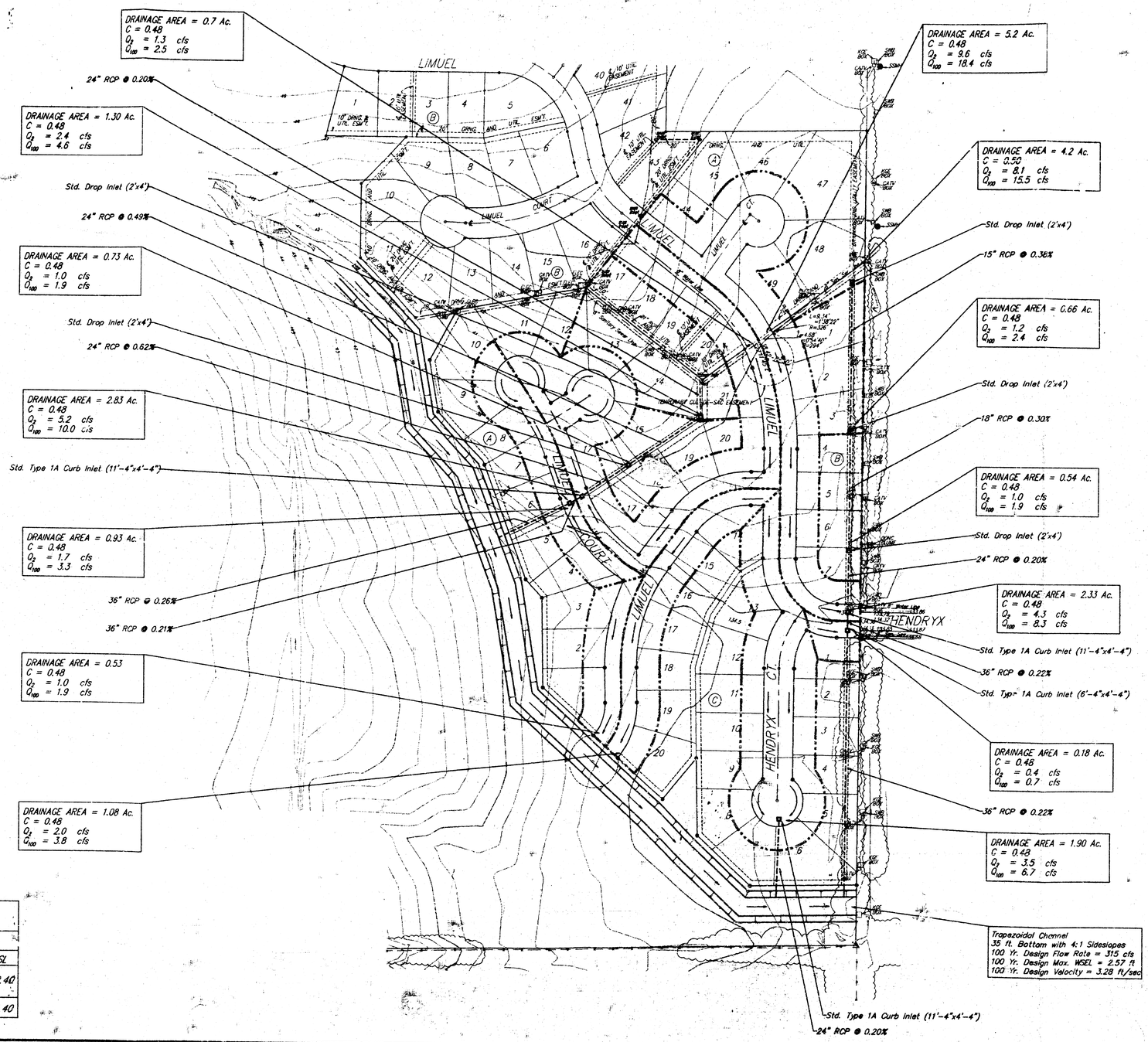


MINIMUM BUILDING PAD ELEV. LOWEST OPENING TO STRUCTURE			
LOT	BLOCK	ELEVATION	
		CITY DATUM	MSL
1, 2, 3, 4, 5, 6, 7, 8, 9, 10	A	145.00	1332.40
6, 7, 8, 9, 20	C	144.00	1331.40

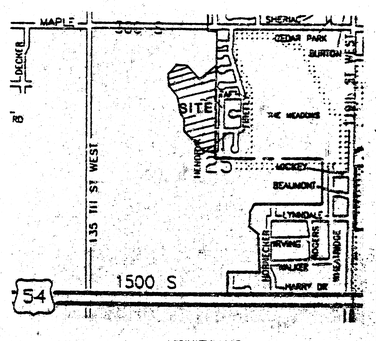


VICINITY MAP

DRAINAGE PLAN
AUBURN HILLS 2nd ADDITION
SEDGWICK COUNTY, KANSAS



MINIMUM BUILDING PAD ELEV. LOWEST OPENING TO STRUCTURE			
LOT	BLOCK	ELEVATION	
		CITY DATUM	MSL
1, 2, 3, 4, 5, 6, 7, 8, 9, 10	A	145.00	1332.40
6, 7, 8, 9, 20	C	144.00	1331.40

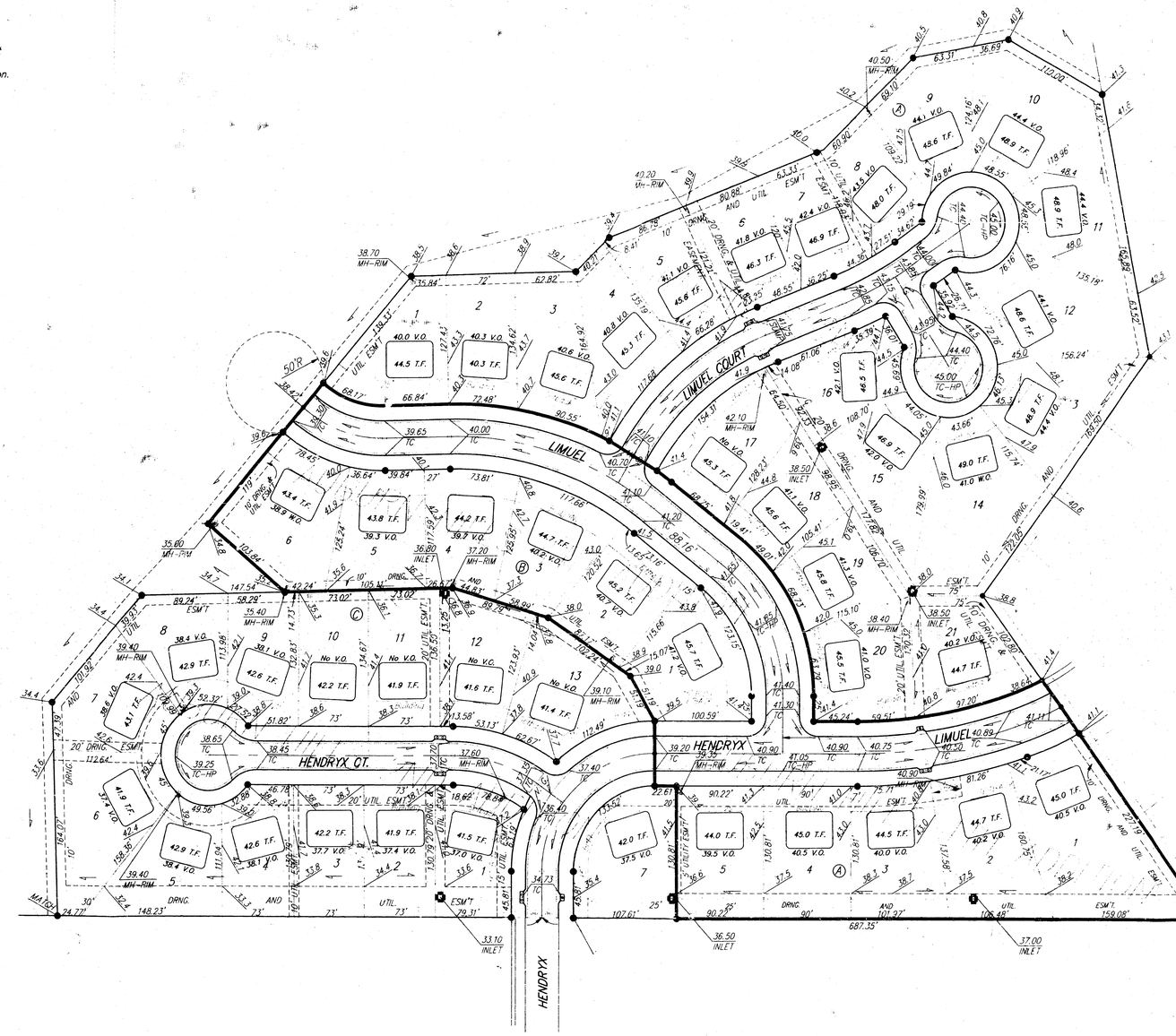


VICINITY MAP

BENCHMARKS:

BM #1 "□" On Top of Curb Located on the South Side of Hendryx 15' West of the East Line of Auburn Hills 2nd Addition. Elev. = 134.32 City Datum

BM #2 "□" On Top of Curb Located at 14.29' North and 38.05' West of the Northeast corner of Lot 13, Block B, Auburn Hills Addition. Elev. = 144.83 City Datum


LIMUEL COURT
LIMUEL
HENDRYX CT.
HENDRYX
50'R
20' DRNG. & UTIL. ESM'T
UTIL. ESM'T
DRNG. ESM'T
ESM'T
DRNG.
UTIL.
AND
MATCH
38.70 MH-RIM
40.20 MH-RIM
40.50 MH-RIM
42.10 MH-RIM
35.60 MH-RIM
37.20 MH-RIM
39.40 MH-RIM
39.10 MH-RIM
39.20 MH-RIM
37.60 MH-RIM
38.50 INLET
36.80 INLET
33.10 INLET
36.50 INLET
37.00 INLET
45.00 TC-HP
39.25 TC-HP
41.05 TC-HP
38.65 TC
39.65 TC
40.00 TC
40.70 TC
41.10 TC
41.20 TC
41.30 TC
41.40 TC
44.40 TC
18.45 TC
37.40 TC
34.73 TC
40.89 TC
40.50 TC
1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18 19 20 21
40.0 V.O. 44.5 T.F.
40.3 V.O. 40.3 T.F.
40.6 V.O. 45.6 T.F.
40.8 V.O. 45.3 T.F.
41.1 V.O. 45.6 T.F.
41.8 V.O. 46.3 T.F.
42.4 V.O. 46.9 T.F.
43.0 V.O. 48.0 T.F.
44.1 V.O. 43.6 T.F.
44.4 V.O. 48.9 T.F.
No V.O. 41.9 T.F.
No V.O. 41.6 T.F.
No V.O. 41.4 T.F.
No V.O. 42.2 T.F.
No V.O. 42.6 T.F.
38.1 V.O. 38.4 T.F.
38.4 V.O. 42.9 T.F.
41.9 T.F.
42.4 T.F.
43.1 T.F.
43.4 T.F. 38.9 W.O.
43.8 T.F. 39.3 V.O.
44.2 T.F. 39.2 V.O.
44.7 T.F. 40.2 V.O.
45.2 T.F. 40.7 V.O.
45.7 T.F. 41.0 V.O.
45.5 T.F. 41.0 V.O.
45.8 T.F. 41.3 V.O.
43.6 T.F. 41.1 V.O.
45.3 T.F. No V.O.
46.5 T.F. 44.9
46.9 T.F. 42.0 V.O.
49.0 T.F. 41.0 W.O.
42.0 T.F. 37.5 V.O.
44.0 T.F. 39.5 V.O.
45.0 T.F. 40.5 V.O.
44.5 T.F. 40.0 V.O.
44.7 T.F. 40.2 V.O.
45.0 T.F. 40.5 V.O.
41.5 T.F. 37.0 V.O.
42.2 T.F. 37.7 V.O.
41.9 T.F. 37.4 V.O.
42.6 T.F. 38.1 V.O.
42.9 T.F. 38.4 V.O.
41.9 T.F.
43.1 T.F.
39.6 V.O.
42.4
42.4
41.9 T.F.
42.4
42.9 T.F.
38.4 V.O.
39.40 MH-RIM
35.84'
72'
62.82'
66.84'
72.48'
90.55'
68.17'
39.65 TC
40.00 TC
40.70 TC
41.10 TC
41.20 TC
41.30 TC
41.40 TC
44.40 TC
18.45 TC
37.40 TC
34.73 TC
40.89 TC
40.50 TC
45.00 TC-HP
39.25 TC-HP
41.05 TC-HP
38.65 TC