

DATE	BY

SUMMARY OF BRIDGE QUANTITIES

ITEM	ITEM QUANTITY	UNIT
Embankment (Granular Fill)	1,695	CY
Excavation (Class III)	886	CY
Concrete Grade 4.0 (AE)	384	CY
Concrete Grade 4.0 (AE)	944	CY
Concrete Masonry Coating	685	SY
Concrete Saddle	685	SY
Graffiti Control System	607	SY
Structural Steel (ASTM A709 Gr 50T2) (Bridges)	602,990	LBS
Structural Steel (ASTM A500, Gr B) (Collision Beam)	20,050	LBS
Expansion Device (Sliding Plate)	127	LF
Headed Stud Anchors	3,600	EA
Reinforcing Steel (Gr. 60)	124,470	LBS
Reinforcing Steel (Epoxy-Coated)	48,820	LBS
Steel Piles (HP 14X89)	10,362	LF
Test Pile (Special) (HP 14X89)	364	LF
Dynamic Pile Testing	4	EA
Electromechanical Devices	8	EA
Steel Bearing Device (EXP)	9,280	LBS
Steel Bearing Device (FIX)	11,800	LBS
Bridge Handrail (Steel-Type 2 & 2A)	288	LF
Abutment Strip Drain	434	SY
Bridge Backwall Protection System	434	SY
Pipe Underdrains (4.0") (Type K)	176	LF
Waterproofing (Deck)	505	SY
Waterproofing (Pier-Bridges)	154	SY

This sheet designed by:



ARCHITECTS ENGINEERS PLANNERS

BRIDGE GENERAL NOTES

RAILROAD BRIDGE DESIGN SPECIFICATIONS:
AREMA Manual for Railway Engineering, 2002.

CONSTRUCTION SPECIFICATIONS
Wichita Central Corridor Railroad Grade Separation Project, 25th Street to Waterman, Wichita,
Kansas-Project Specifications*, HNTB Corporation, 2005.

MATERIAL and TESTING SPECIFICATIONS:
The material and test specifications current as of the publication of the project specifications, will be used.
In cases of discontinuance or material changes to the specification, the engineer will be contacted for
guidance.

REFERENCES:
Kansas-Final Geotechnical Investigation Report*, HNTB Corporation, September 2003.
Wichita Central Corridor Railroad Grade Separation Project, Douglas Avenue to 21st Street, Wichita,
Kansas-Hazardous Materials Screening Report*, HNTB Corporation, September 2000.

BNSF Railway Guidelines, 2002.

BNSF Railway / Union Pacific Railroad Standard Drawings

Engineering and Shop Drawings for Existing Bridges at 2nd Street, 1st Street and Douglas Avenue.

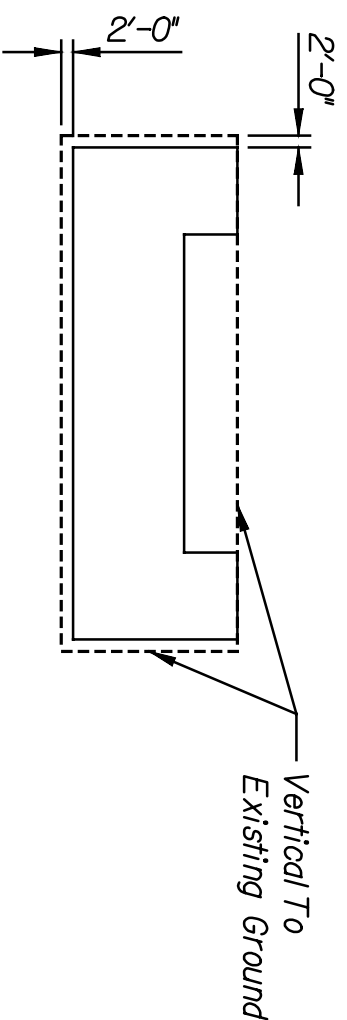
HORIZONTAL and VERTICAL GEOMETRY CONTROL:
Refer to Railroad and Street plans for horizontal and vertical geometry control.

The track profile grade is at the top of rail.

All elevations shown are U.S.G.S. Datum (NGVD 29) City Datum = U.S.G.S. Datum - 1187.41.

STRUCTURAL EXCAVATION:
Structural excavation shall be in accordance with the plans and specifications.

STRUCTURAL BACKFILL:
Structural backfill shall be located within the limits identified in the plans and specifications. Structural
backfill shall meet or exceed the requirements of Embankment.



EXCAVATION FOR PAYMENT

REINFORCING:
All bar bending dimensions and tolerances are in accordance with CRSI's
Manual of Standard Practice.

Reinforcing bars will be designed as follows: SSCOMW
SS = Bar Size (No. 3 to No. 18)

CC = Component Designator, as follows:
A-Abutment F-Footing D-Dowell
P-Pier FB-Pier Beam FC-Pier Column
PW-Pier Wall S-Slab/Deck R-Railing
C-CURB

IN = Bar Mark Sequence (00-99)

Reinforcing Bar Annotation Example:
11A12

A #11 Bar, located in the abutment, 12th bar in bar size/location sequence

EMBANKMENT:

Fill material located within the volume bounded by the back face the
abutment, back face of the wingwall(s), ends of the wingwall(s) and
above the limits of structural backfill shall be classified as embankment.

Excavated materials not considered suitable for use as backfill or embankment
shall be wasted off site. All embankment quantities are anticipated to be from
an approved borrow site provided by the Contractor. Reuse of excavated
materials in the embankment will only be permitted if the Contractor provides
tests verifying the materials proposed for reuse meet the requirements for
compacted granular fill. Embankment materials shall consist of compacted
granular fill with a minimum effective internal friction angle of 32 degrees
when tested by the standard direct shear test AASHTO T-236 utilizing a
sample of the material compacted to 100% of maximum laboratory dry density
of optimum moisture content. For all embankment materials placed on the
project, except for the UPRR track construction work between 17th and 21st
Streets, the moisture content of the fill at the time of placement and compaction
shall be within the range of 3% below to 3% above the optimum moisture
content value determined by the Standard Proctor (ASTM D-698). Embankment
shall be compacted to at least 100% of the material's maximum Standard Proctor
dry density (ASTM D-698). Embankment material shall be free of organic
material, debris and less than 10% by weight shall pass the no. 200 sieve.
The fill shall be placed and compacted in lifts of 8 inches or less in loose
thickness. Where the existing embankment is left in place, new embankment shall
be stair-stepped into the existing embankment. The Contractor is responsible
for furnishing and placing compacted granular fill that meets the design and
performance requirements of the project. Payment for embankment shall be based
on plan quantities. No additional payment will be authorized unless the Engineer
approves embankment beyond the plan limits.

PROTECTIVE SHORING:
Provide protective shoring as required by federal, state and local regulations.
Provide protective shoring as indicated in the plans and specifications.
Additional shoring may be required.

Protective shoring plans & calculations shall be designed and sealed by a
professional engineer licensed in the State of Kansas.

Protective shoring calculations, plans and details shall be submitted eight
(8) weeks prior to commencing shoring operations.

Protective shoring calculations, plans and details shall be submitted to the
Engineer and distributed to the BNSF, UPRR and WUTA for approval.
Protective shoring construction shall not begin until approved by the Engineer
and the railroads.

QUANTITIES:
Items not listed separately in the Summary of Bridge Quantities are subsidiary
to other items.

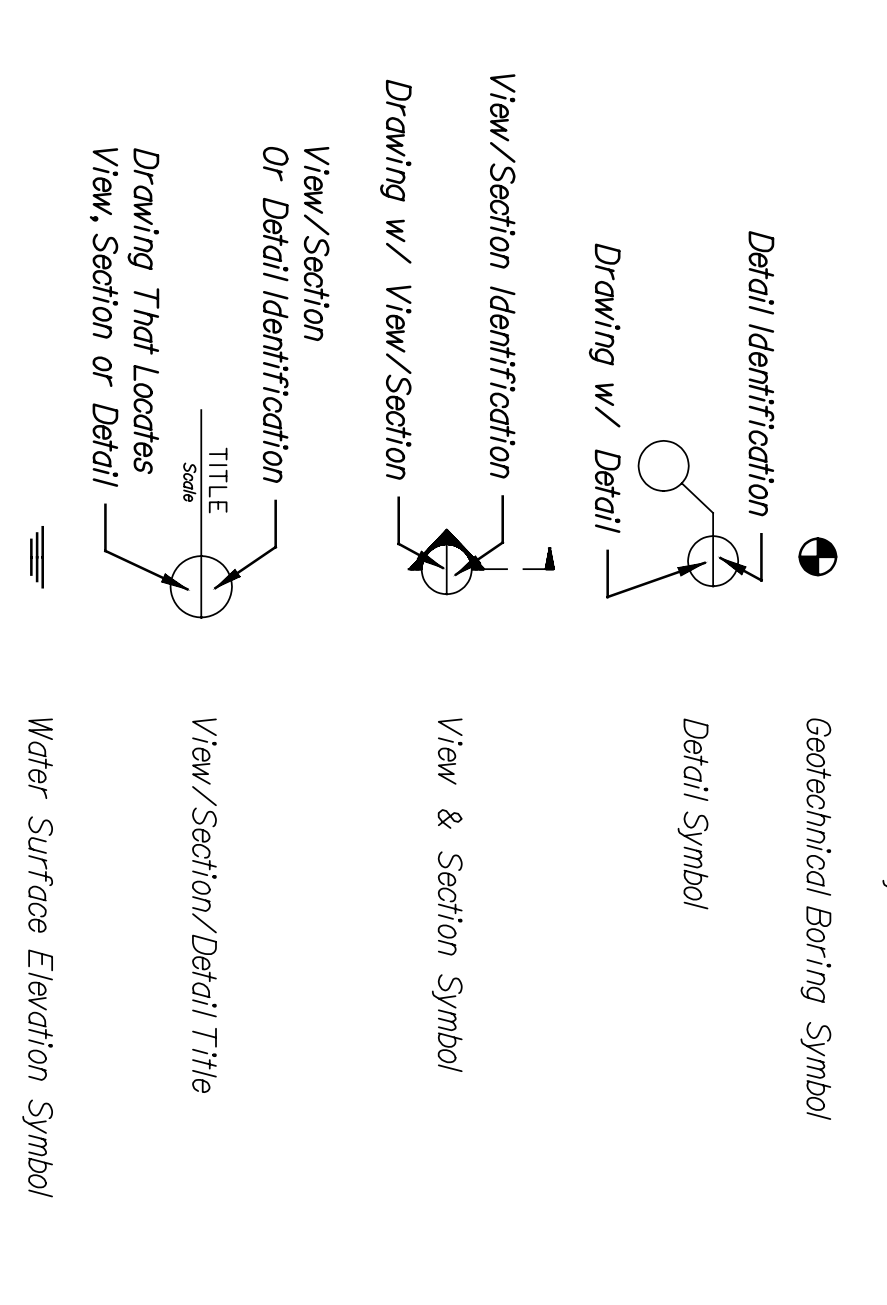
QUALITY CONTROL:
Prior to placing structural steel, verify that the bridge seat elevations are
equal to the plan elevation +/- 1/8" and submit the documentation of the
elevations to the Engineer.

STATE	PROJECT NO.	YEAR	TOTAL SHEETS
KANSAS	472-84071	2005	B23

ABBREVIATIONS:

AASHTO	American Association of State Highway & Transportation Officials	IN.	Inches
ACI	American Concrete Institute	KIP	1000 Pounds
AISC	American Institute of Steel Construction	KSF	Kips per Square Foot
ANSI	American National Standards Institute	L.F.	Linear Feet
AREMA	American Railway Engineering and Maintenance-of-Way Association	Lbs.	Pounds
ASME	American Society of Mechanical Engineers	max.	0.001 inches Minimum Maximum
ASTM	American Society of Testing and Materials	N/A	Not Applicable
AMS	American Welding Society	N.F.	Near Face
B/	Bottom of Burlington Northern and Santa Fe Railway Company Bottom	PCF	Pounds per Cubic Foot
BNSF	Burlington Northern and Santa Fe Railway Company Bottom	PLF	Pounds per Linear Foot
Bm	Bottom	P.L.	Point of Intersection
CRSI	Concrete Reinforcing Steel Institute (www.crsi.org)	P.V.C.	Point of Vertical Curvature
C.F.	Cubic Feet	P.V.I.	Point of Vertical Intersection
C.S.	Curve to Spiral Point	P.V.T.	Point of Vertical Tangency
C.Y.	Cubic Yards	R	Radius
dft	dry film thickness	S.C.	Spiral to Curve Point
E.F.	Each Face	S.S.	Similar
E.S.	Equal Spaces	S.T.	Spiral to Tangent Point
E.W.	Each Way	SSPC	SSPC: The Society for Protective Coatings
Fc	Minimum 28-day Concrete Compressive Strength	SY	Square Yards
F	Fahrenheit	T/	Top of
F.F.	For Face	T.S.	Tangent to Spiral Point
F.F.	For Face	T.P.	Typical
G.W.	Galvanized	U.N.O.	Unless Noted Otherwise
		USACOE	U.S. Army Corps of Engineers
		UPRR	Union Pacific Railroad
		WUTA	Wichita Union Terminal Association

SYMBOLS



SHEET NO.		SCALE AS NOTED		APP'D.	
DESIGNED	C.A.P.	DETAILED	B.L.M.	QUANTITIES	D.U.I.
DESIGN	C.K.	DETAIL	C.K.	QUANT.	D.M.H.
NO.	DATE	REVISIONS		BY	APP'D.
3					
2					
1					

CITY OF WICHITA
13TH STREET
WICHITA CENTRAL CORRIDOR

SUMMARY OF QUANTITIES AND GENERAL NOTES

LOCATION: BNSF BR. 201A
 WICHITA, KS
 LINE SEGMENT 1400