

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
*
* RUN DATE 04/18/94 TIME 16:01:06
*
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
*
*****

```

```

BRADFORD SOUTH
PEAK FLOW FUTURE CONDITION
Q100 - 6HR STORM
FILE: BRAD2

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL     0.  HYDROGRAPH PLOT SCALE

```

```

IT      HYDROGRAPH TIME DATA
        NMIN      2  MINUTES IN COMPUTATION INTERVAL
        IDATE     1  0  STARTING DATE
        ITIME     0000 STARTING TIME
        NQ       181 NUMBER OF HYDROGRAPH ORDINATES
        NDDATE    1  0  ENDING DATE
        NDTIME    0600 ENDING TIME
        ICENT     19  CENTURY MARK

```

```

COMPUTATION INTERVAL 0.03 HOURS
TOTAL TIME BASE      6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	88.	3.30	13.	13.	13.	0.03		

*** NORMAL END OF HEC-1 ***

```

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.

THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION

NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION

KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


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```

```

BRADFORD SOUTH
PEAK FLOW FUTURE CONDITION
Q100 - 6HR STORM
FILE: BRAD2

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT        HYDROGRAPH TIME DATA
          NMIN      2  MINUTES IN COMPUTATION INTERVAL
          IDATE     1  0  STARTING DATE
          ITIME     0000 STARTING TIME
          NQ        181 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1  0  ENDING DATE
          NDTIME    0600 ENDING TIME
          ICENT     19  CENTURY MARK

          COMPUTATION INTERVAL  0.03 HOURS
          TOTAL TIME BASE       6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	88.	3.30	13.	13.	13.	0.03		

*** NORMAL END OF HEC-1 ***

```

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* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 04/18/94 TIME 16:01:06 *
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* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
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*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

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 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1	ID BRADFORD SOUTH
2	ID PEAK FLOW FUTURE CONDITION
3	ID Q100 - 6HR STORM
4	ID FILE: BRAD2
5	IT 2 0 181
6	IO 5 0
7	KK SB1
8	KM COMPUTE RUNOFF HYDROGRAPH
9	BA .033
10	PH 100 0 .87 1.86 3.8 4.6 5.1 6.0
11	LS 0 78.6
12	UD 0.254
13	ZZ

```

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* RUN DATE 04/18/94 TIME 16:01:06
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BRADFORD SOUTH
PEAK FLOW FUTURE CONDITION
Q100 - 6HR STORM
FILE: BRAD2

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN       2  MINUTES IN COMPUTATION INTERVAL
          IDATE      1  0  STARTING DATE
          ITIME      0000 STARTING TIME
          NQ         181 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     1  0  ENDING DATE
          NDTIME     0600 ENDING TIME
          ICENT      19  CENTURY MARK

          COMPUTATION INTERVAL 0.03 HOURS
          TOTAL TIME BASE      6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	88.	3.30	13.	13.	13.	0.03		

*** NORMAL END OF HEC-1 ***

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 04/18/94 TIME 15:52:41 *
*****

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* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
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*****

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

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 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BRADFORD SOUTH									
2	ID	PEAK FLOW PRESENT CONDITION									
3	ID	Q100 - 6HR STORM									
4	ID	FILE: BRAD1									
5	IT	2	0	181							
6	IO	5	0								
7	KK	SB1									
8	KM	COMPUTE RUNOFF HYDROGRAPH									
9	BA	0.05									
10	PH	100	0	.87	1.86	3.8	4.6	5.1	6.0		
11	LS	0	81								
12	UD	0.69									
13	ZZ										

```

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* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
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*
*****

```

```

BRADFORD SOUTH
PEAK FLOW PRESENT CONDITION
Q100 - 6HR STORM
FILE: BRAD1

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA *
          NMIN      2  MINUTES IN COMPUTATION INTERVAL
          IDATE     1  0  STARTING DATE
          ITIME     0000 STARTING TIME
          NQ        181 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1  0  ENDING DATE
          NDTIME    0600 ENDING TIME
          ICENT     19  CENTURY MARK

          COMPUTATION INTERVAL  0.03 HOURS
          TOTAL TIME BASE      6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	84.	3.80	20.	20.	20.	0.05		

*** NORMAL END OF HEC-1 ***

```

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*
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X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

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 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM


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*
*****

```

```

BRADFORD SOUTH
PEAK FLOW PRESENT CONDITION
Q100 - 6HR STORM
FILE: BRAD1

```

```

6 IO OUTPUT CONTROL VARIABLES
      IPRNT      5 PRINT CONTROL
      IPLOT      0 PLOT CONTROL
      QSCAL      0. HYDROGRAPH PLOT SCALE

```

```

IT HYDROGRAPH TIME DATA
      NMIN      2 MINUTES IN COMPUTATION INTERVAL
      IDATE      1 0 STARTING DATE
      ITIME      0000 STARTING TIME
      NQ         181 NUMBER OF HYDROGRAPH ORDINATES
      NDDATE      1 0 ENDING DATE
      NDTIME      0600 ENDING TIME
      ICENT      19 CENTURY MARK

      COMPUTATION INTERVAL 0.03 HOURS
      TOTAL TIME BASE 6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	84.	3.80	20.	20.	20.	0.05		

*** NORMAL END OF HEC-1 ***

THESE CALCS WERE IN THE PLAT FILE.

4-13-94

Proposed Addition

EXISTING DA = 32.0 AC DRAWING TO SOUTH

SOIL TYPE F_b = (B)

CULTIVATED W/O CONSERVATION CN = 81

$$L = \frac{2000 \cdot 81 \cdot 3.7}{100 \cdot 1.016} = 0.69 \text{ HR} = 42 \text{ MIN.}$$

HEC-1 Peak Q₁₀₀ = 84 cfs 134 cfs FUTURE 32.0 AC

AFTER DEVELOPMENT DA = 21.2 AC 33^{PS} IMP.

3.3 AC STREETS	98	323.4	
17.9 AC 1/4 AC LOTS	75	<u>1342.5</u>	
<u>21.2</u>		1665.9	CN = 78.6

$$L = 0.49 \sqrt{\frac{0.138}{18}} \cdot 33 \cdot 1.57 = 0.254 \text{ HRS} = 15.3 \text{ MIN.}$$

HEC-1 Peak Q₁₀₀ 88 cfs

USE E-W LOT LINE TO TYLER 3.7' DEEP IN 930'
 PLUS TYLER GORE 1.5' IN 535' OR 5.2' IN 1515'
 $S = 0.0034 = 1.8' / \text{MI.}$

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
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* VERSION 4.0.1E
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* RUN DATE 04/18/94 TIME 16:01:33
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* (916) 551-1748
*
*****

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```

X   X   XXXXXXX   XXXXX   X
X   X   X       X     X   XX
X   X   X       X     X   X
XXXXXXXX XXXX   X       XXXXX X
X   X   X       X     X   X
X   X   X       X     X   X
X   X   XXXXXXX   XXXXX   XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

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 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	BRADFORD SOUTH									
2	ID	PEAK FLOW FUTURE - 32.0 AC									
3	ID	Q100 - 6HR STORM									
4	ID	FILE: BRAD3									
5	IT	2		0		181					
6	IO	5	0								
7	KK	SB1									
8	KM	COMPUTE RUNOFF HYDROGRAPH									
9	BA	.05									
10	PH	100	0	.87	1.86	3.8	4.6	5.1	6.0		
11	LS	0	78.6								
12	UD	0.254									
13	ZZ										

```

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* MAY 1991
* VERSION 4.0.1E
*
* RUN DATE 04/18/94 TIME 16:01:33
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*
*****

```

```

BRADFORD SOUTH
PEAK FLOW FUTURE - 32.0 AC
Q100 - 6HR STORM
FILE: BRAD3

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5  PRINT CONTROL
          IPLOT      0  PLOT CONTROL
          QSCAL      0.  HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA  *
          NMIN      2  MINUTES IN COMPUTATION INTERVAL
          IDATE     1  0  STARTING DATE
          ITIME     0000 STARTING TIME
          NQ        181 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1  0  ENDING DATE
          NDTIME    0600 ENDING TIME
          ICENT     19  CENTURY MARK

          COMPUTATION INTERVAL  0.03 HOURS
          TOTAL TIME BASE      6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	134.	3.30	19.	19.	19.	0.05		

*** NORMAL END OF HEC-1 ***

```

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* MAY 1991
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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

```

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* MAY 1991 *
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```

BRADFORD SOUTH
PEAK FLOW PRESENT CONDITION
Q100 - 6HR STORM
FILE: BRAD1

```

```

6 IO      OUTPUT CONTROL VARIABLES
          IPRNT      5 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA *
          NMIN      2 MINUTES IN COMPUTATION INTERVAL
          IDATE     1 0 STARTING DATE
          ITIME     0000 STARTING TIME
          NQ        181 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE    1 0 ENDING DATE
          NDDTIME   0600 ENDING TIME
          ICENT     19 CENTURY MARK

          COMPUTATION INTERVAL 0.03 HOURS
          TOTAL TIME BASE      6.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	SB1	84.	3.80	20.	20.	20.	0.05		

*** NORMAL END OF HEC-1 ***

```

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* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 04/18/94 TIME 16:01:06 *
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* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1	ID BRADFORD SOUTH
2	ID PEAK FLOW FUTURE CONDITION
3	ID Q100 - 6HR STORM
4	ID FILE: BRAD2
5	IT 2 0 181
6	IO 5 0
7	KK SB1.
8	KM COMPUTE RUNOFF HYDROGRAPH
9	BA .033
10	PH 100 0 .87 1.86 3.8 4.6 5.1 6.0
11	LS 0 78.6
12	UD 0.254
13	ZZ

Worksheet 2: Time of concentration and peak discharge

Client BRADFORD NORTH By _____ Date _____
 County _____ State _____ Checked _____ Date _____
 Practice _____

Estimating time of concentration

1. Data:

Rainfall distribution type = 1 (I, IA, II, III)
 Drainage area A = 53.3 ac
 Runoff curve number CN = 84.3 (Worksheet 1)
 Watershed slope Y = 0.52 %
 Flow length l = 2900 ft

2. T_c using l, Y, CN and Figure 2-7 = 1.5 hrs
 or using equation 2-5

$$T_c = \frac{1.08 \left(\frac{1000}{CN} - 9 \right)^{0.7}}{1140 Y^{0.5}} = \frac{(2900)^{0.8} (2.86)^{0.7}}{1140 (.52)^{0.5}} = \underline{1.50} \text{ hrs}$$

Estimating peak discharge

1. Frequency yr
2. Rainfall, P (24-hour) in
3. Initial abstraction, I_a in
(Use CN with table 2-4.)
4. Compute I_a/P ratios
5. Unit peak discharge, q^u cfs/ac/in
(Use T_c and I_a/P with exhibit 2-_____)
6. Runoff, Q in
(Use P and CN with figure 2-6 or table 2-2)
7. Peak discharge, q_p cfs
(Where $q_p = q^u A Q$)

Storm #1	Storm #2	Storm #3
2	25	100
3.5	6.1	7.8

0.375	0.375	0.375
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0.107	0.0615	0.0481
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0.255	0.27	0.23
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1.97	4.32	3.94
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24.78	52.17	82.05
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53.3 135.39 193.13

Peak Discharge: Using P100 - 6hr storm AMC III

CN = 84

Drainage Area = 53.3 Acres = 0.08328 mi²

Soil complex \Rightarrow = 84 AMC II = 93 AMC III

L = 2900

Rainfall = 5.90 inches

H = 15

Run-off = 3.07

$L_c = 0.45$

84 4.10

93 5.07

Peak Discharge = $347.5(0.08328)4.10 = 118.65$ for CN = 84

Peak Discharge = $347.5(0.08328)5.07 = 146.72$ cfs CN = 93

USE 150 cfs

$$(1) \frac{5.90 \times 0.08328 \times 540}{12} = 22.518912 \text{ hrs of flow}$$

$$(2) \frac{22.518912}{146.60} = 0.015354 \text{ hrs} = \text{value of 1 unit of flow}$$

$$(3) \frac{150}{60} = 2.5 \text{ cfs} = \text{value of 1 unit of flow}$$

$$(4) \frac{0.015354 \times 2}{2.5} = 0.0122832 \text{ hrs} = \text{value of 1 unit of time}$$