

MID-KANSAS ENGINEERING CONSULTANTS P.A.

3500 NORTH ROCK ROAD BLDG #300 WICHITA, KANSAS 67226 1-316-682-6561

CALCULATIONS & SKETCHES

Proj. No. _____ By FBN Date 9/23/86 Chkd By _____ Date _____ Sheet 1 Of _____

Location Woodland Estates 2nd Reference Lakes East of Woodland Estates 2nd

Drainage Area = 73.5 acres = 0.115 sq mi

CN = 87.0 Tc by SCS: Tc = (L^0.8 (S+1)^0.7) / 1900 Y^0.5

L = hydraulic length of watershed in ft. = 3300'

S = (1000/CN)^-10 (where CN' = retardance factor = CN) = (1000/87)^-10 = 1.49

Y = aver. watershed landscape in percent = 2.0%

From Fig. 3-3, SCS TR-55 Urban Hydrology for Small Watersheds:

L = 0.46 hrs HLM = 100% IMP = 38%

F_HLM = 0.57, F_IMP = 0.83

Tc = (60L / 2.6) x F_HLM x F_IMP = (60 x 0.46 / 2.6) x 0.57 x 0.83 = 22 min = 0.36 hr

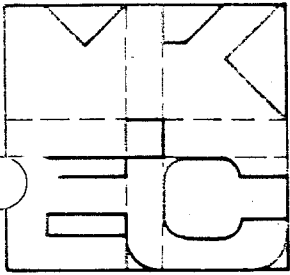
By Rational Formula w/ Tc = 22 min, I_100 = 6.26 in/hr, C_100 = 0.76

Q_100 = (0.76)(6.26)(73.5) = 349.68

for Broadcrested Weir from Merritt, Std. Handbook for Civil Engineers

p. 21-79 Q = C L H^3/2

for H = 1.5, C = 3.08, L = (350 / (1.5^1.5 x 3.08)) = 63.6' approx 64'

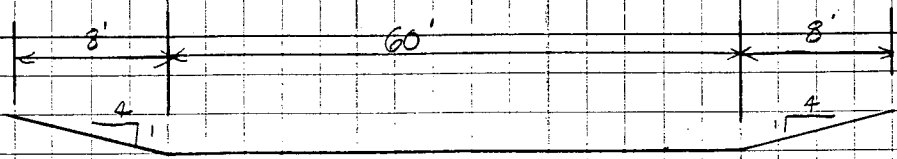


**MID-KANSAS ENGINEERING
CONSULTANTS P.A.**
3500 NORTH ROCK ROAD, BLDG #800
WICHITA, KANSAS 67226 1-316-862-6561
CALCULATIONS & SKETCHES

Proj. No. _____
By FBN Date 9/23/86
Chkd By _____ Date _____
Sheet 2 Of _____

Location Woodland Estates 2nd
Reference Lakes East of Woodland Estates 2nd

Proposed Weir Configuration



Water Surface Elev						Storage acre-ft	
North Lake	South Lake	H ft	L ft	C	Q cfs	North Lake	South Lake
168.00	165.00	0	60		0	0	0
168.50	165.5	0.5	67	2.67	60.4	0.40	0.51
169.00	166.00	1.0	68	2.83	192.4	0.90	1.07
169.50	166.50	1.5	72	3.08	407.4	1.37	1.52
170.00	167.00	2.0	76	3.12	670.7	1.93	2.27

Based on SCS TR-20 Run, $Q = 486$ cfs

$L = \frac{441}{1.5^{2.5} \times 3.08} = 77.93'$ Revise 60' to 75'

Check 80'

H	L	Q	H	L	Q
0.5	79	74.6	0.5	84	79.3
1.0	83	234.9	1.0	88	249.0
1.5	87	492.3	1.5	92	520.6
2.0	91	803.0	2.0	96	847.2

*****80-80 LIST OF INPUT DATA FOR TR-20 HYDROLOGY*****

JOB TR-20	FULLPRINT	SUMMARY	NOLOTS	10	
TITLE 001 LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS				20	
TITLE TR-20, CURVE 2 60 FOOT WEIR					
3 STRUCT 01				40	
8	168.00	0.00	0.0	50	
8	168.50	60.4	0.40	60	
8	169.00	192.40	0.90	70	
8	169.50	407.4	1.37	80	
8	170.00	670.70	1.93	90	
9 ENDTBL				100	
3 STRUCT 02				110	
8	165.00	0.00	0.0	120	
8	165.50	60.4	0.51	130	
8	166.00	192.4	1.07	140	
8	166.50	407.4	1.52	150	
8	170.00	670.70	2.27	160	
9 ENDTBL				170	
6 RUNOFF 1 01 5	0.115	87.0	0.36	1 1 1 1	180
6 RESVOR 2 01 5 6	168.00			1 1 1 1 1	190
6 RESVOR 2 02 6 5	165.00			1 1 1 1 1	200
ENDATA					210
7 INCREM 6	0.10				220
7 COMPUT 7 01 02		7.8	1.0	2 2 01 05	230
ENDCMP 1					240
ENDJOB 2					250

*****END OF 80-80 LIST*****

FILE NO. 1

COMPUTER PROGRAM FOR PROJECT FORMULATION - HYDROLOGY USER NOTES

THE USERS MANUAL FOR THIS PROGRAM IS THE MAY 1982 DRAFT OF TR-20. CHANGES FROM THE 2/14/74 VERSION INCLUDE:

REACH ROUTING - THE MODIFIED ATT-KIN ROUTING PROCEDURE REPLACES THE CONVEX METHOD. INPUT DATA PREPARED FOR PREVIOUS PROGRAM VERSIONS USING CONVEX ROUTING COEFFICIENTS WILL NOT RUN ON THIS VERSION.

THE PREFERRED TYPE OF DATA ENTRY IS CROSS SECTION DATA REPRESENTATIVE OF A REACH. IT IS RECOMMENDED THAT THE OPTIONAL CROSS SECTION DISCHARGE-AREA PLOTS BE OBTAINED WHENEVER NEW CROSS SECTION DATA IS ENTERED. THE PLOTS SHOULD BE CHECKED FOR REASONABLENESS AND ADEQUACY OF INPUT DATA FOR THE COMPUTATION OF "M" VALUES USED IN THE ROUTING PROCEDURE.

GUIDELINES FOR DETERMINING OR ANALYZING REACH LENGTHS AND COEFFICIENTS (X,M) ARE AVAILABLE IN THE USERS MANUAL. SUMMARY TABLE 2 DISPLAYS REACH ROUTING RESULTS AND ROUTING PARAMETERS FOR COMPARISON AND CHECKING.

HYDROGRAPH GENERATION - THE PROCEDURE TO CALCULATE THE INTERNAL TIME INCREMENT AND PEAK TIME OF THE UNIT HYDROGRAPH HAVE BEEN IMPROVED. PEAK DISCHARGES AND TIMES MAY DIFFER FROM THE PREVIOUS VERSION. OUTPUT HYDROGRAPHS ARE STILL INTERPOLATED, PRINTED, AND ROUTED AT THE USER SELECTED MAIN TIME INCREMENT.

INTERMEDIATE PEAKS - METHOD ADDED TO PROVIDE DISCHARGES AT INTERMEDIATE POINTS WITHIN REACHES WITHOUT ROUTING.

OTHER - THIS VERSION CONTAINS SOME ADDITIONS TO THE INPUT AND NUMEROUS MODIFICATIONS TO THE OUTPUT. USER OPTIONS HAVE BEEN MODIFIED AND AUGMENTED ON THE JOB RECORD, RAINTABLES ADDED, ERROR AND WARNING MESSAGES EXPANDED, AND THE SUMMARY TABLES COMPLETELY REVISED. THE HOLDOUT OPTION IS NOT OPERATIONAL AT THIS TIME.

PROGRAM QUESTIONS OR PROBLEMS SHOULD BE DIRECTED TO HYDRAULIC ENGINEERS AT THE SCS NATIONAL TECHNICAL CENTERS:

CHESTER, PA (NORTHEAST) -- 215-499-3933, FORT WORTH, TX (SOUTH) -- 334-5242 (FTS)
LINCOLN, NB (MIDWEST) -- 541-5318 (FTS), PORTLAND, OR (WEST) -- 423-4099 (FTS)
OR HYDROLOGY UNIT, ENGINEERING DIVISION, LANHAM, MD -- 436-7383 (FTS).

PROGRAM CHANGES SINCE MAY 1982:

12/17/82 - CORRECT PEAK RATE FACTOR FOR USER ENTERED DIMHYD
CORRECT REACH ROUTING PEAK TRAVEL TIME PRINTED WITH FULLPRINT OPTION

5/02/83 - CORRECT COMPUTATIONS FOR ---

1. DIVISION OF BASEFLOW IN DIVERT OPERATION
2. HYDROGRAPH VOLUME SPLIT BETWEEN BASEFLOW AND ABOVE BASEFLOW
3. CROSS SECTION DATA PLOTTING POSITION
4. INTERMEDIATE PEAK WHEN "FROM" AREA IS LARGER THAN "THRU" AREA
5. STORAGE ROUTED REACH TRAVEL TIME FOR MULTYPEAK HYDROGRAPH
6. ORDERING "FLOW-FREQ" FILE FROM SUMMARY TABLE #3 DATA
7. BASEFLOW ENTERED WITH READHYD
8. LOW FLOW SPLIT DURING DIVERT PROCEDURE #2 WHEN SECTION RATINGS START AT DIFFERENT ELEVATIONS

ENHANCEMENTS ---

1. REPLACE USER MANUAL ERROR CODES (PAGE 4-9 TO 4-11) WITH MESSAGES
2. LABEL OUTPUT HYDROGRAPH FILES WITH CROSS SECTION/STRUCTURE, ALTERNATE AND STORM NO'S

09/01/83 - CORRECT INPUT AND OUTPUT ERRORS FOR INTERMEDIATE PEAKS
CORRECT COMBINATION OF RATING TABLES FOR DIVERT
CHECK REACH ROUTING PARAMETERS FOR ACCEPTABLE LIMITS
ELIMINATE MINIMUM REACH TRAVEL TIME WHEN ATT-KIN COEFFICIENT EQUALS ONE

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LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
 TR-20, CURVE 2 60 FOOT WEIR

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JOB 1 PASS 1
 PAGE 2

STRUCTURE DATA, STRUCTURE NO. 1

	ELEVATION	DISCHARGE	STORAGE
8	168.00	.00	.00
8	168.50	60.40	.40
8	169.00	192.40	.90
8	169.50	407.40	1.37
8	170.00	670.70	1.93

9 ENDTBL

STRUCTURE DATA, STRUCTURE NO. 2

	ELEVATION	DISCHARGE	STORAGE
8	165.00	.00	.00
8	165.50	60.40	.51
8	166.00	192.40	1.07
8	166.50	407.40	1.52
8	170.00	670.70	2.27

9 ENDTBL

STANDARD CONTROL OPERATION RUNOFF STRUCTURE 1
 OUTPUT HYDROGRAPH = 5
 OUTPUT OPTIONS IN EFFECT PEAK HYD VOL SUM

STANDARD CONTROL OPERATION RESVOR STRUCTURE 1
 INPUT HYDROGRAPH = 5 OUTPUT HYDROGRAPH = 6
 OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

STANDARD CONTROL OPERATION RESVOR STRUCTURE 2
 INPUT HYDROGRAPH = 6 OUTPUT HYDROGRAPH = 5
 OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

DATA FIELD VALUES =	.1150	RECORD ID	180
		87.0000	.3500
DATA FIELD VALUES =	168.0000	RECORD ID	190
		.0000	.0000
DATA FIELD VALUES =	165.0000	RECORD ID	200
		.0000	.0000

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LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
TR-20, CURVE 2 60 FOOT WEIR

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JOB 1 PASS 1
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EXECUTIVE CONTROL OPERATION INCREM MAIN TIME INCREMENT = .10 HOURS RECORD ID 220

EXECUTIVE CONTROL OPERATION COMPUT FROM STRUCTURE 1 TO STRUCTURE 2 RECORD ID 230
STARTING TIME = .00 RAIN DEPTH = 7.80 RAIN DURATION= 1.00 RAIN TABLE NO.= 2 ANT. MOIST. COND= 2
ALTERNATE NO.= 1 STORM NO.= 6 MAIN TIME INCREMENT = .10 HOURS

OPERATION RUNOFF STRUCTURE 1
OUTPUT HYDROGRAPH= 5
AREA= .12 SQ MI INPUT RUNOFF CURVE= 87. TIME OF CONCENTRATION= .35 HOURS
INTERNAL HYDROGRAPH TIME INCREMENT= .0480 HOURS

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.10	441.15	(RUNOFF)
17.65	11.20	(RUNOFF)
19.65	8.98	(RUNOFF)
23.65	6.75	(RUNOFF)

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ MI.
0.00	DISCHG .00 .00 .00 .00 .00 .03 .07	.14 .21 .32	
4.00	DISCHG .45 .60 .74 .88 1.01 1.14 1.27	1.39 1.51 1.63	
5.00	DISCHG 1.74 1.85 1.96 2.07 2.18 2.28 2.38	2.48 2.57 2.67	
6.00	DISCHG 2.76 2.90 3.19 3.52 3.78 3.97 4.13	4.27 4.40 4.52	
7.00	DISCHG 4.63 4.75 4.85 4.96 5.06 5.16 5.26	5.36 5.45 5.54	
8.00	DISCHG 5.64 5.82 6.20 6.67 7.20 7.80 8.30	8.63 8.86 9.05	
9.00	DISCHG 9.21 9.46 9.94 10.48 10.89 11.17 11.49	12.04 12.65 13.09	
10.00	DISCHG 13.40 13.75 14.33 15.04 15.83 17.32 18.77	20.71 22.74 24.92	
11.00	DISCHG 27.32 29.63 32.15 34.74 37.87 42.15 56.58	95.79 149.77 240.06	
12.00	DISCHG 368.73 441.13 373.31 255.14 170.21 120.70 91.02	73.37 62.40 54.63	
13.00	DISCHG 48.19 43.30 39.56 35.60 34.15 31.98 30.11	28.21 26.47 25.07	
14.00	DISCHG 23.90 22.92 21.94 21.04 20.15 19.23 18.38	17.46 16.63 16.11	
15.00	DISCHG 15.86 15.73 15.67 15.59 15.22 14.55 13.99	13.69 13.55 13.47	
16.00	DISCHG 13.44 13.42 13.41 13.41 13.41 13.40 13.24	12.69 12.03 11.61	
17.00	DISCHG 11.40 11.29 11.24 11.21 11.20 11.19 11.19	11.19 11.15 10.79	
18.00	DISCHG 10.12 9.56 9.26 9.11 9.04 9.00 8.98	8.97 8.97 8.97	
19.00	DISCHG 8.97 8.97 8.97 8.97 8.97 8.97 8.97	8.97 8.92 8.56	
20.00	DISCHG 7.90 7.34 7.04 6.89 6.81 6.77 6.75	6.74 6.74 6.74	
21.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74	6.74 6.74 6.74	
22.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74	6.74 6.74 6.74	
23.00	DISCHG 6.74 6.74 6.74 6.75 6.75 6.75 6.75	6.75 6.70 6.34	
24.00	DISCHG 5.67 4.79 3.34 1.87 .93 .47 .23	.11 .05 .02	
25.00	DISCHG .01 .00		

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 464.45 CFS-HRS, 38.38 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 1

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LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
TR-20, CURVE 2 60 FOOT WEIR

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JOB 1 PASS 1
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INPUT HYDROGRAPH= 5 OUTPUT HYDROGRAPH= 6
SURFACE ELEVATION= 168.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.13	430.66	169.54
19.65	8.97	168.07
23.65	6.75	168.06

TIME(HRS)	FIRST HYDROGRAPH POINT =	.00 HOURS	TIME INCREMENT =	10 HOURS	DRAINAGE AREA =	12 SQ. MI.
3.00	DISCHG	.00	.00	.00	.01	.04
3.00	ELEV	168.00	168.00	168.00	168.00	168.00
4.00	DISCHG	.35	.49	.63	.77	.91
4.00	ELEV	168.00	168.00	168.01	168.01	168.01
5.00	DISCHG	1.65	1.76	1.88	1.98	2.09
5.00	ELEV	168.01	168.01	168.02	168.02	168.02
6.00	DISCHG	2.68	2.80	2.99	3.27	3.56
6.00	ELEV	168.02	168.02	168.02	168.03	168.03
7.00	DISCHG	4.54	4.66	4.77	4.88	4.98
7.00	ELEV	168.04	168.04	168.04	168.04	168.04
8.00	DISCHG	5.56	5.69	5.93	6.32	6.79
8.00	ELEV	168.05	168.05	168.05	168.05	168.06
9.00	DISCHG	9.07	9.28	9.61	10.07	10.55
9.00	ELEV	168.08	168.08	168.08	168.08	168.09
10.00	DISCHG	13.12	13.47	13.91	14.50	15.30
10.00	ELEV	168.11	168.11	168.12	168.12	168.13
11.00	DISCHG	25.45	27.77	30.17	32.69	35.47
11.00	ELEV	168.21	168.23	168.25	168.27	168.29
12.00	DISCHG	336.74	426.11	401.23	288.08	190.57
12.00	ELEV	169.34	169.54	169.49	169.22	168.99
13.00	DISCHG	53.11	47.45	42.83	39.18	36.26
13.00	ELEV	168.44	168.39	168.35	168.32	168.30
14.00	DISCHG	24.90	23.75	22.74	21.78	20.87
14.00	ELEV	168.21	168.20	168.19	168.18	168.17
15.00	DISCHG	16.13	15.87	15.74	15.66	15.46
15.00	ELEV	168.13	168.13	168.13	168.13	168.13
16.00	DISCHG	13.48	13.44	13.42	13.41	13.41
16.00	ELEV	168.11	168.11	168.11	168.11	168.11
17.00	DISCHG	11.61	11.41	11.30	11.24	11.22
17.00	ELEV	168.10	168.09	168.09	168.09	168.09
18.00	DISCHG	10.59	10.02	9.55	9.27	9.12
18.00	ELEV	168.09	168.08	168.08	168.08	168.08
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97
19.00	ELEV	168.07	168.07	168.07	168.07	168.07
20.00	DISCHG	8.36	7.79	7.33	7.05	6.89
20.00	ELEV	168.07	168.06	168.06	168.06	168.06
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74
21.00	ELEV	168.06	168.06	168.06	168.06	168.06
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74
22.00	ELEV	168.06	168.06	168.06	168.06	168.06
23.00	DISCHG	6.74	6.74	6.74	6.75	6.75

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LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
TR-20, CURVE 2 60 FOOT WEIR

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JOB 1

PASS 1
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23.00	ELEV	168.06	168.06	168.06	168.06	168.06	168.06	168.06	168.06	168.06	168.06	168.06
24.00	DISCHG	6.14	5.44	4.38	3.02	1.78	.95	.49	.25	.12	.06	.06
24.00	ELEV	168.05	168.05	168.04	168.02	168.01	168.01	168.00	168.00	168.00	168.00	168.00
25.00	DISCHG	.03	.01	.00								
25.00	ELEV	168.00	168.00	168.00								

RUNOFF VOLUME ABOVE BASEFLOW = 6.25 WATERSHED INCHES, 463.90 CFS-HRS, 38.34 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 2
INPUT HYDROGRAPH= 6 OUTPUT HYDROGRAPH= 5
SURFACE ELEVATION= 165.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.15	424.32	166.72
19.66	8.97	165.07
23.66	6.75	165.06

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ. MI.
3.00	DISCHG .00 .00 .00 .00 .00 .00 .02 .05 .10 .16	ELEV 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00	
3.00	DISCHG .25 .36 .49 .63 .77 .90 1.03 1.16 1.29 1.41	ELEV 165.00 165.00 165.00 165.01 165.01 165.01 165.01 165.01 165.01 165.01	
5.00	DISCHG 1.53 1.65 1.76 1.87 1.98 2.09 2.19 2.29 2.39 2.49	ELEV 165.01 165.01 165.01 165.02 165.02 165.02 165.02 165.02 165.02 165.02	
6.00	DISCHG 2.59 2.69 2.82 3.02 3.28 3.55 3.78 3.97 4.14 4.28	ELEV 165.02 165.02 165.02 165.03 165.03 165.03 165.03 165.03 165.03 165.04	
7.00	DISCHG 4.41 4.53 4.65 4.76 4.87 4.98 5.08 5.18 5.28 5.37	ELEV 165.04 165.04 165.04 165.04 165.04 165.04 165.04 165.04 165.04 165.04	
8.00	DISCHG 5.47 5.57 5.73 5.99 6.36 6.82 7.34 7.84 8.27 8.60	ELEV 165.05 165.05 165.05 165.05 165.05 165.06 165.06 165.06 165.06 165.07	
9.00	DISCHG 8.85 9.06 9.31 9.66 10.09 10.51 10.88 11.25 11.69 12.19	ELEV 165.07 165.08 165.08 165.08 165.08 165.09 165.09 165.09 165.09 165.10	
10.00	DISCHG 12.67 13.08 13.48 13.96 14.58 15.40 16.46 17.77 19.37 21.22	ELEV 165.10 165.11 165.11 165.12 165.12 165.13 165.14 165.15 165.16 165.18	
11.00	DISCHG 23.26 25.46 27.77 30.17 32.74 35.58 40.46 53.17 94.58 161.27	ELEV 165.19 165.21 165.23 165.25 165.27 165.30 165.33 165.44 165.63 165.88	
12.00	DISCHG 292.28 410.28 414.29 322.68 212.02 162.59 124.37 93.15 74.52 63.17	ELEV 166.23 166.54 166.59 166.30 166.05 165.89 165.74 165.62 165.55 165.51	
13.00	DISCHG 57.48 52.75 47.75 43.32 39.64 36.61 34.07 31.86 29.86 28.03	ELEV 165.48 165.44 165.40 165.36 165.33 165.30 165.28 165.26 165.25 165.23	
14.00	DISCHG 26.42 25.04 23.86 22.81 21.83 20.90 19.99 19.10 18.21 17.38	ELEV 165.22 165.21 165.20 165.19 165.18 165.17 165.17 165.16 165.15 165.14	
15.00	DISCHG 16.71 16.24 15.96 15.79 15.64 15.38 14.95 14.47 14.06 13.78	ELEV 165.14 165.13 165.13 165.13 165.13 165.13 165.12 165.12 165.12 165.11	
16.00	DISCHG 13.61 13.51 13.46 13.43 13.42 13.41 13.39 13.26 12.95 12.49	ELEV 165.11 165.11 165.11 165.11 165.11 165.11 165.11 165.11 165.11 165.10	
17.00	DISCHG 12.04 11.69 11.47 11.34 11.27 11.23 11.21 11.20 11.19 11.13	ELEV 165.10 165.10 165.09 165.09 165.09 165.09 165.09 165.09 165.09 165.09	
18.00	DISCHG 10.91 10.51 10.03 9.63 9.34 9.17 9.07 9.02 8.99 8.98		

REQ
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LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
TR-20, CURVE 2 60 FOOT WEIR

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JOB 1 PASS 1
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18.00	ELEV	165.09	165.09	165.08	165.08	165.08	165.08	165.08	165.07	165.07	165.07
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.96	8.90
19.00	ELEV	165.07	165.07	165.07	165.07	165.07	165.07	165.07	165.07	165.07	165.07
20.00	DISCHG	8.69	8.29	7.81	7.40	7.12	6.94	6.85	6.79	6.76	6.75
20.00	ELEV	165.07	165.07	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
21.00	ELEV	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
22.00	ELEV	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
23.00	DISCHG	6.74	6.74	6.74	6.74	6.75	6.75	6.75	6.75	6.74	6.68
23.00	ELEV	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
24.00	DISCHG	6.47	6.02	5.29	4.25	3.03	1.94	1.14	.63	.34	.18
24.00	ELEV	165.05	165.05	165.04	165.04	165.03	165.02	165.01	165.01	165.00	165.00
25.00	DISCHG	.09	.04	.02	.01	.00					
25.00	ELEV	165.00	165.00	165.00	165.00	165.00					

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 464.32 CFS-HRS, 38.37 ACRE-FEET; BASEFLOW = .00 CFS

EXECUTIVE CONTROL OPERATION ENDCMP COMPUTATIONS COMPLETED FOR PASS 1 RECORD ID 240

EXECUTIVE CONTROL OPERATION ENDJOB RECORD ID 250

SUMMARY TABLE 1 - SELECTED RESULTS OF STANDARD AND EXECUTIVE CONTROL INSTRUCTIONS IN THE ORDER PERFORMED
(A STAR(*) AFTER THE PEAK DISCHARGE TIME AND RATE (CFS) VALUES INDICATES A FLAT TOP HYDROGRAPH
A QUESTION MARK(?) INDICATES A HYDROGRAPH WITH PEAK AS LAST POINT.)

SECTION/ STRUCTURE ID	STANDARD CONTROL OPERATION	DRAINAGE AREA (SQ MI)	RAIN TABLE #	ANTEC MOIST COND	MAIN TIME INCREM (HR)	PRECIPITATION			RUNOFF AMOUNT (IN)	PEAK DISCHARGE			
						BEGIN (HR)	AMOUNT (IN)	DURATION (HR)		ELEVATION (FT)	TIME (HR)	RATE (CFS)	RATE (CSM)
	ALTERNATE	1	STORM	6									
STRUCTURE 1	RUNOFF	.12	2	2	.10	.0	7.80	24.00	6.26	---	12.10	441.15	3836.1
STRUCTURE 1	RESVOR	.12	2	2	.10	.0	7.80	24.00	6.25	169.54	12.13	430.66	3744.8
STRUCTURE 2	RESVOR	.12	2	2	.10	.0	7.80	24.00	6.26	166.72	12.15	424.32	3689.8

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SUMMARY TABLE 3 - DISCHARGE (CFS) AT XSECTIONS AND STRUCTURES FOR ALL STORMS AND ALTERNATES

XSECTION/ STRUCTURE ID	DRAINAGE AREA (SQ MI)	STORM NUMBERS..... 6
STRUCTURE 2	.12	
ALTERNATE 1		424.32
STRUCTURE 1	.12	
ALTERNATE 1		430.66

*****80-80 LIST OF INPUT DATA FOR TR-20 HYDROLOGY*****

JOB TR-20	FULLPRINT	SUMMARY	NOPLOTS	10		
TITLE 003 LAKEPOINT, N.W. LAKES:	100 YEAR, 24 HOUR EVENT USING SCS			20		
TITLE TR-20, CURVE 2	75-FOOT WEIR			30		
3 STRUCT	01			40		
8	168.00	0.00	0.0	50		
8	168.50	74.6	0.40	60		
8	169.00	234.9	0.90	70		
8	169.50	492.8	1.37	80		
8	170.00	803.0	1.93	90		
9 ENDTBL				100		
3 STRUCT	02			110		
8	165.00	0.00	0.0	120		
8	165.50	74.6	0.51	130		
8	166.00	234.90	1.07	140		
8	166.50	492.3	1.52	150		
8	167.00	803.00	2.27	160		
9 ENDTBL				170		
6 RUNOFF 1	01 5	0.115	87.0	0.36	1 1 1 1	180
6 RESVOR 2	01 5 6	168.00			1 1 1 1 1	190
6 RESVOR 2	02 6 5	165.00			1 1 1 1 1	200
ENDATA						210
7 INCREM 6		0.10				220
7 COMPUT 7	01 02		7.8	1.0	2 2 01 06	230
ENDCMP 1						240
JOB 2						250

*****END OF 80-80 LIST*****

FILE NO. 3

COMPUTER PROGRAM FOR PROJECT FORMULATION - HYDROLOGY USER NOTES

THE USERS MANUAL FOR THIS PROGRAM IS THE MAY 1982 DRAFT OF TR-20. CHANGES FROM THE 2/14/74 VERSION INCLUDE:

REACH ROUTING - THE MODIFIED ATT-KIN ROUTING PROCEDURE REPLACES THE CONVEX METHOD. INPUT DATA PREPARED FOR PREVIOUS PROGRAM VERSIONS USING CONVEX ROUTING COEFFICIENTS WILL NOT RUN ON THIS VERSION.

THE PREFERRED TYPE OF DATA ENTRY IS CROSS SECTION DATA REPRESENTATIVE OF A REACH. IT IS RECOMMENDED THAT THE OPTIONAL CROSS SECTION DISCHARGE-AREA PLOTS BE OBTAINED WHENEVER NEW CROSS SECTION DATA IS ENTERED. THE PLOTS SHOULD BE CHECKED FOR REASONABLENESS AND ADEQUACY OF INPUT DATA FOR THE COMPUTATION OF "M" VALUES USED IN THE ROUTING PROCEDURE.

GUIDELINES FOR DETERMINING OR ANALYZING REACH LENGTHS AND COEFFICIENTS (X,M) ARE AVAILABLE IN THE USERS MANUAL. SUMMARY TABLE 2 DISPLAYS REACH ROUTING RESULTS AND ROUTING PARAMETERS FOR COMPARISON AND CHECKING.

HYDROGRAPH GENERATION - THE PROCEDURE TO CALCULATE THE INTERNAL TIME INCREMENT AND PEAK TIME OF THE UNIT HYDROGRAPH HAVE BEEN IMPROVED. PEAK DISCHARGES AND TIMES MAY DIFFER FROM THE PREVIOUS VERSION. OUTPUT HYDROGRAPHS ARE STILL INTERPOLATED, PRINTED, AND ROUTED AT THE USER SELECTED MAIN TIME INCREMENT.

INTERMEDIATE PEAKS - METHOD ADDED TO PROVIDE DISCHARGES AT INTERMEDIATE POINTS WITHIN REACHES WITHOUT ROUTING.

OTHER - THIS VERSION CONTAINS SOME ADDITIONS TO THE INPUT AND NUMEROUS MODIFICATIONS TO THE OUTPUT. USER OPTIONS HAVE BEEN MODIFIED AND AUGMENTED ON THE JOB RECORD, RAIN TABLES ADDED, ERROR AND WARNING MESSAGES EXPANDED, AND THE SUMMARY TABLES COMPLETELY REVISED. THE HOLDOUT OPTION IS NOT OPERATIONAL AT THIS TIME.

PROGRAM QUESTIONS OR PROBLEMS SHOULD BE DIRECTED TO HYDRAULIC ENGINEERS AT THE SCS NATIONAL TECHNICAL CENTERS:

CHESTER, PA (NORTHEAST) -- 215-499-3933, FORT WORTH, TX (SOUTH) -- 334-5242 (FTS)
LINCOLN, NB (MIDWEST) -- 541-5318 (FTS), PORTLAND, OR (WEST) -- 423-4099 (FTS)
OR HYDROLOGY UNIT, ENGINEERING DIVISION, LANHAM, MD -- 436-7383 (FTS).

PROGRAM CHANGES SINCE MAY 1982:

- 12/17/82 - CORRECT PEAK RATE FACTOR FOR USER ENTERED DIMHYD
CORRECT REACH ROUTING PEAK TRAVEL TIME PRINTED WITH FULLPRINT OPTION
- 5/02/83 - CORRECT COMPUTATIONS FOR ---
 1. DIVISION OF BASEFLOW IN DIVERT OPERATION
 2. HYDROGRAPH VOLUME SPLIT BETWEEN BASEFLOW AND ABOVE BASEFLOW
 3. CROSS SECTION DATA PLOTTING POSITION
 4. INTERMEDIATE PEAK WHEN "FROM" AREA IS LARGER THAN "THRU" AREA
 5. STORAGE ROUTED REACH TRAVEL TIME FOR MULTIPLE PEAK HYDROGRAPH
 6. ORDERING "FLOW-FREQ" FILE FROM SUMMARY TABLE #3 DATA
 7. BASEFLOW ENTERED WITH READHYD
 8. LOW FLOW SPLIT DURING DIVERT PROCEDURE #2 WHEN SECTION RATINGS START AT DIFFERENT ELEVATIONS
- ENHANCEMENTS ---
 1. REPLACE USER MANUAL ERROR CODES (PAGE 4-9 TO 4-11) WITH MESSAGES
 2. LABEL OUTPUT HYDROGRAPH FILES WITH CROSS SECTION/STRUCTURE, ALTERNATE AND STORM NO'S
- 09/01/83 - CORRECT INPUT AND OUTPUT ERRORS FOR INTERMEDIATE PEAKS
CORRECT COMBINATION OF RATING TABLES FOR DIVERT
CHECK REACH ROUTING PARAMETERS FOR ACCEPTABLE LIMITS
ELIMINATE MINIMUM REACH TRAVEL TIME WHEN ATT-KIN COEFFICIENT EQUALS ONE

EQ
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STRUCTURE DATA, STRUCTURE NO. 1

	ELEVATION	DISCHARGE	STORAGE
8	168.00	.00	.00
8	168.50	74.60	.40
8	169.00	234.90	.90
8	169.50	492.80	1.37
8	170.00	803.00	1.93

9 ENDTBL

STRUCTURE DATA, STRUCTURE NO. 2

	ELEVATION	DISCHARGE	STORAGE
8	165.00	.00	.00
8	165.50	74.60	.51
8	166.00	234.90	1.07
8	166.50	492.30	1.52
8	167.00	803.00	2.27

9 ENDTBL

STANDARD CONTROL OPERATION RUNOFF STRUCTURE 1

OUTPUT HYDROGRAPH = 5
OUTPUT OPTIONS IN EFFECT PEAK HYD VOL SUM

DATA FIELD VALUES = .1150

RECORD ID 120
87.0000 .3500

STANDARD CONTROL OPERATION RESVOR STRUCTURE 1

INPUT HYDROGRAPH = 5 OUTPUT HYDROGRAPH = 6
OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

DATA FIELD VALUES = 168.0000

RECORD ID 190
.0000 .0000

STANDARD CONTROL OPERATION RESVOR STRUCTURE 2

INPUT HYDROGRAPH = 6 OUTPUT HYDROGRAPH = 5
OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

DATA FIELD VALUES = 165.0000

RECORD ID 200
.0000 .0000

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EXECUTIVE CONTROL OPERATION INCREM MAIN TIME INCREMENT = .10 HOURS RECORD ID 220

EXECUTIVE CONTROL OPERATION COMPUT FROM STRUCTURE 1 TO STRUCTURE 2 RECORD ID 230
 STARTING TIME = .00 RAIN DEPTH = 7.50 RAIN DURATION = 1.00 RAIN TABLE NO. = 2 ANT. MOIST. COND = 2
 ALTERNATE NO. = 1 STORM NO. = 6 MAIN TIME INCREMENT = .10 HOURS

OPERATION RUNOFF STRUCTURE 1
 OUTPUT HYDROGRAPH = 5
 AREA = .12 SQ MI INPUT RUNOFF CURVE = 87 TIME OF CONCENTRATION = .35 HOURS
 INTERNAL HYDROGRAPH TIME INCREMENT = .0480 HOURS

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.10	441.15	(RUNOFF)
17.65	11.20	(RUNOFF)
19.65	8.98	(RUNOFF)
23.65	6.75	(RUNOFF)

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS				TIME INCREMENT = .10 HOURS				DRAINAGE AREA = .12 SQ MI.			
0.00	DISCHG	.00	.00	.00	.00	.03	.07	.14	.21	.32		
4.00	DISCHG	.45	.60	.74	.88	1.01	1.14	1.27	1.39	1.51	1.63	
5.00	DISCHG	1.74	1.85	1.96	2.07	2.18	2.28	2.38	2.48	2.57	2.67	
6.00	DISCHG	2.76	2.90	3.19	3.52	3.78	3.97	4.13	4.27	4.40	4.52	
7.00	DISCHG	4.63	4.75	4.85	4.96	5.06	5.16	5.26	5.36	5.45	5.54	
8.00	DISCHG	5.64	5.82	6.20	6.67	7.20	7.80	8.30	8.63	8.86	9.05	
9.00	DISCHG	9.21	9.46	9.94	10.48	10.89	11.17	11.49	12.04	12.65	13.09	
10.00	DISCHG	13.40	13.75	14.33	15.04	16.03	17.32	18.77	20.71	22.74	24.92	
11.00	DISCHG	27.32	29.63	32.15	34.74	37.87	42.15	56.58	95.79	149.77	240.06	
12.00	DISCHG	368.73	441.13	373.31	256.14	170.21	120.70	91.02	73.37	62.40	54.63	
13.00	DISCHG	48.19	43.30	39.56	36.60	34.15	31.98	30.11	28.21	26.47	25.07	
14.00	DISCHG	23.90	22.92	21.94	21.04	20.16	19.23	18.38	17.46	16.63	16.11	
15.00	DISCHG	15.86	15.73	15.67	15.59	15.22	14.55	13.99	13.69	13.55	13.47	
16.00	DISCHG	13.44	13.42	13.41	13.41	13.41	13.40	13.24	12.69	12.03	11.61	
17.00	DISCHG	11.40	11.29	11.24	11.21	11.20	11.19	11.19	11.19	11.15	10.79	
18.00	DISCHG	10.12	9.56	9.26	9.11	9.04	9.00	8.98	8.97	8.97	8.97	
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.92	8.56	
20.00	DISCHG	7.90	7.34	7.04	6.89	6.81	6.77	6.75	6.74	6.74	6.74	
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	
23.00	DISCHG	6.74	6.74	6.74	6.75	6.75	6.75	6.75	6.75	6.70	6.34	
24.00	DISCHG	5.67	4.79	3.34	1.87	.93	.47	.23	.11	.05	.02	
25.00	DISCHG	.01	.00									

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 464.45 CFS-HRS, 38.38 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 1

INPUT HYDROGRAPH= 5 OUTPUT HYDROGRAPH= 6
 SURFACE ELEVATION= 168.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.12	435.09	169.39
19.65	8.97	168.06
23.65	6.75	168.05

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ. MI.
3.00	DISCHG .00 .00 .00 .00 .00 .01 .04	ELEV 168.00 168.00 168.00 168.00 168.00 168.00 168.00 168.00 168.00 168.00 168.00	.10 .17 .25
4.00	DISCHG .37 .51 .55 .79 .93 1.05 1.19	ELEV 168.00 168.00 168.00 168.01 168.01 168.01 168.01 168.01 168.01 168.01 168.01	1.31 1.43 1.55
5.00	DISCHG 1.67 1.78 1.89 2.00 2.11 2.21 2.31	ELEV 168.01 168.01 168.01 168.01 168.01 168.01 168.02 168.02 168.02 168.02 168.02	2.41 2.51 2.60
6.00	DISCHG 2.70 2.81 3.02 3.31 3.61 3.84 4.03	ELEV 168.02 168.02 168.02 168.02 168.02 168.03 168.03 168.03 168.03 168.03 168.03	4.18 4.31 4.44
7.00	DISCHG 4.56 4.67 4.78 4.89 5.00 5.10 5.20	ELEV 168.03 168.03 168.03 168.03 168.03 168.03 168.03 168.03 168.04 168.04 168.04	5.30 5.39 5.48
8.00	DISCHG 5.58 5.71 5.97 6.37 6.86 7.42 7.97	ELEV 168.04 168.04 168.04 168.04 168.05 168.05 168.05 168.06 168.06 168.06 168.06	8.40 8.70 8.92
9.00	DISCHG 9.11 9.31 9.65 10.14 10.62 10.98 11.29	ELEV 168.06 168.06 168.06 168.07 168.07 168.07 168.08 168.08 168.08 168.08 168.09	11.70 12.26 12.79
10.00	DISCHG 13.19 13.52 13.97 14.59 15.41 16.51 17.85	ELEV 168.09 168.09 168.09 168.10 168.10 168.11 168.12 168.13 168.14 168.15 168.16	19.49 21.44 23.52
11.00	DISCHG 25.79 28.13 30.53 33.07 35.88 39.48 48.09	ELEV 168.17 168.19 168.20 168.22 168.24 168.26 168.32 168.49 168.67 168.90 168.90	72.55 129.16 204.10
12.00	DISCHG 336.59 431.44 397.83 282.49 194.98 138.54 101.30	ELEV 169.20 169.38 169.32 169.09 168.88 168.70 168.58 168.52 168.46 168.40 168.40	79.53 68.23 59.78
13.00	DISCHG 52.49 46.62 42.11 38.60 35.79 33.42 31.35	ELEV 168.35 168.31 168.28 168.26 168.24 168.22 168.21 168.20 168.19 168.17 168.17	29.44 27.61 26.01
14.00	DISCHG 24.68 23.57 22.58 21.63 20.73 19.83 18.93	ELEV 168.17 168.16 168.15 168.14 168.14 168.13 168.13 168.12 168.12 168.12 168.11	18.05 17.17 16.47
15.00	DISCHG 16.05 15.83 15.72 15.64 15.44 14.96 14.36	ELEV 168.11 168.11 168.11 168.10 168.10 168.10 168.10 168.09 168.09 168.09 168.09	13.91 13.66 13.53
16.00	DISCHG 13.46 13.43 13.42 13.41 13.41 13.41 13.33	ELEV 168.09 168.09 168.09 168.09 168.09 168.09 168.09 168.09 168.09 168.09 168.09	13.01 12.44 11.90
17.00	DISCHG 11.55 11.37 11.28 11.23 11.21 11.20 11.19	ELEV 168.08 168.08 168.08 168.08 168.08 168.08 168.08 168.08 168.08 168.07 168.07	11.19 11.17 11.00
18.00	DISCHG 10.53 9.93 9.48 9.23 9.10 9.03 9.00	ELEV 168.07 168.07 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06	8.98 8.97 8.97
19.00	DISCHG 8.97 8.97 8.97 8.97 8.97 8.97 8.97	ELEV 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06 168.06	8.97 8.95 8.77
20.00	DISCHG 8.30 7.71 7.26 7.00 6.87 6.80 6.77	ELEV 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05	6.75 6.74 6.74
21.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74	ELEV 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05	6.74 6.74 6.74
22.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74	ELEV 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05	6.74 6.74 6.74
23.00	DISCHG 6.74 6.74 6.74 6.75 6.75 6.75 6.75	ELEV 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05 168.05	6.75 6.73 6.55

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23.00	ELEV	168.05	168.05	168.05	168.05	168.05	168.05	168.05	168.05	168.05	168.04
24.00	DISCHG	6.08	5.34	4.23	2.82	1.59	.82	.41	.21	.10	.05
24.00	ELEV	168.04	168.04	168.03	168.02	168.01	168.01	168.00	168.00	168.00	168.00
25.00	DISCHG	.02	.01	.00							
25.00	ELEV	168.00	168.00	168.00							

RUNOFF VOLUME ABOVE BASEFLOW = 6.25 WATERSHED INCHES, 463.73 CFS-HRS, 38.32 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 2
INPUT HYDROGRAPH= 6 OUTPUT HYDROGRAPH= 5
SURFACE ELEVATION= 165.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.14	434.48	166.39
19.66	8.97	165.06
23.66	6.75	165.05

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ. MI.
3.00	DISCHG .00 .00 .00 .00 .00 .00 .02 .06 .11 .19		
3.00	ELEV 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00 165.00		
4.00	DISCHG .28 .40 .54 .68 .81 .95 1.08 1.21 1.33 1.45		
4.00	ELEV 165.00 165.00 165.00 165.00 165.01 165.01 165.01 165.01 165.01 165.01 165.01		
5.00	DISCHG 1.57 1.69 1.80 1.91 2.02 2.12 2.23 2.33 2.43 2.53		
5.00	ELEV 165.01 165.01 165.01 165.01 165.01 165.01 165.01 165.02 165.02 165.02 165.02		
6.00	DISCHG 2.62 2.72 2.87 3.09 3.37 3.64 3.86 4.04 4.20 4.33		
6.00	ELEV 165.02 165.02 165.02 165.02 165.02 165.02 165.03 165.03 165.03 165.03 165.03		
7.00	DISCHG 4.46 4.58 4.69 4.80 4.91 5.01 5.12 5.21 5.31 5.41		
7.00	ELEV 165.03 165.03 165.03 165.03 165.03 165.03 165.03 165.03 165.03 165.04 165.04		
8.00	DISCHG 5.50 5.61 5.78 6.07 6.48 6.98 7.51 8.02 8.42 8.71		
8.00	ELEV 165.04 165.04 165.04 165.04 165.04 165.05 165.05 165.05 165.05 165.06 165.06		
9.00	DISCHG 8.94 9.14 9.40 9.77 10.23 10.66 11.02 11.38 11.83 12.36		
9.00	ELEV 165.06 165.06 165.06 165.07 165.07 165.07 165.07 165.08 165.08 165.08 165.08		
10.00	DISCHG 12.83 13.23 13.62 14.12 14.78 15.67 16.81 18.21 19.91 21.85		
10.00	ELEV 165.09 165.09 165.09 165.09 165.10 165.11 165.11 165.12 165.13 165.15 165.15		
11.00	DISCHG 23.96 26.22 28.56 31.00 33.62 36.68 42.03 55.81 96.39 172.52		
11.00	ELEV 165.16 165.18 165.19 165.21 165.23 165.25 165.28 165.37 165.57 165.81 165.81		
12.00	DISCHG 291.49 421.52 411.84 311.10 215.23 162.70 116.33 88.24 73.26 66.29		
12.00	ELEV 166.11 166.36 166.34 166.15 165.94 165.77 165.63 165.54 165.49 165.44 165.44		
13.00	DISCHG 58.64 51.80 46.20 41.79 38.33 35.53 33.16 31.08 29.16 27.39		
13.00	ELEV 165.39 165.35 165.31 165.28 165.26 165.24 165.22 165.21 165.20 165.18 165.18		
14.00	DISCHG 25.85 24.55 23.44 22.43 21.49 20.58 19.68 18.78 17.90 17.09		
14.00	ELEV 165.17 165.16 165.16 165.15 165.14 165.14 165.13 165.13 165.12 165.11 165.11		
15.00	DISCHG 16.47 16.07 15.85 15.72 15.58 15.29 14.82 14.30 13.91 13.67		
15.00	ELEV 165.11 165.11 165.11 165.11 165.10 165.10 165.10 165.10 165.09 165.09 165.09		
16.00	DISCHG 13.54 13.47 13.44 13.42 13.41 13.41 13.38 13.22 12.85 12.34		
16.00	ELEV 165.09 165.09 165.09 165.09 165.09 165.09 165.09 165.09 165.09 165.09 165.09		
17.00	DISCHG 11.88 11.57 11.39 11.29 11.24 11.21 11.20 11.20 11.19 11.11		
17.00	ELEV 165.08 165.08 165.08 165.08 165.08 165.08 165.08 165.08 165.07 165.07 165.07		
18.00	DISCHG 10.85 10.38 9.87 9.48 9.24 9.11 9.04 9.00 8.98 8.97		

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18.00	ELEV	165.07	165.07	165.07	165.06	165.06	165.06	165.06	165.06	165.06	165.06
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.96	8.88
19.00	ELEV	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
20.00	DISCHG	8.62	8.16	7.65	7.26	7.01	6.88	6.81	6.77	6.75	6.74
20.00	ELEV	165.06	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
21.00	ELEV	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
22.00	ELEV	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05
23.00	DISCHG	6.74	6.74	6.74	6.74	6.75	6.75	6.75	6.75	6.74	6.66
23.00	ELEV	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.05	165.04
24.00	DISCHG	6.40	5.88	5.05	3.90	2.62	1.55	.85	.44	.22	.11
24.00	ELEV	165.04	165.04	165.03	165.03	165.02	165.01	165.01	165.00	165.00	165.00
25.00	DISCHG	.05	.02	.01	.00						
25.00	ELEV	165.00	165.00	165.00	165.00						

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 454.88 CFS-HRS, 38.42 ACRE-FEET; BASEFLOW = .00 CFS

EXECUTIVE CONTROL OPERATION ENDCMP COMPUTATIONS COMPLETED FOR PASS 1 RECORD ID 240

EXECUTIVE CONTROL OPERATION ENDJOB RECORD ID 250

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SUMMARY TABLE 1 - SELECTED RESULTS OF STANDARD AND EXECUTIVE CONTROL INSTRUCTIONS IN THE ORDER PERFORMED
(A STAR(*) AFTER THE PEAK DISCHARGE TIME AND RATE (CFS) VALUES INDICATES A FLAT TOP HYDROGRAPH
A QUESTION MARK(?) INDICATES A HYDROGRAPH WITH PEAK AS LAST POINT.)

SECTION/ STRUCTURE ID	STANDARD CONTROL OPERATION	DRAINAGE AREA (SQ MI)	RAIN TABLE #	ANTEC MOIST COND	MAIN TIME INCREM (HR)	PRECIPITATION			RUNOFF AMOUNT (IN)	PEAK DISCHARGE			
						BEGIN (HR)	AMOUNT (IN)	DURATION (HR)		ELEVATION (FT)	TIME (HR)	RATE (CFS)	RATE (CSM)
ALTERNATE 1 STORM 6													
STRUCTURE 1	RUNOFF	.12	2	2	.10	.0	7.80	24.00	6.26	---	12.10	441.15	3836.1
STRUCTURE 1	RESVOR	.12	2	2	.10	.0	7.80	24.00	6.25	169.39	12.12	435.09	3783.4
STRUCTURE 2	RESVOR	.12	2	2	.10	.0	7.80	24.00	6.26	166.39	12.14	434.48	3778.1

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SUMMARY TABLE 3 - DISCHARGE (CFS) AT XSECTIONS AND STRUCTURES FOR ALL STORMS AND ALTERNATES

XSECTION/ STRUCTURE ID	DRAINAGE AREA (SQ MI)	STORM NUMBERS..... 6
STRUCTURE 2	12	
ALTERNATE 1		434.48
STRUCTURE 1	12	
ALTERNATE 1		435.09

*****80-80 LIST OF INPUT DATA FOR TR-20 HYDROLOGY*****

JOB TR-20	FULLPRINT	SUMMARY	NO PLOTS	10	
TITLE 003 LAKEPOINT, N.W.	LAKES: 100 YEAR, 24 HOUR EVENT	USING SCS		20	
TITLE TR-20, CURVE 2	80-FOOT WEIR			30	
3 STRUCT 01				40	
8	168.00	0.00	0.0	50	
8	168.50	79.3	0.40	60	
8	169.00	249.0	0.90	70	
8	169.50	520.6	1.37	80	
8	170.00	847.2	1.93	90	
9 ENDTBL				100	
3 STRUCT 02				110	
8	165.00	0.00	0.0	120	
8	165.50	79.3	0.51	130	
8	166.00	249.00	1.07	140	
8	166.50	520.6	1.52	150	
8	167.00	847.20	2.27	160	
9 ENDTBL				170	
6 RUNOFF 1 01 5	0.115	87.0	0.36	1 1 1 1	180
6 RESVOR 2 01 5 6	168.00			1 1 1 1 1	190
6 RESVOR 2 02 6 5	165.00			1 1 1 1 1	200
ENDATA					210
7 INCREM 6	0.10				220
7 COMPUT 7 01 02	7.8	1.0	2 2 01 05		230
ENDCMP 1					240
JOB 2					250

*****END OF 80-80 LIST*****

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FILE NO. 3

COMPUTER PROGRAM FOR PROJECT FORMULATION - HYDROLOGY USER NOTES

THE USERS MANUAL FOR THIS PROGRAM IS THE MAY 1982 DRAFT OF TR-20. CHANGES FROM THE 2/14/74 VERSION INCLUDE:

REACH ROUTING - THE MODIFIED ATT-KIN ROUTING PROCEDURE REPLACES THE CONVEX METHOD. INPUT DATA PREPARED FOR PREVIOUS PROGRAM VERSIONS USING CONVEX ROUTING COEFFICIENTS WILL NOT RUN ON THIS VERSION.

THE PREFERRED TYPE OF DATA ENTRY IS CROSS SECTION DATA REPRESENTATIVE OF A REACH. IT IS RECOMMENDED THAT THE OPTIONAL CROSS SECTION DISCHARGE-AREA PLOTS BE OBTAINED WHENEVER NEW CROSS SECTION DATA IS ENTERED. THE PLOTS SHOULD BE CHECKED FOR REASONABLENESS AND ADEQUACY OF INPUT DATA FOR THE COMPUTATION OF "M" VALUES USED IN THE ROUTING PROCEDURE.

GUIDELINES FOR DETERMINING OR ANALYZING REACH LENGTHS AND COEFFICIENTS (X,M) ARE AVAILABLE IN THE USERS MANUAL. SUMMARY TABLE 2 DISPLAYS REACH ROUTING RESULTS AND ROUTING PARAMETERS FOR COMPARISON AND CHECKING.

HYDROGRAPH GENERATION - THE PROCEDURE TO CALCULATE THE INTERNAL TIME INCREMENT AND PEAK TIME OF THE UNIT HYDROGRAPH HAVE BEEN IMPROVED. PEAK DISCHARGES AND TIMES MAY DIFFER FROM THE PREVIOUS VERSION. OUTPUT HYDROGRAPHS ARE STILL INTERPOLATED, PRINTED, AND ROUTED AT THE USER SELECTED MAIN TIME INCREMENT.

INTERMEDIATE PEAKS - METHOD ADDED TO PROVIDE DISCHARGES AT INTERMEDIATE POINTS WITHIN REACHES WITHOUT ROUTING.

OTHER - THIS VERSION CONTAINS SOME ADDITIONS TO THE INPUT AND NUMEROUS MODIFICATIONS TO THE OUTPUT. USER OPTIONS HAVE BEEN MODIFIED AND AUGMENTED ON THE JOB RECORD, RAINTABLES ADDED, ERROR AND WARNING MESSAGES EXPANDED, AND THE SUMMARY TABLES COMPLETELY REVISED. THE HOLDOUT OPTION IS NOT OPERATIONAL AT THIS TIME.

PROGRAM QUESTIONS OR PROBLEMS SHOULD BE DIRECTED TO HYDRAULIC ENGINEERS AT THE SCS NATIONAL TECHNICAL CENTERS:

- CHESTER, PA (NORTHEAST) -- 215-499-3933, FORT WORTH, TX (SOUTH) -- 334-5242 (FTS)
- LINCOLN, NE (MIDWEST) -- 541-5318 (FTS), PORTLAND, OR (WEST) -- 423-4099 (FTS)
- OR HYDROLOGY UNIT, ENGINEERING DIVISION, LANHAM, MD -- 436-7383 (FTS).

PROGRAM CHANGES SINCE MAY 1982:

- 12/17/82 - CORRECT PEAK RATE FACTOR FOR USER ENTERED DIMHYD
CORRECT REACH ROUTING PEAK TRAVEL TIME PRINTED WITH FULLPRINT OPTION
- 5/02/83 - CORRECT COMPUTATIONS FOR ---
 - 1. DIVISION OF BASEFLOW IN DIVERT OPERATION
 - 2. HYDROGRAPH VOLUME SPLIT BETWEEN BASEFLOW AND ABOVE BASEFLOW
 - 3. CROSS SECTION DATA PLOTTING POSITION
 - 4. INTERMEDIATE PEAK WHEN "FROM" AREA IS LARGER THAN "THRU" AREA
 - 5. STORAGE ROUTED REACH TRAVEL TIME FOR MULTYPEAK HYDROGRAPH
 - 6. ORDERING "FLOW-FREQ" FILE FROM SUMMARY TABLE #3 DATA
 - 7. BASEFLOW ENTERED WITH READHYD
 - 8. LOW FLOW SPLIT DURING DIVERT PROCEDURE #2 WHEN SECTION RATINGS START AT DIFFERENT ELEVATIONS
- ENHANCEMENTS ---
 - 1. REPLACE USER MANUAL ERROR CODES (PAGE 4-9 TO 4-11) WITH MESSAGES
 - 2. LABEL OUTPUT HYDROGRAPH FILES WITH CROSS SECTION/STRUCTURE, ALTERNATE AND STORM NO'S
- 09/01/83 - CORRECT INPUT AND OUTPUT ERRORS FOR INTERMEDIATE PEAKS
CORRECT COMBINATION OF RATING TABLES FOR DIVERT
CHECK REACH ROUTING PARAMETERS FOR ACCEPTABLE LIMITS
ELIMINATE MINIMUM REACH TRAVEL TIME WHEN ATT-KIN COEFFICIENT EQUALS ONE

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STRUCTURE DATA, STRUCTURE NO. 1

	ELEVATION	DISCHARGE	STORAGE
8	168.00	.00	.00
8	168.50	79.30	.40
8	169.00	249.00	.90
8	169.50	520.60	1.37
8	170.00	847.20	1.93

9 ENDTBL

STRUCTURE DATA, STRUCTURE NO. 2

	ELEVATION	DISCHARGE	STORAGE
8	165.00	.00	.00
8	165.50	79.30	.51
8	166.00	249.00	1.07
8	166.50	520.60	1.52
8	167.00	847.20	2.27

9 ENDTBL

STANDARD CONTROL OPERATION RUNOFF STRUCTURE 1
OUTPUT HYDROGRAPH = 5
OUTPUT OPTIONS IN EFFECT PEAK HYD VOL SUM

DATA FIELD VALUES = .1150

RECORD ID 180
87.0000 .3500

STANDARD CONTROL OPERATION RESVOR STRUCTURE 1
INPUT HYDROGRAPH = 5 OUTPUT HYDROGRAPH = 6
OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

DATA FIELD VALUES = 168.0000

RECORD ID 150
.0000 .0000

STANDARD CONTROL OPERATION RESVOR STRUCTURE 2
INPUT HYDROGRAPH = 6 OUTPUT HYDROGRAPH = 5
OUTPUT OPTIONS IN EFFECT PEAK HYD ELEV VOL SUM

DATA FIELD VALUES = 165.0000

RECORD ID 200
.0000 .0000

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EXECUTIVE CONTROL OPERATION INCREM MAIN TIME INCREMENT = .10 HOURS

RECORD ID 220

EXECUTIVE CONTROL OPERATION COMPUT FROM STRUCTURE 1 TO STRUCTURE 2

STARTING TIME = .00 RAIN DEPTH = 7.80 RAIN DURATION = 1.00 RAIN TABLE NO. = 2 ANT. MOIST. COND = 2
ALTERNATE NO. = 1 STORM NO. = 6 MAIN TIME INCREMENT = .10 HOURS

RECORD ID 230

OPERATION RUNOFF STRUCTURE 1

OUTPUT HYDROGRAPH = 5

AREA = .12 SQ MI INPUT RUNOFF CURVE = 87 TIME OF CONCENTRATION = .36 HOURS

INTERNAL HYDROGRAPH TIME INCREMENT = .0480 HOURS

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.10	441.15	(RUNOFF)
17.65	11.20	(RUNOFF)
19.65	8.98	(RUNOFF)
23.65	6.75	(RUNOFF)

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ MI.
0.00	DISCHG .00 .00 .00 .00 .00 .03 .07 .14 .21 .32		
4.00	DISCHG .45 .50 .74 .88 1.01 1.14 1.27 1.39 1.51 1.63		
5.00	DISCHG 1.74 1.85 1.96 2.07 2.18 2.28 2.38 2.48 2.57 2.67		
6.00	DISCHG 2.76 2.90 3.19 3.52 3.78 3.97 4.13 4.27 4.40 4.52		
7.00	DISCHG 4.63 4.75 4.85 4.96 5.06 5.16 5.26 5.36 5.45 5.54		
8.00	DISCHG 5.64 5.82 6.20 6.67 7.20 7.80 8.30 8.63 8.86 9.05		
9.00	DISCHG 9.21 9.46 9.94 10.48 10.89 11.17 11.49 12.04 12.65 13.09		
10.00	DISCHG 13.40 13.75 14.33 15.04 16.03 17.32 18.77 20.71 22.74 24.92		
11.00	DISCHG 27.32 29.63 32.15 34.74 37.87 42.15 55.58 95.79 149.77 240.06		
12.00	DISCHG 358.73 441.13 373.31 256.14 170.21 120.70 91.02 73.37 62.40 54.63		
13.00	DISCHG 48.19 43.30 39.56 36.60 34.15 31.98 30.11 28.21 26.47 25.07		
14.00	DISCHG 23.90 22.92 21.94 21.04 20.16 19.23 18.38 17.46 16.63 16.11		
15.00	DISCHG 15.86 15.73 15.67 15.59 15.22 14.55 13.99 13.69 13.55 13.47		
16.00	DISCHG 13.44 13.42 13.41 13.41 13.41 13.40 13.24 12.69 12.03 11.61		
17.00	DISCHG 11.40 11.29 11.24 11.21 11.20 11.19 11.19 11.19 11.15 10.79		
18.00	DISCHG 10.12 9.56 9.26 9.11 9.04 9.00 8.98 8.97 8.97 8.97		
19.00	DISCHG 8.97 8.97 8.97 8.97 8.97 8.97 8.97 8.97 8.92 8.56		
20.00	DISCHG 7.90 7.34 7.04 6.89 6.81 6.77 6.75 6.74 6.74 6.74		
21.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74		
22.00	DISCHG 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74 6.74		
23.00	DISCHG 6.74 6.74 6.74 6.75 6.75 6.75 6.75 6.75 6.70 6.34		
24.00	DISCHG 5.67 4.79 3.34 1.87 .93 .47 .23 .11 .05 .02		
25.00	DISCHG .01 .00		

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 464.45 CFS-HRS, 38.38 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 1

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INPUT HYDROGRAPH= 5 OUTPUT HYDROGRAPH= 6
SURFACE ELEVATION= 168.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.12	436.69	169.35
19.65	8.97	168.06
23.65	6.75	168.04

TIME(HRS)	FIRST HYDROGRAPH POINT =	.00 HOURS	TIME INCREMENT =	.10 HOURS	DRAINAGE AREA =	.12 SQ. MI.
3.00	DISCHG	.00	.00	.00	.01	.05
3.00	ELEV	168.00	168.00	168.00	168.00	168.00
4.00	DISCHG	.37	.51	.66	.80	.93
4.00	ELEV	168.00	168.00	168.00	168.01	168.01
5.00	DISCHG	1.67	1.79	1.90	2.01	2.11
5.00	ELEV	168.01	168.01	168.01	168.01	168.01
6.00	DISCHG	2.70	2.82	3.02	3.32	3.62
6.00	ELEV	168.02	168.02	168.02	168.02	168.02
7.00	DISCHG	4.56	4.68	4.79	4.90	5.00
7.00	ELEV	168.03	168.03	168.03	168.03	168.03
8.00	DISCHG	5.58	5.71	5.98	6.39	6.88
8.00	ELEV	168.04	168.04	168.04	168.04	168.04
9.00	DISCHG	9.11	9.32	9.67	10.15	10.64
9.00	ELEV	168.06	168.06	168.06	168.06	168.07
10.00	DISCHG	13.20	13.54	13.99	14.61	15.44
10.00	ELEV	168.08	168.09	168.09	168.09	168.10
11.00	DISCHG	25.87	28.22	30.63	33.16	35.99
11.00	ELEV	168.16	168.18	168.19	168.21	168.23
12.00	DISCHG	335.80	433.25	396.56	291.20	196.25
12.00	ELEV	169.16	169.34	169.27	169.06	168.84
13.00	DISCHG	52.22	46.39	41.93	38.46	35.68
13.00	ELEV	168.33	168.29	168.26	168.24	168.22
14.00	DISCHG	24.63	23.53	22.54	21.59	20.70
14.00	ELEV	168.16	168.15	168.14	168.14	168.13
15.00	DISCHG	16.03	15.82	15.71	15.64	15.43
15.00	ELEV	168.10	168.10	168.10	168.10	168.10
16.00	DISCHG	13.46	13.43	13.42	13.41	13.41
16.00	ELEV	168.08	168.08	168.08	168.08	168.08
17.00	DISCHG	11.54	11.37	11.28	11.23	11.21
17.00	ELEV	168.07	168.07	168.07	168.07	168.07
18.00	DISCHG	10.51	9.91	9.46	9.22	9.09
18.00	ELEV	168.07	168.06	168.06	168.06	168.06
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97
19.00	ELEV	168.06	168.06	168.06	168.06	168.06
20.00	DISCHG	8.28	7.69	7.24	6.99	6.85
20.00	ELEV	168.05	168.05	168.05	168.04	168.04
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74
21.00	ELEV	168.04	168.04	168.04	168.04	168.04
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74
22.00	ELEV	168.04	168.04	168.04	168.04	168.04
23.00	DISCHG	6.74	6.74	6.74	6.75	6.75

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23.00	ELEV	168.04	168.04	168.04	168.04	168.04	168.04	168.04	168.04	168.04	168.04	168.04
24.00	DISCHG	6.06	5.31	4.19	2.76	1.54	.79	.40	.20	.10	.04	.04
24.00	ELEV	168.04	168.03	168.03	168.02	168.01	168.00	168.00	168.00	168.00	168.00	168.00
25.00	DISCHG	.02	.01	.00								
25.00	ELEV	168.00	168.00	168.00								

RUNOFF VOLUME ABOVE BASEFLOW = 6.25 WATERSHED INCHES, 463.78 CFS-HRS, 38.33 ACRE-FEET; BASEFLOW = .00 CFS

OPERATION RESVOR STRUCTURE 2
INPUT HYDROGRAPH= 6 OUTPUT HYDROGRAPH= 5
SURFACE ELEVATION= 165.00

PEAK TIME(HRS)	PEAK DISCHARGE(CFS)	PEAK ELEVATION(FEET)
12.14	436.80	165.35
19.66	8.97	165.06
23.66	6.75	165.04

TIME(HRS)	FIRST HYDROGRAPH POINT = .00 HOURS	TIME INCREMENT = .10 HOURS	DRAINAGE AREA = .12 SQ. MI.
3.00	DISCHG .00 .00 .00 .00 .00 .00 .02	.06	.12
3.00	ELEV 165.00 165.00 165.00 165.00 165.00 165.00 165.00	165.00 165.00 165.00	165.00
4.00	DISCHG .29 .41 .55 .69 .83 .96 1.09	1.22 1.34 1.46	1.46
4.00	ELEV 165.00 165.00 165.00 165.00 165.01 165.01 165.01	165.01 165.01 165.01	165.01
5.00	DISCHG 1.58 1.70 1.81 1.92 2.03 2.13 2.24	2.34 2.44 2.53	2.53
5.00	ELEV 165.01 165.01 165.01 165.01 165.01 165.01 165.01	165.01 165.02 165.02	165.02
6.00	DISCHG 2.63 2.73 2.88 3.11 3.39 3.66 3.88	4.06 4.21 4.34	4.34
6.00	ELEV 165.02 165.02 165.02 165.02 165.02 165.02 165.02	165.03 165.03 165.03	165.03
7.00	DISCHG 4.47 4.59 4.70 4.81 4.92 5.02 5.12	5.22 5.32 5.42	5.42
7.00	ELEV 165.03 165.03 165.03 165.03 165.03 165.03 165.03	165.03 165.03 165.03	165.03
8.00	DISCHG 5.51 5.62 5.80 6.10 6.52 7.02 7.56	8.06 8.45 8.74	8.74
8.00	ELEV 165.03 165.04 165.04 165.04 165.04 165.04 165.05	165.05 165.05 165.05	165.05
9.00	DISCHG 8.96 9.16 9.42 9.80 10.27 10.70 11.05	11.41 11.87 12.40	12.40
9.00	ELEV 165.06 165.06 165.06 165.06 165.06 165.07 165.07	165.07 165.07 165.07	165.07
10.00	DISCHG 12.88 13.26 13.65 14.16 14.84 15.75 16.90	18.33 20.05 22.01	22.01
10.00	ELEV 165.08 165.08 165.09 165.09 165.09 165.10 165.11	165.12 165.13 165.14	165.14
11.00	DISCHG 24.14 26.41 28.77 31.21 33.85 36.94 42.47	56.90 96.91 175.52	175.52
11.00	ELEV 165.15 165.17 165.18 165.20 165.21 165.23 165.27	165.36 165.55 165.78	165.78
12.00	DISCHG 290.76 424.62 410.75 308.15 217.87 160.86 114.09	87.19 74.59 66.53	66.53
12.00	ELEV 166.08 166.32 166.30 166.11 165.91 165.74 165.60	165.52 165.47 165.42	165.42
13.00	DISCHG 58.20 51.24 45.70 41.39 38.01 35.27 32.94	30.89 28.98 27.23	27.23
13.00	ELEV 165.37 165.32 165.29 165.26 165.24 165.22 165.21	165.19 165.18 165.17	165.17
14.00	DISCHG 25.71 24.43 23.34 22.34 21.41 20.50 19.60	18.71 17.82 17.02	17.02
14.00	ELEV 165.16 165.15 165.15 165.14 165.13 165.13 165.12	165.12 165.11 165.11	165.11
15.00	DISCHG 16.41 16.03 15.82 15.71 15.57 15.27 14.78	14.26 13.88 13.65	13.65
15.00	ELEV 165.10 165.10 165.10 165.10 165.10 165.10 165.09	165.09 165.09 165.09	165.09
16.00	DISCHG 13.53 13.46 13.43 13.42 13.41 13.41 13.38	13.21 12.82 12.30	12.30
16.00	ELEV 165.09 165.08 165.08 165.08 165.08 165.08 165.08	165.08 165.08 165.08	165.08
17.00	DISCHG 11.84 11.54 11.37 11.28 11.23 11.21 11.20	11.19 11.19 11.10	11.10
17.00	ELEV 165.07 165.07 165.07 165.07 165.07 165.07 165.07	165.07 165.07 165.07	165.07
18.00	DISCHG 10.83 10.34 9.83 9.45 9.22 9.09 9.03	9.00 8.98 8.97	8.97

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PAGE 6

18.00	ELEV	165.07	165.07	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
19.00	DISCHG	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.97	8.96	8.88
19.00	ELEV	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06	165.06
20.00	DISCHG	8.60	8.12	7.61	7.22	6.99	6.86	6.80	6.77	6.75	6.74
20.00	ELEV	165.05	165.05	165.05	165.05	165.04	165.04	165.04	165.04	165.04	165.04
21.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
21.00	ELEV	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04
22.00	DISCHG	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74	6.74
22.00	ELEV	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04
23.00	DISCHG	6.74	6.74	6.74	6.74	6.75	6.75	6.75	6.75	6.74	6.66
23.00	ELEV	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04	165.04
24.00	DISCHG	6.38	5.84	4.99	3.80	2.51	1.46	.78	.40	.20	.10
24.00	ELEV	165.04	165.04	165.03	165.02	165.02	165.01	165.00	165.00	165.00	165.00
25.00	DISCHG	.05	.02	.01	.00						
25.00	ELEV	165.00	165.00	165.00	165.00						

RUNOFF VOLUME ABOVE BASEFLOW = 6.26 WATERSHED INCHES, 464.92 CFS-HRS, 38.42 ACRE-FEET; BASEFLOW = .00 CFS

EXECUTIVE CONTROL OPERATION ENDCMP COMPUTATIONS COMPLETED FOR PASS 1 RECORD ID 240

EXECUTIVE CONTROL OPERATION ENDJOB RECORD ID 250

7700 XEQ
REV 09/01/83

LAKEPOINT, N.W. LAKES: 100 YEAR, 24 HOUR EVENT USING SCS
TR-20, CURVE 2 80-FOOT WEIR

20
30

JOB 1 SUMMARY
PAGE 8

SUMMARY TABLE 3 - DISCHARGE (CFS) AT XSECTIONS AND STRUCTURES FOR ALL STORMS AND ALTERNATES

XSECTION/ STRUCTURE ID	DRAINAGE AREA (SQ MI)	STORM NUMBERS..... 6
STRUCTURE 2	12	
ALTERNATE 1		436.80
STRUCTURE 1	12	
ALTERNATE 1		436.69

**PROFESSIONAL
ENGINEERING CONSULTANTS, PA**

1440 E. English
WICHITA, KANSAS 67211

(316) 262-2691

TO City Engineer
455 N. Main - City Hall
Wichita, KS

LETTER OF TRANSMITTAL

DATE	8/31/87	JOB NO.
ATTENTION	Carl Gipson	
RE:	Autumn Chase	
	Storm Sewer Revisions	
	Street Revisions	

WE ARE SENDING YOU Attached Under separate cover via _____ the following items:

- Shop drawings Prints Plans Samples Specifications
 Copy of letter Change order Comps & Quantity Changes

COPIES	DATE	NO.	DESCRIPTION
1			Office Check Plans (full set)
1			Final tracings (Streets) - revised
1			Final tracings (SUIS) - revised
-	-	-	Autumn Chase / Lake Point Drainage Studies
			Street Revision (Prints of Sh. 7, 7A, & 8)

THESE ARE TRANSMITTED as checked below:

- For approval Approved as submitted Resubmit _____ copies for approval
 For your use Approved as noted Submit _____ copies for distribution
 As requested Returned for corrections Return _____ corrected prints
 For review and comment _____
 FOR BIDS DUE _____ 19____ PRINTS RETURNED AFTER LOAN TO US

REMARKS This material reflects the proposed
changes to the referenced s.w.s. improvement.
Supporting drainage comps are enclosed for
your review.

COPY TO File

SIGNED: Burt E Rensberg

+ 390,000

4-1-87 5% SWS No 337

1% x 5 = 5% Project No: 468-76-245-81683-000-000-001

409.5

COLUMN WR

Quantity	Item	Unit Cost	Ext
840 LF	54" RCP		
130 LF	42" RCP		
330 LF	36" RCP		
90 LF	30" RCP		
100 LF	27" RCP		
(190) LF	24" RCP - 151.3 = 33.7		
90 LF	21" RCP		
(20) LF	18" RCP + 151.3 = 171.3		
(410) LF	15" RCP - 185.7 - 174.4 + 140.6 = 190.5		
(4) ea	Curb Inlet, Type 1A (W=4'4", L=6'4") -1+1=4		
(3) ea	Curb Inlet, Type 1A (W=5'4", L=6'4") -1=2		
1 ea	Curb Inlet, Type 1A (W=6'4", L=6'4")		
3 ea	Curb Inlet, Type 1A (W=4'4", L=11'4")		
3 ea	Curb Inlet, Type 1A (W=5'4", L=11'4")		
1 ea	Curb Inlet, Type 1A (W=6'4", L=11'4")		
1 ea	Curb Inlet, Type 1A (W=7'4", L=11'4")		
1 ea	Special Ditch Inlet		
2 ea	Reinforced Concrete Manhole		
1 ea	Ditch Inlet, Type 1A		
1 ea	Special Area Inlet		
LS	South weir		
LS	North weir		
180 SY	Light Stone Riprap		
1030 SY	Heavy Stone Riprap		
710 SF	Stone wall		
1675 CY	Excavation		
35 CY	Compacted Fill		
65 LF	Remove 8" Sanitary Sewer		
(85) SY	Remove concrete pavement		
7 ea	Remove Large Trees		
23 ea	Remove small trees		
	Eng & Inspection		
	Administration		
	Publication		
	Total		
	MKEC \$ 23,425		
	Eng. & Insp. (23,425 + 7%)		
	20 day Start		
	45 working days		
	To be bid &		

(27)

STORM WATER COLLECTION

PROJ. NO.: _____

AREA: _____ DATE: _____

COMPUTED BY: _____ C'K'D BY: _____

DR. AREA		INLET		HYDROLOGY (Q = CIA)										GUTTER FLOW & INLET INTERCEPTION						
				NO.	LOCATION	S (%)	L (FEET)	T _C (MIN.)	C	A (ACRE)	CA	I (IN/HR.)	Q (CFS)	Q _G	GUTTER SLOPE %	PAV'T SLOPE/FT	W	D	Q _{IN}	BYPASS
102		102										2.58	0.06	3/8	7.4	0.23	1.0	1.60		
103		103									1.02	2.62	5/4 mp				2.62			
105		105		(A ₁ + A ₂ + A ₃)	4.12 + 2.42 + 3.63 =						10.17			5/4 mp						
106		106		+ 0.64	106						10.87									
107		107									1.62	0.36	3/8	6.7	0.21	0.9	0.70			
											(L _T = 14) (1/4 L _T = 0.36) (E = 0.54)	7.27	1.6	3/8	9.3	0.29	1.53	5.74		
											(L _T = 40) (1/4 L _T = 0.13) (E = 0.21)	7.80	2.0	3/8	8.6	0.27	1.5	6.30		
108		108									(L _T = 44) (1/4 L _T = 0.11) (E = 0.19)									

a = 57.46 m = 0.82 b = 12.2 HYDRO-35 IDF FORMULA

SOIL GROUP NO. 80

NO.	AREA	LEN	Y	%IMP	%HLM	CN	S	LAG	Fi	Fh	Tc	i	RI	C	Q
101	2.4	1060	1.12	40	90	87.20	1.47	0.25	0.82	0.60	15.00	3.83	2	0.45	4.12
102	1.5	900	1.21	40	90	87.20	1.47	0.21	0.82	0.60	15.00	3.83	2	0.45	2.58
103	0.6	450	0.56	40	95	87.20	1.47	0.18	0.82	0.58	15.00	3.83	2	0.44	1.02
104	1.4	650	1.77	40	85	87.20	1.47	0.13	0.82	0.63	15.00	3.83	2	0.45	2.42
105	2.1	750	1.67	40	90	87.20	1.47	0.15	0.82	0.60	15.00	3.83	2	0.45	3.63
106	0.9	750	1.60	40	95	87.20	1.47	0.16	0.82	0.58	15.00	3.83	2	0.45	1.62
107	4.2	650	1.77	40	50	87.20	1.47	0.13	0.82	0.78	15.00	3.83	2	0.45	7.27
108	1.2	450	1.09	40	50	87.20	1.47	0.13	0.82	0.78	15.00	3.83	2	0.45	2.06
109	0.3	200	1.78	40	75	87.20	1.47	0.05	0.82	0.67	15.00	3.83	2	0.45	0.52
110	0.3	150	0.50	40	95	87.20	1.47	0.08	0.82	0.58	15.00	3.83	2	0.44	0.51
111	0.3	150	1.00	40	95	87.20	1.47	0.05	0.82	0.58	15.00	3.83	2	0.45	0.51

100 j,	164.6000	200	1	9	8			
110 t,	STORM SEWER FOR AUTUMN CHASE REPLAT							
120 i,	107	0.00	0.00	0.00	0.00	1.50	15.00	172.45
130 i,	108	0.00	0.00	0.00	0.00	1.50	15.00	169.30
140 i,	106	0.00	0.00	0.00	0.00	0.90	15.00	168.60
150 i,	109	0.00	0.00	0.00	0.00	2.10	15.00	168.50
160 i,	105	0.00	0.00	0.00	0.00	10.90	15.00	168.10
170 i,	110	0.00	0.00	0.00	0.00	3.50	15.00	168.20
180 i,	103	0.00	0.00	0.00	0.00	2.60	15.00	167.50
190 i,	111	0.00	0.00	0.00	0.00	4.00	15.00	167.50
200 m,	200	165.00						
210 p,	111	200	108.00	36	0.013	90.00	0.00	
220 p,	103	111	38.00	24	0.013	90.00	0.00	
230 p,	110	111	90.00	30	0.013	9.00	0.00	
240 p,	105	110	115.00	24	0.013	115.00	0.00	
250 p,	109	110	69.00	27	0.013	0.00	0.00	
260 p,	106	109	43.00	15	0.013	45.00	0.00	
270 p,	108	109	192.00	18	0.013	0.00	0.00	
280 p,	107	108	141.00	15	0.013	45.00	0.00	
290 e								

Input File: chasesws

STORM SEWER FOR AUTUMN CHASE REPLAT

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

*****										*****									
Tributary Area										Hydrology Summation				Conduit Data					
*****										*****									
Node to	C	Area	Slope	Length	TC(θ)	I(θ)	Q(θ)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC			
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)			
*****										*****									
111	200	0.00	0.00	0.00	0.0	15.00	4.06	4.00	20.05	3.62	3.57	24.73	36"	3.50	100.00	0.51	20.56		
103	111	0.00	0.00	0.00	0.0	15.00	4.06	2.60	15.00	4.06	2.60	2.60	24"	0.83	38.00	0.77	15.77		
110	111	0.00	0.00	0.00	0.0	15.00	4.06	3.50	19.65	3.65	3.15	18.79	30"	3.83	90.00	0.39	20.05		
105	110	0.00	0.00	0.00	0.0	15.00	4.06	10.90	15.00	4.06	10.90	10.90	24"	3.47	115.00	0.55	15.55		
109	110	0.00	0.00	0.00	0.0	15.00	4.06	2.10	18.85	3.71	1.92	5.70	27"	1.43	69.00	0.80	19.65		
106	109	0.00	0.00	0.00	0.0	15.00	4.06	0.90	15.00	4.06	0.90	0.90	15"	0.73	43.00	0.98	15.98		
108	109	0.00	0.00	0.00	0.0	15.00	4.06	1.50	16.92	3.87	1.43	2.93	18"	1.66	192.00	1.93	18.85		
107	108	0.00	0.00	0.00	0.0	15.00	4.06	1.50	15.00	4.06	1.50	1.50	15"	1.22	141.00	1.92	16.92		
*****										*****									

Input File: chasesws

STORM SEWER FOR AUTUMN CHASE REPLAT

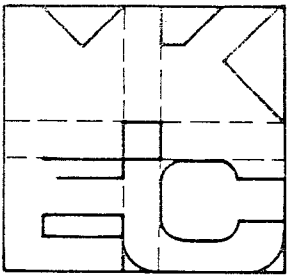
Storm Frequency = 2-Year

* * * H Y D R A U L I C S * * *

```

*****
Node      Hyd-Slope  Friction  Bend.    Transition  Manhole  Deflection  Junction  Total  Hyd-61  Desired  Diff.
      (Ft/Ft)   (Ft)     (Ft)     (Ft)       (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)
*****
107      0.00054   0.0760   0.0000   0.0000     0.0000   0.0000     0.0000   0.0760 165.7350 172.4500  6.71
108      0.00078   0.1495   0.0000   0.0020     0.0000   0.0050     0.0662   0.2227 165.6597 169.3000  3.64
106      0.00019   0.0083   0.0000   0.0000     0.0000   0.0000     0.0000   0.0083 165.4454 168.6000  3.15
109      0.00034   0.0233   0.0000   0.0022     0.0000   0.0000     0.0335   0.0590 165.4370 169.5000  3.06
105      0.00232   0.2670   0.0000   0.0000     0.0000   0.0000     0.0000   0.2670 165.6450 168.1000  2.45
110      0.00210   0.1890   0.0000   0.0041     0.0000   0.1254     0.2119   0.5304 165.3781 168.2000  2.82
103      0.00013   0.0050   0.0000   0.0000     0.0000   0.0000     0.0000   0.0050 164.8527 167.5000  2.65
111      0.00138   0.1485   0.0000   0.0075     0.0000   0.0072     0.0045   0.2477 164.8477 167.5000  2.65
200      0.00000   0.0000   0.0000   0.0000     0.0000   0.0000     0.0000   0.0000 164.6000 165.0000  0.40
*****

```



MID-KANSAS ENGINEERING
CONSULTANTS P.A.

3500 NORTH ROCK ROAD, BLDG. #600
WICHITA, KANSAS 67226 1-316-682-6541

CALCULATIONS & SKETCHES

Proj. No.		Date	12/12/96
By	KLS	Date	
Chkd By		Date	
Sheet	1	Of	

Location _____
Reference _____
Check of Cantilever Retaining Wall @ Woodland Est.

Soil $\phi = 125 \text{pcf}$

$P_a = K_a \delta h$

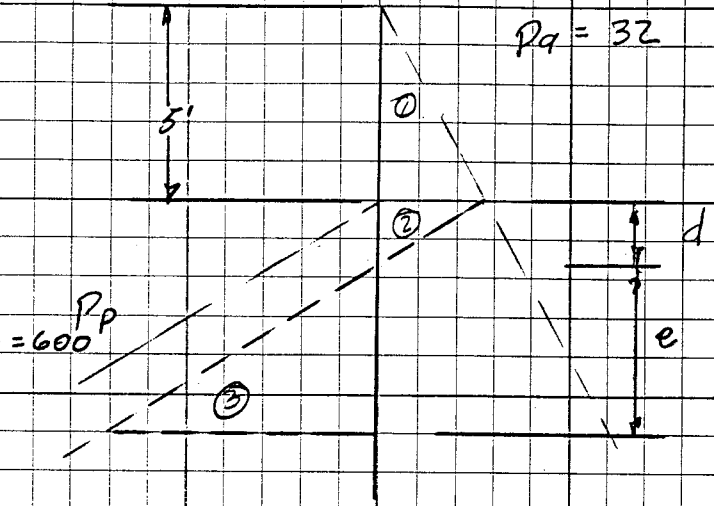
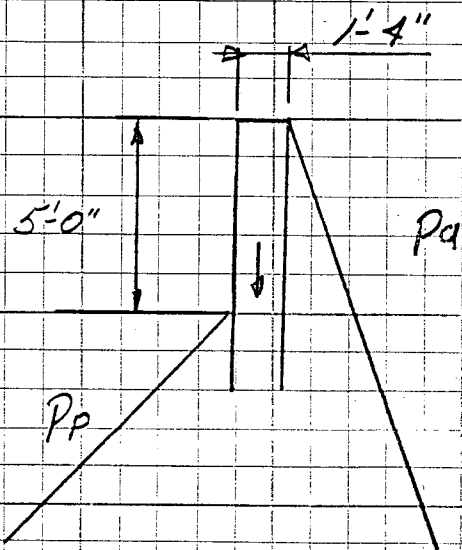
$K_a (\phi = 35, \delta = 10) = 0.25$

$P_a = 0.25 \cdot 125 = 32 \text{pcf}$

$P_p = K_p \delta h$

$K_p (35, 10) = 4.8$

$P_p = 4.8 \cdot 125 = 600 \text{pcf}$



$5 \cdot 32 + d \cdot 32 = d \cdot 600$

$568 = d \cdot 568, d = \frac{100}{568} = 0.28 \text{ ft}$

$\sum M_{\text{about}} = 0$

$(1) + (2) - (3) = 0$

$(d+e + \frac{5}{3}) \cdot (\frac{5^2}{2} \cdot 32)$

$+ (d+e - \frac{2e}{3}) \cdot (\frac{0.28^2}{2} \cdot 568)$

$- (\frac{e}{3}) \cdot (\frac{e^2}{2} \cdot 568) = 0$

$(e+1.95)(400) + (e+1.9)(22)$

$- \frac{1}{2}(\frac{e}{3})(e^2 \cdot 204) = 0$

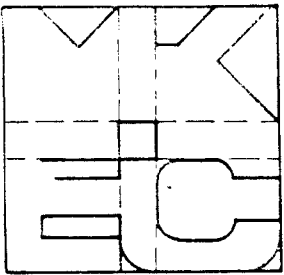
$400e + 780 + 22e + 4.2$

$- e^3 95 = 0$

$422e + 784 - e^3 95 = 0$

$e = 1.0, e = 2.73 \text{ Ft}$

$\text{Factor} = 2.0, e = 2.73 \text{ Ft}$



MID-KANSAS ENGINEERING CONSULTANTS P.A.

1600 NORTH ROCK ROAD, BLDG. 100
WICHITA, KANSAS 67220 TEL: 620-687-6661

CALCULATIONS & SKETCHES

Proj. No.		
By	KDS	Date 12/12/86
Chkd By		Date
Sheet	2	Of

Location _____
 Reference Conti Ret Wall
Woodland Est.

pt. of zero shear

$$\Sigma F = 0$$

$$\textcircled{1} + \textcircled{2} - \textcircled{3} = 0$$

$$\frac{5^2}{2} \cdot 32 + \frac{20^2}{2} \cdot 568$$

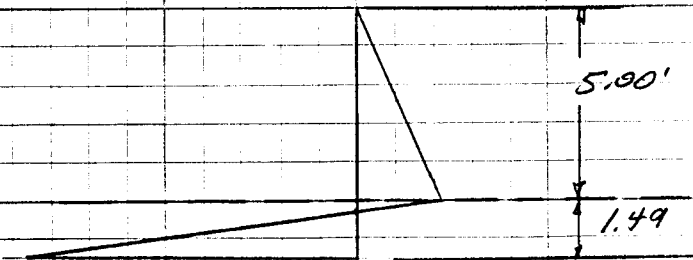
$$- z^2 \cdot 568 = 0$$

$$400 + 18 - z^2 \cdot 284 = 0$$

$$z^2 = \frac{418}{284}$$

$$z = \pm 1.21 = 1.21'$$

1770 ft. Moment in wall



$$\left(\frac{5}{3} + 1.49\right) \cdot 400 + 1.49 \cdot 18$$

$$- \frac{1.21}{3} \cdot \frac{1.21^2}{2} \cdot 568 = M_{max}$$

$$1263 + 25 - 168 = 1120 \text{ lb}\cdot\text{ft}$$

$$M = 13.4 \text{ k}\cdot\text{in}$$

$$A_s = \frac{13.4}{144 \cdot 13} = 0.072 \text{ in}^2/\text{ft}$$

$$A_{s,ult} = \frac{13.4 \cdot 1.7}{4 \cdot 13} = 0.34 \text{ in}^2/\text{ft}$$

#5 @ 12" o.c. adequate.

Check restoring moment of wall
 $t_w = 1.33'$

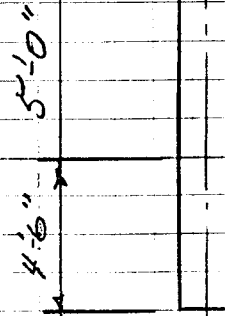
$$8 \cdot 1.33 \cdot .15 = 1.60 \text{ k/ft}$$

$$M = 1600 \cdot .67 = \underline{1072 \text{ lb}\cdot\text{ft}}$$

Mot @ max. bending pt.

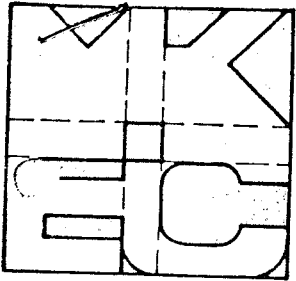
$$1263 + 25 = \underline{1288 \text{ lb}\cdot\text{ft}}$$

-168.0



1158.5

add 1'-6" to previous design



MID-KANSAS ENGINEERING CONSULTANTS P.A.

3500 NORTH ROCK ROAD, BLDG #800
WICHITA, KANSAS 67226 1 316 682 6561

CALCULATIONS & SKETCHES

Proj. No.		Date	
By	FBN	Date	2/26/87
Chkd By		Date	
Sheet	1	Of	

Location: Woodland Estates 2nd

Reference: Lake - between W.E. & W.E. 2nd

<u>Q₁₀₀</u>	<u>Q₁₀₀</u>	<u>T_c</u>
Vin Keridge	56.3	15 min
Area H, Woodland Estates	27.2	17 min
Woodland Estates 2nd	71.6	17 min

$$\text{Total} = (56.3) \frac{7.00}{7.37} + 27.2 + 71.6 = 152.3 \text{ cfs}$$

from Golf Course; Q₁₀₀ = 43.8 cfs T_c = 45 min

$$\text{Total w/ Golf Course: } Q_{100} = \frac{4.38}{7.00} (155.1) + 43.8 = 140.9 \text{ cfs}$$

Use Q₁₀₀ = 152.3 cfs

Use 2 - 48" RCP's @ 0.30%

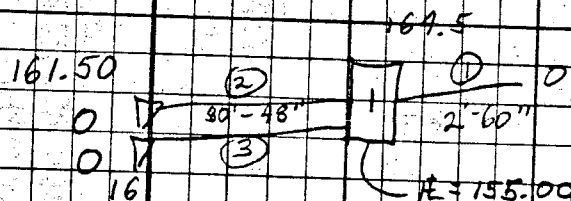
Check Req'd. Opening for Head = 164.5 - 162 = 2.5' by Orifice Formula

$$Q_i = .6A \sqrt{2gh} \Rightarrow A = \frac{152.3}{.6 \sqrt{64.4 \times 2.5}} = 20.00 \text{ sq. ft}$$

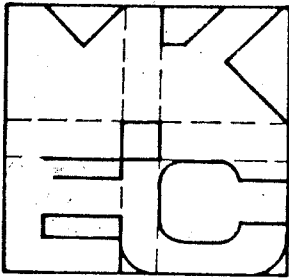
$$\text{Min Width} = 2 \left(\frac{48+10}{12} \right) + 2' + 2(2.5) = 12.67'$$

$$\text{Length} = \frac{2000}{12.67} = 1.58'$$

Check Pipe Size



Based on PIPES Run - Go w/ 48" RCP's for Estimate
WRESTLK - 04T



MID-KANSAS ENGINEERING CONSULTANTS P.A.

3500 NORTH ROCK ROAD, BLDG. #800
WICHITA, KANSAS 67226 1-316-682-6561

CALCULATIONS & SKETCHES

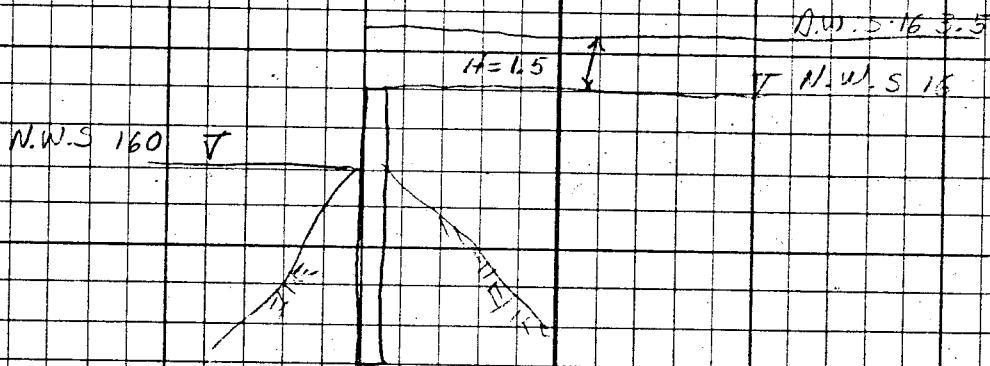
Proj. No.		Date	3/19/87
By	FBN	Date	
Chkd By		Date	
Sheet	2	Of	

Location Woodland Estates 2nd
 Reference Lake between W.E. & W.E. 2nd

Weir
 Design Water Surface 163.5
 Normal Water Surface 162.0
 H = 1.5'

Elev.'s of N. Lake between Gatewood & Woodland Estates

Normal Water Surface = 160
 Design Water Surface = 161.45



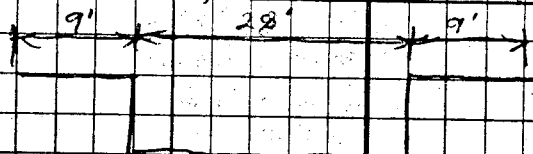
From Merritt, Std. Handbook for Civil Engineers, p. 21-79

Broadcrested weir formula: $Q = CLH^{3/2}$

for Breadth of Weir = 1.33' & $H = 1.5'$, $C = 3.08$

$Q_{100} = 152.3 \text{ cfs}$ $L = \frac{152.3}{1.5^{3/2} \times 3.08} = 26.9'$

For Contracted Weir, $L = 26.9 + (1)(2)(1.5) = 27.2' \approx 28'$



Total $L = 28 + 2(9) = 46'$