



Professional Engineering Consultants, PA.

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LETTER OF TRANSMITTAL

TO: Vicky Huang - Engineering Department 1th Floor
455 N. Main
Wichita, KS 67202
ATTENTION: _____
COPIES TO: FILE

PROJECT NO.: 36-97d09-4366
PROJECT: Chapel Hill Plat
DATE: 4/14/99
FROM: Karen Rand
REFERENCE: Drainage Plan

- WE ARE SENDING YOU: Attached Under separate cover via _____ the following items:
- Shop drawings Prints Plans Samples Specifications
 - Copy of letter Change order AutoCAD Drawing Files - Version 14 on 3.5" diskette(s)

COPIES	DATE	NO.	DESCRIPTION
1			HEC-1 Model for Chapel Hill
1			Revised Four Corner Plan (City Datum)

THESE ARE TRANSMITTED as checked below:

- For approval Approved as submitted Resubmit _____ copies for approval
- For your use Approved as noted Submit _____ copies for distribution
- As requested Returned for corrections Return _____ corrected prints
- For review and comment _____
- FOR BIDS DUE _____ PRINTS RETURNED AFTER LOAN TO US

REMARKS: This is a revised HEC-1 model for Chapel Hill Addition. Due to field conditions, the pond size has been modified as well as the outfall location and size. The new parameters were used in the revised HEC-1 model and the design highwater for the pond increased slightly. This small change in water elevation is not expected to change the minimum openings for the lots adjacent to the pond. A revised Four Corner Plan is also included, using City of Wichita Datum for the elevations. A copy of the four corner plan is also being forwarded to Mike Gable, OCI.

RECEIVED

APR 15 1999

CITY - ENGINEERING

COPY TO _____ SIGNED Karen Rand
If enclosures are not as noted, kindly notify us at once.

Chapel Hill Addition

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*****  
*  
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *  
* MAY 1991 *  
* VERSION 4.0.1E *  
* Lahey F77L-EM/32 version 5.01 *  
* Dodson & Associates, Inc. *  
* RUN DATE 04/13/99 TIME 15:15:06 *  
*****
```

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*****  
*  
* U.S. ARMY CORPS OF ENGINEERS *  
* HYDROLOGIC ENGINEERING CENTER *  
* 609 SECOND STREET *  
* DAVIS, CALIFORNIA 95616 *  
* (916) 551-1748 *  
*****
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X X XXXXXX XXXX X  
X X X X X XX  
X X X X X  
XXXXXX XXXX X XXXX X  
X X X X X  
X X X X X  
X X XXXXXX XXXX XXX
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

Chapel Hill Addition

HEC-1 INPUT

PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1 ID CHAPEL HILL, WICHITA KS
 2 ID 25-, & 100-Year Storms
 3 ID Professional Engineering Consultants
 4 ID Wichita, Ks
 5 ID KER 10/6/98
 6 ID File: T:\DAR\HEC1\CHAPEL.ih1
 7 IT 6 12DEC97 0600 0 13DEC97 1154
 8 IN 30 12DEC97 0600
 9 IO 3 0
 10 JR PREC .78947 1.000

*
 *DIAGRAM
 *

11 KK BAS1 Developed Residential
 12 BA .017
 13 PB 7.8
 14 PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
 15 PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
 16 PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
 17 PC .964 .970 .976 .982 .988 .994 1.000
 18 LS 0 90 0
 19 UD .15

*
 *

20 KK BAS2 Developed Commercial
 21 BA .0334
 22 PB 7.8
 23 PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
 24 PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
 25 PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
 26 PC .964 .970 .976 .982 .988 .994 1.000
 27 LS 0 93 0
 28 UD .15

*
 *

29 KK BAS3 Street & Right of Way
 30 BA .0059
 31 PB 7.8
 32 PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
 33 PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
 34 PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
 35 PC .964 .970 .976 .982 .988 .994 1.000
 36 LS 0 90 0
 37 UD .15

*
 *

38 KK TOTAL
 39 HC 3

*
 *

* 6'6"x3'6" Inlet against pond bank with
 * 58"x34" RCPHE Outlet to K-96 Ditch

Chapel Hill Addition

HEC-1 INPUT

PAGE 2

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
40	KK	POND									
41	RS	1	ELEV	59							
42	SA	2.57	2.96	3.2							
43	SE	59	61.1	62							
44	SQ	0	3.6	10.2	18.8	29	40.5	53.2	67.1	82	88
45	SQ	94.6	99	100	120	150					
46	SE	59.0	59.2	59.4	59.6	59.8	60	60.2	60.4	60.6	60.7
47	SE	61	61.2	61.3	62	63					
	*										
48	ZZ										

Chapel Hill Addition

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW	
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW	
11	BAS1		
	.		
20	.	BAS2	
	.	.	
29	.	.	BAS3
	.	.	.
38	TOTAL.....	.	
	V		
	V		
40	POND		

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

Chapel Hill Addition

```
*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*   Lahey F77L-EM/32 version 5.01 *
*   Dodson & Associates, Inc. *
* RUN DATE 04/13/99 TIME 15:15:06 *
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```
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* U.S. ARMY CORPS OF ENGINEERS *
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* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *
*****
```

```
CHAPEL HILL, WICHITA KS
25-, & 100-Year Storms
Professional Engineering Consultants
Wichita, Ks
KER 10/6/98
File: T:\DAR\HEC1\CHAPEL.ih1
```

```
9 IO      OUTPUT CONTROL VARIABLES
          IPRNT      3 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN       6 MINUTES IN COMPUTATION INTERVAL
          IDATE      12DEC97 STARTING DATE
          ITIME      0600 STARTING TIME
          NQ         300 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     13DEC97 ENDING DATE
          NDTIME     1154 ENDING TIME
          ICENT      19 CENTURY MARK

          COMPUTATION INTERVAL 0.10 HOURS
          TOTAL TIME BASE 29.90 HOURS

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE       DEGREES FAHRENHEIT

JP        MULTI-PLAN OPTION
          NPLAN      1 NUMBER OF PLANS

JR        MULTI-RATIO OPTION
          RATIOS OF PRECIPITATION
          0.79      1.00
```

*** ** ** ** **

```
*****
*
* BAS1 *          Developed Residential
*
*****
```

```
8 IN      TIME DATA FOR INPUT TIME SERIES
          JXMIN      30 TIME INTERVAL IN MINUTES
          JXDATE     12DEC97 STARTING DATE
          JXTIME     600 STARTING TIME
```

SUBBASIN RUNOFF DATA

```
12 BA     SUBBASIN CHARACTERISTICS
          TAREA     0.02 SUBBASIN AREA
```

PRECIPITATION DATA

```
13 PB     STORM      7.80 BASIN TOTAL PRECIPITATION
```

```
14 PI     INCREMENTAL PRECIPITATION PATTERN
```


Chapel Hill Addition

37 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 0.15 LAG

UNIT HYDROGRAPH
10 END-OF-PERIOD ORDINATES

	7.	14.	10.	4.	2.	1.	0.	0.	0.
TOTAL RAINFALL =	7.80,	TOTAL LOSS =	1.19,	TOTAL EXCESS =	6.61				
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW						
+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR			
+ 22.	6.00	(CFS)							
		(INCHES)	3.	1.	1.	1.			
		(AC-FT)	5.373	6.609	6.609	6.609			
			2.	2.	2.	2.			
		CUMULATIVE AREA =	0.01 SQ MI						

*** *** *** *** ***

HYDROGRAPH AT STATION BAS3
FOR PLAN 1, RATIO = 0.79

TOTAL RAINFALL =	6.16,	TOTAL LOSS =	1.16,	TOTAL EXCESS =	5.00	
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
+ 17.	6.00	(CFS)				
		(INCHES)	3.	1.	1.	1.
		(AC-FT)	4.082	5.000	5.000	5.000
			1.	2.	2.	2.
		CUMULATIVE AREA =	0.01 SQ MI			

*** *** *** *** ***

HYDROGRAPH AT STATION BAS3
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL =	7.80,	TOTAL LOSS =	1.19,	TOTAL EXCESS =	6.61	
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
+ 22.	6.00	(CFS)				
		(INCHES)	3.	1.	1.	1.
		(AC-FT)	5.373	6.609	6.609	6.609
			2.	2.	2.	2.
		CUMULATIVE AREA =	0.01 SQ MI			

*** **

* *
38 KK * TOTAL *
* *

39 HC HYDROGRAPH COMBINATION
 ICOMP 3 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION TOTAL
FOR PLAN 1, RATIO = 0.79

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
-----------	------	--	----------------------	--	--	--

Chapel Hill Addition

+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
		(CFS)				
+ 167.	6.00		26.	8.	6.	6.
		(INCHES)	4.226	5.201	5.201	5.201
		(AC-FT)	13.	16.	16.	16.

CUMULATIVE AREA = 0.06 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION TOTAL
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
		(CFS)				
+ 216.	6.00		33.	10.	8.	8.
		(INCHES)	5.512	6.820	6.820	6.820
		(AC-FT)	17.	20.	20.	20.

CUMULATIVE AREA = 0.06 SQ MI

*** **

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*****
*           *
* 40 KK    * POND *
*           *
*****
    
```

HYDROGRAPH ROUTING DATA

41 RS	STORAGE ROUTING										
	NSTPS	1	NUMBER OF SUBREACHES								
	ITYP	ELEV	TYPE OF INITIAL CONDITION								
	RSVVIC	59.00	INITIAL CONDITION								
	X	0.00	WORKING R AND D COEFFICIENT								
42 SA	AREA	2.6	3.0	3.2							
43 SE	ELEVATION	59.00	61.10	62.00							
44 SQ	DISCHARGE	0.	4.	10.	19.	29.	41.	53.	67.	82.	88.
		95.	99.	100.	120.	150.					
46 SE	ELEVATION	59.00	59.20	59.40	59.60	59.80	60.00	60.20	60.40	60.60	60.70
		61.00	61.20	61.30	62.00	63.00					

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	5.80	8.57
ELEVATION	59.00	61.10	62.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.52	1.04	1.57	2.11	2.66	3.21	3.78	4.35	4.63
OUTFLOW	0.00	3.60	10.20	18.80	29.00	40.50	53.20	67.10	82.00	88.00
ELEVATION	59.00	59.20	59.40	59.60	59.80	60.00	60.20	60.40	60.60	60.70

STORAGE	5.51	5.80	6.10	6.40	8.57	11.91
OUTFLOW	94.60	96.80	99.00	100.00	120.00	150.00
ELEVATION	61.00	61.10	61.20	61.30	62.00	63.00

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 0.79

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR

Chapel Hill Addition

		(CFS)			
+	90.	6.20	25.	8.	6.
			(INCHES) 4.134	5.199	5.201
			(AC-FT) 12.	16.	16.
	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE	
			6-HR	24-HR	72-HR
+	(AC-FT)	(HR)			29.90-HR
	5.	6.20	2.	1.	1.
	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE	
			6-HR	24-HR	72-HR
+	(FEET)	(HR)			29.90-HR
	60.79	6.20	59.66	59.25	59.20

CUMULATIVE AREA = 0.06 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.00

		(CFS)			
	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW	
			6-HR	24-HR	72-HR
+	(CFS)	(HR)			29.90-HR
+	101.	6.20	33.	10.	8.
			(INCHES) 5.417	6.817	6.819
			(AC-FT) 16.	20.	20.
	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE	
			6-HR	24-HR	72-HR
+	(AC-FT)	(HR)			29.90-HR
	7.	6.20	2.	1.	1.
	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE	
			6-HR	24-HR	72-HR
+	(FEET)	(HR)			29.90-HR
	61.34	6.20	59.84	59.31	59.25

CUMULATIVE AREA = 0.06 SQ MI

Chapel Hill Addition

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION	
				RATIO 1	RATIO 2
				0.79	1.00
HYDROGRAPH AT					
+ BAS1	0.02	1	FLOW TIME	49. 6.00	64. 6.00
HYDROGRAPH AT					
+ BAS2	0.03	1	FLOW TIME	101. 6.00	130. 6.00
HYDROGRAPH AT					
+ BAS3	0.01	1	FLOW TIME	17. 6.00	22. 6.00
3 COMBINED AT					
+ TOTAL	0.06	1	FLOW TIME	167. 6.00	216. 6.00
ROUTED TO					
+ POND	0.06	1	FLOW TIME	90. 6.20	101. 6.20
** PEAK STAGES IN FEET **					
		1	STAGE TIME	60.79 6.20	61.34 6.20

*** NORMAL END OF HEC-1 ***

Flow Control
Chapel Hill Addition
Pond Outflow

