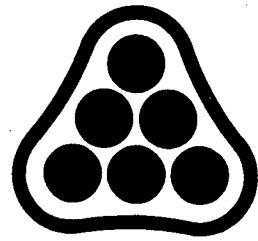


DRAINAGE PLAN  
AND  
SUPPORTING CALCULATIONS

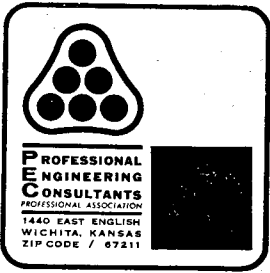
FOR  
CROSS CREEK  
AN ADDITION TO WICHITA, SEDGWICK COUNTY, KANSAS

PREPARED BY  
PROFESSIONAL ENGINEERING CONSULTANTS, P.A.  
ENGINEERS  
WICHITA, KANSAS

NOVEMBER 20, 1987



**P**ROFESSIONAL  
**E**NGINEERING  
**C**ONSULTANTS  
PROFESSIONAL ASSOCIATION



Date 11-12-87 Page 1 of 8

Project Cross Creek Addition

Item Drainage Plan System 100

I HYDROLOGY

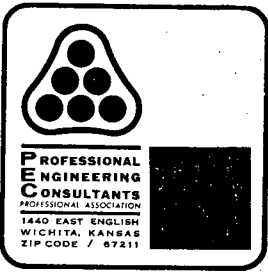
Use Rational Method  $Q = CIA$

1. Determine "C"

<u>Node</u>	<u>Soil Type</u>	<u>Hydrologic Group</u>	<u>Land Use</u>	<u>C<sub>2</sub></u>	<u>C<sub>100</sub></u>
103	I <sub>a</sub> , Rd	D	Res, 1/3 Ac.	0.46	0.73
102	I <sub>a</sub> , Rd	D	Res, 1/3 Ac.	0.46	0.73
101	I <sub>a</sub> , Rd	D	Res, 1/3 Ac.	0.46	0.73
100	(End Section)				

2. Determine "I"

<u>Node</u>	<u>t<sub>c</sub></u>	<u>I<sub>2</sub></u>	<u>I<sub>100</sub></u>
103	15	3.83	7.37
102	15	3.83	7.37
101	15	3.83	7.37
100	(End Section)		



Date 11-13-87 Page 2 of 8

Project Cross Creek Addition

Item Drainage Plan System 100

3 Determine "A"

<u>Node</u>	<u>Plan. Units</u>	<u>Area - SF</u>	<u>Area - Acres</u>
103	621	99,360	2.28
102	1760	281,600	6.46
101	1813	290,080	6.66
100	(End Section)		

4. Determine "Q<sub>2</sub>"

<u>Node</u>	<u>C<sub>2</sub></u>	<u>I<sub>2</sub></u>	<u>A</u>	<u>Q<sub>2</sub></u>
103	0.46	3.83	2.28	4.0
102	0.46	3.83	6.46	11.4
101	0.46	3.83	6.66	11.7
100	(End Section)			

5. Determine "Q<sub>100</sub>"

<u>Node</u>	<u>C<sub>100</sub></u>	<u>I<sub>100</sub></u>	<u>A</u>	<u>Q<sub>100</sub></u>
103	0.76	7.37	2.28	12.8
102	0.76	7.37	6.46	36.2
101	0.76	7.37	6.66	37.3
100	(End Section)			



Date 11.13.87 Page 3 of 8

Project Cross Creek Addition

Item Drainage Plan System 100

II INLET SIZING

<u>Node</u>	<u>Inlet Condition</u>	<u>Q<sub>2</sub></u>	<u>Max. HW</u>	<u>Q<sub>max</sub> (5' Inlet)</u>	<u>Q<sub>max</sub> (10' Inlet)</u>	<u>USE L<sub>f</sub></u>
103	Sump	4.0	0.92'	11.0	22.0	5'
102	Sump	11.4	0.92'	11.0	22.0	10'
101	Sump	11.7	0.92'	11.0	22.0	10'
100	(End Section)					

III FLOOD ROUTING

<u>Node</u>	<u>Inlet Condition</u>	<u>Q<sub>approach</sub>*</u>	<u>Q<sub>intercept</sub>†</u>	<u>Q<sub>bypass</sub></u>	<u>to Node#</u>
103	Sump	4.0	4.0	0.0	-
102	Sump	11.4	11.4	0.0	-
101	Sump	11.7	11.7	0.0	-
100	(End Section)				

\* = Q<sub>2</sub>

† = Input Q in "storm" program.



Date 11.13.87 Page 4 of 8

Project Cross Creek Addition

Item Drainage Plan System 100

IV STREET FLOW - 2YR

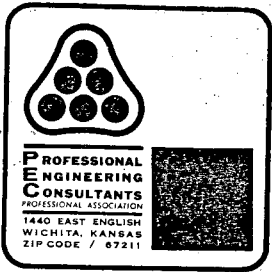
<u>Node</u>	<u>Q<sub>2</sub></u>	<u>Distribution</u>	<u>street slope</u>	<u>d</u>	<u>d<sub>max</sub></u>	<u>Comment</u>
103	4.0	≈ 100% (W) = 4.0	1.2%	0.28'	0.55'	OK
102	11.4	20% (W) = 2.3	1.2%	0.23'	0.30'	OK
		80% (N) = 9.1	1.2%	0.38'	0.55'	OK
101	11.7	5% (S) = 0.6	0.32%	0.18'	0.55'	OK
		95% (N) = 11.1	1.2%	0.42'	0.55'	OK

V STREET FLOW - 100 YR

$$Q_{street} = Q_{100} - Q_{pipe}$$

<u>Location</u>	<u>Contributing Areas</u>	<u>Q<sub>100</sub></u>	<u>Q<sub>pipe</sub></u>	<u>Q<sub>street</sub></u>	<u>street slope</u>	<u>Q<sub>max</sub>*</u>	<u>Comment</u>
Approaching Nodes 101 + 102 from N.	95% 101 =	35.4					
	80% 102 =	29.0					
		<u>64.4</u>	0.0	64.4	1.2%	96.2	OK (Walk Grade = TC + 0.3')
Overtopping Crest South of Cross Creek Court	103	12.8					
	102	36.2					
	101	<u>37.3</u>					
		<u>86.3</u>	27.1	59.2	2.7%	144.3	OK (Walk Grade = T.C. + 0.3')

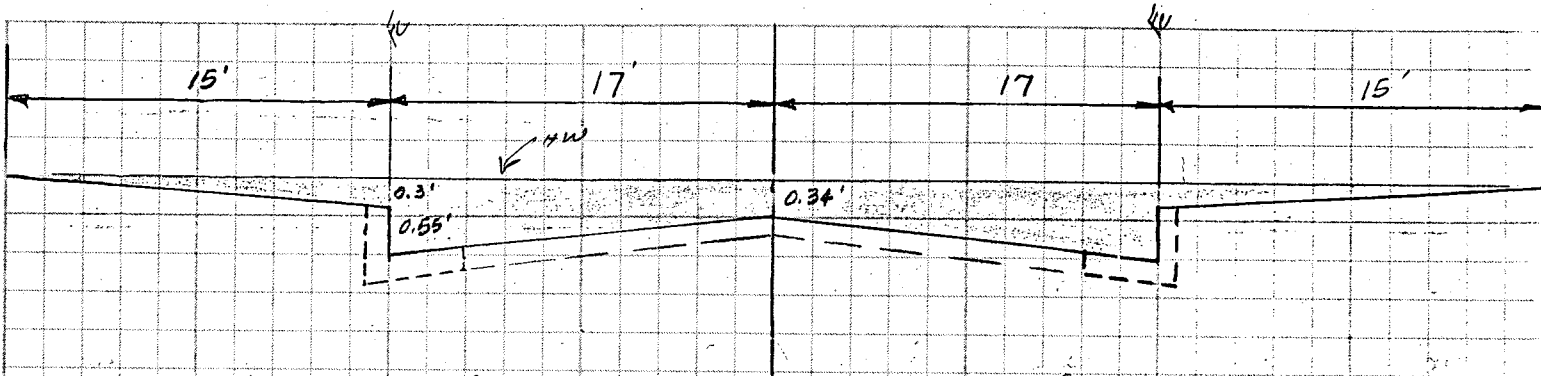
\* From Page 5.



Date 11.13.87 Page 5 of 8

Project Cross Creek Addition

Item Drainage Plan System 100



Determine  $Q_{max}$  in street right-of-way.

Use Manning's Equation  $Q = \frac{1.486}{n} AR^{2/3} S^{1/2}$

$$n = \frac{(2 \times 14.5 \times 0.03) + (2 \times 3.05 \times 0.013) + (2 \times 15 \times 0.016)}{65.1} = \frac{1.4293}{65.1}$$

$$n = 0.021955$$

$$A = (2 \times \frac{1}{2} \times 15 \times 0.3) + (34 \times 0.34) + (2 \times \frac{1}{2} \times 17 \times 0.51)$$

$$A = 24.73$$

$$p = 65.1$$

$$R = A/p = 24.73/65.1 = 0.379877$$

$$R^{2/3} = (0.379877)^{2/3} = 0.5245$$

$$Q = \frac{1.486}{0.021955} \times 24.73 \times 0.5245 \times S^{1/2}$$

$$Q = 877.92 S^{1/2}$$

100 j, 168.0000 100 3 4 3  
 110 t, cross creek addition  
 120 t, drainage plan  
 130 t, storm water sewer system 100 analysis  
 140 i, 103 0.46 2.28 0.00 0.00 4.00 15.00 173.00  
 150 i, 102 0.46 6.46 0.00 0.00 11.40 15.00 173.00  
 160 i, 101 0.46 6.66 0.00 0.00 11.70 15.00 173.00  
 170 a, 100 168.00  
 180 p, 103 102 60.00 15 0.013 100.00 0.00  
 190 p, 102 101 38.00 24 0.013 10.00 0.00  
 200 p, 101 100 150.00 27 0.013 90.00 0.00  
 210 e

Date: 11-13-1987  
Time: 19:25:19

Input File: ccrask100

cross creek addition  
drainage plan  
storm water sewer system 100 analysis

Storm Frequency = 2-Year

\* \* \* HYDROLOGY \* \* \*

Tributary Area							Hydrology Summation					Conduit Data					
Node to	Node	Area (Ac)	Slope (%)	Length (Ft)	TC (Min)	I (In/Hr)	Q (CFS)	TC (Min)	I (In/Hr)	Q (CFS)	Sum Q (CFS)	Size	Velocity (Ft/Sec)	Length (Ft)	TT (Min)	TT+TC (Min)	
103	102	0.46	2.28	0.00	0.0	15.00	4.06	4.00	15.00	4.06	4.00	4.00	15"	3.26	30.00	0.41	15.41
102	101	0.46	6.46	0.00	0.0	15.00	4.06	11.40	15.00	4.06	11.40	15.29	24"	4.27	38.00	0.13	15.13
101	100	0.48	6.65	3.00	0.0	15.00	4.06	11.70	15.13	4.03	11.66	26.93	27"	6.78	153.00	0.37	15.50

Date: 11-13-1987  
Time: 13:25:19

Input File: ccreak100

cross creek addition  
drainage plan  
storm water sewer system 100 analysis

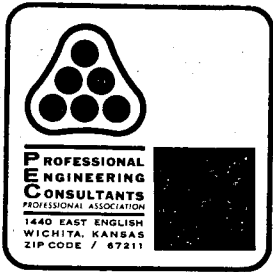
Storm Frequency = 2-Year

\* \* \* HYDRAULICS \* \* \*

```

*****
Node      Hyd-Slope  Friction   Bend      Transition  Manhole  Deflection  Junction  Total  Hyd-51  Desired  Diff.
          (Ft/Ft)    (Ft)      (Ft)      (Ft)        (Ft)     (Ft)        (Ft)     (Ft)   Elevation Elevation (Ft)
*****
100      0.00353    0.3367    0.0000    0.0000     0.0000    0.5000     0.0000    0.3067  171.6510  173.0000  1.35
102      0.00457    0.1737    0.0000    0.0293     0.0000    0.0936     0.8953    1.1838  171.3448  173.0000  1.66
101      0.00757    1.1362    0.0000    0.0346     0.0000    0.0132     0.9770    2.1609  172.1609  173.0000  2.84
100      0.00000    0.0000    0.0000    0.0000     0.0000    0.0000     0.0000    0.0000  168.0000  168.0000  0.00
*****

```



Date 11.13.87 Page 1 of 9

Project Cross Creek Addition

Item Drainage Plan System 200

I HYDROLOGY

Use Rational Method,  $Q = CIA$

1. Determine "c"

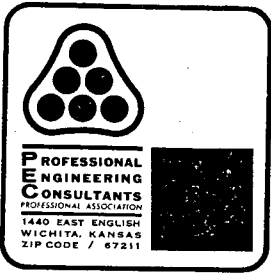
<u>Node</u>	<u>Soil Type</u>	<u>Hydrologic Group</u>	<u>Land Use</u>	<u>C<sub>2</sub></u>	<u>C<sub>100</sub></u>
202	Ia, Rd	D	Res, 1/3 Ac.	0.46	0.73
201	Ia, Rd	D	Res, 1/3 Ac.	0.46	0.73
200	(End section)				

2. Determine "I"

<u>Node</u>	<u>t<sub>c</sub></u>	<u>I<sub>2</sub></u>	<u>I<sub>100</sub></u>
202	15	3.83	7.37
201	15	3.83	7.37
200	(End Section)		

3. Determine "A"

<u>Node</u>	<u>Plan. Units</u>	<u>Area - S.F.</u>	<u>Area - Acres</u>
202	450	72,000	1.65
201	736	117,760	2.70
200	(End Section)		



Date 11.13.87 Page 2 of 9

Project Cross Creek Addition

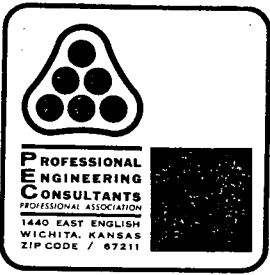
Item Drainage Plan System 200

4. Determine "Q<sub>2</sub>"

<u>Node</u>	<u>C<sub>2</sub></u>	<u>I<sub>2</sub></u>	<u>A</u>	<u>Q<sub>2</sub></u>
202	0.46	3.83	1.65	2.9
201	0.46	3.83	2.70	4.8
200	(End Section)			

5. Determine "Q<sub>100</sub>"

<u>Node</u>	<u>C<sub>100</sub></u>	<u>I<sub>100</sub></u>	<u>A</u>	<u>Q<sub>100</sub></u>
202	0.73	7.37	1.65	8.9
201	0.73	7.37	2.70	14.5
200	(End Section)			



Date 11.13.87 Page 3 of 9

Project Cross Creek Addition

Item Drainage Plan System 200

II INLET SIZING

<u>Node</u>	<u>Inlet Condition</u>	<u>Q<sub>2</sub></u>	<u>Max. HW</u>	<u>Q<sub>max</sub> (5' inlet)</u>	<u>Q<sub>max</sub> (10' inlet)</u>	<u>Use L=</u>
202	Sump	2.9	0.92'	11.0'	22.0'	5'
201	Sump	4.8	0.92'	11.0'	22.0'	5'
200	(End Section)					

III FLOOD ROUTING

<u>Node</u>	<u>Inlet Condition</u>	<u>Q<sub>approach</sub>*</u>	<u>Q<sub>intercept</sub>†</u>	<u>Q<sub>bypass</sub></u>	<u>to Node #</u>
202	Sump	2.9	2.9	0.0	-
201	Sump	4.8	4.8	0.0	-
200	(End Section)				



Date 11-13-87 Page 4 of 9

Project Cross Creek Addition

Item Drainage Plan System 200

IV STREET FLOW - 2 YR

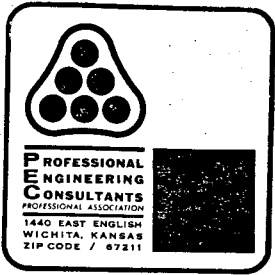
<u>Node</u>	<u>Q<sub>2</sub></u>	<u>Distribution</u>	<u>street slope</u>	<u>d</u>	<u>d<sub>max</sub></u>	<u>Comment</u>
202	2.9	30%(SW) = 0.9 70%(NE) = 2.0	0.32% 0.5%	0.20' 0.32'	0.55' 0.55'	OK OK
201	4.8	30%(SW) = 1.4 70%(NE) = 3.4	0.32% 0.5%	0.24' 0.31'	0.55' 0.55'	OK OK

I STREET FLOW - 100-YR

$(Q_{street} = Q_{100} - Q_{pipe})$

<u>Location</u>	<u>Contributing Areas</u>	<u>Q<sub>100</sub></u>	<u>Q<sub>pipe</sub></u>	<u>Q<sub>street</sub></u>	<u>street slope</u>	<u>Q<sub>max</sub>*</u>	<u>Comment</u>
Approaching	70% 202	6.2					
Nodes 201, 202	70% 201	10.2					
(from east)	overflow from system 100	59.2					
	(Q <sub>100</sub> - Q <sub>2</sub> ) = 86.3 - 27.1	<u>75.6</u>	0.0	75.6	0.5%	83.9	OK (Walk Grade = TC + 0.4')

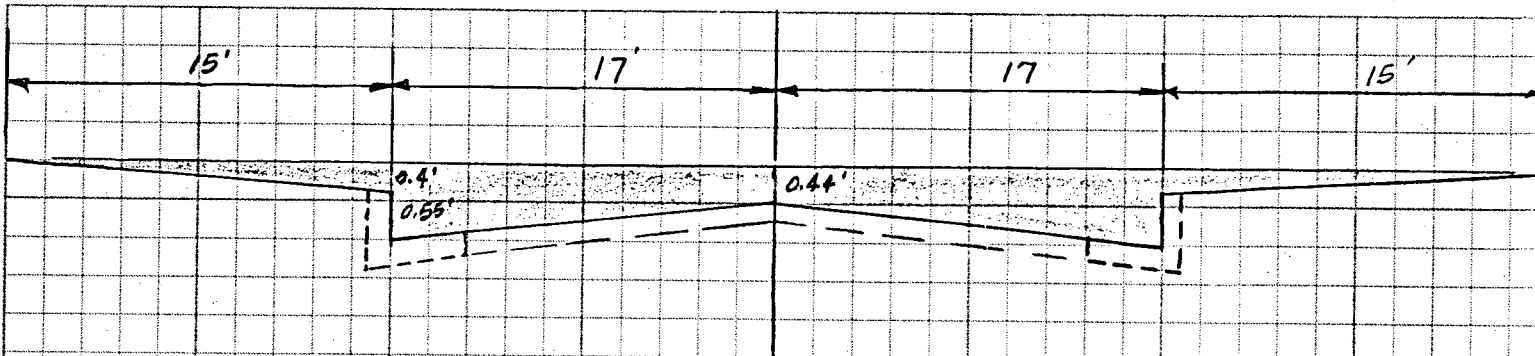
\* From Page 5



Date 11.13.87 Page 5 of 9

Project Cross Creek Addition

Item Drainage Plan System 200



Determine  $Q_{max}$  in street right-of-way.

Use Mannings Equation  $Q = \frac{1.486}{n} AR^{2/3} s^{1/2}$

$$n = \frac{(2 \times 14.5 \times 0.03) + (2 \times 3.05 \times 0.013) + (2 \times 15 \times 0.016)}{65.1} = \frac{1.4293}{65.1}$$

$$n = 0.021955$$

$$A = (2 \times \frac{1}{2} \times 15 \times 0.4) + (34 \times 0.44) + (2 \times \frac{1}{2} \times 17 \times 0.51)$$

$$A = 29.63$$

$$p = 65.1$$

$$R = A/p = 29.63/65.1 = 0.455146$$

$$R^{2/3} = (0.455146)^{2/3} = 0.5917$$

$$Q = \frac{1.486}{0.021955} \times 29.63 \times 0.5917 \times s^{1/2}$$

$$Q = 1,186.6 s^{1/2}$$



Date 11.13.87 Page 6 of 9

Project Cross Creek Addition

Item Drainage Plan System 200

VI OVERFLOW CHANNEL

Design overflow channel at storm sewer location

$$\begin{aligned} \Sigma Q_{100} &= 100\% \text{ 202} = 8.9 \text{ cfs} \\ &100\% \text{ 201} = 14.5 \\ &\text{overflow from?} \\ &\text{System 100} \} \\ &\hline &82.5 \end{aligned}$$

$$Q_{\text{pipe}} = Q_2 = \underline{7.7}$$

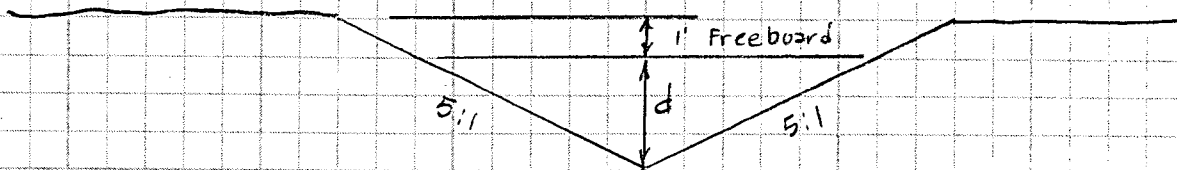
$$Q_{\text{overflow}} = 74.8 \text{ cfs}$$

Use Manning's Equation

$$Q = \frac{1.486}{n} AR^{2/3} S^{1/2}$$

$$AR^{2/3} = \frac{Q \times n}{1.486 \times S^{1/2}} = \frac{74.8 \times 0.03}{1.486 \times (0.01)^{1/2}}$$

$$AR^{2/3} = 15.10$$



<u>d</u>	<u>A</u>	<u>p</u>	<u>R</u>	<u>R<sup>2/3</sup></u>	<u>AR<sup>2/3</sup></u>
2.0	20.0	20.396	0.981	0.987	19.7
1.9	18.05	19.376	0.932	0.9538	17.2
1.8	16.20	18.356	0.883	0.920	14.9

← USE d = 1.8'

$$V = \frac{Q}{A} = \frac{74.8}{16.2} = 4.6$$

W = (1.8 + 1.0) x 5 x 2 = 28'  
USE 30' EASEMENT

100 i, 165.0000 200 0 3 2

110 t, cross creek addition

120 t, drainage plan

130 t, storm water sewer system 200 analysis

140 i, 202 0.46 1.65 0.00 0.00 2.96 15.00 169.50

150 i, 201 0.46 2.70 0.00 0.00 4.80 15.00 169.50

160 m, 200 165.00

170 c, 202 201 40.00 15 0.013 20.00 0.00

180 p, 201 200 240.00 18 0.013 90.00 0.00

190 e

Date: 11-16-1987  
Time: 09:26:01

Input File: ccreek200

cross creek addition  
drainage plan  
storm water sewer system 200 analysis

Storm Frequency = 2-Year

\*\*\* HYDROLOGY \*\*\*

Tributary Area										Hydrology Summation				Conduit Data			
Node to	C	Area	Slope	Length	TC(Q)	I(Q)	Q(Q)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC	
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)	(In)	(Ft/Sec)	(Ft)	(Min)	(Min)	
202	201	0.46	1.65	0.00	0.0	15.00	4.06	2.90	15.00	4.06	2.90	2.90	15"	2.36	40.00	0.28	15.28
201	200	0.46	2.70	0.00	0.0	15.00	4.06	4.80	15.28	4.06	4.77	7.87	18"	4.54	240.00	0.92	16.20

Date: 11-14-1997  
Time: 09:25:01

Input File: ccreek200

cross creek addition  
drainage plan  
storm water sewer system 200 analysis

Storm Frequency = 2-Year

\* \* \* HYDRAULICS \* \* \*

```

*****
Node      Hyd-Slope  Friction   Bend      Transition  Manhole  Deflection  Junction  Total  Hyd-01  Desired  Diff.
          (Ft/Ft)   (Ft)      (Ft)      (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)
*****
202      0.00292   0.0505    0.0000    0.0000     0.0000   0.0000    0.0000   0.0000  166.753E  169.5000   2.55
201      0.00532   1.2700    0.0000    0.0205     0.0000   0.0071    0.5669   1.9726  166.676E  169.5000   2.63
200      0.00500   0.0000    0.0000    0.0000     0.0000   0.0000    0.0000   0.0000  165.0000  165.0000   0.00
*****

```

## 1.0 INTRODUCTION

This study covers a small portion of the floodplain of the East Branch Gypsum Creek lying in the east one-half of Section 17, T27S, R2E of the Sixth Principal Meridian, Sedgwick County, Kansas. The study has been performed to establish the minimum floodway width and location for the platting of Cross Creek, an addition to Wichita, Sedgwick County, Kansas.

This property was previously studied as presented in the report prepared by Mid-Kansas Engineering Consultants (M.K.E.C.) for the Lakepoint Development. The current study was necessary (1) to extend the study further upstream and (2) to reflect the intent to maintain the stream valley in a more natural condition in its final developed state.

## 2.0 AREA STUDIED

The area studied lies within the corporate limits of the City of Wichita, Kansas, and is to be platted as Cross Creek. The tract is bounded by residential subdivisions on the south, and west, and by undeveloped land to the north, and by Beech Aircraft property on the east.

The flooding source studied in this report is the East Branch Gypsum Creek from its north crossing of Gatewood Street north to the west line of Webb Road.

## 3.0 ENGINEERING METHODS

### 3.1 Hydrology

The estimate of the 100-year flood discharge for the East Branch Gypsum Creek was established as 2000 cfs. This figure was taken from a previous report prepared by Mid-Kansas Engineering Consultants for the Lakepoint Development. The contributing area for the basin at a point 800 feet north of Central Avenue is 2.18 square miles.

### 3.2 Hydraulic Analyses

#### A. Cross-section information.

Data used in this study was taken from a 1"=100' topographic map using a two foot contour interval. This map was produced from aerial photographs taken June 28, 1985, and was prepared by Miles Air Photo and Survey, Manhattan, Kansas, with ground control by Professional Engineering Consultants, Wichita. Based on measurements made in the field, the stream channel proper was modeled as a twenty-foot flat bottom channel, two feet deep having nearly vertical banks.

Construction plans for the 5-10'x4' reinforced concrete beneath Gatewood Street also provided information.

#### B. Roughness Coefficients

The following values for Manning's roughness coefficient "n" were used:

0.035 - Lake sections (channel)

0.040 - Landscaped area adjacent to lake or channel. Brush and small trees are to be cleared. Scattered large trees will remain. All areas are to be mowed, as they will be in backyards.

0.070 - Natural channel section, which presently contains large trees within the channel.

- C. Computation Method  
Water surface profiles were computed using the computer program "HEC-2, Water Surface Profiles," as developed by the Corps of Engineers, U.S. Army.  
The starting water-surface elevation for each profile was established analyzing the flow through the 5-10'x4' RCB culvert using the FHWA's Hydraulic Design Series #5, "Hydraulic Design of the Highway Culverts."  
All elevations in this report are based on City of Wichita Datum.  
The results of this study are valid only in the case of unobstructed flows, with floodplain conditions similar to those assumed in the report.

#### 4.0 HYDRAULIC RESULTS

##### 4.1 Floodway Limits

After several trials, the final computation models four profiles. The first is for the natural condition, the second and third are for encroached conditions using the equal conveyance reduction principle with targets of zero and one foot of rise, respectively, and the fourth being for encroached conditions with floodway limits fixed.

For the fourth profile, the floodway limits established by the third profile were used with the exception of Section 43.5. A slight adjustment to the south was required at this location to provide more buildable lot area on the north side.

The platted floodway limits will be as wide or wider as those computed by HEC-2.

##### 4.2 Floodplain Limits

Floodplain limits were not delineated, as they are not a platting requirement. Minimum pad elevations for each lot abutting the East Branch Gypsum Creek were established as two feet above the natural stream profile elevations for the 100-year storm, which provides one foot of freeboard above the encroached condition.

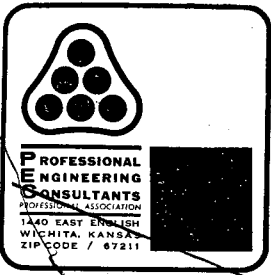
##### 4.3 Other Notes

The natural channel upstream from Station 57.1 is severely restricted. Man-made channel improvements will be required in this reach. This improved channel was modeled as a twenty foot flat bottom trapezoidal channel having 4:1 side slopes and an improved Manning's n of 0.040. The right-of-way requirements are estimated to be 140 feet wide as shown in Figure A.

It should be noted that under existing conditions, flows on the order of 2000 cfs would overtop Webb Road due to inadequate culvert capacity.

5.0      SUPPORTING INFORMATION

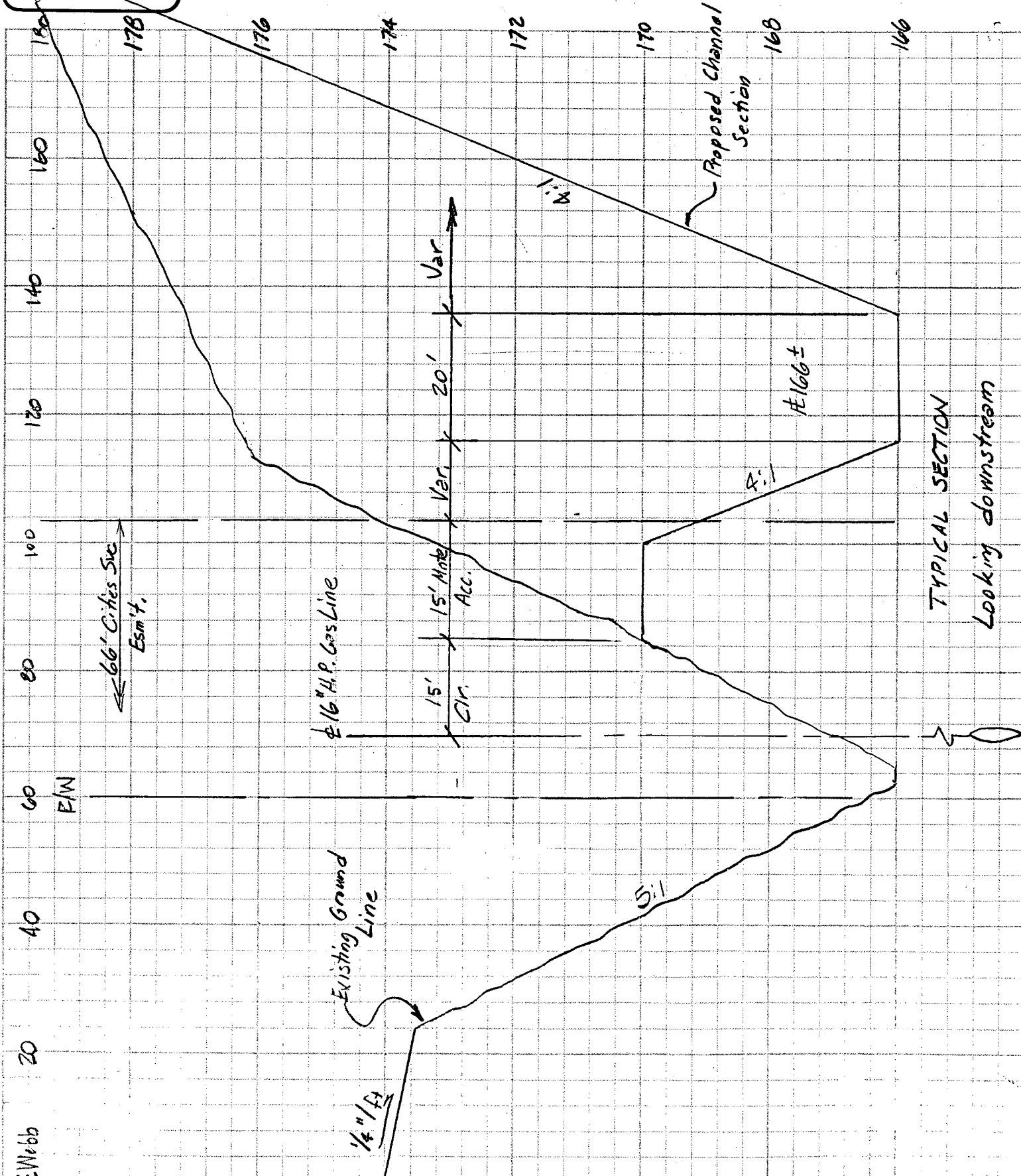
A printout of Computer results follows. A map showing the computed floodway limits is given as Exhibit 1.



Date 11-17-87 Page      of     

Project Cross Creek

Item Design Section - E. Br. Gypsum Creek



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*****  
*           E D I T  2           *  
*   FOR HEC2 DATA PREPARATION   *  
*   IBM VERSION (DECEMBER 1983)  *  
*****
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CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4										
T2 NATURAL CONDITIONS; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2										
T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87										
J1	-10	2	0	0	0	0	0	0	167.20	
J2	1		-1					0		
J3	110	150	200							
NC	0.04	0.04	0.035							
QT	4	2000	2000	2000	2000					
ET			0.4	10.4	9.1				915	1075
X1	37.55	10	915	1075	10	10	10			
GR170.00		735	168.00	785	166.00	860	164.00	900	162.10	915
GR158.10		931	158.10	1059	162.10	1075	164.00	1125	170.00	1150
ET			0.4	10.4	9.1				835	1100
X1	39.6	13	835	1100	200	200	200			
GR170.00		750	168.00	805	166.00	820	164.00	835	162.10	855
GR158.10		871	158.10	936	162.10	950	164.00	1100	166.00	1250
GR166.00		1250	166.00	1304	170.00	1320				
NC			0.070							
ET			0.4	10.4	9.1				990	1175
X1	40.8	9	989	1011	215	195	210			
X4	3	162.00	990	162.00	1010	164.00	1011			
GR170.00		955	168.00	970	166.00	980	164.00	989	164.00	1115
GR164.00		1235	166.00	1300	166.00	1304	170.00	1320		
ET			9.1	10.4	9.1				900	1120
X1	43.5	12	989	1011	170	170	170			
GR170.00		855	168.00	870	166.00	885	164.00	900	164.00	989
GR162.00		990	162.00	1010	164.00	1011	164.00	1180	166.00	1235
GR166.00		1274	170.00	1290						
ET			0.4	10.4	9.1				835	1025
X1	46.0	11	989	1011	250	250	250			
GR170.00		740	168.00	760	166.00	775	164.00	800	164.00	989
GR162.00		990	162.00	1010	164.00	1011	166.00	1035	166.00	1174
GR170.00		1190								
ET			0.4	10.4	9.1				990	1115
X1	46.75	12	989	1011	70	70	70			
GR170.00		935	168.00	950	166.00	965	164.00	980	164.00	989
GR162.00		990	162.00	1010	164.00	1011	164.00	1030	166.00	1120
GR166.00		1264	170.00	1280						
ET			0.4	10.4	9.1				970	1070
X1	48.8	9	970	990	210	200	205			
X4	2	162.00	971	162.00	989					
GR170.00		900	168.00	915	166.00	955	164.00	970	164.00	990
GR166.00		1100	168.00	1240	170.00	1275	172.00	1360		
ET			0.4	10.4	9.1				985	1070
X1	51.1	11	989	1011	235	225	230			
GR170.00		900	168.00	935	166.00	970	166.00	989	164.00	990
GR164.00		1010	166.00	1011	166.00	1070	168.00	1130	170.00	1160
GR172.00		1250								
ET			0.4	10.4	9.1				990	1095
X1	52.5	11	989	1011	140	140	140			
GR170.00		880	168.00	960	166.00	985	166.00	989	164.00	990
GR164.00		1010	166.00	1011	166.00	1100	168.00	1150	170.00	1165
GR172.00		1180								
ET			0.4	10.4	9.1				975	1035
X1	54.75	8	989	1011	210	210	210			
X4	3	164.00	990	164.00	1010	166.00	989			

GR170.00	940	168.00	950	166.00	1011	166.00	1025	168.00	1055
GR170.00	1065	172.00	1075	174.00	1100				
ET		0.4	10.4	9.1				970	1105
X1 57.1	6	970	1000	235	235	235			
GR170.00	970	168.00	980	166.00	1000	168.00	1185	170.00	1200
GR176.00	1225								
NC 0.040	0.040	0.040							
ET		0.4	10.4	9.1				965	1035
X1 58.35	7	965	1035	125	125	125			
X4 3	168.0	1040.0	168.0	1080.0	174.0	1100.0			
GR172.00	955	170.00	965	168.00	975	168.00	1035	170.00	1050
GR176.00	1140	180.0	1225						
ET		0.4	10.4	9.1				975	1055
X1 60.0	9	975	1050	155	155	155			
GR 174.0	935	172.00	970	170.00	975	168.00	985	168.00	1005
GR170.00	1050	168.00	1060	168.00	1080	177.00	1115		
EJ									

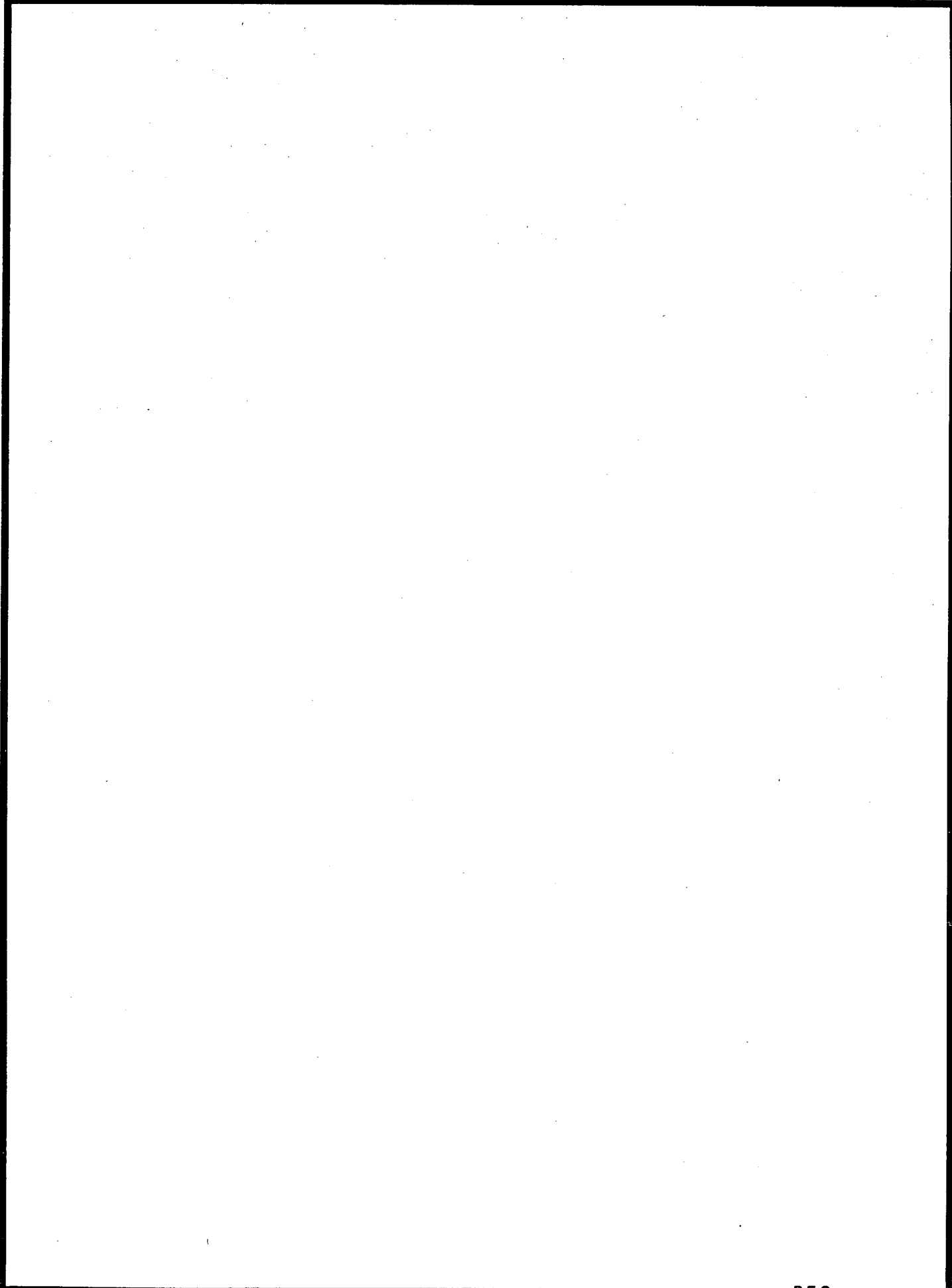
T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 F/W ENCR.-0'; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87  
 J1 -10 3 0 0 0 0 0 0 167.20  
 J2 2 -1 0

T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 F/W ENCR.-1'; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87  
 J1 -10 4 0 0 0 0 0 0 167.20  
 J2 3 -1 0

T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 SET F/W ENCR. STA'S.; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87  
 J1 -10 5 0 0 0 0 0 0 167.20  
 J2 15 -1 0

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SUMMARY OF ERRORS

- T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4
- T2 NATURAL CONDITIONS; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2
- T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87
  
- T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4
- T2 F/W ENCR.-0'; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2
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- T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4
- T2 SET F/W ENCR. STA'S.; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2
- T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87

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*****
* WATER SURFACE PROFILES *
* VERSION OF NOVEMBER 1976 *
* UPDATED MAY 1984 *
* IBM-PC-YT VERSION AUGUST 1985 *
* RUN DATE 11-17-87 TIME 20:20:02 *
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* THE HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET, SUITE D *
* DAVIS, CALIFORNIA 95616 *
* (916) 440-2105 (FTS) 448-2105 *
*****

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11-17-87 20:20:03

PAGE 1

THIS RUN EXECUTED 11-17-87

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 HEC2 RELEASE DATED NOV 76 UPDATED MAY 1984  
 ERROR CORR - 01,02,03,04,05,06  
 MODIFICATION - 50,51,52,53,54,55,56  
 IBM-PC-XT VERSION AUGUST 1985  
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T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 NATURAL CONDITIONS; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87

J1	ICHECK	INQ	NINV	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	-10.	2.	0.	0.	.000000	.00	.0	0.	167.200	.000
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	1.000	.000	-1.000	.000	.000	.000	.000	.000	.000	.000
J3	VARIABLE CODES FOR SUMMARY PRINTOUT									
	110.000	150.000	200.000	.000	.000	.000	.000	.000	.000	.000

11-17-87 20:20:03

PAGE 2

SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST
*PROF 1									
*SECNO 37.550									
37.55	9.10	167.20	.00	167.20	167.22	.02	.00	.00	162.10
2000.	86.	1765.	149.	177.	1392.	229.	0.	0.	162.10
.00	.49	1.27	.65	.040	.035	.040	.000	158.10	815.00
.000050	10.	10.	10.	0	0	0	.00	323.33	1138.33
*SECNO 39.600									
39.60	9.12	167.22	.00	.00	167.23	.02	.01	.00	164.00
2000.	19.	1775.	206.	39.	1513.	400.	9.	2.	164.00
.05	.50	1.17	.51	.040	.035	.040	.000	158.10	810.92
.000075	200.	200.	200.	0	0	0	.00	497.92	1308.84
*SECNO 40.800									
40.80	5.21	167.21	.00	.00	167.27	.06	.03	.00	164.00
2000.	35.	176.	1789.	24.	113.	870.	16.	4.	164.00
.08	1.50	1.56	2.06	.040	.070	.040	.000	162.00	973.97
.000706	215.	210.	195.	2	0	0	.00	334.86	1308.83
*SECNO 43.500									
43.50	5.32	167.32	.00	.00	167.37	.04	.10	.00	164.00
2000.	592.	151.	1258.	337.	115.	745.	20.	5.	164.00
.11	1.75	1.31	1.69	.040	.070	.040	.000	162.00	875.07
.000483	170.	170.	170.	2	0	0	.00	404.22	1279.30
*SECNO 46.000									
46.00	5.45	167.45	.00	.00	167.50	.06	.14	.00	164.00
2000.	1483.	178.	340.	721.	118.	264.	26.	8.	164.00
.14	2.06	1.51	1.29	.040	.070	.040	.000	162.00	764.14
.000619	250.	250.	250.	2	0	0	.00	415.65	1179.79
*SECNO 46.750									
46.75	5.45	167.45	.00	.00	167.58	.13	.07	.00	164.00
2000.	248.	333.	1419.	76.	118.	498.	28.	8.	164.00
.15	3.29	2.82	2.85	.040	.070	.040	.000	162.00	954.15
.002174	70.	70.	70.	2	0	0	.00	315.64	1269.79

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PAGE 3

SECNO	DEPTH	CWSEL	CRWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST
*SECNO 48.800									
48.80	5.89	167.89	.00	.00	168.04	.15	.47	.00	164.00
2000.	225.	364.	1411.	79.	116.	443.	31.	10.	164.00
.17	2.84	3.14	3.18	.040	.070	.040	.000	162.00	917.17
.002459	210.	205.	200.	2	0	0	.00	315.25	1232.42
*SECNO 51.100									
51.10	4.55	168.55	.00	.00	168.86	.31	.82	.00	166.00
2000.	461.	397.	1142.	105.	98.	246.	34.	11.	166.00
.18	4.37	4.05	4.64	.040	.070	.040	.000	164.00	925.36
.005710	235.	230.	225.	2	0	0	.00	212.91	1138.27
*SECNO 52.500									
52.50	5.17	169.17	.00	.00	169.35	.18	.49	.00	166.00
2000.	248.	317.	1435.	94.	112.	396.	35.	12.	166.00
.19	2.63	2.84	3.62	.040	.070	.040	.000	164.00	913.16
.002361	140.	140.	140.	2	0	0	.00	245.62	1158.78
*SECNO 54.750									
54.75	5.64	169.64	.00	.00	170.11	.46	.76	.00	166.00
2000.	593.	595.	812.	110.	122.	137.	38.	13.	166.00
.20	5.39	4.87	5.92	.040	.070	.040	.000	164.00	941.76
.006158	210.	210.	210.	2	0	0	.00	121.47	1063.24
*SECNO 57.100									
3280 CROSS SECTION		57.10 EXTENDED		.50 FEET					
57.10	4.50	170.50	.00	.00	170.62	.12	.51	.00	170.00
2000.	0.	118.	1882.	0.	85.	671.	41.	14.	166.00
.23	.00	1.39	2.80	.040	.070	.040	.000	166.00	970.00
.001105	235.	235.	235.	2	0	0	.00	232.09	1202.09
*SECNO 58.350									
3301 HV CHANGED MORE THAN HVINS									
3685 20 TRIALS ATTEMPTED WSEL,CWSEL									
3693 PROBABLE MINIMUM SPECIFIC ENERGY									

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PAGE 4

SECNO	DEPTH	CWSEL	CRISW	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

3720 CRITICAL DEPTH ASSUMED

58.35	2.54	170.54	170.54	.00	171.55	1.00	.35	.00	170.00
2000.	1.	1435.	564.	1.	168.	85.	42.	14.	168.00
.23	1.98	8.53	6.61	.040	.040	.040	.000	168.00	962.28
.016471	125.	125.	125.	20	11	0	.00	126.20	1088.48

\*SECNO 60.000

3301 HV CHANGED MORE THAN HVINS

60.00	4.17	172.17	.00	.00	172.54	.37	.99	.00	170.00
2000.	13.	1260.	726.	6.	258.	149.	43.	15.	170.00
.24	2.20	4.89	4.88	.040	.040	.040	.000	168.00	967.06
.003360	155.	155.	155.	3	0	0	.00	129.15	1096.21

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PAGE 5

THIS RUN EXECUTED 11-17-87

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 HEC2 RELEASE DATED NOV 76 UPDATED MAY 1984  
 ERROR CORR - 01,02,03,04,05,06  
 MODIFICATION - 50,51,52,53,54,55,56  
 IBM-PC-XT VERSION AUGUST 1985  
 \*\*\*\*\*

T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 F/W ENCR.-0'; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87

J1	ICHECK	INQ	NINW	IDIR	STRT	METRIC	EVINS	Q	WSEL	FQ
	-10.	3.	0.	0.	.000000	.00	.0	0.	167.200	.000
J2	NPROF	IPLOT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	2.000	.000	-1.000	.000	.000	.000	.000	.000	.000	.000

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PAGE 6

SECNO	DEPTH	CWSEL	CRISW	WSELK	EG	HV	HL	CLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*PROF 2

\*SECNO 37.550

37.55	9.10	167.20	.00	167.20	167.22	.02	.00	.00	162.10
2000.	86.	1765.	149.	177.	1392.	229.	0.	0.	162.10
.00	.49	1.27	.65	.040	.035	.040	.000	158.10	815.00
.000050	10.	10.	10.	0	0	0	.00	323.33	1138.33

\*SECNO 39.600

2800 NAT Q1= 2304.96 WSEL= 167.22 ENC Q1= 2304.96 WSEL= 167.22 RATIO= .0000  
 NAT Q1= 2309. RATIOS LOB,CH,ROB= .0097 .8871 .1032 WSEL= 167.22

3470 ENCROACHMENT STATIONS= 819.5 1302.1 TYPE= 4 TARGET= .002

39.60	9.12	167.22	.00	167.22	167.23	.02	.01	.00	164.00
2000.	18.	1777.	205.	34.	1513.	395.	9.	2.	164.00
.05	.54	1.17	.52	.040	.035	.040	.000	158.10	819.49
.000075	200.	200.	200.	0	0	0	.00	482.58	1302.07

\*SECNO 40.800

40.80	5.21	167.21	.00	167.21	167.27	.06	.03	.00	164.00
2000.	35.	176.	1789.	24.	113.	869.	15.	4.	164.00
.08	1.50	1.56	2.06	.040	.070	.040	.000	162.00	973.97
.000706	215.	210.	195.	0	0	0	.00	334.86	1308.83

\*SECNO 43.500

3470 ENCROACHMENT STATIONS= 900.0 1120.0 TYPE= 1 TARGET= 220.000

43.50	5.31	167.31	.00	167.32	167.42	.11	.15	.00	164.00
2000.	794.	229.	977.	295.	115.	361.	19.	5.	164.00
.10	2.70	1.99	2.71	.040	.070	.040	.000	162.00	900.00
.001120	170.	170.	170.	2	0	0	.00	220.00	1120.00

\*SECNO 46.000

46.00	5.56	167.56	.00	167.45	167.61	.05	.19	.00	164.00
2000.	1472.	173.	356.	745.	120.	282.	24.	7.	164.00
.14	1.98	1.44	1.26	.040	.070	.040	.000	162.00	763.33
.000548	250.	250.	250.	2	0	0	.00	416.89	1180.22

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PAGE 7

SECNO	DEPTH	CWSEL	CRIBS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST
*SECNO 46.750									
46.75	5.55	167.55	.00	167.45	167.67	.12	.06	.00	164.00
2000.	247.	319.	1434.	79.	120.	526.	26.	7.	164.00
.14	3.11	2.66	2.73	.040	.070	.040	.000	162.00	953.34
.001876	70.	70.	70.	2	0	0	.00	316.88	1270.22
*SECNO 48.800									
48.80	5.95	167.95	.00	167.89	168.09	.14	.42	.00	164.00
2000.	227.	357.	1416.	82.	117.	457.	29.	9.	164.00
.16	2.76	3.05	3.10	.040	.070	.040	.000	162.00	916.04
.002288	210.	205.	200.	2	0	0	.00	320.31	1236.35
*SECNO 51.100									
51.10	4.56	168.56	.00	168.55	168.87	.31	.78	.00	166.00
2000.	462.	396.	1142.	106.	98.	247.	32.	10.	166.00
.17	4.36	4.03	4.62	.040	.070	.040	.000	164.00	925.19
.005634	235.	230.	225.	2	0	0	.00	213.21	1138.41
*SECNO 52.500									
2800 NAT Q1=	411.56	WSEL=	169.17	ENC Q1=	411.56	WSEL=	169.17	RATIO=	.0000
NAT Q1=	412.	RATIOS LOB,CH,ROB=	.1240	.1587	.7173	WSEL=	169.17		
3470 ENCROACHMENT STATIONS= 935.0 1155.3 TYPE= 4 TARGET= .000									
52.50	5.17	169.17	.00	169.17	169.35	.18	.49	.00	166.00
2000.	248.	317.	1435.	89.	112.	395.	34.	11.	166.00
.19	2.80	2.84	3.63	.040	.070	.040	.000	164.00	934.97
.002357	140.	140.	140.	2	0	0	.00	220.38	1155.35
*SECNO 54.750									
54.75	5.65	169.65	.00	169.64	170.11	.47	.76	.00	166.00
2000.	592.	596.	812.	110.	122.	137.	36.	12.	166.00
.20	5.39	4.88	5.93	.040	.070	.040	.000	164.00	941.78
.006184	210.	210.	210.	0	0	0	.00	121.43	1063.22
*SECNO 57.100									
2800 NAT Q1=	601.61	WSEL=	170.50	ENC Q1=	601.61	WSEL=	170.50	RATIO=	.0000
NAT Q1=	602.	RATIOS LOB,CH,ROB=	.0000	.0591	.9409	WSEL=	170.50		

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SECNO	DEPTH	CWSEL	CRWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

3280 CROSS SECTION 57.10 EXTENDED .50 FEET

3470 ENCROACHMENT STATIONS=	970.0	1200.0	TYPE=	4	TARGET=	.000
57.10	4.51	170.51	.00	170.50	170.63	.12 .51 .00 170.00
2000.	0.	116.	1884.	0.	85.	671. 39. 13. 166.00
.22	.00	1.36	2.81	.040	.070	.040 .000 166.00 970.00
.001109	235.	235.	235.	0	0	0 .00 230.00 1200.00

\*SECNO 58.350

3301 HV CHANGED MORE THAN HVINS

7185 MINIMUM SPECIFIC ENERGY

3720 CRITICAL DEPTH ASSUMED

58.35	2.54	170.54	170.54	170.54	171.55	1.00	.35	.00	170.00
2000.	1.	1435.	564.	1.	168.	85.	41.	13.	168.00
.23	1.98	8.54	6.61	.040	.040	.040	.000	168.00	962.28
.016478	125.	125.	125.	19	11	0	.00	126.20	1088.48

\*SECNO 60.000

3301 HV CHANGED MORE THAN HVINS

60.00	4.17	172.17	.00	172.17	172.54	.37	.99	.00	170.00
2000.	13.	1260.	726.	6.	258.	149.	42.	14.	170.00
.23	2.20	4.89	4.88	.040	.040	.040	.000	168.00	967.08
.003364	155.	155.	155.	0	0	0	.00	129.12	1096.20

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THIS RUN EXECUTED 11-17-87

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 HEC2 RELEASE DATED NOV 76 UPDATED MAY 1984  
 ERROR CORR - 01,02,03,04,05,06  
 MODIFICATION - 50,51,52,53,54,55,56  
 IBM-PC-XT VERSION AUGUST 1985  
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T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 F/W ENCR.-1'; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87

J1	ICHECK	INQ	NINW	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	-10.	4.	0.	0.	.000000	.00	.0	0.	167.200	.000
J2	NPROF	IPL0T	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	3.000	.000	-1.000	.000	.000	.000	.000	.000	.000	.000

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*PROF 3

\*SECNO 37.550

2800 NAT Q1= 2821.37 WSEL= 167.20 ENC Q1= 2985.01 WSEL= 168.20 RATIO= -.0580  
 NAT Q1= 3518. RATIOS LOB,CH,ROB= .0643 .8484 .0873 WSEL= 168.20

3470 ENCROACHMENT STATIONS= 915.0 1075.0 TYPE= 4 TARGET= .152  
 37.55 9.10 167.20 .00 167.20 167.23 .03 .00 .00 162.10  
 2000. 0. 2000. 0. 0. 1392. 0. 0. 0. 162.10  
 .00 .00 1.44 .00 .040 .035 .040 .000 158.10 915.00  
 .000070 10. 10. 10. 0 0 0 .00 160.00 1075.00

\*SECNO 39.600

2800 NAT Q1= 2304.96 WSEL= 167.22 ENC Q1= 2680.51 WSEL= 168.22 RATIO= -.1629  
 NAT Q1= 3197. RATIOS LOB,CH,ROB= .0146 .8385 .1469 WSEL= 168.22

3470 ENCROACHMENT STATIONS= 835.0 1100.0 TYPE= 4 TARGET= .161  
 39.60 9.12 167.22 .00 167.22 167.25 .03 .02 .00 164.00  
 2000. 0. 2000. 0. 0. 1514. 0. 7. 1. 164.00  
 .04 .00 1.32 .00 .040 .035 .040 .000 158.10 835.00  
 .000098 200. 200. 200. 1 0 0 .00 265.00 1100.00

\*SECNO 40.800

2800 NAT Q1= 752.76 WSEL= 167.21 ENC Q1= 752.76 WSEL= 168.21 RATIO= .0000  
 NAT Q1= 1207. RATIOS LOB,CH,ROB= .0232 .0737 .9031 WSEL= 168.21

3470 ENCROACHMENT STATIONS= 989.0 1176.7 TYPE= 4 TARGET= .376  
 40.80 5.14 167.14 .00 167.21 167.30 .16 .05 .00 164.00  
 2000. 0. 254. 1746. 0. 111. 520. 12. 2. 164.00  
 .06 .00 2.29 3.36 .040 .070 .040 .000 162.00 989.00  
 .001821 215. 210. 195. 2 0 0 .00 187.67 1176.67

\*SECNO 43.500

2800 NAT Q1= 910.01 WSEL= 167.32 ENC Q1= 910.01 WSEL= 168.32 RATIO= .0000  
 NAT Q1= 1456. RATIOS LOB,CH,ROB= .2948 .0631 .6421 WSEL= 168.32

3470 ENCROACHMENT STATIONS= 949.9 1169.2 TYPE= 4 TARGET= .375  
 43.50 5.43 167.43 .00 167.32 167.53 .10 .23 .00 164.00  
 2000. 340. 225. 1435. 134. 117. 542. 15. 3. 164.00  
 .08 2.54 1.92 2.65 .040 .070 .040 .000 162.00 949.89  
 .001011 170. 170. 170. 2 0 0 .00 219.35 1169.24

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SECNO	DEPTH	CSSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*SECNO 46.000  
 2800 NAT Q1= 803.88 WSEL= 167.45 ENC Q1= 803.88 WSEL= 168.45 RATIO= .0000  
 NAT Q1= 1327. RATIOS LOB,CH,ROB= .6991 .0715 .2294 WSEL= 168.45

3470 ENCROACHMENT STATIONS= 836.7 1025.1 TYPE= 4 TARGET= .394  
 46.00 5.67 167.67 .00 167.45 167.79 .12 .27 .00 164.00  
 2000. 1639. 257. 103. 559. 123. 44. 19. 4. 164.00  
 .10 2.93 2.10 2.37 .040 .070 .040 .000 162.00 836.71  
 .001134 250. 250. 250. 2 0 0 .00 188.42 1025.13

\*SECNO 46.750  
 2800 NAT Q1= 428.94 WSEL= 167.45 ENC Q1= 428.94 WSEL= 168.45 RATIO= .0000  
 NAT Q1= 780. RATIOS LOB,CH,ROB= .1208 .1217 .7575 WSEL= 168.45

3470 ENCROACHMENT STATIONS= 989.0 1114.7 TYPE= 4 TARGET= .450  
 46.75 5.56 167.56 .00 167.45 167.94 .39 .15 .00 164.00  
 2000. 0. 481. 1519. 0. 120. 289. 20. 4. 164.00  
 .11 .00 4.00 5.25 .040 .070 .040 .000 162.00 989.00  
 .005095 70. 70. 70. 2 0 0 .00 125.67 1114.67

\*SECNO 48.800  
 2800 NAT Q1= 403.36 WSEL= 167.89 ENC Q1= 403.36 WSEL= 168.89 RATIO= .0000  
 NAT Q1= 738. RATIOS LOB,CH,ROB= .1313 .1297 .7390 WSEL= 168.89

3470 ENCROACHMENT STATIONS= 970.0 1069.1 TYPE= 4 TARGET= .453  
 48.80 6.43 168.43 .00 167.89 168.81 .38 .86 .00 164.00  
 2000. 0. 457. 1543. 0. 127. 293. 22. 5. 164.00  
 .12 .00 3.61 5.26 .040 .070 .040 .000 162.00 970.00  
 .003671 210. 205. 200. 2 0 0 .00 99.14 1069.14

\*SECNO 51.100  
 2800 NAT Q1= 264.68 WSEL= 168.55 ENC Q1= 264.68 WSEL= 169.55 RATIO= .0000  
 NAT Q1= 493. RATIOS LOB,CH,ROB= .2570 .1493 .5937 WSEL= 169.55

3470 ENCROACHMENT STATIONS= 983.7 1071.4 TYPE= 4 TARGET= .463  
 51.10 5.31 169.31 .00 168.55 169.91 .60 1.11 .00 166.00  
 2000. 86. 565. 1349. 17. 115. 200. 24. 5. 166.00  
 .13 4.93 4.92 6.74 .040 .070 .040 .000 164.00 983.74  
 .006823 235. 230. 225. 2 0 0 .00 87.64 1071.38

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*SECNO 52.500  
 2800 NAT Q1= 411.56 WSEL= 169.17 ENC Q1= 411.56 WSEL= 170.17 RATIO= .0000  
 NAT Q1= 698. RATIOS LOB,CH,ROB= .1692 .1263 .7045 WSEL= 170.17  
 3280 CROSS SECTION 52.50 EXTENDED .16 FEET

3470 ENCROACHMENT STATIONS= 989.0 1094.4 TYPE= 4 TARGET= .410  
 52.50 6.16 170.16 .00 169.17 170.45 .29 .54 .00 166.00  
 2000. 0. 394. 1606. 0. 133. 347. 25. 6. 166.00  
 .14 .00 2.95 4.63 .040 .070 .040 .000 164.00 989.00  
 .002483 140. 140. 140. 2 0 0 .00 105.36 1094.36

\*SECNO 54.750  
 2800 NAT Q1= 254.86 WSEL= 169.64 ENC Q1= 254.86 WSEL= 170.64 RATIO= .0000  
 NAT Q1= 405. RATIOS LOB,CH,ROB= .3286 .2467 .4247 WSEL= 170.64  
 3280 CROSS SECTION 54.75 EXTENDED .57 FEET

3470 ENCROACHMENT STATIONS= 973.0 1033.2 TYPE= 4 TARGET= .370  
 54.75 6.57 170.57 .00 169.64 171.26 .69 .80 .00 166.00  
 2000. 453. 798. 749. 67. 143. 99. 27. 6. 166.00  
 .15 6.80 5.60 7.55 .040 .070 .040 .000 164.00 972.97  
 .006635 210. 210. 210. 2 0 0 .00 60.26 1033.23

\*SECNO 57.100  
 2800 NAT Q1= 601.61 WSEL= 170.50 ENC Q1= 601.61 WSEL= 171.50 RATIO= .0000  
 NAT Q1= 928. RATIOS LOB,CH,ROB= .0000 .0620 .9380 WSEL= 171.50  
 3280 CROSS SECTION 57.10 EXTENDED 1.59 FEET

3301 HV CHANGED MORE THAN HVINS

3470 ENCROACHMENT STATIONS= 970.0 1105.4 TYPE= 4 TARGET= .352  
 57.10 5.59 171.59 .00 170.50 171.76 .17 .51 .00 170.00  
 2000. 0. 182. 1818. 0. 118. 529. 29. 6. 166.00  
 .17 .00 1.54 3.44 .040 .070 .040 .000 166.00 970.00  
 .001051 235. 235. 235. 2 0 0 .00 135.42 1105.42

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SECNO	DEPTH	CWSEL	CRISW	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*SECNO 58.350  
 2800 NAT Q1= 155.83 WSEL= 170.54 ENC Q1= 199.72 WSEL= 171.54 RATIO= -.2816  
 NAT Q1= 298. RATIOS LOB,CH,ROB= .0062 .6700 .3239 WSEL= 171.54

3301 HV CHANGED MORE THAN EVINS

3685 20 TRIALS ATTEMPTED WSEL,CWSEL  
 3693 PROBABLE MINIMUM SPECIFIC ENERGY  
 3720 CRITICAL DEPTH ASSUMED

3470 ENCROACHMENT STATIONS=	965.0	1035.0	TYPE=	4	TARGET=	.330			
58.35	3.07	171.07	171.07	170.54	172.55	1.48	.34	.00	170.00
2000.	0.	2000.	0.	0.	205.	0.	31.	7.	168.00
.17	.00	9.75	.00	.040	.040	.040	.000	168.00	965.00
.017826	125.	125.	125.	20	11	0	.00	70.00	1035.00

\*SECNO 60.000  
 2800 NAT Q1= 345.03 WSEL= 172.17 ENC Q1= 345.03 WSEL= 173.17 RATIO= .0000  
 NAT Q1= 532. RATIOS LOB,CH,ROB= .0179 .6253 .3568 WSEL= 173.17

3301 HV CHANGED MORE THAN EVINS

3470 ENCROACHMENT STATIONS=	975.0	1055.5	TYPE=	4	TARGET=	.351			
60.00	5.11	173.11	.00	172.17	173.64	.52	1.09	.00	170.00
2000.	0.	1926.	74.	0.	328.	20.	32.	7.	170.00
.18	.00	5.88	3.66	.040	.040	.040	.000	168.00	975.00
.003717	155.	155.	155.	2	0	0	.00	80.52	1055.52

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THIS RUN EXECUTED 11-17-87

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 HEC2 RELEASE DATED NOV 76 UPDATED MAY 1984  
 ERROR CORR - 01,02,03,04,05,06  
 MODIFICATION - 50,51,52,53,54,55,56  
 IBM-PC-XT VERSION AUGUST 1985  
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T1 CROSS CREEK HYDRAULIC STUDY PEC PROJ. NO. 36-87424-4  
 T2 SET F/W ENCR. STA'S.; 100-YR. Q=2000 CFS; SET FLOODWAY LIMITS; BLOCK 2  
 T3 EAST BRANCH GYPSUM CREEK W/ CHIMP MWB 11-17-87

J1	ICHECK	INQ	NINW	IDIR	STRT	METRIC	HVINS	Q	WSEL	FQ
	-10.	5.	0.	0.	.000000	.00	.0	0.	167.200	.000
J2	NPROF	IPLT	PRFVS	XSECV	XSECH	FN	ALLDC	IBW	CHNIM	ITRACE
	15.000	.000	-1.000	.000	.000	.000	.000	.000	.000	.000

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*PROF 4

\*SECNO 37.550

3470 ENCROACHMENT STATIONS=	915.0	1075.0	TYPE=	1	TARGET=	160.000
37.55	9.10	167.20	.00	167.20	167.23	.03 .00 .00 162.10
2000.	0.	2000.	0.	0.	1392.	0. 0. 0. 100000.00
.00	.00	1.44	.00	.040	.035	.040 .000 158.10 915.00
.000070	10.	10.	10.	0	0	0 .00 160.00 1075.00

\*SECNO 39.600

3470 ENCROACHMENT STATIONS=	835.0	1100.0	TYPE=	1	TARGET=	265.000
39.60	9.12	167.22	.00	167.22	167.25	.03 .02 .00 164.00
2000.	0.	2000.	0.	0.	1514.	0. 7. 1. 100000.00
.04	.00	1.32	.00	.040	.035	.040 .000 158.10 835.00
.000098	200.	200.	200.	0	0	0 .00 265.00 1100.00

\*SECNO 40.800

3470 ENCROACHMENT STATIONS=	990.0	1175.0	TYPE=	1	TARGET=	185.000
40.80	5.13	167.13	.00	167.21	167.30	.17 .05 .00100000.00
2000.	0.	244.	1756.	0.	107.	514. 12. 2. 164.00
.06	.00	2.29	3.42	.040	.070	.040 .000 162.00 990.00
.001891	215.	210.	195.	2	0	0 .00 185.00 1175.00

\*SECNO 43.500

3470 ENCROACHMENT STATIONS=	900.0	1120.0	TYPE=	1	TARGET=	220.000
43.50	5.43	167.43	.00	167.32	167.53	.10 .23 .00 164.00
2000.	796.	225.	979.	305.	117.	374. 14. 3. 164.00
.08	2.61	1.91	2.62	.040	.070	.040 .000 162.00 900.00
.001002	170.	170.	170.	2	0	0 .00 220.00 1120.00

\*SECNO 46.000

3470 ENCROACHMENT STATIONS=	835.0	1025.0	TYPE=	1	TARGET=	190.000
46.00	5.67	167.67	.00	167.45	167.79	.12 .26 .00 164.00
2000.	1643.	255.	102.	566.	123.	43. 19. 4. 164.00
.10	2.90	2.08	2.35	.040	.070	.040 .000 162.00 835.00
.001113	250.	250.	250.	2	0	0 .00 190.00 1025.00

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SECNO	DEPTH	CWSEL	CRIWS	WSELK	EG	HV	HL	GLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*SECNO 46.750

3470 ENCROACHMENT STATIONS=	990.0	1115.0	TYPE=	1	TARGET=	125.000
46.75	5.54	167.54	.00	167.45	167.94	.40 .15 .00100000.00
2000.	0.	461.	1539.	0.	115.	288. 20. 4. 164.00
.11	.00	3.99	5.34	.040	.070	.040 .000 162.00 990.00
.005296	70.	70.	70.	2	0	0 .00 125.00 1115.00

\*SECNO 48.800

3470 ENCROACHMENT STATIONS=	970.0	1070.0	TYPE=	1	TARGET=	100.000
48.80	6.44	168.44	.00	167.89	168.81	.37 .87 .00 164.00
2000.	0.	453.	1547.	0.	127.	297. 22. 5. 164.00
.12	.00	3.58	5.21	.040	.070	.040 .000 162.00 970.00
.003595	210.	205.	200.	2	0	0 .00 100.00 1070.00

\*SECNO 51.100

3470 ENCROACHMENT STATIONS=	985.0	1070.0	TYPE=	1	TARGET=	85.000
51.10	5.32	169.32	.00	168.55	169.94	.63 1.13 .00 166.00
2000.	63.	591.	1346.	13.	115.	196. 24. 5. 166.00
.13	4.77	5.14	6.88	.040	.070	.040 .000 164.00 985.00
.007446	235.	230.	225.	2	0	0 .00 85.00 1070.00

\*SECNO 52.500

3470 ENCROACHMENT STATIONS=	990.0	1095.0	TYPE=	1	TARGET=	105.000
52.50	6.20	170.20	.00	169.17	170.49	.29 .55 .00100000.00
2000.	0.	371.	1629.	0.	129.	353. 25. 6. 166.00
.14	.00	2.87	4.62	.040	.070	.040 .000 164.00 990.00
.002430	140.	140.	140.	2	0	0 .00 105.00 1095.00

\*SECNO 54.750

3470 ENCROACHMENT STATIONS=	975.0	1035.0	TYPE=	1	TARGET=	60.000
54.75	6.60	170.60	.00	169.64	171.28	.68 .79 .00 166.00
2000.	397.	798.	805.	59.	143.	107. 27. 6. 166.00
.15	6.67	5.57	7.51	.040	.070	.040 .000 164.00 975.00
.006510	210.	210.	210.	2	0	0 .00 60.00 1035.00

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SECNO	DEPTH	CWSEL	CRWS	WSELK	EG	HV	HL	OLOSS	BANK ELEV
Q	QLOB	QCH	QROB	ALOB	ACH	AROB	VOL	TWA	LEFT/RIGHT
TIME	VLOB	VCH	VROB	XNL	XNCH	XNR	WTN	ELMIN	SSTA
SLOPE	XLOBL	XLCH	XLOBR	ITRIAL	IDC	ICONT	CORAR	TOPWID	ENDST

\*SECNO 57.100

3301 HV CHANGED MORE THAN HVINS

3470 ENCROACHMENT STATIONS=	970.0	1105.0	TYPE=	1	TARGET=	135.000
57.10	5.61	171.61	.00	170.50	171.78	.17 .50 .00 170.00
2000.	0.	182.	1818.	0.	118.	530. 29. 6. 166.00
.17	.00	1.54	3.43	.040	.070	.040 .000 166.00 970.00
.001043	235.	235.	235.	2	0	0 .00 135.00 1105.00

\*SECNO 58.350

3301 HV CHANGED MORE THAN HVINS

3685 20 TRIALS ATTEMPTED WSEL, CWSEL  
 3693 PROBABLE MINIMUM SPECIFIC ENERGY  
 3720 CRITICAL DEPTH ASSUMED

3470 ENCROACHMENT STATIONS=	965.0	1035.0	TYPE=	1	TARGET=	70.000
58.35	3.07	171.07	171.07	170.54	172.55	1.48 .34 .00 170.00
2000.	0.	2000.	0.	0.	205.	0. 31. 7. 100000.00
.17	.00	9.76	.00	.040	.040	.040 .000 168.00 965.00
.017839	125.	125.	125.	20	11	0 .00 70.00 1035.00

\*SECNO 60.000

3301 HV CHANGED MORE THAN HVINS

3470 ENCROACHMENT STATIONS=	975.0	1055.0	TYPE=	1	TARGET=	80.000
60.00	5.11	173.11	.00	172.17	173.64	.53 1.09 .00 170.00
2000.	0.	1936.	64.	0.	328.	18. 32. 7. 170.00
.18	.00	5.90	3.55	.040	.040	.040 .000 168.00 975.00
.003731	155.	155.	155.	3	0	0 .00 80.00 1055.00

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THIS RUN EXECUTED 11-17-87

\*\*\*\*\*  
 HEC2 RELEASE DATED NOV 76 UPDATED MAY 1984  
 ERROR CORR - 01,02,03,04,05,06  
 MODIFICATION - 50,51,52,53,54,55,56  
 IBM-PC-XT VERSION AUGUST 1985  
 \*\*\*\*\*

NOTE- ASTERISK (\*) AT LEFT OF CROSS-SECTION NUMBER INDICATES MESSAGE IN SUMMARY OF ERRORS LIST

EAST BRANCH GYPSUM CR

SUMMARY PRINTOUT TABLE 110

SECNO	CWSEL	DIFKNS	EG	TOPWID	QLOB	QCH	QROB	PERENC	STENCL	STCHL	STCHR	STENCR
37.550	167.20	.00	167.22	323.33	86.34	1765.06	148.60	.00	.00	915.00	1075.00	.00
37.550	167.20	.00	167.22	323.33	86.34	1765.06	148.60	.00	.00	915.00	1075.00	.00
37.550	167.20	.00	167.23	160.00	.00	2000.00	.00	.15	915.00	915.00	1075.00	1075.00
37.550	167.20	.00	167.23	160.00	.00	2000.00	.00	160.00	915.00	915.00	1075.00	1075.00
39.600	167.22	.00	167.23	497.92	19.28	1774.83	205.88	.00	.00	835.00	1100.00	.00
39.600	167.22	.00	167.23	482.58	18.17	1777.27	204.56	.00	819.49	835.00	1100.00	1302.07
39.600	167.22	.01	167.25	265.00	.00	2000.00	.00	.16	835.00	835.00	1100.00	1100.00
39.600	167.22	.01	167.25	265.00	.00	2000.00	.00	265.00	835.00	835.00	1100.00	1100.00
40.800	167.21	.00	167.27	334.86	35.23	175.57	1789.20	.00	.00	989.00	1011.00	.00
40.800	167.21	.00	167.27	334.86	35.23	175.57	1789.20	.00	.00	989.00	1011.00	.00
40.800	167.14	-.07	167.30	187.67	.00	254.43	1745.57	.38	989.00	989.00	1011.00	1176.67
40.800	167.13	-.07	167.30	185.00	.00	244.37	1755.63	185.00	990.00	989.00	1011.00	1175.00
43.500	167.32	.00	167.37	404.22	591.67	150.81	1257.52	.00	.00	989.00	1011.00	.00
43.500	167.31	-.01	167.42	220.00	794.28	228.68	977.05	220.00	900.00	989.00	1011.00	1120.00
43.500	167.43	.10	167.53	219.35	340.12	225.32	1434.56	.38	949.89	989.00	1011.00	1169.24
43.500	167.43	.11	167.53	220.00	796.04	224.59	979.37	220.00	900.00	989.00	1011.00	1120.00
46.000	167.45	.00	167.50	415.65	1482.97	177.51	339.52	.00	.00	989.00	1011.00	.00
46.000	167.56	.11	167.61	416.89	1471.84	172.66	355.50	.00	.00	989.00	1011.00	.00
46.000	167.67	.22	167.79	188.42	1639.27	257.31	103.42	.39	836.71	989.00	1011.00	1025.13
46.000	167.67	.23	167.79	190.00	1643.29	254.99	101.72	190.00	835.00	989.00	1011.00	1025.00
46.750	167.45	.00	167.58	315.64	248.32	332.60	1419.08	.00	.00	989.00	1011.00	.00
46.750	167.55	.11	167.67	316.88	246.67	319.34	1433.99	.00	.00	989.00	1011.00	.00
46.750	167.56	.11	167.94	125.67	.00	481.22	1518.78	.45	989.00	989.00	1011.00	1114.67
46.750	167.54	.10	167.94	125.00	.00	461.01	1538.99	125.00	990.00	989.00	1011.00	1115.00
48.800	167.89	.00	168.04	315.25	224.69	363.84	1411.47	.00	.00	970.00	990.00	.00
48.800	167.95	.05	168.09	320.31	226.81	356.71	1416.49	.00	.00	970.00	990.00	.00
48.800	168.43	.54	168.81	99.14	.00	456.93	1543.07	.45	970.00	970.00	990.00	1069.14
48.800	168.44	.55	168.81	100.00	.00	453.41	1546.59	100.00	970.00	970.00	990.00	1070.00

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SECNO	CWSEL	DIFKWS	EG	TOPWID	QLOB	QCH	QROB	PERENC	STENCL	STCHL	STCHR	STENCR
51.100	168.55	.00	168.86	212.91	461.19	397.28	1141.53	.00	.00	989.00	1011.00	.00
51.100	168.56	.01	168.87	213.21	461.79	395.99	1142.22	.00	.00	989.00	1011.00	.00
51.100	169.31	.76	169.91	87.64	85.89	565.37	1348.73	.46	983.74	989.00	1011.00	1071.38
51.100	169.32	.77	169.94	85.00	63.26	590.93	1345.80	85.00	985.00	989.00	1011.00	1070.00
52.500	169.17	.00	169.35	245.62	248.01	317.38	1434.61	.00	.00	989.00	1011.00	.00
52.500	169.17	.00	169.35	220.38	248.03	317.26	1434.71	.00	934.97	989.00	1011.00	1155.35
52.500	170.16	.99	170.45	105.36	.00	394.04	1605.96	.41	989.00	989.00	1011.00	1094.36
52.500	170.20	1.03	170.49	105.00	.00	371.04	1628.96	105.00	990.00	989.00	1011.00	1095.00
54.750	169.64	.00	170.11	121.47	592.63	595.08	812.29	.00	.00	989.00	1011.00	.00
54.750	169.65	.00	170.11	121.43	592.30	595.60	812.10	.00	.00	989.00	1011.00	.00
54.750	170.57	.92	171.26	60.26	453.02	797.65	749.33	.37	972.97	989.00	1011.00	1033.23
54.750	170.60	.96	171.28	60.00	396.81	797.91	805.28	60.00	975.00	989.00	1011.00	1035.00
57.100	170.50	.00	170.62	232.09	.00	118.17	1881.83	.00	.00	970.00	1000.00	.00
57.100	170.51	.01	170.63	230.00	.00	115.88	1884.12	.00	970.00	970.00	1000.00	1200.00
57.100	171.59	1.09	171.76	135.42	.00	181.64	1818.36	.35	970.00	970.00	1000.00	1105.42
57.100	171.61	1.11	171.78	135.00	.00	182.34	1817.66	135.00	970.00	970.00	1000.00	1105.00
* 58.350	170.54	.00	171.55	126.20	1.47	1434.88	563.66	.00	.00	965.00	1035.00	.00
* 58.350	170.54	.00	171.55	126.20	1.46	1434.90	563.63	.00	.00	965.00	1035.00	.00
* 58.350	171.07	.53	172.55	70.00	.00	2000.00	.00	.33	965.00	965.00	1035.00	1035.00
* 58.350	171.07	.53	172.55	70.00	.00	2000.00	.00	70.00	965.00	965.00	1035.00	1035.00
60.000	172.17	.00	172.54	129.15	13.38	1260.21	726.41	.00	.00	975.00	1050.00	.00
60.000	172.17	.00	172.54	129.12	13.36	1260.21	726.43	.00	.00	975.00	1050.00	.00
60.000	173.11	.94	173.64	80.52	.00	1926.19	73.81	.35	975.00	975.00	1050.00	1055.52
60.000	173.11	.94	173.64	80.00	.00	1935.86	64.14	80.00	975.00	975.00	1050.00	1055.00

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EAST BRANCH GYPSUM CR

SUMMARY PRINTOUT TABLE 150

SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10K+S	VCH	AREA	.01K
37.550	.00	.00	.00	158.10	2000.00	167.20	.00	167.22	.50	1.27	1798.08	2821.37
37.550	.00	.00	.00	158.10	2000.00	167.20	.00	167.22	.50	1.27	1798.08	2821.37
37.550	.00	.00	.00	158.10	2000.00	167.20	.00	167.23	.70	1.44	1392.01	2390.03
37.550	.00	.00	.00	158.10	2000.00	167.20	.00	167.23	.70	1.44	1391.99	2390.01
39.600	200.00	.00	.00	158.10	2000.00	167.22	.00	167.23	.75	1.17	1951.08	2304.96
39.600	200.00	.00	.00	158.10	2000.00	167.22	.00	167.23	.75	1.17	1942.12	2303.46
39.600	200.00	.00	.00	158.10	2000.00	167.22	.00	167.25	.98	1.32	1514.29	2016.74
39.600	200.00	.00	.00	158.10	2000.00	167.22	.00	167.25	.98	1.32	1513.71	2015.49
40.800	210.00	.00	.00	162.00	2000.00	167.21	.00	167.27	7.06	1.56	1005.55	752.76
40.800	210.00	.00	.00	162.00	2000.00	167.21	.00	167.27	7.06	1.56	1005.55	752.75
40.800	210.00	.00	.00	162.00	2000.00	167.14	.00	167.30	18.21	2.29	630.98	468.66
40.800	210.00	.00	.00	162.00	2000.00	167.13	.00	167.30	18.91	2.29	620.57	459.87
43.500	170.00	.00	.00	162.00	2000.00	167.32	.00	167.37	4.83	1.31	1197.07	910.01
43.500	170.00	.00	.00	162.00	2000.00	167.31	.00	167.42	11.20	1.99	770.48	597.74
43.500	170.00	.00	.00	162.00	2000.00	167.43	.00	167.53	10.11	1.92	793.39	628.93
43.500	170.00	.00	.00	162.00	2000.00	167.43	.00	167.53	10.02	1.91	796.43	631.70
46.000	250.00	.00	.00	162.00	2000.00	167.45	.00	167.50	6.19	1.51	1102.74	803.88
46.000	250.00	.00	.00	162.00	2000.00	167.56	.00	167.61	5.48	1.44	1147.73	854.46
46.000	250.00	.00	.00	162.00	2000.00	167.67	.00	167.79	11.34	2.10	725.56	593.78
46.000	250.00	.00	.00	162.00	2000.00	167.67	.00	167.79	11.13	2.08	731.75	599.43
46.750	70.00	.00	.00	162.00	2000.00	167.45	.00	167.58	21.74	2.82	691.71	428.94
46.750	70.00	.00	.00	162.00	2000.00	167.55	.00	167.67	18.76	2.66	725.69	461.79
46.750	70.00	.00	.00	162.00	2000.00	167.56	.00	167.94	50.95	4.00	409.42	280.19
46.750	70.00	.00	.00	162.00	2000.00	167.54	.00	167.94	52.96	3.99	403.86	274.83
48.800	205.00	.00	.00	162.00	2000.00	167.89	.00	168.04	24.59	3.14	638.33	403.36
48.800	205.00	.00	.00	162.00	2000.00	167.95	.00	168.09	22.88	3.05	656.18	418.10
48.800	205.00	.00	.00	162.00	2000.00	168.43	.00	168.81	36.71	3.61	419.92	330.11
48.800	205.00	.00	.00	162.00	2000.00	168.44	.00	168.81	35.95	3.58	423.62	333.57
51.100	230.00	.00	.00	164.00	2000.00	168.55	.00	168.86	57.10	4.05	449.40	264.68
51.100	230.00	.00	.00	164.00	2000.00	168.56	.00	168.87	56.34	4.03	451.37	266.46
51.100	230.00	.00	.00	164.00	2000.00	169.31	.00	169.91	68.23	4.92	332.57	242.13
51.100	230.00	.00	.00	164.00	2000.00	169.32	.00	169.94	74.46	5.14	323.99	231.77
52.500	140.00	.00	.00	164.00	2000.00	169.17	.00	169.35	23.61	2.84	602.06	411.56
52.500	140.00	.00	.00	164.00	2000.00	169.17	.00	169.35	23.57	2.84	595.72	411.96
52.500	140.00	.00	.00	164.00	2000.00	170.16	.00	170.45	24.83	2.95	480.02	401.33
52.500	140.00	.00	.00	164.00	2000.00	170.20	.00	170.49	24.30	2.87	482.14	405.72

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SECNO	XLCH	ELTRD	ELLC	ELMIN	Q	CWSEL	CRWS	EG	10K*S	VCH	AREA	.01K	
54.750	210.00	.00	.00	164.00	2000.00	169.64	.00	170.11	61.58	4.87	369.52	254.86	
54.750	210.00	.00	.00	164.00	2000.00	169.65	.00	170.11	61.84	4.88	369.04	254.33	
54.750	210.00	.00	.00	164.00	2000.00	170.57	.00	171.26	66.35	5.60	308.44	245.53	
54.750	210.00	.00	.00	164.00	2000.00	170.60	.00	171.28	65.10	5.57	310.05	247.89	
57.100	235.00	.00	.00	166.00	2000.00	170.50	.00	170.62	11.05	1.39	756.01	601.61	
57.100	235.00	.00	.00	166.00	2000.00	170.51	.00	170.63	11.09	1.36	755.59	600.68	
57.100	235.00	.00	.00	166.00	2000.00	171.59	.00	171.76	10.51	1.54	647.03	616.94	
57.100	235.00	.00	.00	166.00	2000.00	171.61	.00	171.78	10.43	1.54	647.98	619.28	
*	58.350	125.00	.00	.00	168.00	2000.00	170.54	170.54	171.55	164.71	8.53	254.16	155.83
*	58.350	125.00	.00	.00	168.00	2000.00	170.54	170.54	171.55	164.78	8.54	254.13	155.81
*	58.350	125.00	.00	.00	168.00	2000.00	171.07	171.07	172.55	178.26	9.75	205.03	149.80
*	58.350	125.00	.00	.00	168.00	2000.00	171.07	171.07	172.55	178.39	9.76	204.98	149.74
60.000	155.00	.00	.00	168.00	2000.00	172.17	.00	172.54	33.60	4.89	412.52	345.03	
60.000	155.00	.00	.00	168.00	2000.00	172.17	.00	172.54	33.64	4.89	412.35	344.80	
60.000	155.00	.00	.00	168.00	2000.00	173.11	.00	173.64	37.17	5.88	347.81	328.04	
60.000	155.00	.00	.00	168.00	2000.00	173.11	.00	173.64	37.31	5.90	346.29	327.41	

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EAST BRANCH GYPSUM CR

SUMMARY PRINTOUT TABLE 150

SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH
37.550	2000.00	167.20	.00	.00	.00	323.33	.00
37.550	2000.00	167.20	.00	.00	.00	323.33	.00
37.550	2000.00	167.20	.00	.00	.00	160.00	.00
37.550	2000.00	167.20	.00	.00	.00	160.00	.00
39.600	2000.00	167.22	.00	.02	.00	497.92	200.00
39.600	2000.00	167.22	.00	.02	.00	482.58	200.00
39.600	2000.00	167.22	.01	.02	.01	265.00	200.00
39.600	2000.00	167.22	.00	.02	.01	265.00	200.00
40.800	2000.00	167.21	.00	-.01	.00	334.86	210.00
40.800	2000.00	167.21	.00	-.01	.00	334.86	210.00
40.800	2000.00	167.14	-.07	-.08	-.07	187.67	210.00
40.800	2000.00	167.13	-.01	-.09	-.07	185.00	210.00
43.500	2000.00	167.32	.00	.12	.00	404.22	170.00
43.500	2000.00	167.31	-.01	.10	-.01	220.00	170.00
43.500	2000.00	167.43	.11	.29	.10	219.35	170.00
43.500	2000.00	167.43	.00	.30	.11	220.00	170.00
46.000	2000.00	167.45	.00	.12	.00	415.65	250.00
46.000	2000.00	167.56	.11	.25	.11	416.89	250.00
46.000	2000.00	167.67	.12	.25	.22	188.42	250.00
46.000	2000.00	167.67	.00	.24	.23	190.00	250.00
46.750	2000.00	167.45	.00	.00	.00	315.64	70.00
46.750	2000.00	167.55	.11	.00	.11	316.88	70.00
46.750	2000.00	167.56	.00	-.12	.11	125.67	70.00
46.750	2000.00	167.54	-.01	-.13	.10	125.00	70.00
48.800	2000.00	167.89	.00	.44	.00	315.25	205.00
48.800	2000.00	167.95	.05	.39	.05	320.31	205.00
48.800	2000.00	168.43	.48	.87	.54	99.14	205.00
48.800	2000.00	168.44	.01	.90	.55	100.00	205.00
51.100	2000.00	168.55	.00	.66	.00	212.91	230.00
51.100	2000.00	168.56	.01	.61	.01	213.21	230.00
51.100	2000.00	169.31	.75	.89	.76	87.64	230.00
51.100	2000.00	169.32	.00	.88	.77	85.00	230.00
52.500	2000.00	169.17	.00	.62	.00	245.62	140.00
52.500	2000.00	169.17	.00	.61	.00	220.38	140.00
52.500	2000.00	170.16	.99	.84	.99	105.36	140.00
52.500	2000.00	170.20	.04	.88	1.03	105.00	140.00

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SECNO	Q	CWSEL	DIFWSP	DIFWSX	DIFKWS	TOPWID	XLCH	
54.750	2000.00	169.64	.00	.47	.00	121.47	210.00	
54.750	2000.00	169.65	.00	.47	.00	121.43	210.00	
54.750	2000.00	170.57	.92	.41	.92	60.26	210.00	
54.750	2000.00	170.60	.03	.40	.96	60.00	210.00	
57.100	2000.00	170.50	.00	.86	.00	232.09	235.00	
57.100	2000.00	170.51	.01	.86	.01	230.00	235.00	
57.100	2000.00	171.59	1.08	1.02	1.09	135.42	235.00	
57.100	2000.00	171.61	.02	1.01	1.11	135.00	235.00	
*	58.350	2000.00	170.54	.00	.04	.00	126.20	125.00
*	58.350	2000.00	170.54	.00	.04	.00	126.20	125.00
*	58.350	2000.00	171.07	.53	-.52	.53	70.00	125.00
*	58.350	2000.00	171.07	.00	-.54	.53	70.00	125.00
60.000	2000.00	172.17	.00	1.62	.00	129.15	155.00	
60.000	2000.00	172.17	.00	1.62	.00	129.12	155.00	
60.000	2000.00	173.11	.94	2.04	.94	80.52	155.00	
60.000	2000.00	173.11	.00	2.04	.94	80.00	155.00	

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SUMMARY OF ERRORS AND SPECIAL NOTES

CAUTION	SECNO=	58.350	PROFILE= 1	CRITICAL DEPTH ASSUMED
CAUTION	SECNO=	58.350	PROFILE= 1	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION	SECNO=	58.350	PROFILE= 1	20 TRIALS ATTEMPTED TO BALANCE WSEL
CAUTION	SECNO=	58.350	PROFILE= 2	CRITICAL DEPTH ASSUMED
CAUTION	SECNO=	58.350	PROFILE= 2	MINIMUM SPECIFIC ENERGY
CAUTION	SECNO=	58.350	PROFILE= 3	CRITICAL DEPTH ASSUMED
CAUTION	SECNO=	58.350	PROFILE= 3	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION	SECNO=	58.350	PROFILE= 3	20 TRIALS ATTEMPTED TO BALANCE WSEL
CAUTION	SECNO=	58.350	PROFILE= 4	CRITICAL DEPTH ASSUMED
CAUTION	SECNO=	58.350	PROFILE= 4	PROBABLE MINIMUM SPECIFIC ENERGY
CAUTION	SECNO=	58.350	PROFILE= 4	20 TRIALS ATTEMPTED TO BALANCE WSEL

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FLOODWAY DATA, EAST BRANCH GYPSUM CR  
 PROFILE NO. 2

STATION	WIDTH	FLOODWAY		WATER SURFACE ELEVATION		
		SECTION AREA	MEAN VELOCITY	WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
37.550	323.	1798.	1.1	167.2	167.2	.0
39.600	483.	1942.	1.0	167.2	167.2	.0
40.800	335.	1006.	2.0	167.2	167.2	.0
43.500	220.	770.	2.6	167.3	167.3	.0
46.000	417.	1148.	1.7	167.5	167.4	.1
46.750	317.	726.	2.8	167.5	167.4	.1
48.800	320.	656.	3.0	168.0	167.9	.1
51.100	213.	451.	4.4	168.6	168.6	.0
52.500	220.	596.	3.4	169.2	169.2	.0
54.750	121.	369.	5.4	169.6	169.6	.0
57.100	230.	756.	2.6	170.5	170.5	.0
58.350	126.	254.	7.9	170.5	170.5	.0
60.000	129.	412.	4.9	172.2	172.2	.0

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PAGE 26

FLOODWAY DATA, EAST BRANCH GYPSUM CR  
 PROFILE NO. 3

STATION	WIDTH	FLOODWAY		WATER SURFACE ELEVATION		
		SECTION AREA	MEAN VELOCITY	WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
37.550	160.	1392.	1.4	167.2	167.2	.0
39.600	265.	1514.	1.3	167.2	167.2	.0
40.800	188.	631.	3.2	167.2	167.2	.0
43.500	219.	793.	2.5	167.4	167.3	.1
46.000	188.	726.	2.8	167.6	167.4	.2
46.750	126.	409.	4.9	167.5	167.4	.1
48.800	99.	420.	4.8	168.4	167.9	.5
51.100	88.	333.	6.0	169.4	168.6	.8
52.500	105.	480.	4.2	170.2	169.2	1.0
54.750	60.	308.	6.5	170.5	169.6	.9
57.100	135.	647.	3.1	171.6	170.5	1.1
58.350	70.	205.	9.8	171.0	170.5	.5
60.000	81.	348.	5.8	173.1	172.2	.9

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PAGE 27

FLOODWAY DATA, EAST BRANCH GYPSUM CR  
 PROFILE NO. 4

STATION	WIDTH	FLOODWAY SECTION AREA	MEAN VELOCITY	WATER SURFACE ELEVATION		
				WITH FLOODWAY	WITHOUT FLOODWAY	DIFFERENCE
37.550	160.	1392.	1.4	167.2	167.2	.0
39.600	265.	1514.	1.3	167.2	167.2	.0
40.800	185.	621.	3.2	167.2	167.2	.0
43.500	220.	796.	2.5	167.4	167.3	.1
46.000	190.	732.	2.7	167.6	167.4	.2
46.750	125.	404.	5.0	167.5	167.4	.1
48.800	100.	424.	4.7	168.4	167.9	.5
51.100	85.	324.	6.2	169.4	168.6	.8
52.500	105.	482.	4.1	170.2	169.2	1.0
54.750	60.	310.	6.5	170.6	169.6	1.0
57.100	135.	648.	3.1	171.6	170.5	1.1
58.350	70.	205.	9.8	171.0	170.5	.5
60.000	80.	346.	5.8	173.1	172.2	.9

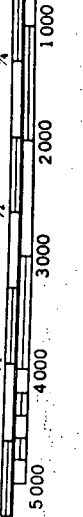
(Joins sheet 28)

R.



1 Mile  
5000 Feet

Scale 1:20,000



(Joins sheet 44) 1 2 370 000 FEET

## EXHIBIT NO. 1

## SOIL LEGEND

<u>SYMBOL</u>	<u>HYDROLOGIC GROUP</u>	<u>NAME</u>
Aa	B	Albion-Shellabarger sandy loams, 1 to 4 percent slopes
Ab	B	Albion and Shellabarger sandy loams, 7 to 15 percent slopes
Ba	C	Blanket silt loam, 0 to 1 percent slopes
Bb	C	Blanket silt loam, 1 to 3 percent slopes
Ca	B	Canadian fine sandy loam
Cb	B	Canadian-Waldeck fine sandy loams
Cc	D	Carwile fine sandy loam
Cd	B	Clark-Ost clay loams, 1 to 4 percent slopes
Ce	C	Cline silty clay, 3 to 6 percent slopes
Ea	B	Elandco silt loam
Eb	B	Elandco silt loam, occasionally flooded
Ec	B	Elandco silt loam, frequently flooded
Fa	B	Farnum loam, 0 to 1 percent slopes
Fb	B	Farnum loam, 1 to 3 percent slopes
Fc	B	Farnum loam, sandy substratum, 0 to 1 percent slopes
Ga	D	Goessel silty clay, 0 to 1 percent slopes
Gb	D	Goessel silty clay, 1 to 2 percent slopes
Ia	D	Irwin silty clay loam, 1 to 3 percent slopes
Ib	D	Irwin silty clay loam, 3 to 6 percent slopes
Ic	D	Irwin silty clay loam, 2 to 6 percent slopes, eroded
La	C	Lesho loam
Lb	A	Lincoln soils
Ma	B	Milan loam, 1 to 3 percent slopes
Mb	B	Milan form, 3 to 6 percent slopes
Mc	B	Milan clay loam, 2 to 6 percent slopes, eroded
Na	B	Naron fine sandy loam
Oc	D	Owens clay loam, 1 to 3 percent slopes
Od	D	Owens-Rock outcrop complex, 3 to 10 percent slopes
Pa		Pits
Pb	D	Plevna fine sandy loam
Pc	A	Pratt loamy fine sand, undulating
Pd	A	Pratt-Tivoli complex, rolling
Ra	D	Renfrow silty clay loam, 1 to 3 percent slopes
Rb	D	Renfrow silty clay loam, 3 to 6 percent slopes
Rc	D	Renfrow-Owens clay loams, 1 to 4 percent slopes
Rd	D	Rosehill silty clay, 1 to 3 percent slopes
Sa	B	Shellabarger sandy loam, 1 to 3 percent slopes
Sb	B	Shellabarger sandy loam, 3 to 6 percent slopes
Sc	B	Shellabarger sandy loam, 3 to 6 percent slopes, eroded
Ta	D	Tabler silty clay loam
Tb	D	Tabler-Drummond complex
Ua	B	Urban land-Canadian complex
Ub	B	Urban land-Elandco complex
Uc	B	Urban land-Farnum complex, 0 to 3 percent slopes
Ud	D	Urban land-Irwin complex, 1 to 3 percent slopes
Ue	D	Urban land-Tabler complex
Va	B	Vanoss silt loam, 0 to 1 percent slopes
Vb	B	Vanoss silt loam, 1 to 3 percent slopes
Vc	B	Vanoss silt loam, 3 to 6 percent slopes
Vd	B	Vanoss silt loam, 3 to 6 percent slopes, eroded
Ve	D	Vernon sandy loam, 1 to 3 percent slopes
Vf	D	Vernon sandy loam, 3 to 6 percent slopes
Wa	C	Waldeck sandy loam
Wb	D	Waurika silt loam

## ATTACHMENT D

## DRAINAGE CRITERIA

## CITY OF WICHITA, KANSAS

RECOMMENDED RUNOFF COEFFICIENTS FOR RATIONAL METHOD  
AND PERCENT IMPERVIOUS FOR UNIT HYDROGRAPH METHOD

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		2	5	10	100
<b>1. Business:</b>					
Downtown Areas	95	0.84	0.85	0.87	0.91
Neighborhood Areas	70	0.68	0.69	0.73	0.80
<b>2. Residential:</b>					
<u>Single Family (Soil Group D)</u>					
1/8 Acre	50	0.57	0.61	0.66	0.79
1/4 Acre	38	0.50	0.54	0.62	0.76
1/3 Acre	30	0.46	0.50	0.59	0.73
1/2 Acre	25	0.42	0.48	0.56	0.72
3/4 Acre	22	0.42	0.46	0.55	0.71
1 Acre	20	0.41	0.45	0.54	0.71
<u>Multi-Family (Soil Group D)</u>					
Multi-Unit (detached)	60	0.62	0.66	0.72	0.82
Multi-Unit (attached)	65	0.64	0.68	0.73	0.83
Apartments	75	0.70	0.73	0.79	0.86
<u>Single Family (Soil Group C)</u>					
1/8 Acre	50	0.55	0.58	0.64	0.73
1/4 Acre	38	0.48	0.51	0.57	0.68
1/3 Acre	30	0.43	0.46	0.53	0.65
1/2 Acre	25	0.40	0.43	0.50	0.63
3/4 Acre	22	0.39	0.42	0.49	0.62
1 Acre	20	0.37	0.40	0.48	0.61
<u>Multi-Family (Soil Group C)</u>					
Multi-Unit (detached)	60	0.60	0.63	0.69	0.77
Multi-Unit (attached)	65	0.63	0.66	0.71	0.79
Apartments	75	0.68	0.72	0.77	0.83
<u>Single-Family (Soil Group B)</u>					
1/8 Acre	50	0.52	0.54	0.59	0.67
1/4 Acre	38	0.44	0.46	0.52	0.61
1/3 Acre	30	0.39	0.41	0.47	0.57
1/2 Acre	25	0.36	0.38	0.44	0.54
3/4 Acre	22	0.34	0.36	0.42	0.52
1 Acre	20	0.33	0.35	0.40	0.51
<u>Multi-Family (Soil Group B)</u>					
Multi-Unit (detached)	60	0.58	0.60	0.65	0.72
Multi-Unit (attached)	65	0.61	0.64	0.68	0.75
Apartments	75	0.67	0.70	0.74	0.80

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
<u>Single Family (Soil Group A)</u>					
1/8 Acre	50	0.47	0.50	0.54	0.60
1/4 Acre	38	0.39	0.41	0.45	0.52
1/3 Acre	30	0.33	0.35	0.39	0.47
1/2 Acre	25	0.30	0.31	0.35	0.44
3/4 Acre	22	0.28	0.29	0.33	0.42
1 Acre	20	0.26	0.28	0.32	0.40
<u>Multi-Family (Soil Group A)</u>					
Multi-Unit (detached)	60	0.55	0.57	0.61	0.67
Multi-Unit (attached)	65	0.58	0.60	0.64	0.70
Apartments	75	0.65	0.68	0.72	0.77
3. Industrial:					
Light Areas	70	0.68	0.69	0.73	0.80
Heavy Areas	80	0.74	0.76	0.79	0.84
4. Playgrounds:	15	0.33	0.35	0.42	0.55
5. Schools:	40	0.49	0.51	0.56	0.66
6. Railroad Yard Areas:	30	0.43	0.45	0.50	0.62
7. Undeveloped Urban Areas: Offsite Flow Analysis (when land use not defined)	45	0.52	0.54	0.59	0.68
8. Streets:					
Paved	99	0.87	0.88	0.90	0.93
Gravel	00	0.24	0.26	0.33	0.48
9. Drive, Parking Lots and Walks:	96	0.87	0.87	0.88	0.89
10. Roofs:	90	0.80	0.85	0.90	0.93
11. Urban Lawn Areas (See Note No. 1 below):					
<u>Soil Group A</u>					
Slope less than 1%	00	0.08	0.09	0.13	0.23
Slope 1% to 4%	00	0.12	0.13	0.17	0.27
Slope more than 4%	00	0.16	0.17	0.21	0.31
<u>Soil Group B</u>					
Slope less than 1%	00	0.16	0.18	0.24	0.37
Slope 1% to 4%	00	0.20	0.22	0.28	0.41
Slope more than 4%	00	0.24	0.26	0.32	0.45
<u>Soil Group C</u>					
Slope less than 1%	00	0.24	0.27	0.35	0.51
Slope 1% to 4%	00	0.26	0.29	0.37	0.53
Slope more than 4%	00	0.28	0.31	0.39	0.55

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
<u>Soil Group D</u>					
Slope less than 1%	00	0.28	0.33	0.43	0.63
Slope 1% to 4%	00	0.30	0.35	0.45	0.65
Slope more than 4%	00	0.32	0.37	0.47	0.67

Note No. 1: Coefficients shown in the above table are for pervious open space areas with thick turf which includes pervious areas in parks and cemeteries. Coefficients shown above must be increased 0.02 for use with agricultural pasture areas. Coefficients shown above must be reduced by 0.04 for use with agricultural cultivated areas. Group A soils are well-drained, coarse textured sands with high infiltration rates. Group B soils are moderately well-drained, moderately coarse textured soils with moderate infiltration rates. Group C soils are moderately poor-drained, moderately fine textured soils with slow infiltration rates. Group D soils are poor-drained, fine textured soils with very slow infiltration rates.

GENERAL NOTE: These Rational Formula Coefficients may not be valid for basins 320 acres or larger.

April 15, 1986

ATTACHMENT A  
DRAINAGE CRITERIA MANUAL

CITY OF WICHITA, KANSAS

RAINFALL INTENSITY TABLE FOR SEDGWICK COUNTY, KANSAS

The following tabulation contains rainfall intensity in inches per hour as derived from ESSA Weather Bureau Technical Paper 40 Modified to NWS Hydro-35, 1977 During First Hour

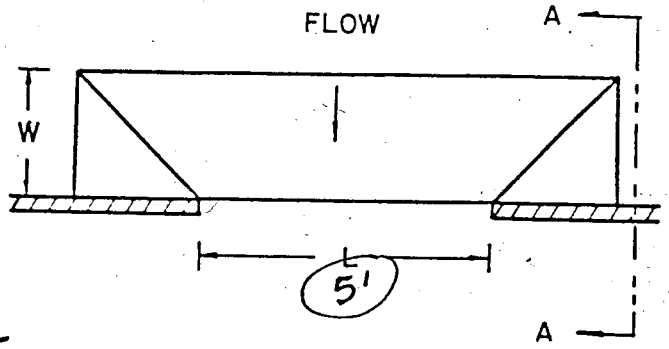
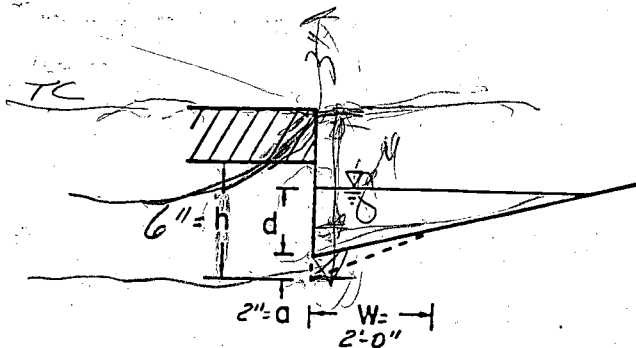
DURATION IN MINUTES	RETURN PERIODS OF						
	1-YR	2-YR	5-YR	10-YR	25-YR	50-YR	100-YR
5	4.18	5.57	6.53	7.41	8.52	9.48	10.32
6	3.99	5.32	6.25	7.09	8.16	9.09	9.89
7	3.81	5.09	5.99	6.81	7.84	8.74	9.50
8	3.66	4.89	5.75	6.55	7.55	8.42	9.15
9	3.52	4.70	5.54	6.31	7.28	8.13	8.83
10	3.39	4.52	5.34	6.09	7.04	7.86	8.54
11	3.27	4.36	5.16	5.89	6.81	7.61	8.27
12	3.18	4.21	4.99	5.71	6.60	7.38	8.02
13	3.05	4.08	4.84	5.53	6.41	7.17	7.79
14	2.96	3.95	4.69	5.37	6.23	6.97	7.57
15	2.87	3.83	4.56	5.22	6.06	6.78	7.37
16	2.78	3.72	4.43	5.08	5.90	6.60	7.18
17	2.71	3.61	4.31	4.95	5.75	6.44	7.00
18	2.63	3.51	4.20	4.83	5.61	6.29	6.84
19	2.56	3.42	4.10	4.71	5.47	6.14	6.68
20	2.50	3.33	4.00	4.60	5.35	6.00	6.53
21	2.44	3.25	3.90	4.50	5.23	5.87	6.39
22	2.38	3.17	3.81	4.40	5.12	5.75	6.26
23	2.32	3.10	3.73	4.31	5.01	5.63	6.13
24	2.27	3.03	3.65	4.22	4.91	5.52	6.01
25	2.22	2.96	3.57	4.13	4.81	5.41	5.90
26	2.20	2.90	3.50	4.05	4.72	5.31	5.79
27	2.16	2.84	3.43	3.98	4.63	5.21	5.69
28	2.14	2.78	3.37	3.90	4.55	5.12	5.59
29	2.11	2.72	3.30	3.83	4.47	5.03	5.49
30	2.08	2.67	3.24	3.76	4.39	4.94	5.40
31	2.05	2.62	3.19	3.70	4.32	4.86	5.32
32	2.02	2.57	3.10	3.63	4.25	4.79	5.22
33	1.99	2.52	3.05	3.57	4.18	4.71	5.14
34	1.96	2.48	3.01	3.51	4.11	4.63	5.07
35	1.93	2.44	2.98	3.46	4.05	4.56	5.00
36	1.91	2.39	2.93	3.41	3.99	4.50	4.93
37	1.89	2.35	2.88	3.36	3.93	4.43	4.86
38	1.87	2.32	2.84	3.31	3.87	4.37	4.79
39	1.85	2.28	2.80	3.26	3.82	4.31	4.73
40	1.83	2.24	2.76	3.22	3.76	4.25	4.66
41	1.81	2.21	2.72	3.17	3.71	4.19	4.60
42	1.79	2.18	2.68	3.13	3.66	4.13	4.54
43	1.77	2.14	2.64	3.09	3.61	4.08	4.49
44	1.75	2.11	2.61	3.05	3.57	4.03	4.43
45	1.73	2.08	2.57	3.01	3.52	3.98	4.38

ATTACHMENT A CONTINUED  
Page 2

<u>DURATION IN MINUTES</u>	<u>RETURN PERIODS OF</u>						
	<u>1-YR</u>	<u>2-YR</u>	<u>5-YR</u>	<u>10-YR</u>	<u>25-YR</u>	<u>50-YR</u>	<u>100-YR</u>
46	1.70	2.05	2.54	2.97	3.48	3.93	4.33
47	1.67	2.02	2.50	2.93	3.44	3.88	4.28
48	1.66	2.00	2.47	2.90	3.39	3.84	4.23
49	1.64	1.97	2.44	2.86	3.35	3.79	4.18
50	1.61	1.95	2.41	2.83	3.32	3.75	4.13
51	1.59	1.92	2.38	2.79	3.28	3.71	4.09
52	1.56	1.89	2.35	2.76	3.24	3.67	4.05
53	1.54	1.86	2.33	2.73	3.20	3.63	4.00
54	1.52	1.84	2.30	2.70	3.17	3.59	3.96
55	1.50	1.81	2.27	2.67	3.14	3.55	3.92
56	1.47	1.79	2.25	2.64	3.10	3.51	3.88
57	1.45	1.76	2.22	2.61	3.07	3.48	3.84
58	1.43	1.74	2.20	2.59	3.04	3.44	3.81
59	1.42	1.72	2.18	2.56	3.01	3.41	3.77
60	1.40	1.69	2.15	2.53	2.98	3.37	3.73
61	1.38	1.67	2.13	2.51	2.95	3.34	3.70
62	1.36	1.65	2.11	2.48	2.92	3.31	3.67
63	1.34	1.63	2.09	2.46	2.89	3.28	3.63
64	1.33	1.61	2.07	2.44	2.86	3.25	3.60
65	1.31	1.59	2.05	2.41	2.84	3.22	3.57
66	1.30	1.57	2.03	2.39	2.81	3.19	3.54
67	1.28	1.56	2.01	2.37	2.79	3.16	3.51
68	1.26	1.54	1.99	2.35	2.76	3.13	3.48
69	1.25	1.52	1.97	2.33	2.74	3.10	3.45
70	1.24	1.50	1.95	2.31	2.71	3.08	3.42
71	1.22	1.49	1.93	2.28	2.69	3.05	3.39
72	1.21	1.47	1.92	2.26	2.67	3.02	3.36
73	1.20	1.46	1.90	2.25	2.64	3.00	3.34
74	1.18	1.44	1.88	2.23	2.63	2.98	3.31
75	1.17	1.43	1.86	2.21	2.61	2.95	3.29
76	1.16	1.41	1.85	2.19	2.58	2.93	3.26
77	1.15	1.40	1.83	2.17	2.55	2.90	3.24
78	1.13	1.38	1.82	2.15	2.53	2.88	3.22
79	1.12	1.37	1.80	2.14	2.50	2.86	3.19
80	1.11	1.36	1.79	2.12	2.48	2.84	3.16
81	1.10	1.34	1.77	2.10	2.46	2.82	3.13
82	1.09	1.33	1.76	2.08	2.43	2.79	3.10
83	1.08	1.32	1.74	2.06	2.41	2.76	3.07
84	1.07	1.31	1.73	2.04	2.39	2.74	3.04
85	1.06	1.30	1.72	2.02	2.37	2.71	3.01
86	1.05	1.28	1.70	2.00	2.34	2.69	2.99
87	1.04	1.27	1.69	1.99	2.32	2.66	2.96
88	1.03	1.26	1.68	1.97	2.30	2.64	2.93
89	1.02	1.25	1.68	1.95	2.28	2.62	2.91
90	1.01	1.24	1.66	1.93	2.26	2.59	2.88

<u>DURATION IN MINUTES</u>	<u>RETURN PERIODS OF</u>						
	<u>1-YR</u>	<u>2-YR</u>	<u>5-YR</u>	<u>10-YR</u>	<u>25-YR</u>	<u>50-YR</u>	<u>100-YR</u>
91	1.00	1.23	1.65	1.92	2.24	2.57	2.86
92	1.00	1.22	1.63	1.90	2.22	2.55	2.83
93	0.99	1.21	1.62	1.89	2.20	2.53	2.81
94	0.98	1.20	1.61	1.87	2.19	2.51	2.79
95	0.97	1.19	1.59	1.85	2.17	2.49	2.76
96	0.96	1.18	1.58	1.84	2.15	2.46	2.74
97	0.96	1.17	1.57	1.82	2.13	2.44	2.72
98	0.95	1.16	1.56	1.81	2.12	2.42	2.70
99	0.94	1.15	1.54	1.80	2.10	2.41	2.67
100	0.93	1.14	1.53	1.78	2.08	2.39	2.65
101	0.93	1.13	1.52	1.77	2.07	2.39	2.65
102	0.92	1.13	1.51	1.75	2.05	2.35	2.61
103	0.91	1.12	1.50	1.74	2.04	2.33	2.59
104	0.90	1.11	1.49	1.73	2.02	2.31	2.57
105	0.90	1.10	1.47	1.72	2.01	2.30	2.55
106	0.89	1.09	1.46	1.70	1.99	2.28	2.54
107	0.88	1.09	1.45	1.69	1.98	2.26	2.52
108	0.88	1.08	1.44	1.68	1.96	2.25	2.50
109	0.87	1.07	1.43	1.67	1.95	2.23	2.48
110	0.87	1.06	1.42	1.65	1.93	2.21	2.46
111	0.86	1.06	1.41	1.64	1.92	2.20	2.45
112	0.85	1.05	1.40	1.63	1.91	2.18	2.43
113	0.85	1.04	1.39	1.62	1.89	2.17	2.41
114	0.84	1.03	1.38	1.61	1.88	2.15	2.40
115	0.84	1.03	1.37	1.60	1.87	2.14	2.38
116	0.83	1.02	1.36	1.59	1.86	2.12	2.36
117	0.82	1.01	1.36	1.58	1.84	2.11	2.35
118	0.82	1.01	1.35	1.57	1.83	2.09	2.33
119	0.81	1.00	1.34	1.56	1.82	2.08	2.32
120	0.81	0.99	1.33	1.55	1.81	2.07	2.30

<u>DURATION IN HOURS</u>	<u>RETURN PERIODS OF</u>						
	<u>1-YR</u>	<u>2-YR</u>	<u>5-YR</u>	<u>10-YR</u>	<u>25-YR</u>	<u>50-YR</u>	<u>100-YR</u>
2	0.81	0.99	1.33	1.55	1.81	2.07	2.30
3	0.59	0.72	0.97	1.13	1.32	1.51	1.68
4	0.47	0.58	0.78	0.91	1.06	1.21	1.35
5	0.40	0.49	0.66	0.77	0.89	1.02	1.14
6	0.35	0.42	0.57	0.67	0.78	0.89	0.99
8	0.28	0.34	0.46	0.53	0.62	0.71	0.79
10	0.23	0.29	0.39	0.45	0.52	0.60	0.67
12	0.20	0.25	0.33	0.39	0.45	0.52	0.58
18	0.15	0.18	0.24	0.28	0.33	0.38	0.42
24	0.12	0.15	0.20	0.23	0.27	0.31	0.34



DEF. SKETCH, C.D.W. TYPE 1A INLET

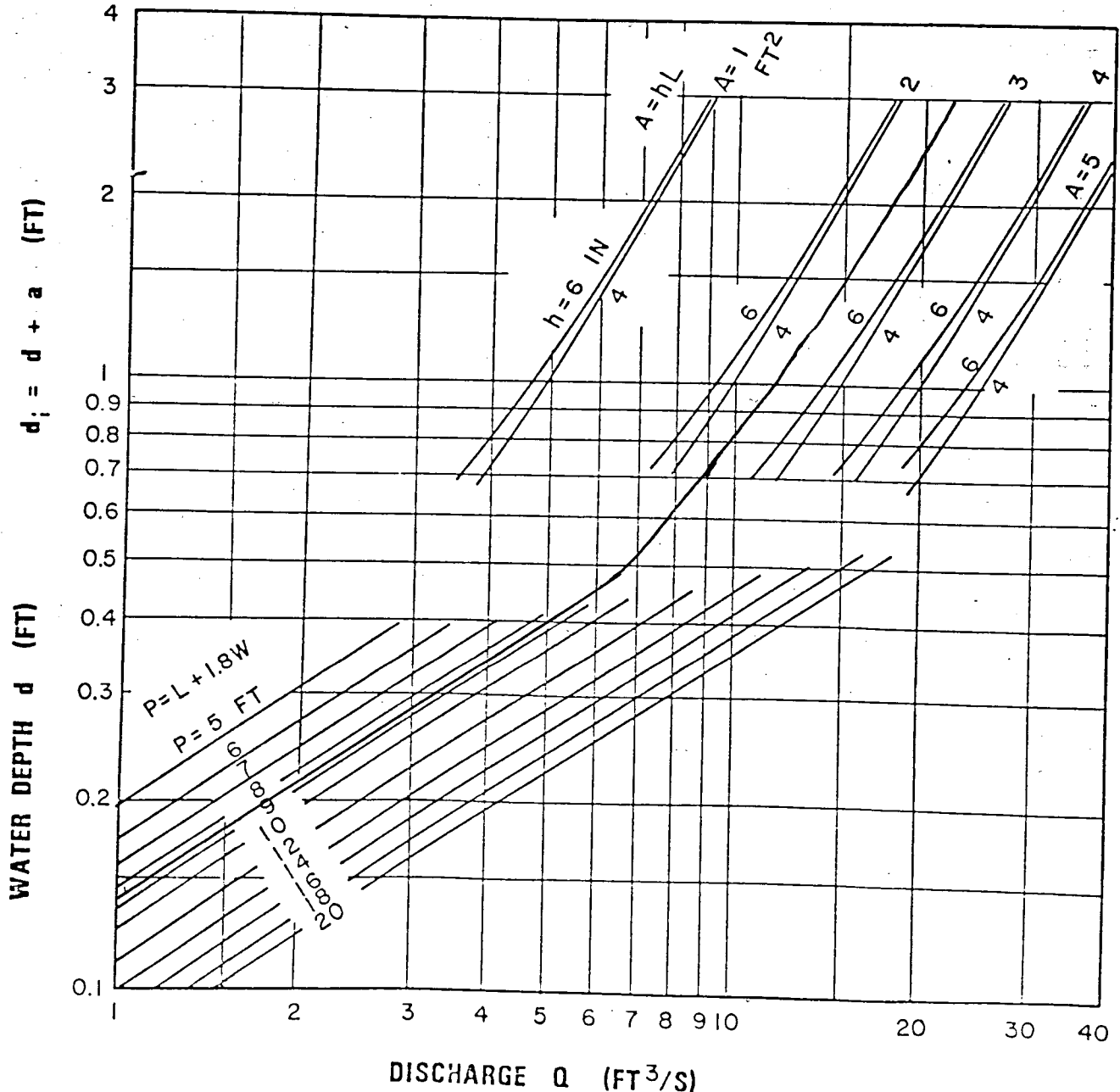
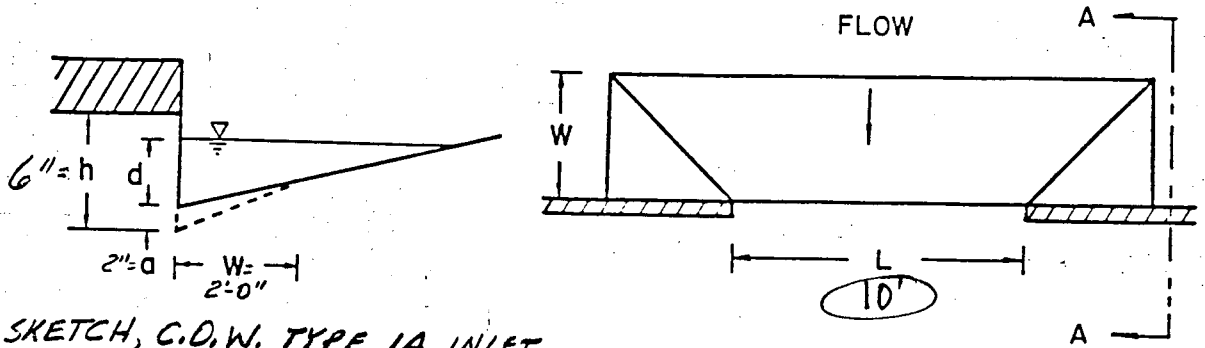


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, FHWA, MAR, 1974



DEF. SKETCH, C.D.W. TYPE 1A INLET

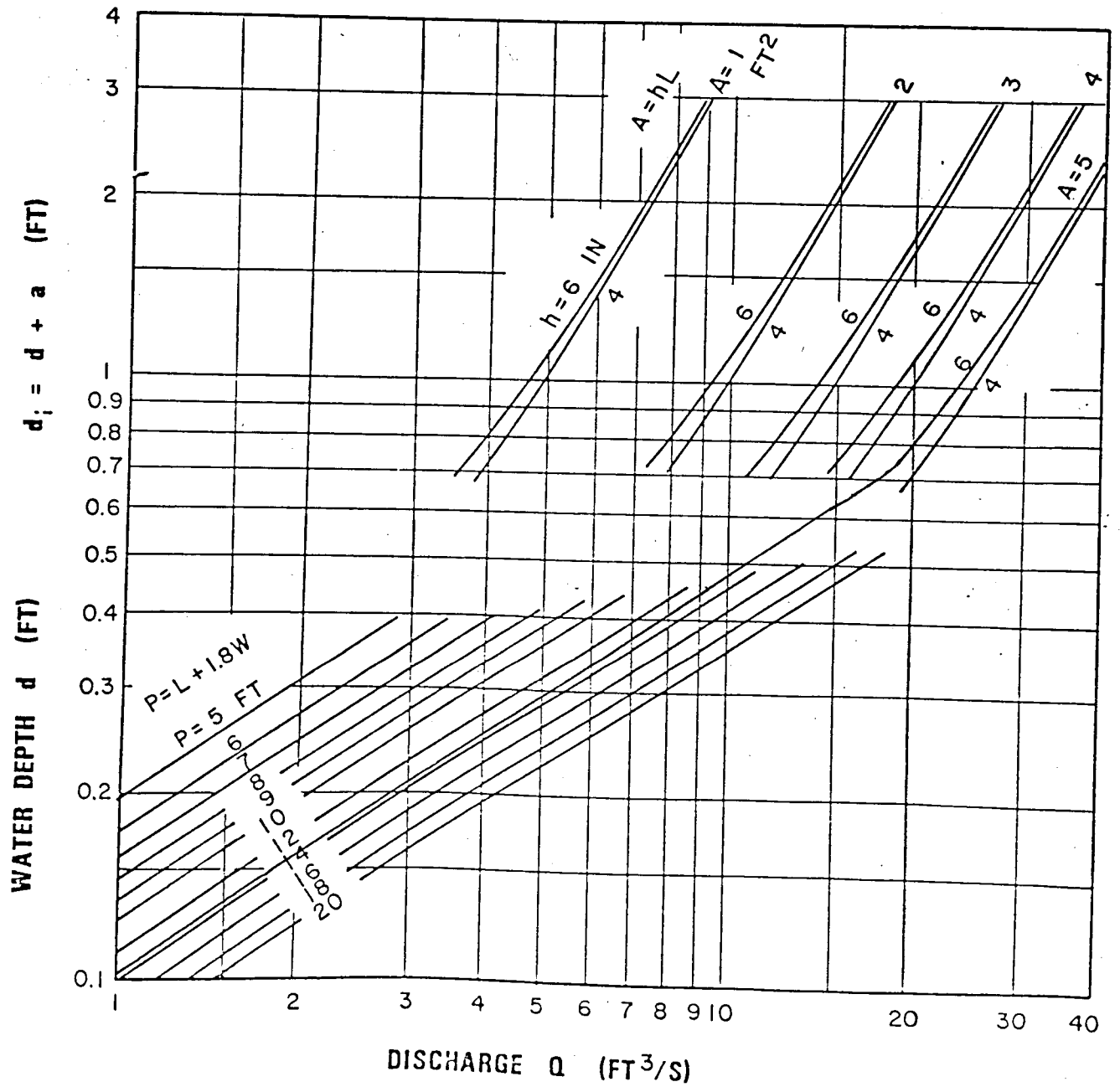
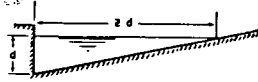


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR., 1974

Chart 1

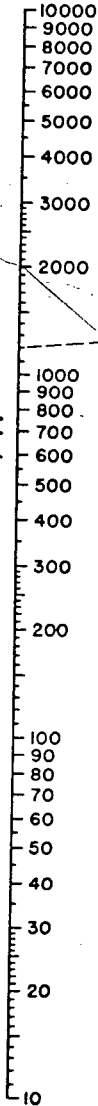


EQUATION:  $Q = 0.56 \left(\frac{z}{n}\right) S^{1/2} d^{2.48}$   
 $n$  IS ROUGHNESS COEFFICIENT IN MANNING  
 FORMULA APPROPRIATE TO MATERIAL IN  
 BOTTOM OF CHANNEL  
 $z$  IS RECIPROCAL OF CROSS SLOPE  
 REFERENCE: H. R. B. PROCEEDINGS 1948,  
 PAGE 150, EQUATION (14)

EXAMPLE (SEE INSTRUCTION 1)

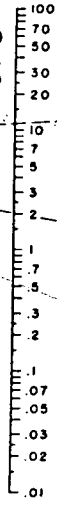
GIVEN:  $S = 0.03$   
 $z = 24$   
 $n = .02$  }  $z/n = 1200$   
 $Q = 20 \text{ CFS}$   
 FIND:  $d = 0.22$  BY FOLLOWING  
 DASHED LINES

RATIO  $z/n$

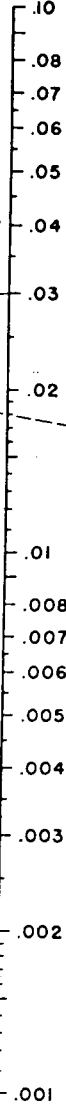


TURNING LINE

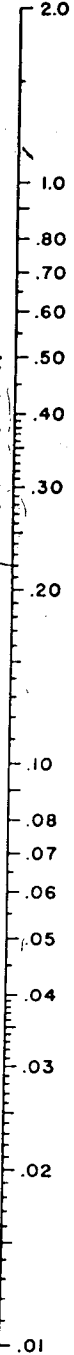
DISCHARGE (Q) IN CFS



SLOPE OF CHANNEL (S) IN FT./FT.



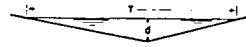
DEPTH AT CURB OR DEEPEST POINT (d) IN FT.



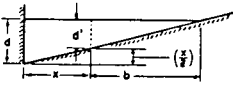
INSTRUCTIONS

1. CONNECT  $z/n$  RATIO WITH SLOPE (S) AND CONNECT DISCHARGE (Q) WITH POINT WHERE LINE CROSSES TURNING LINE. READ DEPTH AT CURB (d). Q CAN BE FOUND FROM d BY CONNECTING d WITH CROSSING OF TURNING LINE.

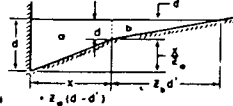
2. FOR SHALLOW V-SHAPED CHANNEL AS SHOWN USE NOMOGRAPH AS EXPLAINED IN INSTRUCTION 1 BUT WITH  $z = \frac{T}{d}$ .



3. TO DETERMINE DISCHARGE  $Q_x$  IN PORTION OF CHANNEL HAVING WIDTH  $x$ : DETERMINE DEPTH  $d$  FOR TOTAL DISCHARGE IN ENTIRE SECTION AS EXPLAINED IN 1. THEN USE NOMOGRAPH TO DETERMINE  $Q_b$  IN SECTION OF WIDTH  $d$  FOR DEPTH  $d' = d - \left(\frac{x}{z}\right)$ . THEN  $Q_x = Q - Q_b$ .



4. TO DETERMINE DISCHARGE ( $Q_c$ ) IN COMPOSITE SECTION: FOLLOW INSTRUCTION 3. TO OBTAIN DISCHARGE ( $Q_b$ ) IN SECTION  $b$  AT ASSUMED DEPTH  $d$  BASED ON AN EXTENSION OF SLOPE RATIO  $z_a$  TO INTERSECT WATER SURFACE; OBTAIN  $Q_b$  FOR SLOPE RATIO  $z_b$  AND DEPTH  $d'$ ;  $d' = d - \frac{a}{z_b}$ . THEN  $Q_c = Q_b + Q_b$ .





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Project Cross Creek

Item Drainage

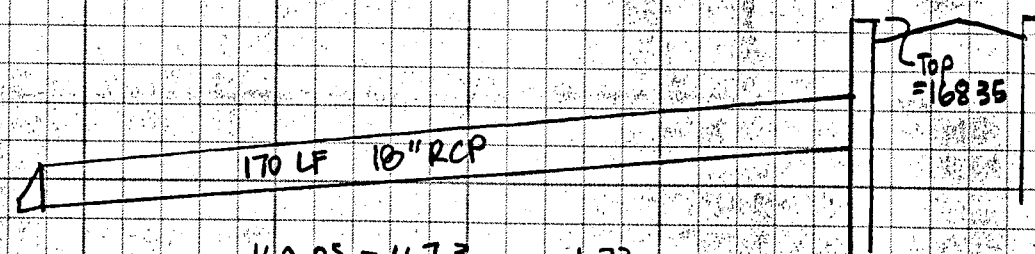
**PROBLEM:** Investigate possibility of reducing drainage easement width from 20' to 10' in Lot Block Cross Creek in order to increase buildable area.

**PROCEDURE:** In order to reduce easement size, overland flow must be decreased and/or section of overflow swale altered.

To decrease flow in overflow channel, the flow in pipe must be increased.

Flow in pipe can be increased with increased head @ inlets: Max HW<sub>100</sub> = 169.0'

$Q_{DWS_{100}} = 167.3$



Hyd. Slope =  $\frac{169.05 - 167.3}{170} = \frac{1.73}{170} = 1.02\%$

Capacity of 18" @ 1.02% = 12 cfs

Determine  $Q_{100}$  approaching sump.

$Q_{100 \text{ total}} = 12.8 + 36.2 + 37.3 + 8.9 + 14.5 = 109.7$

$Q_{\text{entering SWS Line \#1}} = 29 \text{ cfs}$  (from p. 2)

$Q_{\text{approaching sump}} = 109.7 - 29 = 80.7 \text{ cfs}$

$Q_{\text{overflow channel}} = 80.7 - 12 \text{ cfs (pipe)} = 67.3 \text{ cfs}$



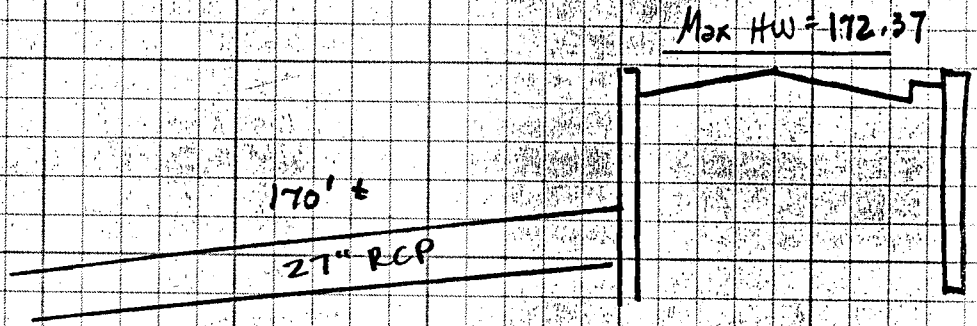
Date 4.27.89 Page 2 of 7

Project CROSS CREEK

Item DRAINAGE

Line #1 Analysis

DWS<sub>100</sub> = 169.3



$$\text{Hyd. Slope} = \frac{172.37 - 169.3}{170} = \frac{3.07}{170} = 1.81\%$$

Capacity of 27" SWS = 41 cfs

Check capacity of 3 inlets (1-10' + 2-5')

10' Top = 172.87  
 HW<sub>max</sub> = 172.37  
 d = 11" - 0.5' = 0.42' ∴ cap. = 9 cfs

5' Top = 172.50  
 HW<sub>max</sub> = 172.37  
 d = 11" - 0.13' = 0.79' ∴ cap. = 10 cfs

Total inlet capacity Line #1 = 9 + 10 + 10 = 29 cfs

Total intercept = smaller of inlet cap or pipe  
 " " = 29 cfs.

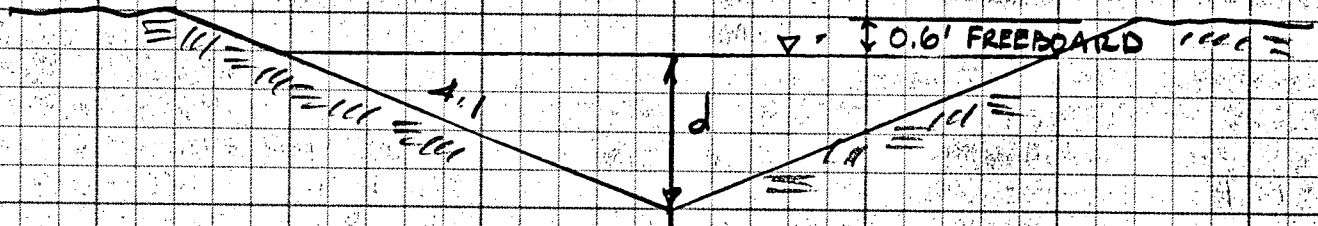


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Project CROSS CREEK

Item DRAINAGE

Design Section for 67.3 cfs  
channel slope = 1%



Manning's Eq'n -  $Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$   
 $67.3 = \frac{1.486}{0.03} A R^{2/3} (0.01)^{1/2}$   
 $A R^{2/3} = 13.59$

<u>d</u>	<u>A</u>	<u>P</u>	<u>R</u>	<u>R<sup>2/3</sup></u>	<u>A R<sup>2/3</sup></u>
2.0	16.00	16.49	0.97	0.98	15.68
1.9	14.44	15.67	0.92	0.95	13.68 ←

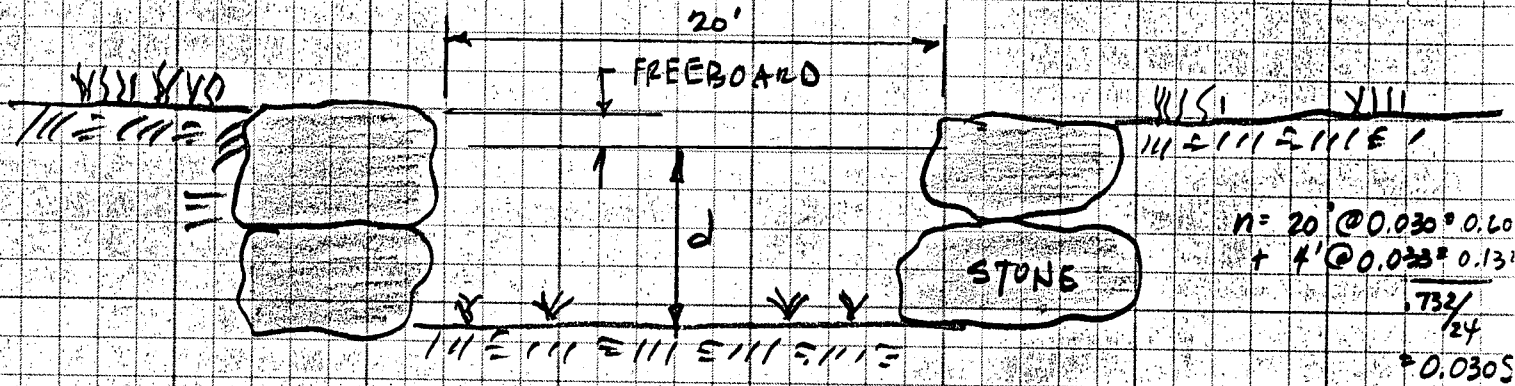
USE  $d = 1.9$

$V = Q/A = 67.3/14.4 = 4.67$   
 $W = (1.9 + 0.6) \times 4 \times 2 = 20.0$



Date 4.27.89 Page 4 of 7  
 Project CROSS CREEK  
 Item DRAINAGE

ALTERNATE SECTION



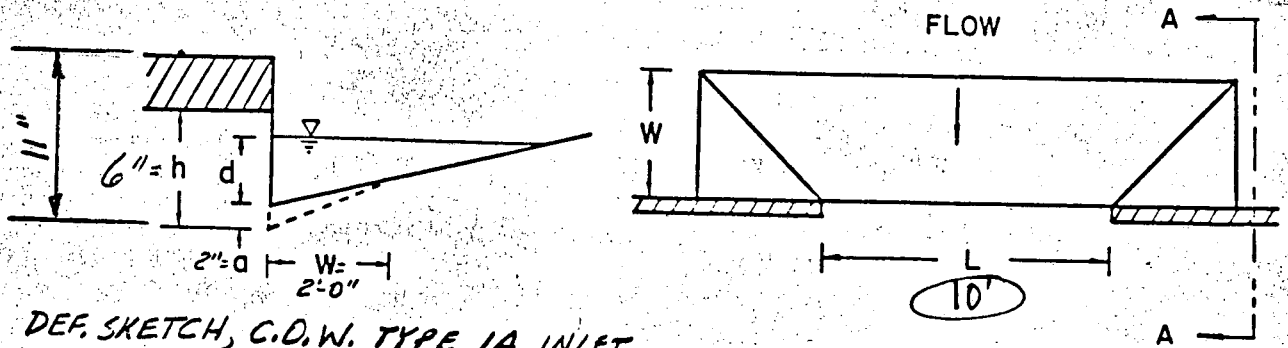
Mannings Eq'n  $Q = \frac{1.486}{n} AR^{2/3} S^{1/2}$   
 $67.3 = \frac{1.486}{0.0305} AR^{2/3} (0.01)^{1/2}$   
 $67.3 = 48.72 \times AR^{2/3} \times 0.10$   
 $AR^{2/3} = 13.81$

d	A	P	R	R <sup>2/3</sup>	AR <sup>2/3</sup>
1.0	20.0	22.0	0.909	0.938	18.76
0.9	18.0	21.8	0.826	0.880	15.8
0.8	16.0	21.6	0.741	0.819	13.10

USE  $d = 0.8'$   $V = Q/A = 67.3/16 = 4.2$



6/7



DEF. SKETCH, C.D.W. TYPE 1A INLET

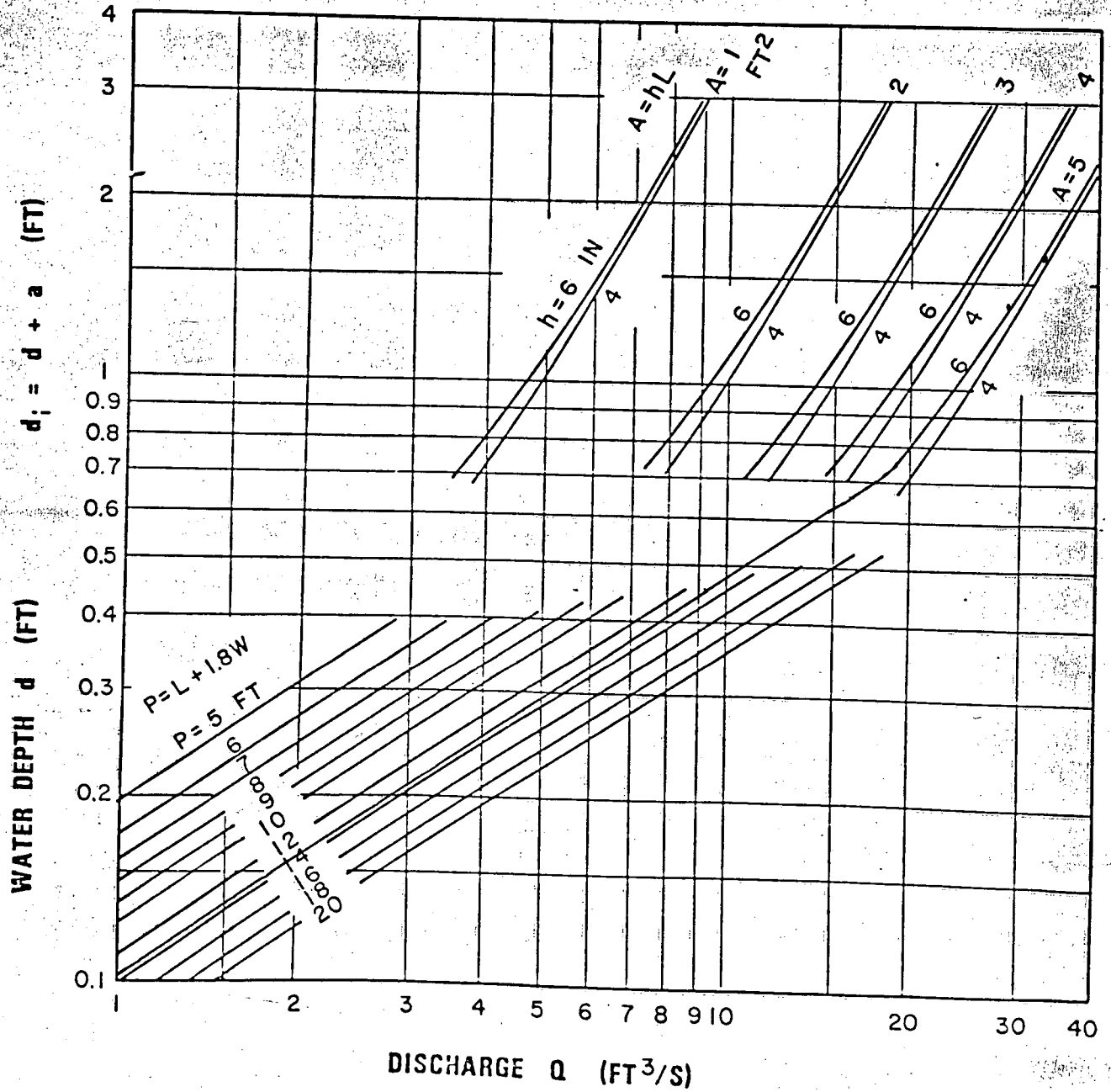
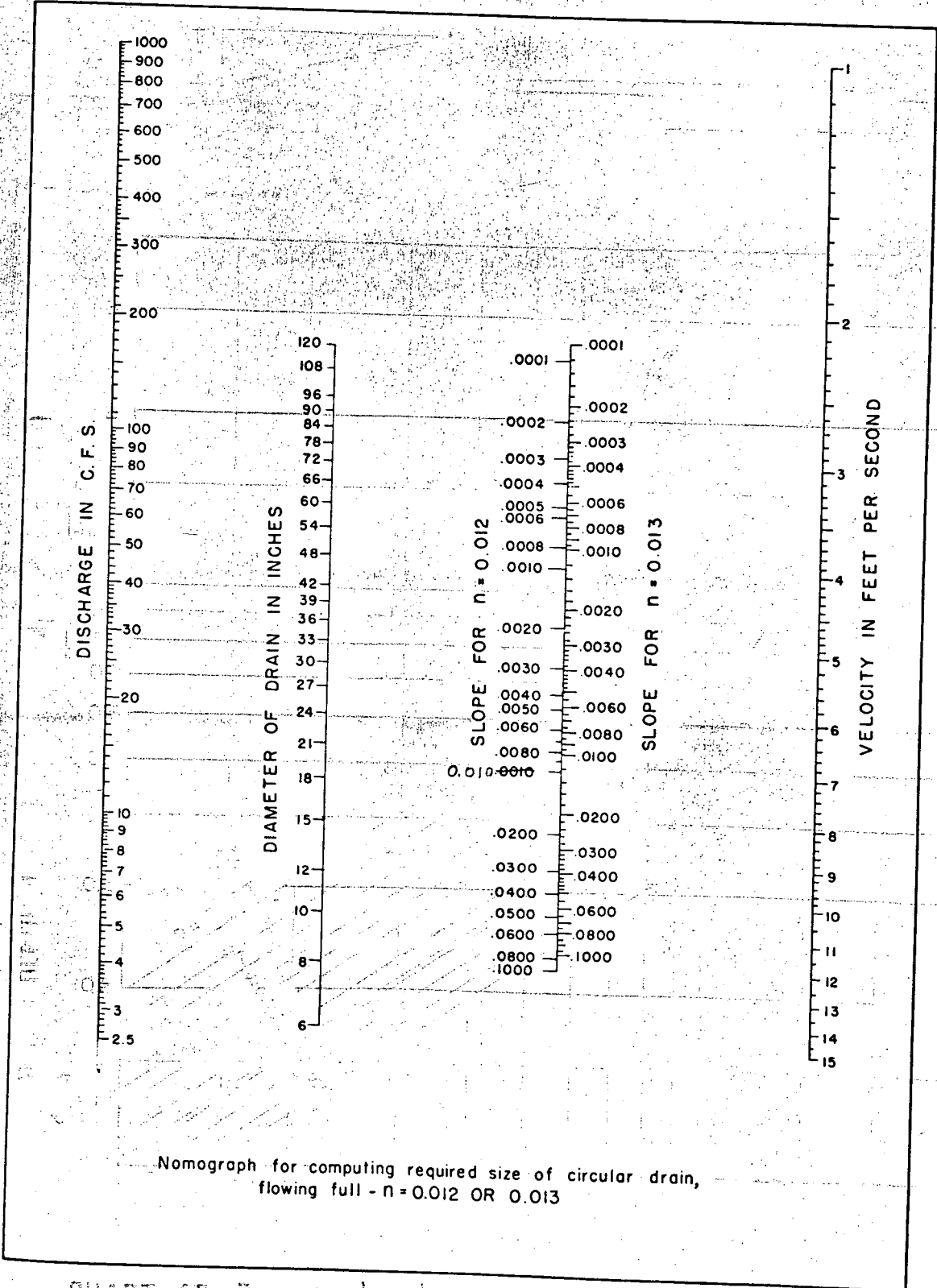


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR., 1974



Nomograph for computing required size of circular drain, flowing full - n = 0.012 OR 0.013