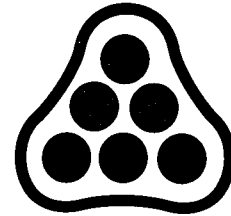
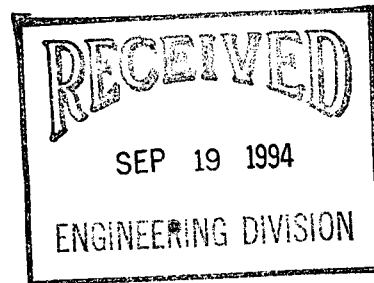


September 14, 1994

Mr. Michael E. Lindebak, P.E.
City Engineer
City Hall - 7th Floor
455 N. Main
Wichita, KS 67202



PROFESSIONAL
ENGINEERING
CONSULTANTS
PROFESSIONAL ASSOCIATION

Reference: Construction of Dam in Reserve "A" of Fairfield Estates
PEC Project No. 36-94575-2185

Dear Mr. Lindebak:

In a May 26, 1994 letter to Mr. Ralph Rudy, Polo Sales Center and Development, you informed Mr. Rudy that the dam structure built in Reserve "A" is not in compliance with the submitted drainage plan, and may increase the risk of flooding in the area. In accordance with your request, Mr. Rudy hired PEC to perform a hydraulic analysis of the drainage system as it was constructed.

The addition of the dam clearly altered the stage-storage and stage-discharge relationships of the drainage system. A field survey of the reserve and drainage structures was performed to provide the information necessary to redefine the aforementioned relationships, which were subsequently revised in the original HEC-1 data input. An as-built contour map of the reserve is enclosed as Figure 1. The HEC-1 program was then run with the new storage and discharge data. The output from the original HEC-1 run and the revised run are included herein. The results of the revised run are as follows:

1. The peak 100-year discharge for developed conditions increased from 427 cfs to 455 cfs. However, this peak is still less than 493 cfs, the 100-year peak for pre-developed conditions.
2. The 100-year design water surface elevation rose from 171.04 to 171.46 feet, city datum.

The low-opening of houses surrounding the pond were also surveyed. The lowest opening was found to be 173.09. This allows 1.63 feet of freeboard between the 100-year high water elevation and minimum opening on this house.

September 14, 1994
PEC Project No. 36-94575-2185
Page 2

Given that the dam did not increase the post-developed flow beyond the pre-developed discharge, and that the 100-year water elevation did not rise significantly, we request that the dam be allowed to stay without any liability for damages caused by flooding being placed on Mr. Rudy.

Very truly yours,

PROFESSIONAL ENGINEERING CONSULTANTS, P.A.

Darwin R. Cronk

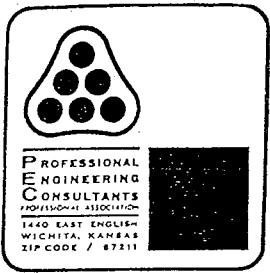
Darwin R. Cronk, E.I.T.
Design Engineer

DRC:blc

cc: Ralph Rudy
Bruce Pedersen, President
Hartmoor Homeowner Association
1308 N. Rutland Circle
Wichita, KS 67206

Encl.: As noted

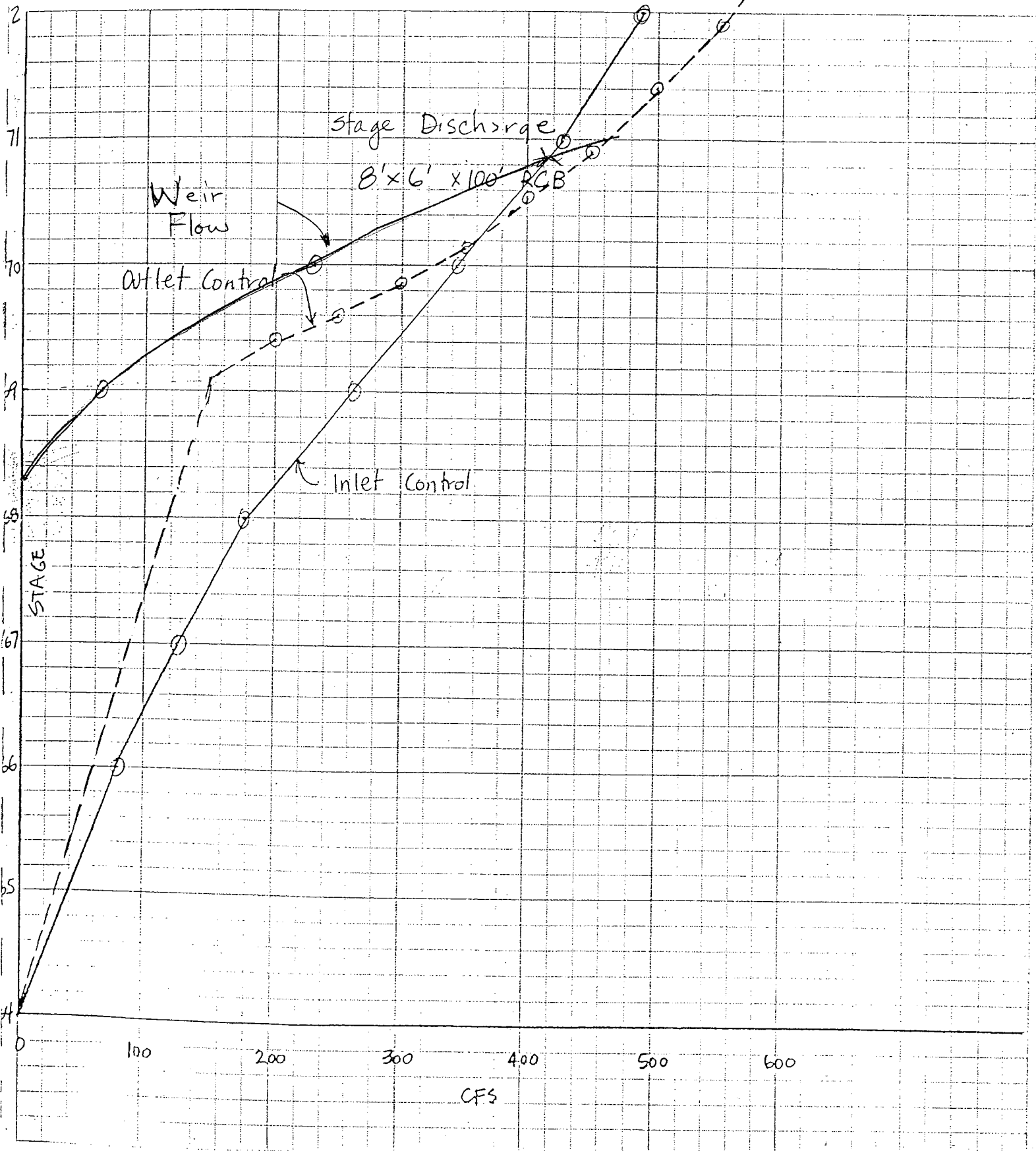
All lots ^{around the lake} have been developed.



Date 11-8-85 Page 21 of 36

Project Fairfield Estates

Item Detention Area #3



PROPOSED
 8x6x100' RCB
 PAGE 1 W/ HYD.

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41						
1	ID	1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41						
1	ID	FAIRFIELD	ESTATES	DRAINAGE	PLAN																																											
2	ID	DETENTION	AREA	NO	3	RESERVOIR	STAGE	STORAGE	ANALYSIS																																							
3	ID	100	YEAR	DEVELOPED	CONDITIONS																																											
4	IT	6	08NOV85	800	0	08NOV85	1600																																									
5	IO	4	0	0																																												
6	JP	1																																														
7	KK	1100	OUTFLOW	HYDROGRAPH	FROM	COLEMAN	DETENTION	EASIN																																								
8	IN	15	08NOV85	1100																																												
9	OI	12	13	23	75	210	225	222	207	60	44																																					
10	OI	35	28	24	21	20	18	18	18	18	17																																					
11	OI	17																																														
12	KK	100	SYSTEM	100																																												
13	IN	5	08NOV85	1130																																												
14	OI	0	16	30	44	35	25	15	5	0																																						
15	KK	200	SYSTEM	200																																												
16	OI	0	9	18	27	22	16	10	4	0																																						
17	KK	300	SYSTEM	300																																												
18	OI	0	18	30	44	35	25	15	5	0																																						
19	KK	400	SYSTEM	400																																												
20	OI	0	8	17	25	19	14	9	4	0																																						
21	KK	500	SYSTEM	500																																												
22	OI	0	18	35	54	51	40	28	17	6	0																																					
23	KK	600	SYSTEM	600																																												
24	OI	0	55	120	190	200	155	113	70	27	0																																					
25	KK	900	SYSTEM	900																																												
26	OI	0	20	40	60	47	34	20	7	0																																						
27	KK	1000	SYSTEM	1000																																												
28	OI	0	18	38	56	43	32	19	6	0																																						
29	KK	1200	DEVELOP	COND.	INFLOW	HYDROGRAPH	COMBINATION																																									
30	KO	3	2	0	0	0	0	0	0	0																																						
31	HC	9	0																																													
32	KK	1300	STAGE-STORAGE	THRU	RESERVOIR																																											
33	KM	PLAN1	8'X4'	X100'	RCB	OUTFALL																																										
34	KP	1																																														
35	KO	3	2	0	0	0	0	0	0	0																																						
36	RS	1	ELEV	164.0	0																																											
37	SA	0	0.07	0.19	0.41	0.94	1.52	1.96	2.18	2.36																																						
38	SO	0	29	58	88	120	150	325	425	490																																						
39	SE	164	165	166	167	168	169	170	171	172																																						
40	SS	172	250	3.0	1.5																																											
41	ZZ																																															

ORIGINAL
 HEC-1 RUN

 FLOOD HYDROGRAPH PACKAGE HEC-1 (JEM XI 512K VERSION) -OCT 1, 1984
 U.S. ARMY CORPS OF ENGINEERS, THE HYDROLOGIC ENGINEERING CENTER, 609 SECOND STREET, DAVIS, CA. 95616

FAIRFIELD ESTATES DRAINAGE PLAN
 DETENTION AREA NO 3 RESERVOIR STAGE STORAGE ANALYSIS
 100 YEAR DEVELOPED CONDITIONS

5 IO OUTPUT CONTROL VARIABLES

IFRNT 4 PRINT CONTROL
 IFLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

11 HYDROGRAPH TIME DATA

NMIN 6 MINUTES IN COMPUTATION INTERVAL
 IDATE ENOV85 STARTING DATE
 ITIME 0800 STARTING TIME
 NQ 81 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE ENOV85 ENDING DATE
 NDTIME 1600 ENDING TIME

COMPUTATION INTERVAL .10 HOURS
 TOTAL TIME BASE 8.00 HOURS

ENGLISH UNITS

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF RUNOFF
 1.00

 *

7 KK * 1100 * OUTFLOW HYDROGRAPH FROM COLEMAN DETENTION BASIN
 *

8 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 15 TIME INTERVAL IN MINUTES
 JXDATE ENOV85 STARTING DATE
 JXTIME 1100 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
 TAREA .00 SUBBASIN AREA

24/36

12 KK * 100 * SYSTEM 100

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA .00 SUBBASIN AREA

15 KK * 200 * SYSTEM 200

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA .00 SUBBASIN AREA

17 KK * 300 * SYSTEM 300

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA .00 SUBBASIN AREA

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA .00 SUBBASIN AREA

0 BA SUBBASIN CHARACTERISTICS
TAREA .00 SUBBASIN AREA

*** ** ** ** **

 * * * * *
 19 KK * * * * * SYSTEM 400
 * * * * *

13 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 5 TIME INTERVAL IN MINUTES
 JXDATE NOV85 STARTING DATE
 JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA
 SUBBASIN CHARACTERISTICS
 TAREA .00 SUBBASIN AREA

 * * * * *
 21 KK * * * * * SYSTEM 500
 * * * * *

13 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 5 TIME INTERVAL IN MINUTES
 JXDATE NOV85 STARTING DATE
 JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA
 SUBBASIN CHARACTERISTICS
 TAREA .00 SUBBASIN AREA

 * * * * *
 23 KK * * * * * SYSTEM 600
 * * * * *

13 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 5 TIME INTERVAL IN MINUTES
 JXDATE NOV85 STARTING DATE
 JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA
 SUBBASIN CHARACTERISTICS
 TAREA .00 SUBBASIN AREA

*** ** ** **

0 BA SUBBASIN CHARACTERISTICS .00 SUBBASIN AREA

25 KK * 900 * SYSTEM 900

13 IN TIME DATA FOR INPUT TIME SERIES

JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS .00 SUBBASIN AREA

27 KK * 1000 * SYSTEM 1000

13 IN TIME DATA FOR INPUT TIME SERIES

JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS .00 SUBBASIN AREA

29 KK * 1200 * DEVELOP COND. INFLOW HYDROGRAPH COMBINATION

30 KO OUTPUT CONTROL VARIABLES

IPRNT 3 PRINT CONTROL
IPLUT 2 PLOT CONTROL

27/36

HYDROGRAPH AT STATION 1200
FOR PLAN 1, RATIO = 1.00

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	24-HR (INCHES) (AC-FT)	72-HR (CFS)	8.00-HR (CFS)
559.	3.80	125.	97.	97.	97.
		.000	.000	.000	.000
		62.	64.	64.	64.

CUMULATIVE AREA = .00 SQ MI

STATION 1200

DAHRMN PER	(0) OUTFLOW												
	0.	50.	100.	150.	200.	250.	300.	350.	400.	450.	500.	550.	600.
80800	1.0												
80806	2.0												
80812	3.0												
80818	4.0												
80824	5.0												
80830	6.0												
80836	7.0												
80842	8.0												
80848	9.0												
80854	10.0												
80900	11.0												
80906	12.0												
80912	13.0												
80918	14.0												
80924	15.0												
80930	16.0												
80936	17.0												
80942	18.0												
80948	19.0												
80954	20.0												
81000	21.0												
81006	22.0												
81012	23.0												
81018	24.0												
81024	25.0												
81030	26.0												
81036	27.0												
81042	28.0												
81048	29.0												
81054	30.0												
81100	31.0												
81106	32.0												
81112	33.0												
81118	34.0												
81124	35.0												
81130	36.0												
81136	37.0												
81142	38.0												
81148	39.0												
81154	40.0												
81200	41.0												
81206	42.0												
81212	43.0												
81218	44.0												
81224	45.0												
81230	46.0												
81236	47.0												
81242	48.0												
81248	49.0												
81254	50.0												
81300	51.0												
81306	52.0												
81312	53.0												
81318	54.0												
81324	55.0												
81330	56.0												

 * * * 1300 * STAGE-STORAGE THRU RESERVOIR
 * * *

FLAN1 8'X6'X100' RCB OUTFALL
 *** ** ** ** ** ** ** ** ** ** ** **
 34 KP PLAN 1 FOR STATION 1300 STAGE-STORAGE THRU RESERVOIR
 35 KO OUTPUT CONTROL VARIABLES

3 PRINT CONTROL
 2 FLOT CONTROL
 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA
 STORAGE ROUTING
 NSTPS
 ITYP 1 NUMBER OF SUBREACHES
 ELEV TYPE OF INITIAL CONDITION
 RSVRIC 164.00 INITIAL CONDITION
 X .00 WORKING R AND D COEFFICIENT

STATION	AREA	DISCHARGE	ELEVATION	SPILLWAY	CREL	SPWID	CUBW	EXPW
37 SA	.0	.1	.2	.4	.9	1.5	2.0	2.4
38 SO	0.	29.	58.	88.	120.	150.	325.	490.
39 SE	164.00	165.00	166.00	167.00	168.00	169.00	170.00	171.00
40 SS	172.00	173.00	174.00	175.00	176.00	177.00	178.00	179.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE ELEVATION	OUTFLOW ELEVATION	STORAGE	OUTFLOW
.00	.02	.15	.44
164.00	165.00	166.00	167.00
168.00	169.00	170.00	171.00
171.00	172.00	173.00	174.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	OUTFLOW	STORAGE	OUTFLOW
.00	.02	.15	.44
29.00	58.00	88.00	120.00
165.00	166.00	167.00	168.00
169.00	170.00	171.00	172.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 0. TO 29.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

31/36

HYDROGRAPH AT STATION 1300
FOR PLAN 1, RATIO = 1.00

32/36

PEAK FLOW (CFS)	TIME (HR)	5-HR MAXIMUM AVERAGE FLOW	24-HR MAXIMUM AVERAGE FLOW	72-HR MAXIMUM AVERAGE FLOW	8.00-HR MAXIMUM AVERAGE STORAGE
427.	4.00	125.	97.	97.	97.
		.000	.000	.000	.000
		62.	64.	64.	64.
PEAK STORAGE (AC-FT)	6.	6-HR 1.	24-HR 1.	72-HR 1.	8.00-HR 1.
PEAK STAGE (FEET)	4.00	6-HR 167.17	24-HR 166.48	72-HR 166.48	8.00-HR 166.48
CUMULATIVE AREA		.00 SQ MI			
1					
2					
3					
4					
5					
6					
7					
8					
9					
10					
11					
12					
13					
14					
15					
16					
17					
18					
19					
20					
21					
22					
23					
24					
25					
26					
27					
28					
29					
30					
31					
32					
33					
34					
35					
36					
37					
38					
39					
40					
41					
42					
43					
44					
45					
46					
47					
48					
49					
50					
51					
52					
53					
54					
55					
56					
57					

STATION 1300

DAHRMN PER	100.		200.		300.		400.		500.		600.		0.		0.		0.		0.	
	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW	(I) INFLOW	(O) OUTFLOW
80800	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.	0.
80805	101																			
80812	2.10																			
80818	3.1																			
80824	4.10																			
80830	5.1																			
80836	6.1																			
80842	7.1																			
80848	8.1																			
80854	9.1																			
80900	10.1																			
80906	11.1																			
80912	12.1																			
80918	13.1																			
80924	14.1																			
80930	15.1																			
80936	16.1																			
80942	17.1																			
80948	18.1																			
80954	19.1																			
81000	20.1																			
81006	21.1																			
81012	22.1																			
81018	23.1																			
81024	24.1																			
81030	25.1																			
81036	26.1																			
81042	27.1																			
81048	28.1																			
81054	29.1																			
81100	30.1																			
81106	31.1																			
81112	32.1																			
81118	33.1																			
81124	34.01																			
81130	35.1																			
81136	36.1																			
81142	37.1																			
81148	38.																			
81154	39.																			
81200	40.																			
81206	41.																			
81212	42.																			
81218	43.																			
81224	44.																			
81230	45.																			
81236	46.																			
81242	47.																			
81248	48.																			
81254	49.																			
81300	50.																			
81306	51.																			
81312	52.																			
81318	53.																			
81324	54.																			
81330	55.																			

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

RATIOS APPLIED TO FLOWS

RATIO 1
1.00

OPERATION STATION AREA PLAN

HYDROGRAPH AT 1100 .00 1 FLOW TIME 224. 4.30

HYDROGRAPH AT 100 .00 1 FLOW TIME 39. 3.80

HYDROGRAPH AT 200 .00 1 FLOW TIME 24. 3.80

HYDROGRAPH AT 300 .00 1 FLOW TIME 24. 3.80

HYDROGRAPH AT 400 .00 1 FLOW TIME 21. 3.80

HYDROGRAPH AT 500 .00 1 FLOW TIME 52. 3.80

HYDROGRAPH AT 600 .00 1 FLOW TIME 196. 3.80

HYDROGRAPH AT 900 .00 1 FLOW TIME 52. 3.80

HYDROGRAPH AT 1000 .00 1 FLOW TIME 48. 3.80

9 COMBINED AT 1200 .00 1 FLOW TIME 559. 3.80

ROUTED TO 1300 .00 1 FLOW TIME 427. 4.00

** PEAK STAGES IN FEET **
 1 STAGE 171.04
 TIME 4.00

REVISED
HEC-1 RUN

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* Lahey F77L-EM/32 version 5.01 *
* Dodson & Associates, Inc. *
* RUN DATE 09/12/94 TIME 16:44:09 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *

```

X X XXXXXX XXXX X
X X X X X XX
X X X X X X
XXXXXX XXXX X XXXX X
X X X X X X
X X X X X X
X X XXXXXX XXXX XXX
    
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	FAIRFIELD ESTATES DRAINAGE STUDY 9-13-94									
2	ID	REVISED FROM FAIRFIELD ESTATES DRAINAGE PLAN 1985									
3	ID	100 YEAR DEVELOPED CONDITIONS									
4	IT	6	08NOV85	800	0	08NOV85	1600				
5	IO	4	0	0							
6	JP	1									
7	KK	1100	OUTFLOW HYDROGRAPH FROM COLEMAN DETENTION BASIN								
8	IN	15	08NOV85	1100							
9	QI	12	13	23	75	210	225	222	207	60	44
10	QI	35	28	24	21	20	18	18	18	18	17
11	QI	17									
12	KK	100	SYSTEM 100								
13	IN	5	08NOV85	1130							
14	QI	0	16	30	44	35	25	15	5	0	
15	KK	200	SYSTEM 200								
16	QI	0	9	18	27	22	16	10	4	0	
17	KK	300	SYSTEM 300								
18	QI	0	9	18	27	22	16	10	4	0	
19	KK	400	SYSTEM 400								
20	QI	0	8	17	25	19	14	9	4	0	
21	KK	500	SYSTEM 500								
22	QI	0	18	35	54	51	40	28	17	6	0
23	KK	600	SYSTEM 600								
24	QI	0	55	120	190	200	155	113	70	27	0
25	KK	900	SYSTEM 900								
26	QI	0	20	40	60	47	34	20	7	0	
27	KK	1000	SYSTEM 1000								
28	QI	0	18	38	56	43	32	19	6	0	
29	KK	1200	DEVELOP COND. INFLOW HYDROGRAPH COMBINATION								
30	KO	3	2								
31	HC	9									

```

32      KK      1300      STAGE-STORAGE THRU RESERVIOR
33      KM      PLAN1    8'x 6' x 100' RCB OUTFALL
34      KP      1
35      KO      3      2
36      RS      1      ELEV 168.0
37      SA      1.22    1.43  1.61  1.79  2.01
38      SE      168    169    170    171    172
39      SQ      0      65    233    425    490
40      SE      168.25  169    170    171    172
41      SS      172    250    3      1.5
42      ZZ
    
```

```

1*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
* Lahey F77L-EM/32 version 5.01
* Dodson & Associates, Inc.
* RUN DATE 09/12/94 TIME 16:44:09
*****
    
```

```

*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
*
*****
    
```

FAIRFIELD ESTATES DRAINAGE STUDY 9-13-94
 REVISED FROM FAIRFIELD ESTATES DRAINAGE PLAN 1985
 100 YEAR DEVELOPED CONDITIONS

```

5 IO      OUTPUT CONTROL VARIABLES
          IPRNT      4 PRINT CONTROL
          IPLOT      0 PLOT CONTROL
          QSCAL      0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN      6 MINUTES IN COMPUTATION INTERVAL
          IDATE      8NOV85 STARTING DATE
          ITIME      0800 STARTING TIME
          NQ         81 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     8NOV85 ENDING DATE
          NDTIME     1600 ENDING TIME
          ICENT      19 CENTURY MARK

          COMPUTATION INTERVAL 0.10 HOURS
          TOTAL TIME BASE 8.00 HOURS
    
```

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

```

JP        MULTI-PLAN OPTION
          NPLAN      1 NUMBER OF PLANS
    
```

```

JR        MULTI-RATIO OPTION
          RATIOS OF RUNOFF
          1.00
    
```

*** **

```

*****
*
* 7 KK      1100      OUTFLOW HYDROGRAPH FROM COLEMAN DETENTION BASIN
*
*****
    
```

```

8 IN      TIME DATA FOR INPUT TIME SERIES
          JXMIN      15 TIME INTERVAL IN MINUTES
          JXDATE     8NOV85 STARTING DATE
          JXTIME     1100 STARTING TIME
    
```

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

* *
12 KK * 100 * SYSTEM 100
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

* *
15 KK * 200 * SYSTEM 200
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

* *
17 KK * 300 * SYSTEM 300
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

19 KK * *
* 400 * SYSTEM 400
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

21 KK * *
* 500 * SYSTEM 500
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

23 KK * *
* 600 * SYSTEM 600
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

25 KK * *
* 900 * SYSTEM 900
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

* *
27 KK * 1000 * SYSTEM 1000
* *

13 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 5 TIME INTERVAL IN MINUTES
JXDATE 8NOV85 STARTING DATE
JXTIME 1130 STARTING TIME

SUBBASIN RUNOFF DATA

0 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

*** **

* *
29 KK * 1200 * DEVELOP COND. INFLOW HYDROGRAPH COMBINATION
* *

30 KO OUTPUT CONTROL VARIABLES
IPRNT 3 PRINT CONTROL
IPLOT 2 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

31 HC HYDROGRAPH COMBINATION
ICOMP 9 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

HYDROGRAPH AT STATION 1200
FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	8.00-HR	
+ 559.	3.80	(CFS)	125.	97.	97.	97.
		(INCHES)	0.000	0.000	0.000	0.000
		(AC-FT)	62.	64.	64.	64.

CUMULATIVE AREA = 0.00 SQ MI

1 STATION 1200

DAHRMN PER	(O) OUTFLOW												
	0.	50.	100.	150.	200.	250.	300.	350.	400.	450.	500.	550.	600.
80800	1.0
80806	2.0
80812	3.0
80818	4.0
80824	5.0
80830	6.0
80836	7.0
80842	8.0
80848	9.0
80854	10.0

32 KK * 1300 * STAGE-STORAGE THRU RESERVIOR
 * *

PLAN1 8'x 6' x 100' RCB OUTFALL

*** **

34 KP PLAN 1 FOR STATION 1300 STAGE-STORAGE THRU RESERVIOR

35 KO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 2 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

HYDROGRAPH ROUTING DATA

36 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP ELEV TYPE OF INITIAL CONDITION
 RSVRIC 168.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

37 SA AREA 1.2 1.4 1.6 1.8 2.0

38 SE ELEVATION 168.00 169.00 170.00 171.00 172.00

39 SQ DISCHARGE 0. 65. 233. 425. 490.

40 SE ELEVATION 168.25 169.00 170.00 171.00 172.00

41 SS SPILLWAY
 CREL 172.00 SPILLWAY CREST ELEVATION
 SPWID 250.00 SPILLWAY WIDTH
 COQW 3.00 WEIR COEFFICIENT
 EXPW 1.50 EXPONENT OF HEAD

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	1.32	2.84	4.54	6.44
ELEVATION	168.00	169.00	170.00	171.00	172.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.31	1.32	2.84	4.54	6.44
OUTFLOW	0.00	0.00	65.00	233.00	425.00	490.00
ELEVATION	168.00	168.25	169.00	170.00	171.00	172.00

*** **

HYDROGRAPH AT STATION 1300
 FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	8.00-HR
455.	4.00	124.	95.	95.	95.
		(INCHES) 0.000	0.000	0.000	0.000
		(AC-FT) 61.	63.	63.	63.

PEAK STORAGE + (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	8.00-HR
5.	4.00	2.	1.	1.	1.

PEAK STAGE + (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	8.00-HR
171.46	4.00	169.27	169.03	169.03	169.03

CUMULATIVE AREA = 0.00 SQ MI

1 STATION 1300

0.	100.	(I) INFLOW, 200.	(O) OUTFLOW 300.	400.	500.	600.	0.	0.	0.	0.	0.	0.
							(S) STORAGE					

DAHRMN PER	0.	0.	0.	0.	0.	0.	0.	2.	4.	6.	0.	0.	0.
80800	10I
80806	20I
80812	30I
80818	40I
80824	50I
80830	6.I
80836	7.I
80842	8.I
80848	9.I
80854	10.I
80900	11.I
80906	12.I
80912	13.I
80918	14.I
80924	15.I
80930	16.I
80936	17.I
80942	18.I
80948	19.I
80954	20.I
81000	21.I
81006	22.I
81012	23.I
81018	24.I
81024	25.I
81030	26.I
81036	27.I
81042	28.I
81048	29.I
81054	30.I
81100	31.I
81106	32.I
81112	33.I
81118	34.OI
81124	35.OI
81130	36. I
81136	37. 0	.	I
81142	38. 0
81148	39. 0
81154	40. 0
81200	41. I.O
81206	42. I.O
81212	43. I
81218	44. I 0
81224	45. IO
81230	46. I
81236	47. I
81242	48. IO
81248	49. IO
81254	50. I 0
81300	51. I 0
81306	52. I O
81312	53. IO
81318	54. I
81324	55. IO
81330	56. I
81336	57. I
81342	58. IO
81348	59. I
81354	60. I
81400	61. I
81406	62. I
81412	63. IO
81418	64. I
81424	65. I
81430	66. I
81436	67. I
81442	68. I
81448	69. I
81454	70. I
81500	71. I
81506	72. I
81512	73. I
81518	74. I
81524	75. I
81530	76. I
81536	77. I
81542	78. I

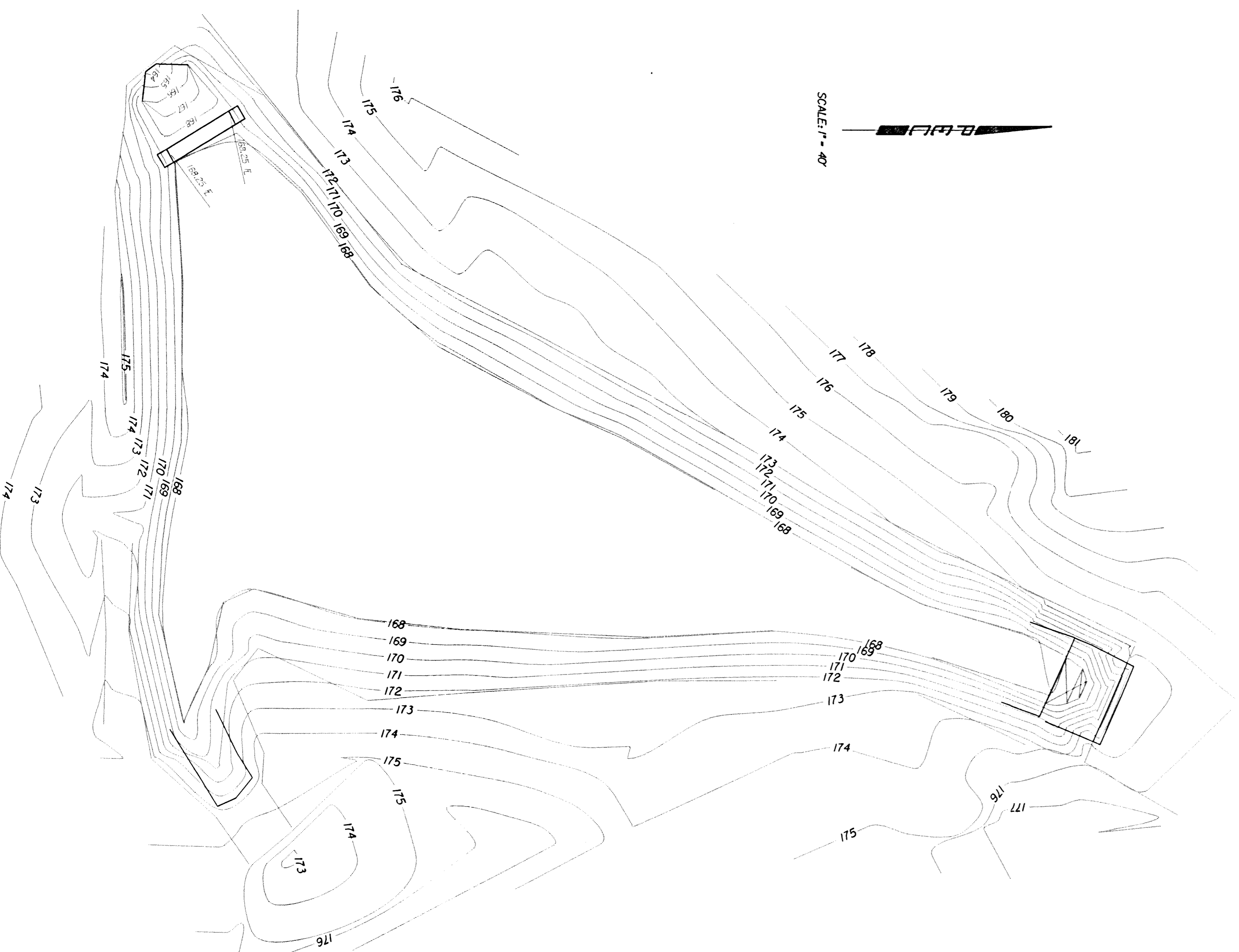
81548 79. I . : : : : : S . : : : : :
 81554 80. I . : : : : : S . : : : : :
 81600 81. I - - - - - S - - - - -

1
1

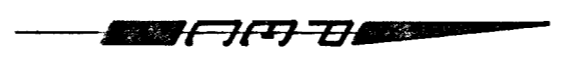
PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO FLOWS	
				RATIO 1	
				1.00	
HYDROGRAPH AT +	1100	0.00	1	FLOW TIME	224. 4.30
HYDROGRAPH AT +	100	0.00	1	FLOW TIME	39. 3.80
HYDROGRAPH AT +	200	0.00	1	FLOW TIME	24. 3.80
HYDROGRAPH AT +	300	0.00	1	FLOW TIME	24. 3.80
HYDROGRAPH AT +	400	0.00	1	FLOW TIME	21. 3.80
HYDROGRAPH AT +	500	0.00	1	FLOW TIME	52. 3.80
HYDROGRAPH AT +	600	0.00	1	FLOW TIME	196. 3.80
HYDROGRAPH AT +	900	0.00	1	FLOW TIME	52. 3.80
HYDROGRAPH AT +	1000	0.00	1	FLOW TIME	48. 3.80
9 COMBINED AT +	1200	0.00	1	FLOW TIME	559. 3.80
ROUTED TO +	1300	0.00	1	FLOW TIME	455. 4.00
** PEAK STAGES IN FEET **					
	1	STAGE	171.46		
		TIME	4.00		

*** NORMAL END OF HEC-1 ***



SCALE: 1" = 40'



CONTOUR MAP
RESERVE A
FAIRFIELD
ESTATES