

Date _____ Page _____ of _____

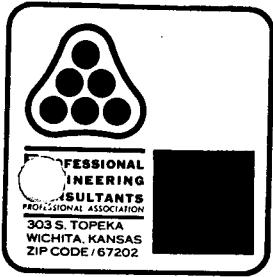
Project HORSESHOE LK.

Item HYDROLOGY

<u>BASIN</u>	<u>AREA (AC.)</u>	<u>C₂</u>	<u>C₁₀₀</u>	<u>L₂</u>	<u>L₁₀₀</u>	<u>Q₂</u>	<u>Q₁₀₀</u>
1A	1.1	0.58	0.72	}	}	2.4	5.8
1B	0.8	0.58	0.72			1.8	4.2
2A	1.15	0.58	0.72			2.5	6.1
2B	1.6	0.58	0.72			3.5	8.5
3A	2.8	0.48	0.68			5.1	14.0
3B	2.8	0.48	0.68			5.1	14.0
4A	1.6	0.48	0.68			2.9	8.0
4B	2.8	0.48	0.68			5.1	14.0

Hydrologic Calculations for Lots 10 thru 14, Block 2, have been omitted due to the unknown nature of how these lots will be developed.

It is assumed that further drainage computation and analysis will be performed at such time as the lots ^{are} to be developed, and will be done so on an individual basis.



Date _____ Page _____ of _____

Project _____

Item STREET FLOW

2-Year

<u>Q</u>	<u>Basin</u>	<u>Slope</u>	<u>d</u>	<u>dmax</u>	
0.7x5.1 = 3.6	70% of 3A	0.43%	0.32'	0.30'	OK
0.6x5.1 = 3.1	60% of 4B	0.36%	0.31'	0.30'	OK

BY INSPECTION, ROLL-TYPE CAN BE USED FOR ALL LOTS.

100-YR.

<u>Basin</u>	<u>Q</u>	<u>Slope</u>	<u>Qmax</u>	
3A-70%	0.7x14 = 9.8	0.43%	27.8	OK

Note: Above Street width uses 29' Bk-Bk, not 35'.
Used to show that most conservative conditions are more than adequate.

By Inspection, 0.3' wk-Gd is OK for all streets.



Date _____ Page _____ of _____

Project _____

Item Inlet/Pipe Sizing

<u>Node</u>	<u>Q₂</u>	<u>Location</u>	<u>Size</u>	<u>d</u>	<u>d_{max}</u>	
301	5.1	Sump	5'	0.4'	0.55'	OK

By Inspection, all inlets will be 5'-Type 1A curb inlets.

Pipes

<u>Node</u>	<u>Q₂</u>	<u>T_c</u>	
100	φ		
101	2.4	15	STARTING HGL = 130
102	1.8	15	
103	42.0	23	

200	φ		
201	2.5	15	starting HGL = 130
202	3.5	15	

300	φ		
301	5.1	15	Starting HGL = 133
302	5.1	15	

Date: 07-24-1995
Time: 10:12:15

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #1
DRC 7-24-95

Input File: horssysl.stm

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

*****										*****										*****				
Tributary Area										Hydrology Summation					Conduit Data									
Node to	C	Area	Slope	Length	TC(0)	I(0)	Q(0)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC								
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)								
*****										*****										*****				
103	102	.00	.00	.00	.0	23.00	3.10	42.00	23.00	3.10	42.00	42.00	42"	4.37	120.00	.46	23.46							
102	101	.00	.00	.00	.0	15.00	3.83	1.80	23.46	3.07	1.44	43.44	42"	4.52	30.00	.11	23.57							
101	100	.00	.00	.00	.0	15.00	3.83	2.40	23.57	3.06	1.92	45.36	42"	4.71	160.00	.57	24.13							
*****										*****										*****				

Date: 07-24-1995
 Time: 10:12:15

Input File: horssysl.stm

HORSESHOE LAKE DRAINAGE PLAN
 SYSTEM #1
 DRC 7-24-95

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

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*****
Node   Hyd-Slope  Friction  Bend   Transition  Manhole  Deflection  Junction  Total   Hyd-Gl   Desired  Diff.
      (Ft/Ft)    (Ft)      (Ft)   (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)
*****
100    .00000     .0000    .0000   .0000       .0000    .0000      .0000    .0000  130.0000  130.0000  .00
101    .00203     .3252    .0000   .0029       .0000    .0000      .0670    .3950  130.3951  139.0000  8.60
102    .00186     .0559    .0000   .0021       .0000    .0000      .0504    .1084  130.5035  139.0000  8.50
103    .00174     .2091    .0000   .0000       .0000    .0000      .0000    .2091  130.7126  135.1000  4.39
*****
    
```

07-24-1995

Date: 07-24-1995
Time: 10:28:12

Input File: horssys2.stm

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #2
DRC 7-24-95

Storm Frequency = 2-Year

* * * H Y D R O L O G Y * * *

Tributary Area										Hydrology Summation				Conduit Data				
Node to	C	Area	Slope	Length	TC(0)	I(0)	Q(0)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC		
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)		
202	201	.00	.00	.00	.0	15.00	3.83	3.50	15.00	3.83	3.50	3.50	15"	2.85	50.00	.29	15.29	
201	200	.00	.00	.00	.0	15.00	3.83	2.50	15.29	3.80	2.48	5.98	15"	4.87	50.00	.17	15.46	

07-24-1995

Date: 07-24-1995
Time: 10:11:12

Input File: horssys2.stm

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #2
DRC 7-24-95

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

```
*****  
Node   Hyd-Slope  Friction  Bend   Transition  Manhole  Deflection  Junction  Total  Hyd-Gl  Desired  Diff.  
      (Ft/Ft)   (Ft)     (Ft)   (Ft)        (Ft)     (Ft)        (Ft)     (Ft)   Elevation Elevation (Ft)  
*****  
200    .00000     .0000    .0000   .0000       .0000     .0000       .0000     .0000  130.0000  130.0000   .00  
201    .00856     .4282    .0000   .0242       .0000     .0000       .5140     .9665  130.9665  139.0000   8.03  
202    .00294     .1468    .0000   .0000       .0000     .0000       .0000     .1468  131.1133  139.0000   7.89  
*****
```

Date: 07-24-1995
Time: 10:20:27

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #3
DRC 7-24-95

Input File: horssys3.stm

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

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*****
Tributary Area          Hydrology Summation          Conduit Data
*****
Node to C Area Slope Length TC(0) I(0) Q(0) TC I Q Sum Q Size Velocity Length TT TT+TC
Node (Ac) (%) (Ft) (Min) (In/Hr) (CFS) (Min) (In/Hr) (CFS) (CFS) (Ft/Sec) (Ft) (Min) (Min)
*****
302 301 .00 .00 .00 .0 15.00 3.83 5.10 15.00 3.83 5.10 5.10 15" 4.16 50.00 .20 15.20
301 300 .00 .00 .00 .0 15.00 3.83 5.10 15.20 3.81 5.07 10.17 18" 5.75 50.00 .14 15.35
*****

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07-24-1995

Date: 07-24-1995
Time: 10:20:27

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #3
DRC 7-24-95

Input File: horssys3.stm

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

```
*****  
Node   Hyd-Slope  Friction  Bend   Transition  Manhole  Deflection  Junction  Total  Hyd-Gl  Desired  Diff.  
      (Ft/Ft)   (Ft)     (Ft)   (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)  
*****  
300    .00000      .0000    .0000   .0000       .0000    .0000      .0000    .0000  133.0000  133.0000   .00  
301    .00937      .4686    .0000   .0246       .0000    .0000      .8147    1.3080  134.3080  139.5000   5.19  
302    .00623      .3117    .0000   .0000       .0000    .0000      .0000    .3117  134.6196  139.5000   4.88  
*****
```

Date: 07-24-1995
Time: 10:24:59

Input File: horssys4.stm

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #4
DRC 7-24-95

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

***** Tributary Area *****										***** Hydrology Summation *****				***** Conduit Data *****			
Node to	C	Area	Slope	Length	TC(0)	I(0)	Q(0)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC	
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)	
402	401	.00	.00	.00	.0	15.00	3.83	5.10	15.00	3.83	5.10	5.10	15"	4.16	50.00	.20	15.20
401	400	.00	.00	.00	.0	15.00	3.83	2.90	15.20	3.81	2.88	7.98	15"	6.50	50.00	.13	15.33

07-24-1995

Date: 07-24-1995
Time: 10:24:59

Input File: horssys4.stm

HORSESHOE LAKE DRAINAGE PLAN
SYSTEM #4
DRC 7-24-95

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

```
*****  
Node   Hyd-Slope  Friction  Bend   Transition  Manhole  Deflection  Junction  Total  Hyd-Gl  Desired  Diff.  
      (Ft/Ft)   (Ft)     (Ft)   (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)  
*****  
400    .00000     .0000    .0000   .0000       .0000     .0000      .0000     .0000  133.0000  133.0000   .00  
401    .01527     .7635    .0000   .0389       .0000     .0000      .8329     1.6353  134.6353  139.5000   4.86  
402    .00623     .3117    .0000   .0000       .0000     .0000      .0000     .3117  134.9470  139.5000   4.55  
*****
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Date _____ Page _____ of _____

Project _____

Item _____

Lots in Blocks 1, 3, and 4 and Lots 1 thru 9, Block 2, lie within an area defined as Floodplain by FEMA's FIRM map, Panel 200328 0005B. Based on a restudy of the Big Slough by Baughman Co., Les Eck's Lake (west of Northshore Blvd.) has a B.F.E. of 1323.0 or 136.0 City Datum. Horseshoe Lake (east of North Shore Blvd) has a lower BFE, but because the only outlet is a 30" CMP in 21st St., ~~for~~ the emergency overflow for the lake is to be 21st St. at elev. 1323.0 (136.0 City), the min. opening and low floor for both lakes will be the same; 137.0.

Additionally, the three proposed ponds will be provided a 36" equalizer pipe to maintain elevations equal to the larger lakes.

ATTACHMENT D

DRAINAGE CRITERIA

CITY OF WICHITA, KANSAS

RECOMMENDED RUNOFF COEFFICIENTS FOR RATIONAL METHOD
AND PERCENT IMPERVIOUS FOR UNIT HYDROGRAPH METHOD

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
1. Business:					
Downtown Areas	95	0.84	0.85	0.87	0.91
Neighborhood Areas	70	0.68	0.69	0.73	0.80
2. Residential:					
<u>Single Family (Soil Group D)</u>					
1/8 Acre	50	0.57	0.61	0.66	0.79
1/4 Acre	38	0.50	0.54	0.62	0.76
1/3 Acre	30	0.46	0.50	0.59	0.73
1/2 Acre	25	0.42	0.48	0.56	0.72
3/4 Acre	22	0.42	0.46	0.55	0.71
1 Acre	20	0.41	0.45	0.54	0.71
<u>Multi-Family (Soil Group D)</u>					
Multi-Unit (detached)	60	0.62	0.66	0.72	0.82
Multi-Unit (attached)	65	0.64	0.68	0.73	0.83
Apartments	75	0.70	0.73	0.79	0.86
<u>Single Family (Soil Group C)</u>					
1/8 Acre	50	0.55	0.58	0.64	0.73
1/4 Acre	38	0.48	0.51	0.57	0.68
1/3 Acre	30	0.43	0.46	0.53	0.65
1/2 Acre	25	0.40	0.43	0.50	0.63
3/4 Acre	22	0.39	0.42	0.49	0.62
1 Acre	20	0.37	0.40	0.48	0.61
<u>Multi-Family (Soil Group C)</u>					
Multi-Unit (detached)	60	0.60	0.63	0.69	0.77
Multi-Unit (attached)	65	0.63	0.66	0.71	0.79
Apartments	75	0.68	0.72	0.77	0.83
<u>Single-Family (Soil Group B)</u>					
1/8 Acre	50	0.52	0.54	0.59	0.67
1/4 Acre	38	0.44	0.46	0.52	0.61
1/3 Acre	30	0.39	0.41	0.47	0.57
1/2 Acre	25	0.36	0.38	0.44	0.54
3/4 Acre	22	0.34	0.36	0.42	0.52
1 Acre	20	0.33	0.35	0.40	0.51
<u>Multi-Family (Soil Group B)</u>					
Multi-Unit (detached)	60	0.58	0.60	0.65	0.72
Multi-Unit (attached)	65	0.61	0.64	0.68	0.75
Apartments	75	0.67	0.70	0.74	0.80

Land Use or face Characteristics	Percent Impervious	Frequency			
		2	5	10	100
<u>Single Family (Soil Group A)</u>					
1/8 Acre	50	0.47	0.50	0.54	0.60
1/4 Acre	38	0.39	0.41	0.45	0.52
1/3 Acre	30	0.33	0.35	0.39	0.47
1/2 Acre	25	0.30	0.31	0.35	0.44
3/4 Acre	22	0.28	0.29	0.33	0.42
1 Acre	20	0.26	0.28	0.32	0.40
<u>Multi-Family (Soil Group A)</u>					
Multi-Unit (detached)	60	0.55	0.57	0.61	0.67
Multi-Unit (attached)	65	0.58	0.60	0.64	0.70
Apartments	75	0.65	0.68	0.72	0.77
3. Industrial:					
Light Areas	70	0.68	0.69	0.73	0.80
Heavy Areas	80	0.74	0.76	0.79	0.84
4. Playgrounds:	15	0.33	0.35	0.42	0.55
5. Schools:	40	0.49	0.51	0.56	0.66
6. Railroad Yard Areas:	30	0.43	0.45	0.50	0.62
Undeveloped Urban Areas:					
Offsite Flow Analysis (when land use not defined)	45	0.52	0.54	0.59	0.68
8. Streets:					
Paved	99	0.87	0.88	0.90	0.93
Gravel	00	0.24	0.26	0.33	0.48
9. Drive, Parking Lots and Walks:	96	0.87	0.87	0.88	0.89
10. Roofs:	90	0.80	0.85	0.90	0.93
11. Urban Lawn Areas (See Note No. 1 below):					
<u>Soil Group A</u>					
Slope less than 1%	00	0.08	0.09	0.13	0.23
Slope 1% to 4%	00	0.12	0.13	0.17	0.27
Slope more than 4%	00	0.16	0.17	0.21	0.31
<u>Soil Group B</u>					
Slope less than 1%	00	0.16	0.18	0.24	0.37
Slope 1% to 4%	00	0.20	0.22	0.28	0.41
Slope more than 4%	00	0.24	0.26	0.32	0.45
<u>Soil Group C</u>					
Slope less than 1%	00	0.24	0.27	0.35	0.51
Slope 1% to 4%	00	0.26	0.29	0.37	0.53
Slope more than 4%	00	0.28	0.31	0.39	0.55

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
<u>Soil Group D</u>					
Slope less than 1%	00	0.28	0.33	0.43	0.63
Slope 1% to 4%	00	0.30	0.35	0.45	0.65
Slope more than 4%	00	0.32	0.37	0.47	0.67

Note No. 1: Coefficients shown in the above table are for pervious open space areas with thick turf which includes pervious areas in parks and cemeteries. Coefficients shown above must be increased 0.02 for use with agricultural pasture areas. Coefficients shown above must be reduced by 0.04 for use with agricultural cultivated areas. Group A soils are well-drained, coarse textured sands with high infiltration rates. Group B soils are moderately well-drained, moderately coarse textured soils with moderate infiltration rates. Group C soils are moderately poor-drained, moderately fine textured soils with slow infiltration rates. Group D soils are poor-drained, fine textured soils with very slow infiltration rates.

GENERAL NOTE: These Rational Formula Coefficients may not be valid for basins 320 acres or larger.

RAINFALL INTENSITY TABLE

SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
0:05	4.77	5.52	6.56	7.32	8.44	9.32	10.20
0:06	4.53	5.26	6.27	7.02	8.11	8.96	9.81
0:07	4.33	5.04	6.03	6.76	7.82	8.65	9.48
0:08	4.16	4.85	5.82	6.52	7.55	8.36	9.17
0:09	4.00	4.67	5.61	6.30	7.30	8.09	8.87
0:10	3.85	4.50	5.42	6.08	7.06	7.82	8.58
0:11	3.71	4.34	5.23	5.88	6.83	7.56	8.30
0:12	3.58	4.19	5.06	5.69	6.60	7.32	8.04
0:13	3.45	4.05	4.90	5.51	6.40	7.10	7.79
0:14	3.34	3.92	4.75	5.34	6.21	6.89	7.57
0:15	3.23	3.80	4.61	5.19	6.04	6.70	7.36
0:16	3.13	3.69	4.48	5.05	5.88	6.53	7.17
0:17	3.03	3.58	4.36	4.92	5.73	6.37	7.00
0:18	2.94	3.48	4.25	4.80	5.60	6.22	6.84
0:19	2.86	3.39	4.14	4.69	5.47	6.09	6.70
0:20	2.78	3.30	4.05	4.58	5.35	5.96	6.56
0:21	2.70	3.21	3.95	4.48	5.24	5.84	6.43
0:22	2.63	3.14	3.87	4.39	5.14	5.72	6.30
0:23	2.56	3.06	3.78	4.30	5.04	5.61	6.19
0:24	2.50	2.99	3.71	4.21	4.94	5.51	6.07
0:25	2.44	2.93	3.63	4.13	4.85	5.41	5.97
0:26	2.38	2.86	3.56	4.05	4.76	5.31	5.86
0:27	2.33	2.80	3.49	3.98	4.68	5.22	5.76
0:28	2.28	2.75	3.43	3.91	4.59	5.13	5.66
0:29	2.23	2.69	3.36	3.84	4.52	5.04	5.57
0:30	2.19	2.64	3.30	3.77	4.44	4.96	5.48
0:31	2.14	2.59	3.24	3.71	4.37	4.88	5.39
0:32	2.10	2.54	3.19	3.64	4.30	4.80	5.31
0:33	2.06	2.50	3.14	3.58	4.23	4.73	5.22
0:34	2.02	2.45	3.08	3.53	4.16	4.65	5.14
0:35	1.99	2.41	3.03	3.47	4.10	4.58	5.07
0:36	1.95	2.37	2.99	3.42	4.03	4.51	4.99
0:37	1.92	2.33	2.94	3.36	3.97	4.45	4.92
0:38	1.89	2.30	2.89	3.31	3.91	4.38	4.84
0:39	1.86	2.26	2.85	3.27	3.86	4.32	4.77
0:40	1.83	2.23	2.81	3.22	3.80	4.26	4.71
0:41	1.80	2.19	2.77	3.17	3.75	4.20	4.64
0:42	1.77	2.16	2.73	3.13	3.70	4.14	4.58
0:43	1.75	2.13	2.69	3.08	3.65	4.08	4.52
0:44	1.72	2.10	2.65	3.04	3.60	4.03	4.46
0:45	1.70	2.07	2.62	3.00	3.55	3.97	4.40

RAINFALL INTENSITY TABLE

SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
0:46	1.67	2.04	2.58	2.96	3.50	3.92	4.34
0:47	1.65	2.01	2.55	2.92	3.46	3.87	4.29
0:48	1.63	1.98	2.51	2.88	3.41	3.82	4.23
0:49	1.60	1.96	2.48	2.85	3.37	3.78	4.18
0:50	1.58	1.93	2.45	2.81	3.33	3.73	4.13
0:51	1.56	1.91	2.42	2.78	3.29	3.68	4.08
0:52	1.54	1.88	2.39	2.74	3.25	3.64	4.03
0:53	1.52	1.86	2.36	2.71	3.21	3.60	3.98
0:54	1.50	1.84	2.33	2.68	3.17	3.55	3.94
0:55	1.48	1.81	2.30	2.65	3.13	3.51	3.89
0:56	1.46	1.79	2.28	2.62	3.10	3.47	3.85
0:57	1.45	1.77	2.25	2.59	3.06	3.43	3.80
0:58	1.43	1.75	2.23	2.56	3.03	3.40	3.76
0:59	1.41	1.73	2.20	2.53	3.00	3.36	3.72
1:00	1.39	1.71	2.18	2.50	2.96	3.32	3.68
1:05	1.32	1.62	2.06	2.37	2.81	3.15	3.49
1:10	1.25	1.53	1.96	2.25	2.67	3.00	3.33
1:15	1.18	1.46	1.87	2.15	2.55	2.86	3.17
1:20	1.13	1.39	1.78	2.05	2.44	2.74	3.04
1:25	1.07	1.33	1.70	1.97	2.34	2.63	2.91
1:30	1.03	1.27	1.63	1.89	2.24	2.52	2.80
1:35	0.98	1.22	1.57	1.81	2.16	2.43	2.69
1:40	0.94	1.17	1.51	1.75	2.08	2.34	2.60
1:45	0.91	1.13	1.46	1.69	2.01	2.26	2.51
1:50	0.87	1.09	1.41	1.63	1.94	2.18	2.42
1:55	0.84	1.05	1.36	1.57	1.88	2.11	2.35
2:00	0.81	1.02	1.32	1.52	1.82	2.05	2.28
2:05	0.79	0.98	1.28	1.48	1.76	1.99	2.21
2:10	0.76	0.95	1.24	1.43	1.71	1.93	2.14
2:15	0.74	0.92	1.20	1.39	1.67	1.88	2.08
2:20	0.72	0.90	1.17	1.36	1.62	1.82	2.03
2:25	0.70	0.87	1.14	1.32	1.58	1.78	1.98
2:30	0.68	0.85	1.11	1.29	1.54	1.73	1.93
2:35	0.66	0.83	1.08	1.25	1.50	1.69	1.88
2:40	0.64	0.81	1.05	1.22	1.46	1.65	1.83
2:45	0.62	0.79	1.03	1.19	1.43	1.61	1.79
2:50	0.61	0.77	1.00	1.17	1.40	1.57	1.75
2:55	0.59	0.75	0.98	1.14	1.37	1.54	1.71
3:00	0.58	0.73	0.96	1.12	1.34	1.51	1.68

RAINFALL INTENSITY TABLE

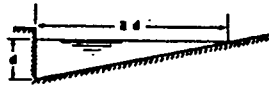
SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
3:15	0.54	0.69	0.90	1.05	1.26	1.42	1.58
3:30	0.51	0.65	0.85	0.99	1.19	1.34	1.49
3:45	0.48	0.61	0.80	0.94	1.12	1.27	1.41
4:00	0.46	0.58	0.76	0.89	1.07	1.21	1.34
4:15	0.44	0.55	0.73	0.85	1.02	1.15	1.28
4:30	0.42	0.53	0.70	0.81	0.98	1.10	1.23
4:45	0.40	0.51	0.67	0.78	0.94	1.06	1.18
5:00	0.38	0.49	0.64	0.75	0.90	1.02	1.13
5:15	0.37	0.47	0.62	0.72	0.87	0.98	1.09
5:30	0.35	0.45	0.60	0.70	0.83	0.94	1.05
5:45	0.34	0.44	0.58	0.67	0.81	0.91	1.01
6:00	0.33	0.42	0.56	0.65	0.78	0.88	0.98
6:30	0.31	0.40	0.52	0.61	0.73	0.83	0.92
7:00	0.30	0.38	0.50	0.58	0.69	0.78	0.87
7:30	0.28	0.36	0.47	0.55	0.66	0.74	0.83
8:00	0.27	0.34	0.45	0.52	0.62	0.70	0.78
8:30	0.26	0.33	0.43	0.50	0.60	0.67	0.75
9:00	0.25	0.31	0.41	0.48	0.57	0.64	0.72
9:30	0.24	0.30	0.39	0.46	0.55	0.62	0.69
10:00	0.23	0.29	0.38	0.44	0.52	0.59	0.66
10:30	0.22	0.28	0.36	0.42	0.50	0.57	0.63
11:00	0.21	0.27	0.35	0.41	0.49	0.55	0.61
11:30	0.21	0.26	0.34	0.39	0.47	0.53	0.59
12:00	0.20	0.25	0.33	0.38	0.45	0.51	0.57
13:00	0.19	0.24	0.31	0.36	0.43	0.48	0.53
14:00	0.18	0.22	0.29	0.34	0.40	0.45	0.50
15:00	0.17	0.21	0.27	0.32	0.38	0.43	0.47
16:00	0.16	0.20	0.26	0.30	0.36	0.40	0.45
17:00	0.15	0.19	0.25	0.29	0.34	0.38	0.43
18:00	0.15	0.18	0.24	0.27	0.33	0.37	0.41
19:00	0.14	0.18	0.23	0.26	0.31	0.35	0.39
20:00	0.14	0.17	0.22	0.25	0.30	0.34	0.37
21:00	0.13	0.16	0.21	0.24	0.29	0.32	0.36
22:00	0.13	0.16	0.20	0.23	0.28	0.31	0.34
23:00	0.12	0.15	0.19	0.22	0.27	0.30	0.33
24:00	0.12	0.15	0.19	0.22	0.26	0.29	0.32

NOMOGRAPH FOR FLOW IN TRIANGULAR CHANNELS

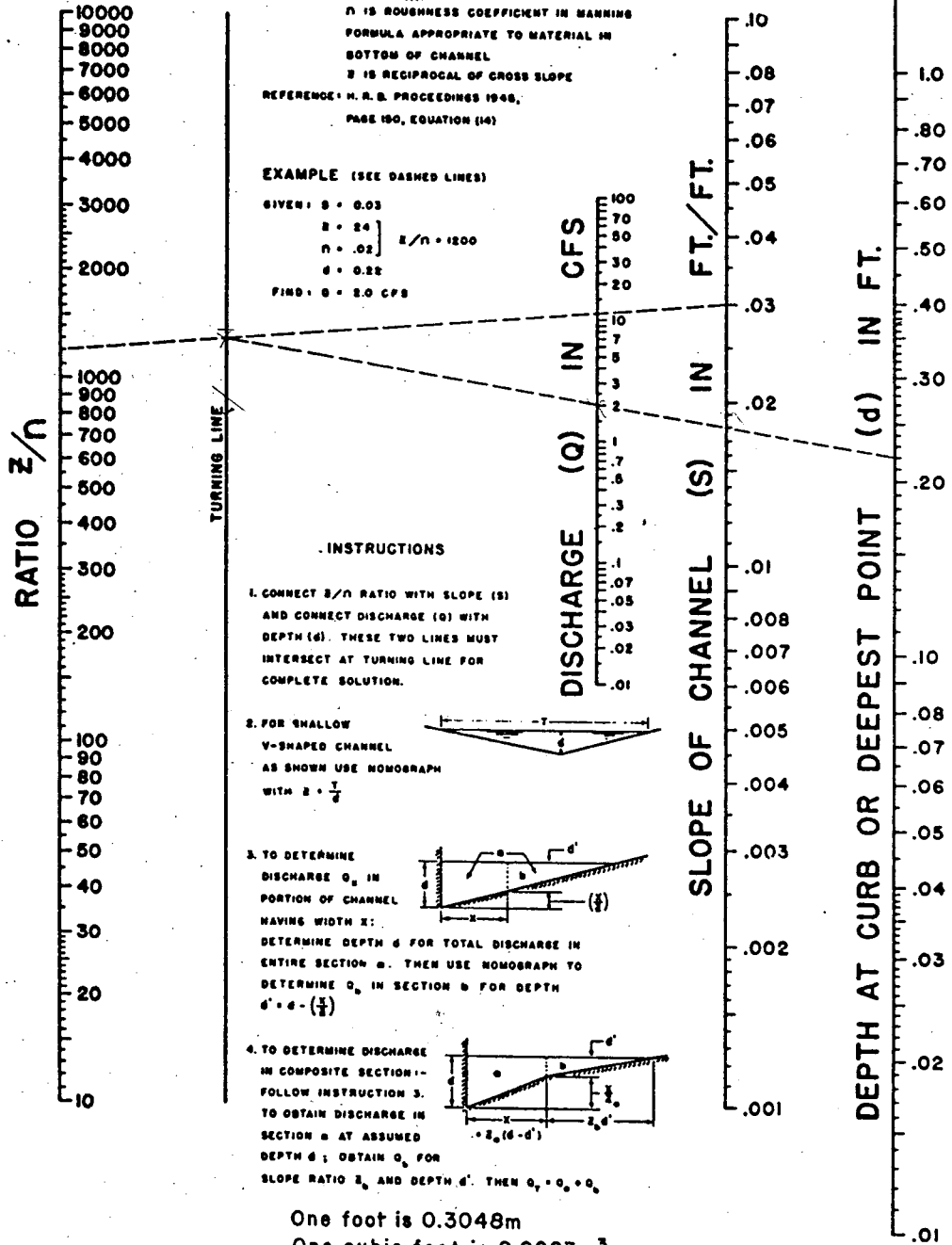
$Z = 32$
 $n = .016$
 $Z/n = 2000$



EQUATION: $Q = 0.85 \left(\frac{Z}{n}\right) S^{1/2} d^{5/2}$
 n IS ROUGHNESS COEFFICIENT IN MANNING FORMULA APPROPRIATE TO MATERIAL IN BOTTOM OF CHANNEL
 Z IS RECIPROCAL OF CROSS SLOPE
REFERENCE: N. R. S. PROCEEDINGS 1948, PAGE 150, EQUATION (14)

EXAMPLE (SEE DASHED LINES)

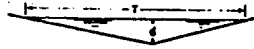
GIVEN: $S = 0.03$
 $Z = 32$
 $n = .016$ } $Z/n = 2000$
 $S = 0.03$
FIND: $Q = 2.0$ CFS



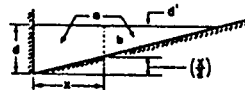
INSTRUCTIONS

1. CONNECT Z/n RATIO WITH SLOPE (S) AND CONNECT DISCHARGE (Q) WITH DEPTH (d). THESE TWO LINES MUST INTERSECT AT TURNING LINE FOR COMPLETE SOLUTION.

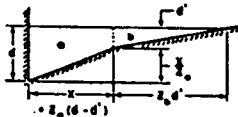
2. FOR SHALLOW V-SHAPED CHANNEL AS SHOWN USE NOMOGRAPH WITH $Z = \frac{1}{S}$



3. TO DETERMINE DISCHARGE Q_1 IN PORTION OF CHANNEL HAVING WIDTH X : DETERMINE DEPTH d FOR TOTAL DISCHARGE IN ENTIRE SECTION a . THEN USE NOMOGRAPH TO DETERMINE Q_2 IN SECTION b FOR DEPTH $d' = d - (\frac{X}{Z})$



4. TO DETERMINE DISCHARGE IN COMPOSITE SECTION -- FOLLOW INSTRUCTION 3. TO OBTAIN DISCHARGE IN SECTION a AT ASSUMED DEPTH d ; OBTAIN Q_2 FOR SLOPE RATIO S_2 AND DEPTH d' . THEN $Q_1 = Q_2 \cdot Q_3$



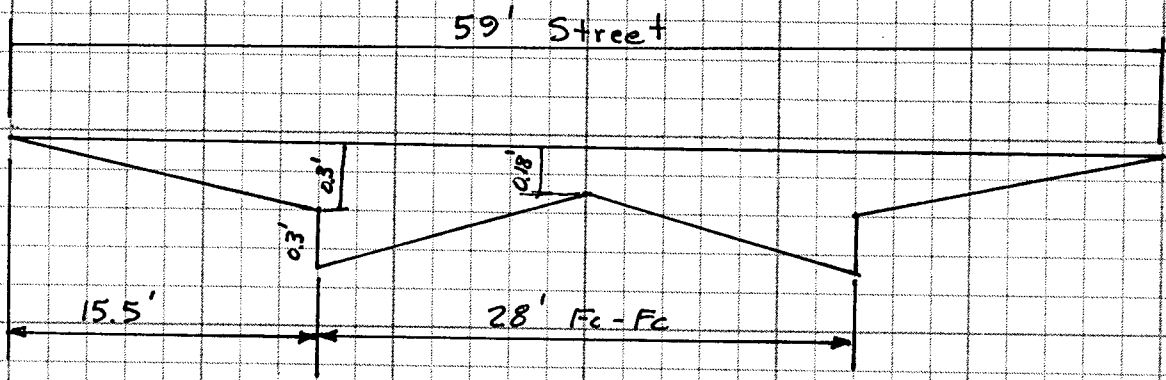
One foot is 0.3048m
 One cubic foot is 0.0283m³



Date _____ Page _____ of _____

Project _____

Item _____



$$n = \frac{2(.03 \times 15) + 2(.013 \times 2.8) + 2(.016 \times 12)}{2(2.8 + 15 + 12)} = 0.02277$$

$$A = (0.18 \times 28) + 2\left(\frac{1}{2} \times 0.3 \times 15.5\right) + 2\left(\frac{1}{2} \times 0.42 \times 12\right)$$

$$= 15.57$$

$$P = 2(2.8 + 15 + 12) = 59.6$$

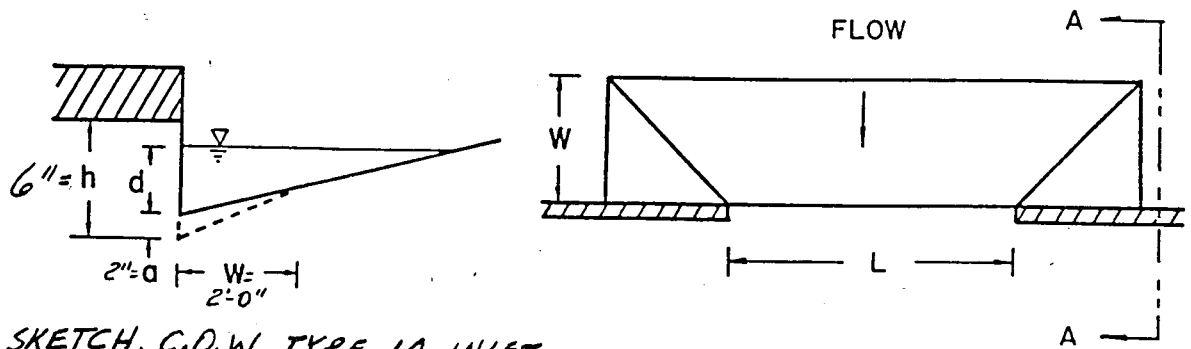
$$R = \frac{A}{P} = \frac{15.57}{59.6} = 0.26$$

$$R^{2/3} = 0.41$$

$$Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$$

$$Q = \frac{1.486}{.02277} \times 15.57 \times 0.41 \times S^{1/2}$$

$$Q = 416 \sqrt{S}$$



DEF. SKETCH, C.D.W. TYPE 1A INLET

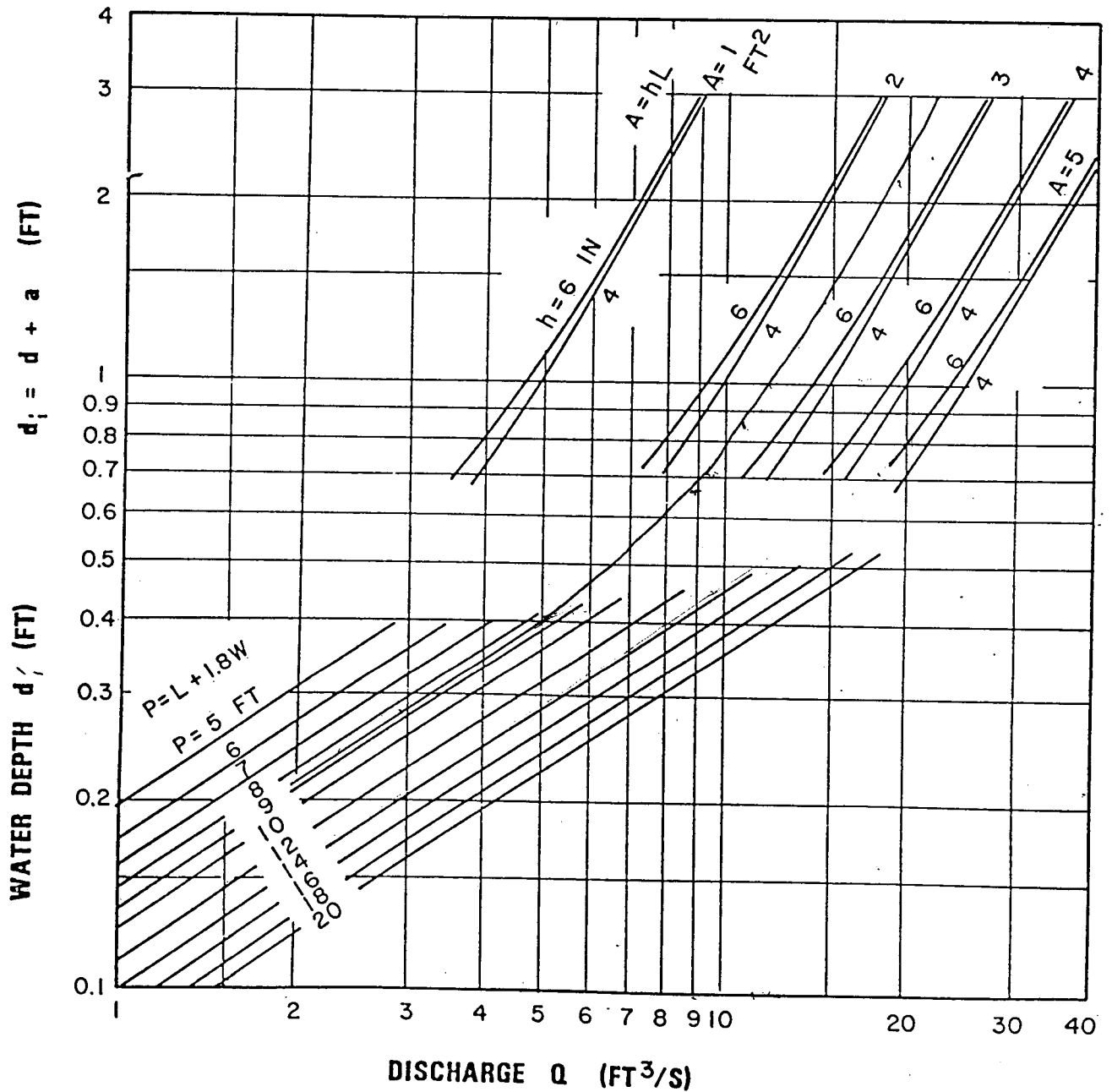


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR., 1984