



Date _____ Page _____ of _____

Project STERLING FARMS 6TH

Item HYDROLOGY

<u>BASIN</u>	<u>AREA</u>	<u>C₂</u>	<u>L₂</u>	<u>q₂</u>	<u>C₁₀₀</u>	<u>L₁₀₀</u>	<u>q₁₀₀</u>
1A (60% B / 40% C)	4.55	0.46	380	7.9	0.64	7.36	21.4
1B (80% B / 20% C)	1.87	0.45	}	3.2	0.62	}	8.5
R1 (80% B / 20% C)	1.96	0.45		3.4	0.62		8.9
2A (70% C / 30% B)	1.63	0.47		2.9	0.66		7.9
2B (60% C / 40% B)	2.42	0.46		4.2	0.65		11.6
2C (C)	1.36	0.48		2.5	0.68		6.8

$t_c = 15 \text{ min. (assumed)}$
 $1/4 \text{ ac. Residential}$



Date _____ Page _____ of _____

Project Sterling Farms 6th

Item Inlet Capacity

<u>Node</u>	<u>Basin</u>	<u>Q₂</u>	<u>L (ft)</u>	<u>d</u>	<u>d_{max}</u>	<u>Com.</u>
102	1A	21.4 (Q ₁₀₀)	10	0.82	0.85	Std. Cb. + 0.3'
103	1B	3.2	5	0.3'	0.3'	OK
104-108	R1	0.7/ea.	4'x2'	d=0.5', Q=17 cfs		OK
201	2C	6.8 (Q ₁₀₀)	5	0.52'	0.6'	Roll. Cb. + 0.3'
202	2A	2.9	5	0.29'	0.3'	OK
203	2B	4.2	5	0.35	0.3'	OK



Date _____ Page _____ of _____

Project STERLING FARMS COTH

Item 2-YR. STREET FLOW @ 100-Yr.

2 - Yr.

<u>BASIN</u>	<u>Q₂ approaching</u>	<u>Node #</u>	<u>Slope</u>	<u>X-Slope</u>	<u>d</u>	<u>d_{max}</u>
80% 1A	0.8(7.9) = 6.3	102 (south)	0.32%	} 3/8" / 1'	0.42'	0.55'
80% 1B	0.8(3.2) = 2.6	103 (south)	0.32%		0.30'	0.30'
50% 2A	0.5(2.9) = 1.5	202 (south)	0.32%		0.25'	0.30'
90% 2B	0.9(4.2) = 3.8	203 (East)	0.57%		0.31'	0.30'
80% 2C	0.8(2.5) = 2.0	201 (South)	0.32%		0.27'	0.30'

100 - Yr.

<u>Basins</u>	<u>Q₁₀₀</u>	<u>S</u>	<u>Q_{max}</u>
80% 1A & 1B	0.8(21.4 + 8.5) = 23.9 (south)	0.32%	30.0

By Insp., all remaining basins are OK.



Date _____ Page _____ of _____

Project STERLING FARMS 6TH

Item PIPE SIZING

<u>SYSTEM 100</u>		<u>SYSTEM 200</u>	
<u>Node</u>	<u>Q_i</u>	<u>Node</u>	<u>Q_i</u>
100	∅ Outlet	200	∅ Outlet
101	∅ MH	201	6.8 (Q ₁₀₀)
102	2.4 (Q ₁₀₀)	202	2.9
103	8.5 (Q ₁₀₀)	203	4.2
104	0.7		
105	0.7		
106	0.7		
107	0.7		
108	0.6		

Static = 1346.4
 DW₁₀₀ = 1349.9

STARTING HGL = 1348.4 for Storm

Sterling Farms 6th Addition
Storm Water Sewer System #1
DRC 5/22/95

Input File: c:\output\ster6th1.stm

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

*****										*****										*****									
Tributary Area										Hydrology Summation										Conduit Data									
Node to	C	Area	Slope	Length	TC(0)	I(0)	Q(0)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC													
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)													
*****										*****										*****									
108	107	.00	.00	.00	.0	15.00	3.83	.60	15.00	3.83	.60	.60	12"	.76	140.00	3.05	18.05												
107	106	.00	.00	.00	.0	15.00	3.83	.70	18.05	3.51	.64	1.24	12"	1.58	155.00	1.63	19.69												
106	105	.00	.00	.00	.0	15.00	3.83	.70	19.69	3.36	.61	1.86	15"	1.51	115.00	1.27	20.96												
105	104	.00	.00	.00	.0	15.00	3.83	.70	20.96	3.25	.60	2.45	15"	2.00	135.00	1.13	22.08												
104	103	.00	.00	.00	.0	15.00	3.83	.70	22.08	3.17	.58	3.03	18"	1.71	125.00	1.22	23.30												
103	102	.00	.00	.00	.0	15.00	3.83	8.50	15.00	3.83	8.50	10.45	24"	3.33	40.00	.20	15.20												
102	101	.00	.00	.00	.0	15.00	3.83	21.40	15.20	3.81	21.27	31.72	36"	4.49	130.00	.48	15.68												
101	100	.00	.00	.00	.0	.00	.00	.00	15.68	3.75	.00	31.72	36"	4.49	65.00	.24	15.92												
*****										*****										*****									

Input File: c:\output\ster6th1.stm

Sterling Farms 6th Addition
Storm Water Sewer System #1
DRC 5/22/95

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

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*****
Node   Hyd-Slope  Friction  Bend  Transition  Manhole  Deflection  Junction  Total  Hyd-Gl  Desired  Diff.
      (Ft/Ft)   (Ft)     (Ft)   (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)
*****
100    .00000     .0000    .0000   .0000       .0000    .0000      .0000    .0000  1348.4000 1348.4000  .00
102    .00226     .2941    .0000   .0141       .0000    .0000      .6667    .9749  1349.6170 1352.7000  3.08
101    .00226     .1470    .0000   .0000       .0156    .0681      .0113    .2421  1348.6420 1350.8000  2.16
103    .00213     .0854    .0000   .0126       .0000    .0000      .3823    .4802  1350.0970 1352.7000  2.60
104    .00083     .1040    .0000   .0033       .0000    .0000      .0119    .1192  1350.2160 1351.0000  .78
105    .00144     .1944    .0000   .0026       .0000    .0048      .0586    .2604  1350.4770 1351.0000  .52
106    .00083     .0949    .0000   .0007       .0000    .0052      .0312    .1320  1350.6090 1351.0000  .39
107    .00121     .1882    .0000   .0030       .0000    .0012      .0633    .2557  1350.8640 1351.0000  .14
108    .00028     .0397    .0000   .0000       .0000    .0000      .0000    .0397  1350.9040 1351.0000  .10
*****

```

Date: 05-22-1995
Time: 09:45:37

Sterling Farms 6th Addition
Storm Water Sewer System #2
DRC 5/22/95

Input File: c:\output\ster6th2.stm

Storm Frequency = 2-Year

* * * HYDROLOGY * * *

*****										*****									
Tributary Area					Hydrology Summation					Conduit Data									
Node to	C	Area	Slope	Length	TC(0)	I(0)	Q(0)	TC	I	Q	Sum Q	Size	Velocity	Length	TT	TT+TC			
Node		(Ac)	(%)	(Ft)	(Min)	(In/Hr)	(CFS)	(Min)	(In/Hr)	(CFS)	(CFS)		(Ft/Sec)	(Ft)	(Min)	(Min)			
*****										*****									
203	202	.00	.00	.00	.0	15.00	3.83	4.20	15.00	3.83	4.20	4.20	18"	2.38	50.00	.35	15.35		
202	201	.00	.00	.00	.0	15.00	3.83	2.90	15.35	3.79	2.87	7.07	18"	4.00	50.00	.21	15.56		
201	200	.00	.00	.00	.0	15.00	3.83	6.80	15.56	3.77	6.69	13.76	24"	4.38	150.00	.57	16.13		
*****										*****									

Date: 05-22-1995
 Time: 09:45:37

Input File: c:\output\ster6th2.stm

Sterling Farms 6th Addition
 Storm Water Sewer System #2
 DRC 5/22/95

Storm Frequency = 2-Year

* * * HYDRAULICS * * *

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*****
Node   Hyd-Slope  Friction  Bend  Transition  Manhole  Deflection  Junction  Total  Hyd-Gl  Desired  Diff.
      (Ft/Ft)   (Ft)     (Ft)   (Ft)        (Ft)     (Ft)       (Ft)     (Ft)   Elevation Elevation (Ft)
*****
200    .00000     .0000    .0000   .0000       .0000    .0000      .0000    .0000  1348.4000 1348.4000   .00
201    .00370     .5547    .0000   .0049       .0000    .0541      .4257    1.0395  1349.4390 1352.5000   3.06
202    .00453     .2265    .0000   .0161       .0000    .0713      .3375    .6515  1350.0910 1352.6000   2.51
203    .00160     .0799    .0000   .0000       .0000    .0000      .0000    .0799  1350.1710 1352.6000   2.43
*****
    
```

ATTACHMENT D

DRAINAGE CRITERIA

CITY OF WICHITA, KANSAS

RECOMMENDED RUNOFF COEFFICIENTS FOR RATIONAL METHOD
AND PERCENT IMPERVIOUS FOR UNIT HYDROGRAPH METHOD

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		2	5	10	100
1. Business:					
Lowntown Areas	95	0.84	0.85	0.87	0.91
Neighborhood Areas	70	0.68	0.69	0.73	0.80
2. Residential:					
<u>Single Family (Soil Group D)</u>					
1/8 Acre	50	0.57	0.61	0.66	0.79
1/4 Acre	38	0.50	0.54	0.62	0.76
1/3 Acre	30	0.46	0.50	0.59	0.73
1/2 Acre	25	0.42	0.48	0.56	0.72
3/4 Acre	22	0.42	0.46	0.55	0.71
1 Acre	20	0.41	0.45	0.54	0.71
<u>Multi-Family (Soil Group D)</u>					
Multi-Unit (detached)	60	0.62	0.66	0.72	0.82
Multi-Unit (attached)	65	0.64	0.68	0.73	0.83
Apartments	75	0.70	0.73	0.79	0.86
<u>Single Family (Soil Group C)</u>					
1/8 Acre	50	0.55	0.58	0.64	0.73
1/4 Acre	38	0.48	0.51	0.57	0.68
1/3 Acre	30	0.43	0.46	0.53	0.65
1/2 Acre	25	0.40	0.43	0.50	0.63
3/4 Acre	22	0.39	0.42	0.49	0.62
1 Acre	20	0.37	0.40	0.48	0.61
<u>Multi-Family (Soil Group C)</u>					
Multi-Unit (detached)	60	0.60	0.63	0.69	0.77
Multi-Unit (attached)	65	0.63	0.66	0.71	0.79
Apartments	75	0.68	0.72	0.77	0.83
<u>Single-Family (Soil Group B)</u>					
1/8 Acre	50	0.52	0.54	0.59	0.67
1/4 Acre	38	0.44	0.46	0.52	0.61
1/3 Acre	30	0.39	0.41	0.47	0.57
1/2 Acre	25	0.36	0.38	0.44	0.54
3/4 Acre	22	0.34	0.36	0.42	0.52
1 Acre	20	0.33	0.35	0.40	0.51
<u>Multi-Family (Soil Group B)</u>					
Multi-Unit (detached)	60	0.58	0.60	0.65	0.72
Multi-Unit (attached)	65	0.61	0.64	0.68	0.75
Apartments	75	0.67	0.70	0.74	0.80

Land Use or face Characteristics	Percent Impervious	Frequency			
		2	5	10	100
<u>Single Family (Soil Group A)</u>					
1/8 Acre	50	0.47	0.50	0.54	0.60
1/4 Acre	38	0.39	0.41	0.45	0.52
1/3 Acre	30	0.33	0.35	0.39	0.47
1/2 Acre	25	0.30	0.31	0.35	0.44
3/4 Acre	22	0.28	0.29	0.33	0.42
1 Acre	20	0.26	0.28	0.32	0.40
<u>Multi-Family (Soil Group A)</u>					
Multi-Unit (detached)	60	0.55	0.57	0.61	0.67
Multi-Unit (attached)	65	0.58	0.60	0.64	0.70
Apartments	75	0.65	0.68	0.72	0.77
3. Industrial:					
Light Areas	70	0.68	0.69	0.73	0.80
Heavy Areas	80	0.74	0.76	0.79	0.84
4. Playgrounds:	15	0.33	0.35	0.42	0.55
5. Schools:	40	0.49	0.51	0.56	0.66
6. Railroad Yard Areas:	30	0.43	0.45	0.50	0.62
Undeveloped Urban Areas: Offsite Flow Analysis (when land use not defined)	45	0.52	0.54	0.59	0.68
8. Streets:					
Paved	99	0.87	0.88	0.90	0.93
Gravel	00	0.24	0.26	0.33	0.48
9. Drive, Parking Lots and Walks:	96	0.87	0.87	0.88	0.89
10. Roofs:	90	0.80	0.85	0.90	0.93
11. Urban Lawn Areas (See Note No. 1 below):					
<u>Soil Group A</u>					
Slope less than 1%	00	0.08	0.09	0.13	0.23
Slope 1% to 4%	00	0.12	0.13	0.17	0.27
Slope more than 4%	00	0.16	0.17	0.21	0.31
<u>Soil Group B</u>					
Slope less than 1%	00	0.16	0.18	0.24	0.37
Slope 1% to 4%	00	0.20	0.22	0.28	0.41
Slope more than 4%	00	0.24	0.26	0.32	0.45
<u>Soil Group C</u>					
Slope less than 1%	00	0.24	0.27	0.35	0.51
Slope 1% to 4%	00	0.26	0.29	0.37	0.53
Slope more than 4%	00	0.28	0.31	0.39	0.55

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
<u>Soil Group D</u>					
Slope less than 1%	00	0.28	0.33	0.43	0.63
Slope 1% to 4%	00	0.30	0.35	0.45	0.65
Slope more than 4%	00	0.32	0.37	0.47	0.67

Note No. 1: Coefficients shown in the above table are for pervious open space areas with thick turf which includes pervious areas in parks and cemeteries. Coefficients shown above must be increased 0.02 for use with agricultural pasture areas. Coefficients shown above must be reduced by 0.04 for use with agricultural cultivated areas. Group A soils are well-drained, coarse textured sands with high infiltration rates. Group B soils are moderately well-drained, moderately coarse textured soils with moderate infiltration rates. Group C soils are moderately poor-drained, moderately fine textured soils with slow infiltration rates. Group D soils are poor-drained, fine textured soils with very slow infiltration rates.

GENERAL NOTE: These Rational Formula Coefficients may not be valid for basins 320 acres or larger.

RAINFALL INTENSITY TABLE

SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
0:05	4.77	5.52	6.56	7.32	8.44	9.32	10.20
0:06	4.53	5.26	6.27	7.02	8.11	8.96	9.81
0:07	4.33	5.04	6.03	6.76	7.82	8.65	9.48
0:08	4.16	4.85	5.82	6.52	7.55	8.36	9.17
0:09	4.00	4.67	5.61	6.30	7.30	8.09	8.87
0:10	3.85	4.50	5.42	6.08	7.06	7.82	8.58
0:11	3.71	4.34	5.23	5.88	6.83	7.56	8.30
0:12	3.58	4.19	5.06	5.69	6.60	7.32	8.04
0:13	3.45	4.05	4.90	5.51	6.40	7.10	7.79
0:14	3.34	3.92	4.75	5.34	6.21	6.89	7.57
0:15	3.23	3.80	4.61	5.19	6.04	6.70	7.36
0:16	3.13	3.69	4.48	5.05	5.88	6.53	7.17
0:17	3.03	3.58	4.36	4.92	5.73	6.37	7.00
0:18	2.94	3.48	4.25	4.80	5.60	6.22	6.84
0:19	2.86	3.39	4.14	4.69	5.47	6.09	6.70
0:20	2.78	3.30	4.05	4.58	5.35	5.96	6.56
0:21	2.70	3.21	3.95	4.48	5.24	5.84	6.43
0:22	2.63	3.14	3.87	4.39	5.14	5.72	6.30
0:23	2.56	3.06	3.78	4.30	5.04	5.61	6.19
0:24	2.50	2.99	3.71	4.21	4.94	5.51	6.07
0:25	2.44	2.93	3.63	4.13	4.85	5.41	5.97
0:26	2.38	2.86	3.56	4.05	4.76	5.31	5.86
0:27	2.33	2.80	3.49	3.98	4.68	5.22	5.76
0:28	2.28	2.75	3.43	3.91	4.59	5.13	5.66
0:29	2.23	2.69	3.36	3.84	4.52	5.04	5.57
0:30	2.19	2.64	3.30	3.77	4.44	4.96	5.48
0:31	2.14	2.59	3.24	3.71	4.37	4.88	5.39
0:32	2.10	2.54	3.19	3.64	4.30	4.80	5.31
0:33	2.06	2.50	3.14	3.58	4.23	4.73	5.22
0:34	2.02	2.45	3.08	3.53	4.16	4.65	5.14
0:35	1.99	2.41	3.03	3.47	4.10	4.58	5.07
0:36	1.95	2.37	2.99	3.42	4.03	4.51	4.99
0:37	1.92	2.33	2.94	3.36	3.97	4.45	4.92
0:38	1.89	2.30	2.89	3.31	3.91	4.38	4.84
0:39	1.86	2.26	2.85	3.27	3.86	4.32	4.77
0:40	1.83	2.23	2.81	3.22	3.80	4.26	4.71
0:41	1.80	2.19	2.77	3.17	3.75	4.20	4.64
0:42	1.77	2.16	2.73	3.13	3.70	4.14	4.58
0:43	1.75	2.13	2.69	3.08	3.65	4.08	4.52
0:44	1.72	2.10	2.65	3.04	3.60	4.03	4.46
0:45	1.70	2.07	2.62	3.00	3.55	3.97	4.40

RAINFALL INTENSITY TABLE

SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
0:46	1.67	2.04	2.58	2.96	3.50	3.92	4.34
0:47	1.65	2.01	2.55	2.92	3.46	3.87	4.29
0:48	1.63	1.98	2.51	2.88	3.41	3.82	4.23
0:49	1.60	1.96	2.48	2.85	3.37	3.78	4.18
0:50	1.58	1.93	2.45	2.81	3.33	3.73	4.13
0:51	1.56	1.91	2.42	2.78	3.29	3.68	4.08
0:52	1.54	1.88	2.39	2.74	3.25	3.64	4.03
0:53	1.52	1.86	2.36	2.71	3.21	3.60	3.98
0:54	1.50	1.84	2.33	2.68	3.17	3.55	3.94
0:55	1.48	1.81	2.30	2.65	3.13	3.51	3.89
0:56	1.46	1.79	2.28	2.62	3.10	3.47	3.85
0:57	1.45	1.77	2.25	2.59	3.06	3.43	3.80
0:58	1.43	1.75	2.23	2.56	3.03	3.40	3.76
0:59	1.41	1.73	2.20	2.53	3.00	3.36	3.72
1:00	1.39	1.71	2.18	2.50	2.96	3.32	3.68
1:05	1.32	1.62	2.06	2.37	2.81	3.15	3.49
1:10	1.25	1.53	1.96	2.25	2.67	3.00	3.33
1:15	1.18	1.46	1.87	2.15	2.55	2.86	3.17
1:20	1.13	1.39	1.78	2.05	2.44	2.74	3.04
1:25	1.07	1.33	1.70	1.97	2.34	2.63	2.91
1:30	1.03	1.27	1.63	1.89	2.24	2.52	2.80
1:35	0.98	1.22	1.57	1.81	2.16	2.43	2.69
1:40	0.94	1.17	1.51	1.75	2.08	2.34	2.60
1:45	0.91	1.13	1.46	1.69	2.01	2.26	2.51
1:50	0.87	1.09	1.41	1.63	1.94	2.18	2.42
1:55	0.84	1.05	1.36	1.57	1.88	2.11	2.35
2:00	0.81	1.02	1.32	1.52	1.82	2.05	2.28
2:05	0.79	0.98	1.28	1.48	1.76	1.99	2.21
2:10	0.76	0.95	1.24	1.43	1.71	1.93	2.14
2:15	0.74	0.92	1.20	1.39	1.67	1.88	2.08
2:20	0.72	0.90	1.17	1.36	1.62	1.82	2.03
2:25	0.70	0.87	1.14	1.32	1.58	1.78	1.98
2:30	0.68	0.85	1.11	1.29	1.54	1.73	1.93
2:35	0.66	0.83	1.08	1.25	1.50	1.69	1.88
2:40	0.64	0.81	1.05	1.22	1.46	1.65	1.83
2:45	0.62	0.79	1.03	1.19	1.43	1.61	1.79
2:50	0.61	0.77	1.00	1.17	1.40	1.57	1.75
2:55	0.59	0.75	0.98	1.14	1.37	1.54	1.71
3:00	0.58	0.73	0.96	1.12	1.34	1.51	1.68

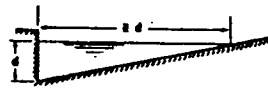
RAINFALL INTENSITY TABLE

SEDGWICK COUNTY
KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
IN INCHES PER HOUR.

DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
3:15	0.54	0.69	0.90	1.05	1.26	1.42	1.58
3:30	0.51	0.65	0.85	0.99	1.19	1.34	1.49
3:45	0.48	0.61	0.80	0.94	1.12	1.27	1.41
4:00	0.46	0.58	0.76	0.89	1.07	1.21	1.34
4:15	0.44	0.55	0.73	0.85	1.02	1.15	1.28
4:30	0.42	0.53	0.70	0.81	0.98	1.10	1.23
4:45	0.40	0.51	0.67	0.78	0.94	1.06	1.18
5:00	0.38	0.49	0.64	0.75	0.90	1.02	1.13
5:15	0.37	0.47	0.62	0.72	0.87	0.98	1.09
5:30	0.35	0.45	0.60	0.70	0.83	0.94	1.05
5:45	0.34	0.44	0.58	0.67	0.81	0.91	1.01
6:00	0.33	0.42	0.56	0.65	0.78	0.88	0.98
6:30	0.31	0.40	0.52	0.61	0.73	0.83	0.92
7:00	0.30	0.38	0.50	0.58	0.69	0.78	0.87
7:30	0.28	0.36	0.47	0.55	0.66	0.74	0.83
8:00	0.27	0.34	0.45	0.52	0.62	0.70	0.78
8:30	0.26	0.33	0.43	0.50	0.60	0.67	0.75
9:00	0.25	0.31	0.41	0.48	0.57	0.64	0.72
9:30	0.24	0.30	0.39	0.46	0.55	0.62	0.69
10:00	0.23	0.29	0.38	0.44	0.52	0.59	0.66
10:30	0.22	0.28	0.36	0.42	0.50	0.57	0.63
11:00	0.21	0.27	0.35	0.41	0.49	0.55	0.61
11:30	0.21	0.26	0.34	0.39	0.47	0.53	0.59
12:00	0.20	0.25	0.33	0.38	0.45	0.51	0.57
13:00	0.19	0.24	0.31	0.36	0.43	0.48	0.53
14:00	0.18	0.22	0.29	0.34	0.40	0.45	0.50
15:00	0.17	0.21	0.27	0.32	0.38	0.43	0.47
16:00	0.16	0.20	0.26	0.30	0.36	0.40	0.45
17:00	0.15	0.19	0.25	0.29	0.34	0.38	0.43
18:00	0.15	0.18	0.24	0.27	0.33	0.37	0.41
19:00	0.14	0.18	0.23	0.26	0.31	0.35	0.39
20:00	0.14	0.17	0.22	0.25	0.30	0.34	0.37
21:00	0.13	0.16	0.21	0.24	0.29	0.32	0.36
22:00	0.13	0.16	0.20	0.23	0.28	0.31	0.34
23:00	0.12	0.15	0.19	0.22	0.27	0.30	0.33
24:00	0.12	0.15	0.19	0.22	0.26	0.29	0.32

NOMOGRAPH FOR FLOW IN TRIANGULAR CHANNELS



EQUATION: $Q = 0.58 \left(\frac{S}{n}\right)^{2/3} d^{5/3}$
 n IS ROUGHNESS COEFFICIENT IN MANNING
 FORMULA APPROPRIATE TO MATERIAL IN
 BOTTOM OF CHANNEL
 S IS RECIPROCAL OF CROSS SLOPE
 REFERENCE: M. R. B. PROCEEDINGS 1948,
 PAGE 150, EQUATION (14)

EXAMPLE (SEE DASHED LINES)

GIVEN: $S = 0.03$
 $S = \frac{24}{1000}$
 $n = .02$ } $S/n = 1200$
 $Q = 0.22$
 FIND: $Q = 2.0$ CFS

RATIO S/n

10000
9000
8000
7000
6000
5000
4000
3000
2000
1000
900
800
700
600
500
400
300
200
100
90
80
70
60
50
40
30
20
10

TURNING LINE

DISCHARGE (Q) IN CFS

100
70
50
30
20
10
7
5
3
2
1
.7
.5
.3
.2
.1
.07
.05
.03
.02
.01

SLOPE OF CHANNEL (S) IN FT./FT.

.10
.08
.07
.06
.05
.04
.03
.02
.01
.008
.007
.006
.005
.004
.003
.002
.001

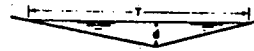
DEPTH AT CURB OR DEEPEST POINT (d) IN FT.

2.0
1.0
.80
.70
.60
.50
.40
.30
.20
.10
.08
.07
.06
.05
.04
.03
.02
.01
.01

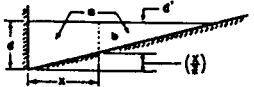
INSTRUCTIONS

1. CONNECT S/n RATIO WITH SLOPE (S) AND CONNECT DISCHARGE (Q) WITH DEPTH (d). THESE TWO LINES MUST INTERSECT AT TURNING LINE FOR COMPLETE SOLUTION.

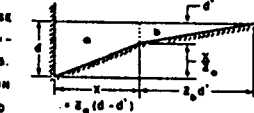
2. FOR SHALLOW V-SHAPED CHANNEL AS SHOWN USE NOMOGRAPH WITH $S = \frac{1}{4}$



3. TO DETERMINE DISCHARGE Q_1 IN PORTION OF CHANNEL HAVING WIDTH x : DETERMINE DEPTH d FOR TOTAL DISCHARGE IN ENTIRE SECTION a . THEN USE NOMOGRAPH TO DETERMINE Q_2 IN SECTION b FOR DEPTH $d' = d \left(\frac{x}{a}\right)$



4. TO DETERMINE DISCHARGE IN COMPOSITE SECTION - FOLLOW INSTRUCTION 3. TO OBTAIN DISCHARGE IN SECTION a AT ASSUMED DEPTH d ; OBTAIN Q_2 FOR SLOPE RATIO S_2 AND DEPTH d' . THEN $Q_T = Q_1 + Q_2$



One foot is 0.3048m
 One cubic foot is 0.0283m³

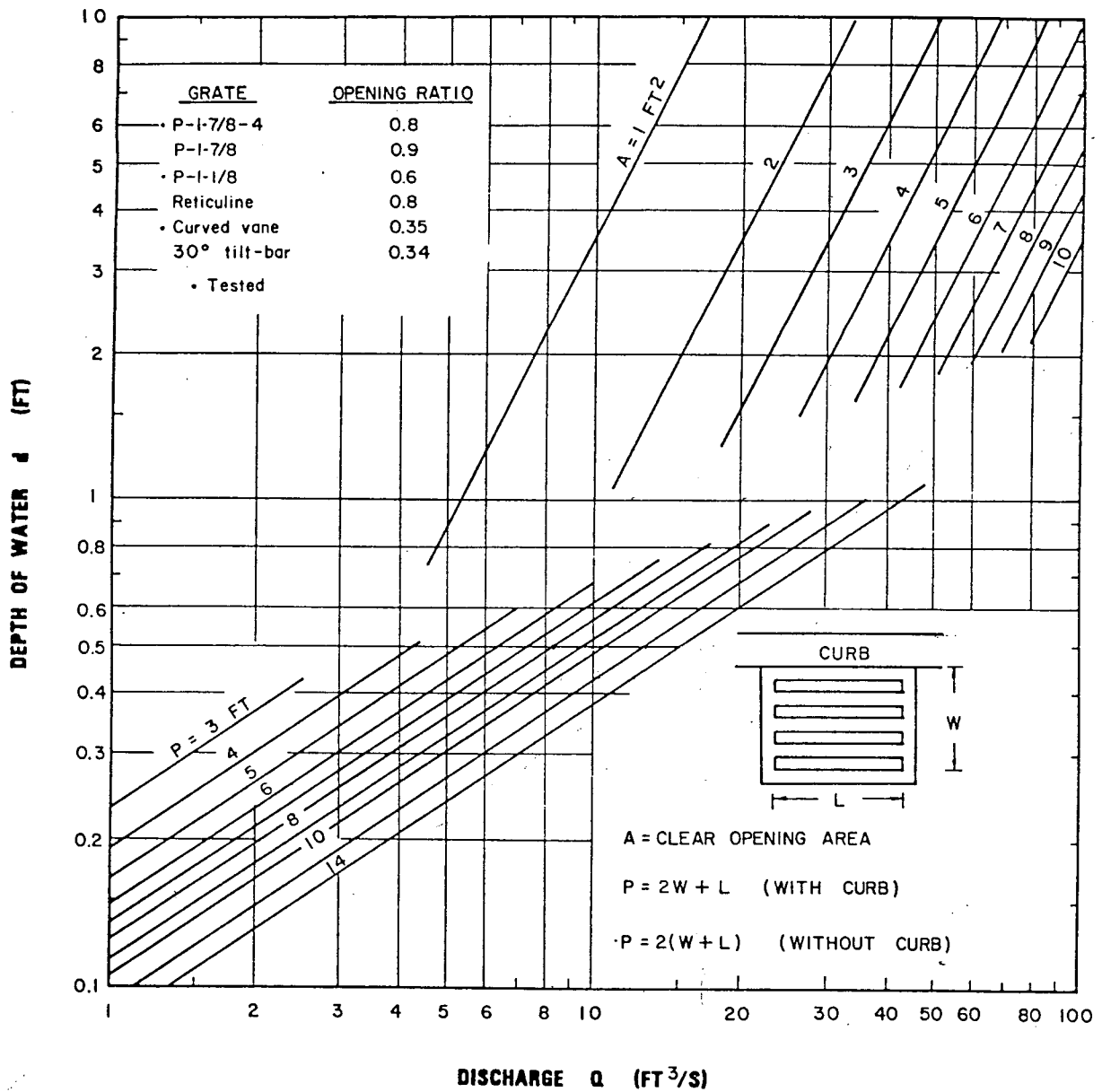
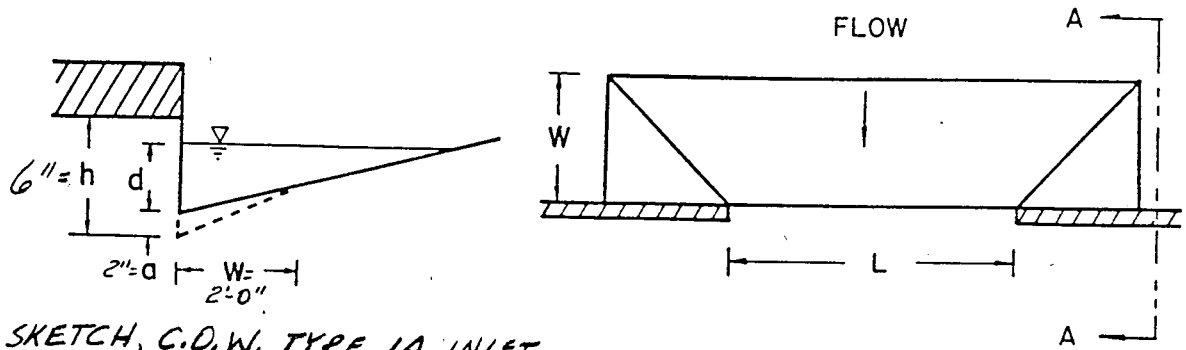


CHART 11. Grate inlet capacity in sump conditions.

From: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR 1984



DEF. SKETCH, C.D.W. TYPE 1A INLET

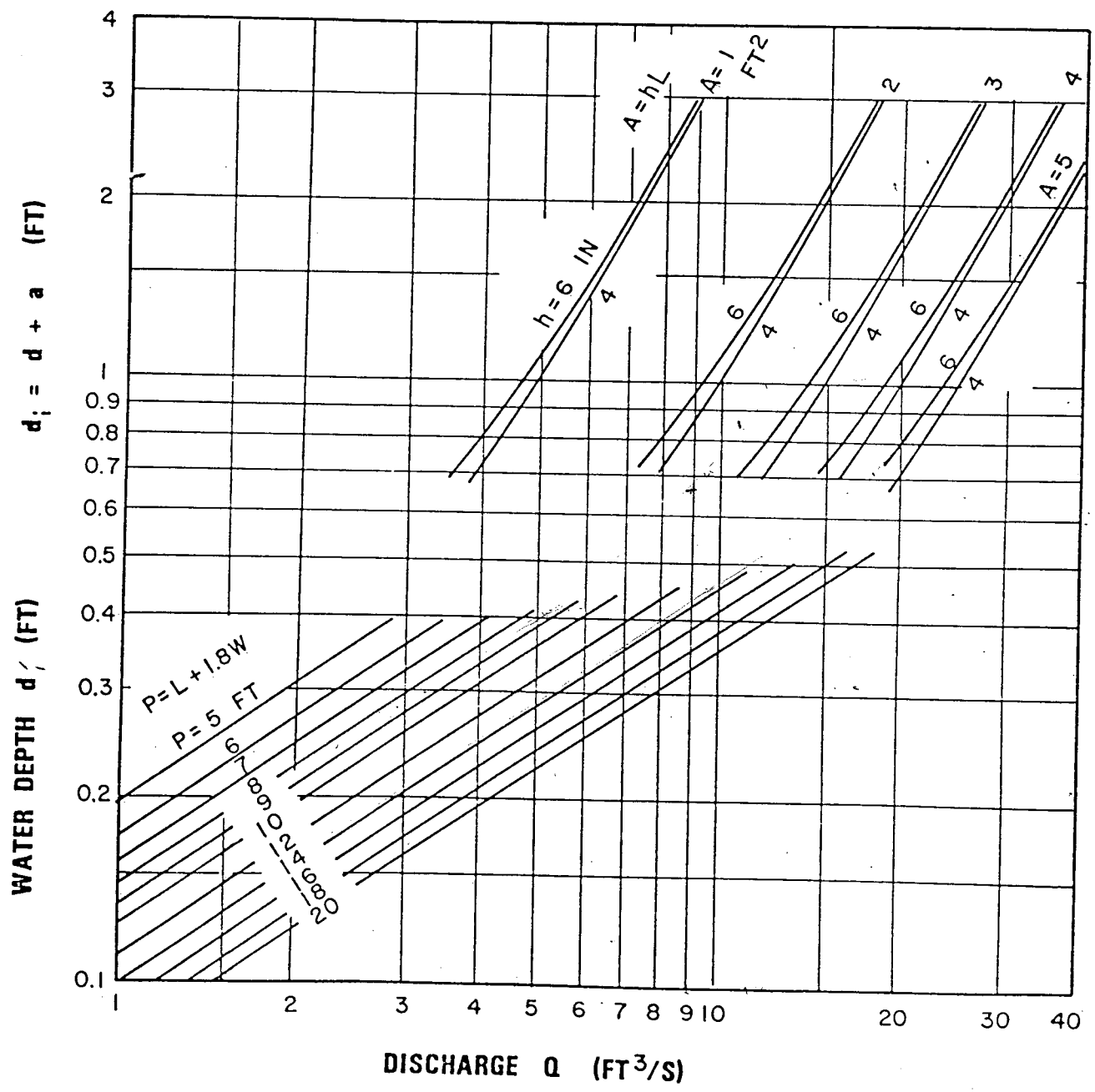
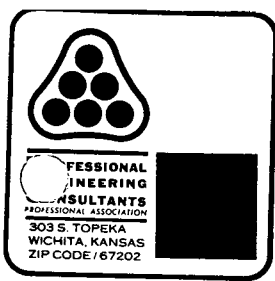


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR., 1984

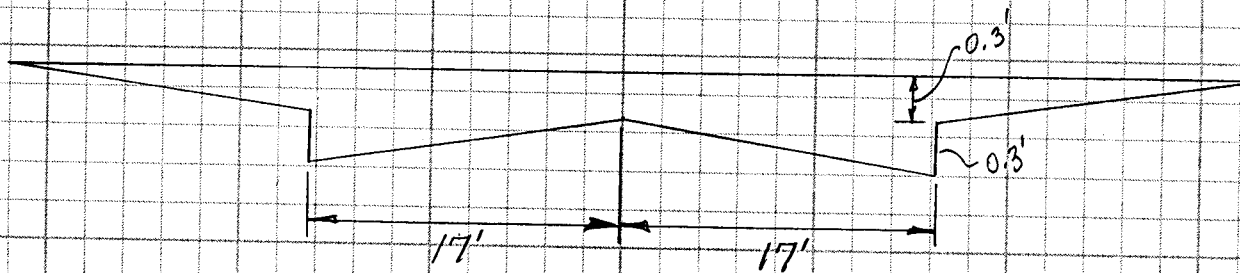


Date _____ Page _____ of _____

Project _____

Item _____

35' BK-BK, Roll Cb.
0.3' Wk.-Gd.



$$A = (.08)(34) + 2\left(\frac{1}{2}\right)(17)(.52) = 11.56 \text{ ft}^2$$

$$n = \frac{30(.016) + 2(2.3)(.013)}{(30 + 4.6)} = \frac{0.5398}{34.6} = 0.0156$$

$$R^{2/3} = \left(\frac{A}{P}\right)^{2/3} = \left(\frac{11.56}{34.6}\right)^{2/3} = 0.48$$

$$Q = \frac{1.49}{n} A R^{2/3} S^{1/2}$$

$$Q = \frac{1.49}{.0156} (11.56)(0.48) S^{1/2}$$

$$Q = 530 S^{1/2}$$

Note: 100-yr. Street Conveyance has been determined using only the street width. Flow depths up to the walk grade are used, but conveyance of the parking is neglected due the relatively high 'n' value.