



Professional Engineering Consultants, P.A.

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Angel Acres 2nd Addition **Wichita, Sedgwick County, Kansas**

04/20/00

Angel Acres 2nd Addition is a residential development at 50th Street South and Seneca in Wichita, Kansas. The plat is 19.4 acres, which will be split into 44 lots, streets and a detention pond. The 19 acres consists of primarily Canadian fine sandy loam soil that is nearly level in the S ½, N ½, S ½, NW ¼ Section 20, T28S, R1E of the 6th P.M. The drainage plan, supporting computations and data for the Angel Acres 2nd Addition drainage plan are presented herein.

Hydrology

The existing landscape is nearly level cultivated agricultural field. The surrounding area is predominately flat so very little offsite drainage was included in the study of this property. An existing residential development borders this property on the south. It appears that a small portion of the plat to the south (South Seneca Gardens 2nd Addition) drains onto Angel Acres 2nd. Drainage for this small portion has been accounted for in this drainage plan. Currently most of the plat drains to the west to a low point in the field on the east side of Seneca. During a large storm, storm water may flow into Seneca and then north until it reaches the ditch to the west. About 4 acres of the plat drains to the east. This corresponds with the South Wichita Drainage Study by Van Doren, Hazard and Stallings. That study also illustrates a proposed detention pond (pond 7) to the east of this plat.

The proposed drainage plan for Angel Acres 2nd follows the VHS study and discharges water to the east to an apparent low area east of McLean Blvd right-of-way. For the western part of the property a detention pond is proposed. The pond would be a groundwater pond, to enable adequate depth for a storm sewer system outfall and provide fill material for site grading. The pond will have a controlled discharge into the 7'x3' RCB that is under Seneca north of this plat. The pond will serve to provide detention for the western portion of this plat and will reduce runoff significantly.

The pond level will be controlled by the groundwater elevation but the pond outlet elevation was used as the static water surface elevation for storage volume calculations.

Runoff coefficients were estimated based on tables presented in the Design Aids section and existing land use. A map showing the basin boundaries, drainage calculations and HEC-HMS outputs are included.

The analysis made is based on the available site data which includes the following: 1" = 100' topographic map with 1' contours of the site and adjacent areas, USGS topographic map, Sedgwick County Soil Survey Map and references noted herein.

Storm Sewer Design

The preliminary design for a storm sewer system has been completed at this time. The storm sewer system was designed for the 10-year storm event and the condition that water would be contained within the curb area. The 100-year storm event will be contained within the street right-of-way and adjacent reserves and easements.

Anticipating the development of the adjacent land to the north minimized storm sewer requirements. If the north tier of lots is graded toward the street, backyard storm sewer can be eliminated. The same thing can be done on the property to the north if that land is developed similarly. Excess excavation from the pond can be used to slope the ground with a gentle slope from the north property line of Angel Acres 2nd to the north onto the existing farmland. This decreases the amount of storm sewer required and allows the ground to continue to be farmed at this time.

Design Aids

This section includes material used to assist in designing the drainage system. A 1"=100' scale drainage plan map is enclosed in the pocket.

References

Interim Drainage and Storm Sewer Policy for Design Criteria and Documentation, City of Wichita, Kansas, 1985.

Rainfall Intensity Tables for Counties in Kansas, Kansas Department of Transportation, June 1997.

Rainfall Maps based on Storm Intensity and Duration, Dodson and Associates, Inc., Hydrologists and Civil Engineers.

Soil Survey of Sedgwick County, Kansas, US Department of Agriculture, Soil Conservation Service, 1979.

Existing Conditions

Basin	Area (acres)	t_c	CN	Q_2 (cfs)	Q_{10} (cfs)	Q_{100} (cfs)
West	19.28	65 min	71	8.6	20.8	40.3
East	4.03	23 min	71	3	6.8	13.1

Developed Conditions

Time of Concentration = 15 minutes

CN = 75

Basin	Area (acres)	Q_2 (cfs)	Q_{10} (cfs)	Q_{100} (cfs)
West				
A	4.44	4.9	10.4	18.7
B	1.14	1.3	2.7	4.8
C	4.56	5	10.7	19.2
D	0.18	0.2	0.4	0.8
E	0.19	0.2	0.4	0.8
F	0.7	0.8	1.6	3
G	0.99	1.1	2.3	4.2
H	2.38	2.6	5.6	10
I	0.75	0.8	1.8	3.2
J	2.75	3	6.5	11.6
K	1.2	1.3	2.8	5.1
Total	19.28	21.2	45.2	81.4
Pond Discharge		3.2	7.9	15.4

East				
L	1.77	2	4.1	7.5
M	0.83	0.9	1.9	3.5
N	0.88	1	2.1	3.7
O	0.55	0.6	1.3	2.3
Total Out		4.5	9.4	17



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Project Angel Acres 2nd 36-00081
Item Drainage Plan

Date 3/16/00
By KER

Existing Conditions

Pasture / field with one house & some trees & shrubs adjacent to an existing residential development

Water currently ponds on the property east of Seneca in a low spot

The eastern part of the property is relatively flat with no apparent drainage outlet.

Ground water is about 76" according to one soil boring taken at the edge of the property and Spawmore Ave.

Time of Concentration

West

A = 19.28 Ac

Length = 1950'

slope = 0.5%

Velocity = 0.5 ft/s

$t_c = 65$ min

East

Area = 4.03 acres

Length = 700'

slope = 0.5%

$t_c = 23$ min

Soil Types (Sedgwick County Soil Survey Sh. No 58)

Ca - "B" Canadian fine sandy loam, nearly level

Cb - "B & C" Canadian - Waldeck fine sandy loam
70% B & 30% C

CN = 71

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 11

Start of Simulation : 11Jan00 0000 Basin Model : Existing Conditions
End of Simulation : 12Jan00 0600 Precip Model : 2-Year Storm
Execution Time : 19Apr00 1140 Control Specs : 24 Hour Rainfall

Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
West	8.6003	11 Jan 00 1230	1.6996	0.030125
East	2.9531	11 Jan 00 1210	0.35527	0.006296875
Sink-1	10.028	11 Jan 00 1230	2.0549	0.036421875

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 12

Start of Simulation : 11Jan00 0000 Basin Model : Existing Conditions
End of Simulation : 12Jan00 0600 Precip Model : 10-Year Storm
Execution Time : 19Apr00 1140 Control Specs : 24 Hour Rainfall

Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
West	20.845	11 Jan 00 1230	3.7481	0.030125
East	6.7594	11 Jan 00 1210	0.78346	0.006296875
Sink-1	23.893	11 Jan 00 1230	4.5316	0.036421875

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 13

Start of Simulation : 11Jan00 0000	Basin Model : Existing Conditions
End of Simulation : 12Jan00 0600	Precip Model : 100-Year Storm
Execution Time : 19Apr00 1141	Control Specs : 24 Hour Rainfall

Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
West	40.314	11 Jan 00 1230	7.0398	0.030125
East	13.146	11 Jan 00 1200	1.4715	0.006296875
Sink-1	45.956	11 Jan 00 1220	8.5113	0.036421875

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 3

Start of Simulation : 11Jan00 0000 Basin Model : Developed Conditions
 End of Simulation : 12Jan00 0600 Precip Model : 2-Year Storm
 Execution Time : 19Apr00 1051 Control Specs : 24 Hour Rainfall

Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
A	4.9126	11 Jan 00 1200	0.47901	0.0069375
B	1.2613	11 Jan 00 1200	0.12299	0.00178125
C	5.0454	11 Jan 00 1200	0.49195	0.007125
D	0.19916	11 Jan 00 1200	0.019419	0.00028125
E	0.21022	11 Jan 00 1200	0.020498	0.000296875
F	0.77451	11 Jan 00 1200	0.075519	0.00109375
G	1.0954	11 Jan 00 1200	0.10681	0.001546875
H	2.6333	11 Jan 00 1200	0.25676	0.00371875
I	0.82983	11 Jan 00 1200	0.080913	0.001171875
J	3.0427	11 Jan 00 1200	0.29668	0.004296875
K	1.3277	11 Jan 00 1200	0.12946	0.001875
L	1.9584	11 Jan 00 1200	0.19095	0.002765625
M	0.91835	11 Jan 00 1200	0.089544	0.001296875
N	0.97210	11 Jan 00 1200	0.094785	0.001372787
O	0.60315	11 Jan 00 1200	0.058811	0.000851763
Reservoir 2	3.8344	11 Jan 00 1300	2.4688	0.036412058

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 4

Start of Simulation : 11Jan00 0000 Basin Model : Developed Conditions
 End of Simulation : 12Jan00 0600 Precip Model : 10-Year Storm
 Execution Time : 19Apr00 1050 Control Specs : 24 Hour Rainfall

Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
A	10.429	11 Jan 00 1200	0.99153	0.0069375
B	2.6777	11 Jan 00 1200	0.25458	0.00178125
C	10.711	11 Jan 00 1200	1.0183	0.007125
D	0.42279	11 Jan 00 1200	0.040197	0.00028125
E	0.44628	11 Jan 00 1200	0.042430	0.000296875
F	1.6442	11 Jan 00 1200	0.15632	0.00109375
G	2.3253	11 Jan 00 1200	0.22108	0.001546875
H	5.5902	11 Jan 00 1200	0.53150	0.00371875
I	1.7616	11 Jan 00 1200	0.16749	0.001171875
J	6.4593	11 Jan 00 1200	0.61412	0.004296875
K	2.8186	11 Jan 00 1200	0.26798	0.001875
L	4.1574	11 Jan 00 1200	0.39527	0.002765625
M	1.9495	11 Jan 00 1200	0.18535	0.001296875
N	2.0636	11 Jan 00 1200	0.19620	0.001372787
O	1.2804	11 Jan 00 1200	0.12174	0.000851763
Reservoir 2	9.9479	11 Jan 00 1240	5.1081	0.036412050

HMS * Summary of Results

Project : angelacres2nd

Run Name : Run 2

Start of Simulation : 11Jan00 0000 Basin Model : Developed Conditions
 End of Simulation : 12Jan00 0600 Precip Model : 100-Year Storm
 Execution Time : 19Apr00 1051 Control Specs : 24 Hour Rainfall


Hydrologic Element	Discharge Peak (cfs)	Time of Peak	Total Volume (ac ft)	Drainage Area (sq mi)
A	18.733	11 Jan 00 1200	1.7888	0.0069375
B	4.8098	11 Jan 00 1200	0.45930	0.00178125
C	19.239	11 Jan 00 1200	1.8372	0.007125
D	0.75944	11 Jan 00 1200	0.072521	0.00028125
E	0.80163	11 Jan 00 1200	0.076550	0.000296875
F	2.9534	11 Jan 00 1200	0.28202	0.00109375
G	4.1769	11 Jan 00 1200	0.39886	0.001546875
H	10.041	11 Jan 00 1200	0.95888	0.00371875
I	3.1643	11 Jan 00 1200	0.30217	0.001171875
J	11.603	11 Jan 00 1200	1.1080	0.004296875
K	5.0629	11 Jan 00 1200	0.48347	0.001875
L	7.4678	11 Jan 00 1200	0.71312	0.002765625
M	3.5019	11 Jan 00 1200	0.33440	0.001296875
N	3.7068	11 Jan 00 1200	0.35397	0.001372787
O	2.3000	11 Jan 00 1200	0.21963	0.000851763
Reservoir 2	17.915	11 Jan 00 1240	9.2379	0.036412050

Detention Pond Design

<u>Elev</u>	<u>Area</u>	<u>Storage Volume</u> (total)	
78	1.39 Ac	0	Ac-ft
79	1.53 Ac	1.46	Ac-ft
80	1.68 Ac	3.065	Ac-ft
81	1.83 Ac	4.82	Ac-ft
82	2.03 Ac	6.75	Ac-ft

Rating Curve for 24" RCP

$L = 275'$
 $\# \text{ In} = 78.0$
 $\# \text{ Out} = 76.84$

Tailwater
 3' channel 
 $n = 0.012$
 $s = 0.002$

<u>Q</u>	<u>Elevation</u>	<u>Elev.</u>	<u>Q (cfs)</u>
0 cfs	78	78	0
6 cfs	79.21	79	5
12 cfs	79.88	80	13.4
18 cfs	80.4	81	19.8
24 cfs	82.43	82	22.7
30 cfs	84.60		

Basins A-K are directed to pond
 L-N are directed off-site

2-yr	HW = 78.6	$Q_{\text{out}} = 3.2 \text{ cfs}$
10-yr	HW = 79.3	$Q_{\text{out}} = 7.9 \text{ cfs}$
100-yr	HW = 80.4	$Q_{\text{out}} = 15.4 \text{ cfs}$

HMS * Summary of Results for Reservoir-2

Project : angelacres2nd

Run Name : Run 6

Start of Simulation : 11Jan00 0000 Basin Model : Pond Only
End of Simulation : 12Jan00 0600 Precip Model : 2-Year Storm
Execution Time : 19Apr00 1536 Control Specs : 24 Hour Rainfall

Computed Results

Peak Inflow : 21.332 (cfs) Date/Time of Peak Inflow : 11 Jan 00 1200
Peak Outflow : 3.1724 (cfs) Date/Time of Peak Outflow : 11 Jan 00 1300
Total Inflow : 1.29 (in) Peak Storage : 0.92633(ac-ft)
Total Outflow : 1.27 (in) Peak Elevation : 78.634(ft)

HMS * Summary of Results for Reservoir-2

Project : angelacres2nd

Run Name : Run 8

Start of Simulation : 11Jan00 0000 Basin Model : Pond Only
End of Simulation : 12Jan00 0600 Precip Model : 10-Year Storm
Execution Time : 19Apr00 1537 Control Specs : 24 Hour Rainfall

Computed Results

Peak Inflow : 45.285 (cfs) Date/Time of Peak Inflow : 11 Jan 00 1200
Peak Outflow : 7.8837 (cfs) Date/Time of Peak Outflow : 11 Jan 00 1250
Total Inflow : 2.68 (in) Peak Storage : 2.0110(ac-ft)
Total Outflow : 2.63 (in) Peak Elevation : 79.343(ft)

HMS * Summary of Results for Reservoir-2

Project : angelacres2nd

Run Name : Run 10

Start of Simulation : 11Jan00 0000 Basin Model : Pond Only
End of Simulation : 12Jan00 0600 Precip Model : 100-Year Storm
Execution Time : 19Apr00 1538 Control Specs : 24 Hour Rainfall

Computed Results

Peak Inflow : 81.345 (cfs) Date/Time of Peak Inflow : 11 Jan 00 1200
Peak Outflow : 15.386 (cfs) Date/Time of Peak Outflow : 11 Jan 00 1240
Total Inflow : 4.83 (in) Peak Storage : 3.6873(ac-ft)
Total Outflow : 4.76 (in) Peak Elevation : 80.355(ft)

CURRENT DATE: 04-18-2000
 CURRENT TIME: 16:46:30

FILE DATE: 04-18-2000
 FILE NAME: ANGEL2ND

FHWA CULVERT ANALYSIS
 HY-8, VERSION 6.1

C U L V N O.	SITE DATA			CULVERT SHAPE, MATERIAL, INLET				
	INLET ELEV. (ft)	OUTLET ELEV. (ft)	CULVERT LENGTH (ft)	BARRELS SHAPE MATERIAL	SPAN (ft)	RISE (ft)	MANNING n	INLET TYPE
1	78.00	76.84	275.00	1 RCP	2.00	2.00	.012	CONVENTIONAL
2								
3								
4								
5								
6								

SUMMARY OF CULVERT FLOWS (cfs)

FILE: ANGEL2ND

DATE: 04-18-2000

ELEV (ft)	TOTAL	1	2	3	4	5	6	ROADWAY	ITR
78.00	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
79.21	6.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
79.88	12.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
80.40	18.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
82.43	24.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
84.60	30.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
87.25	36.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
89.41	40.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
94.34	48.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
98.58	54.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
103.28	60.0	0.0	0.0	0.0	0.0	0.0	0.0	0.00	0
0.00	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	OVERTOPPING

SUMMARY OF ITERATIVE SOLUTION ERRORS

FILE: ANGEL2ND

DATE: 04-18-2000

HEAD ELEV (ft)	HEAD ERROR (ft)	TOTAL FLOW (cfs)	FLOW ERROR (cfs)	% FLOW ERROR
78.00	0.000	0.00	0.00	0.00
79.21	0.000	6.00	0.00	0.00
79.88	0.000	12.00	0.00	0.00
80.40	0.000	18.00	0.00	0.00
82.43	0.000	24.00	0.00	0.00
84.60	0.000	30.00	0.00	0.00
87.25	0.000	36.00	0.00	0.00
89.41	0.000	40.00	0.00	0.00
94.34	0.000	48.00	0.00	0.00
98.58	0.000	54.00	0.00	0.00
103.28	0.000	60.00	0.00	0.00

<1> TOLERANCE (ft) = 0.010

<2> TOLERANCE (%) = 1.000

CURRENT DATE: 04-18-2000
CURRENT TIME: 16:46:30

FILE DATE: 04-18-2000
FILE NAME: ANGEL2ND

TAILWATER

***** REGULAR CHANNEL CROSS SECTION *****

BOTTOM WIDTH 3.00 ft
SIDE SLOPE H/V (X:1) 0.0
CHANNEL SLOPE V/H (ft/ft) 0.002
MANNING'S n (.01-0.1) 0.012
CHANNEL INVERT ELEVATION 76.59 ft
CULVERT NO.1 OUTLET INVERT ELEVATION 76.84 ft

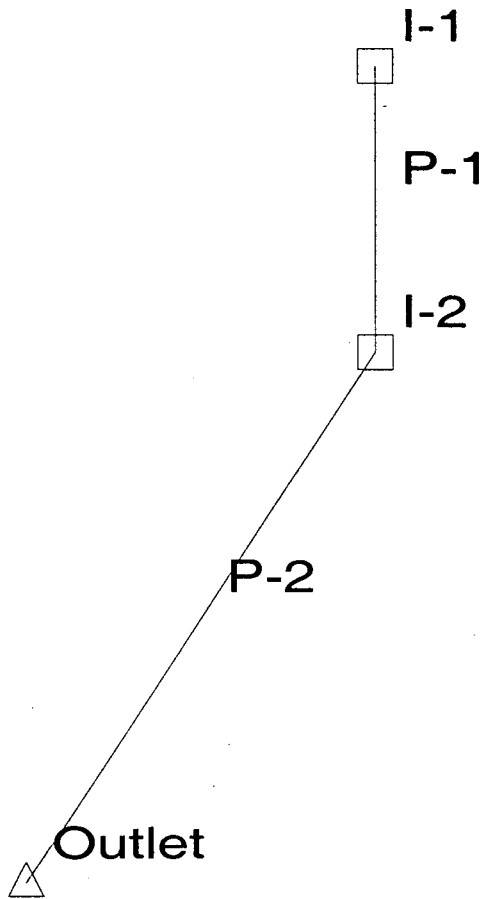
***** UNIFORM FLOW RATING CURVE FOR DOWNSTREAM CHANNEL

FLOW (cfs)	W.S.E. (ft)	FROUDE NUMBER	DEPTH (ft)	VEL. (f/s)	SHEAR (psf)
0.00	76.59	0.000	0.00	0.00	0.00
6.00	77.21	0.715	0.62	3.21	0.08
12.00	77.60	0.693	1.01	3.96	0.13
18.00	77.95	0.668	1.36	4.42	0.17
24.00	78.28	0.644	1.69	4.75	0.21
30.00	78.59	0.623	2.00	5.00	0.25
36.00	78.90	0.603	2.31	5.20	0.29
40.00	79.10	0.591	2.51	5.31	0.31
48.00	79.50	0.568	2.91	5.50	0.36
54.00	79.79	0.553	3.20	5.62	0.40
60.00	80.09	0.539	3.50	5.72	0.44

ROADWAY OVERTOPPING DATA

ROADWAY SURFACE PAVED
EMBANKMENT TOP WIDTH 40.00 ft
CREST LENGTH 300.00 ft
OVERTOPPING CREST ELEVATION 84.00 ft

SWS Line No. 1



----- Beginning Calculation Cycle -----

Discharge: 10.40 cfs at node I-1
 Discharge: 13.10 cfs at node I-2
 Discharge: 13.10 cfs at node Outlet
 Beginning iteration 1
 Discharge: 10.40 cfs at node I-1
 Discharge: 13.10 cfs at node I-2
 Discharge: 13.10 cfs at node Outlet
 Discharge Convergence Achieved in 1 iterations: relative error: 0.0
 Warning: No Duration data exists in IDF Table
 Information: P-1 Surcharged condition
 ----- Calculations Complete -----

** Analysis Options **

Friction method: Manning's Formula
 HGL Convergence Test: 0.001000
 Maximum Network Traversals: 5
 Number of Pipe Profile Steps: 5
 Discharge Convergence Test: 0.001000
 Maximum Design Passes: 3

----- Network Quick View -----

Label	Length	Size	Discharge	Hydraulic Grade	
				Upstream	Downstream
P-1	35.30	18 inch	10.40	81.40	81.05
P-2	85.00	18 inch	13.10	80.62	79.30

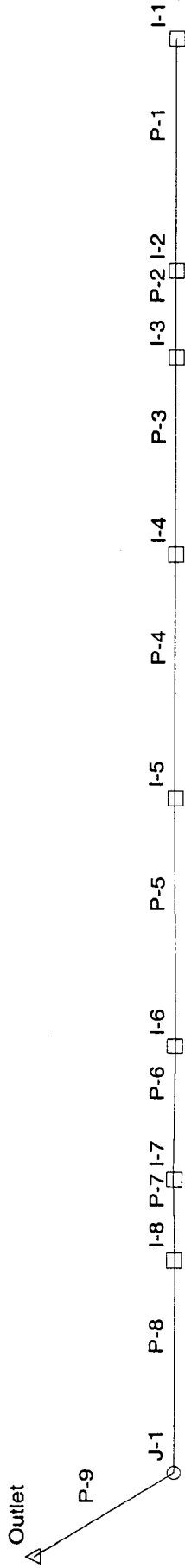
Label	Discharge	Elevations		
		Ground	Upstream HGL	Downstream HGL
I-1	10.40	81.97	81.40	81.40
I-2	13.10	81.97	81.05	80.62
Outlet	13.10	83.00	79.30	79.30

Elapsed: 0 minute(s) 0 second(s)

Pipe Report

Pipe	Upstream Node	Downstream Node	Additional Flow (cfs)	Discharge (cfs)	Length (ft)	Constructed Slope (ft/ft)	Section Size	Roughness	Capacity (cfs)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Upstream Ground Elevation (ft)	Downstream Ground Elevation (ft)	Upstream Cover (ft)	Downstream Cover (ft)	Upstream HGL (ft)	Downstream HGL (ft)	Average Velocity (ft/s)
P-1	I-1	I-2	10.40	10.40	35.30	0.003966	18 inch	0.013	6.61	78.46	78.32	81.97	81.97	2.01	2.15	81.40	81.05	5.89
P-2	I-2	Outlet	2.70	13.10	85.00	0.004941	18 inch	0.013	7.38	78.22	77.80	81.97	83.00	2.25	3.70	80.62	79.30	7.41
	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

SWS Line No. 2



Storm Sewer System Design for
10-yr Storm

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----- Beginning Calculation Cycle -----
Discharge: 2.82 cfs at node I-1
Discharge: 9.28 cfs at node I-2
Discharge: 14.87 cfs at node I-3
Discharge: 16.63 cfs at node I-4
Discharge: 18.96 cfs at node I-5
Discharge: 20.60 cfs at node I-6
Discharge: 21.05 cfs at node I-7
Discharge: 21.47 cfs at node I-8
Discharge: 21.47 cfs at node J-1
Discharge: 21.47 cfs at node Outlet
Beginning iteration 1
Discharge: 2.82 cfs at node I-1
Discharge: 9.28 cfs at node I-2
Discharge: 14.87 cfs at node I-3
Discharge: 16.63 cfs at node I-4
Discharge: 18.96 cfs at node I-5
Discharge: 20.60 cfs at node I-6
Discharge: 21.05 cfs at node I-7
Discharge: 21.47 cfs at node I-8
Discharge: 21.47 cfs at node J-1
Discharge: 21.47 cfs at node Outlet
Discharge Convergence Achieved in 1 iterations: relative error: 0.0
Warning: No Duration data exists in IDF Table
Information: P-9 Surcharged condition
Information: P-8 Surcharged condition
Information: P-7 Surcharged condition
Information: P-6 Surcharged condition
Information: P-5 Surcharged condition
Information: P-4 Surcharged condition
Information: P-3 Surcharged condition
Information: P-2 Surcharged condition
Information: P-1 Surcharged condition
----- Calculations Complete -----

```

```

** Analysis Options **
Friction method: Manning's Formula
HGL Convergence Test: 0.001000
Maximum Network Traversals: 5
Number of Pipe Profile Steps: 5
Discharge Convergence Test: 0.001000
Maximum Design Passes: 3

```

----- Network Quick View -----

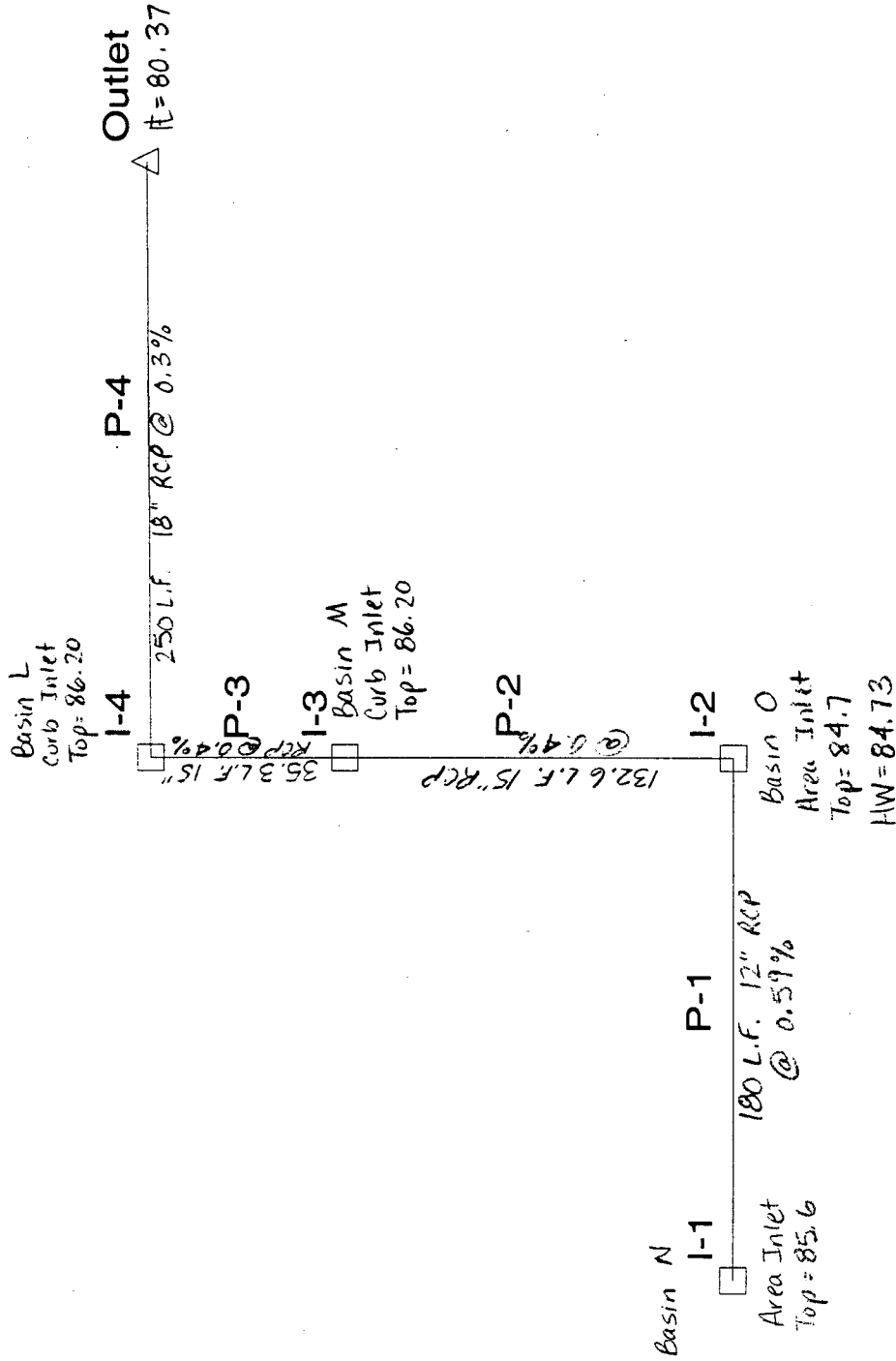
Label	Length	Size	Discharge	Hydraulic Grade	
				Upstream	Downstream
P-1	193.30	15 inch	2.82	84.60	84.23
P-2	37.30	18 inch	9.28	84.01	83.72
P-3	191.30	24 inch	14.87	83.55	82.72
P-4	270.00	30 inch	16.63	82.63	82.19
P-5	270.00	30 inch	18.96	82.07	81.50
P-6	103.30	30 inch	20.60	81.36	81.10
P-7	37.40	30 inch	21.05	80.96	80.86
P-8	325.00	30 inch	21.47	80.71	79.82
P-9	135.00	30 inch	21.47	79.67	79.30

Label	Discharge	Elevations		
		Ground	Upstream HGL	Downstream HGL
Outlet	21.47	81.00	79.82	79.67
I-1	2.82	84.70	84.60	84.60
I-2	9.28	85.06	84.23	84.01
I-3	14.87	85.06	83.72	83.55

I-4	16.63	84.20	82.72	82.63
I-5	18.96	83.30	82.19	82.07
I-6	20.60	83.20	81.50	81.36
I-7	21.05	83.78	81.10	80.96
I-8	21.47	83.78	80.86	80.71

apsed: 0 minute(s) 1 second(s)

SWS Line No. 3



----- Beginning Calculation Cycle -----

Discharge: 2.10 cfs at node I-1
 Discharge: 3.40 cfs at node I-2
 Discharge: 5.30 cfs at node I-3
 Discharge: 9.50 cfs at node I-4
 Discharge: 9.50 cfs at node Outlet

Beginning iteration 1

Discharge: 2.10 cfs at node I-1
 Discharge: 3.40 cfs at node I-2
 Discharge: 5.30 cfs at node I-3
 Discharge: 9.50 cfs at node I-4
 Discharge: 9.50 cfs at node Outlet

Discharge Convergence Achieved in 1 iterations: relative error: 0.0

** Problem: Flooding in system

Warning: No Duration data exists in IDF Table

Information: P-3 Surcharged condition

Information: P-2 Surcharged condition

Information: I-2 The hydraulic grade exceeds the Rim/Ground elevation

Information: I-2 Flooding condition.

Information: P-1 Surcharged condition

----- Calculations Complete -----

** Analysis Options **

Friction method: Manning's Formula

HGL Convergence Test: 0.001000

Maximum Network Traversals: 5

Number of Pipe Profile Steps: 5

Discharge Convergence Test: 0.001000

Maximum Design Passes: 3

----- Network Quick View -----

Label	Length	Size	Discharge	Hydraulic Grade	
				Upstream	Downstream
P-1	180.00	12 inch	2.10	85.33	84.70
P-2	132.60	15 inch	3.40	84.73	84.36
P-3	35.30	15 inch	5.30	84.22	83.98
P-4	250.00	18 inch	9.50	83.75	81.56

Label	Discharge	Elevations		
		Ground	Upstream HGL	Downstream HGL
I-1	2.10	85.60	85.33	85.33
I-2	3.40	84.70	84.70	84.70
I-3	5.30	86.20	84.36	84.22
I-4	9.50	86.20	83.98	83.75
Outlet	9.50	86.00	81.56	81.56

Elapsed: 0 minute(s) 1 second(s)

Pipe Report

Pipe	Upstream Node	Downstream Node	Additional Flow (cfs)	Discharge (cfs)	Length (ft)	Constructed Slope (ft/ft)	Section Size	Roughness	Capacity (cfs)	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	Upstream Ground Elevation (ft)	Downstream Ground Elevation (ft)	Upstream Cover (ft)	Downstream Cover (ft)	Upstream HGL (ft)	Downstream HGL (ft)	Average Velocity (ft/s)
P-1	I-1	I-2	2.10	2.10	180.00	0.005889	12 inch	0.013	2.73	83.16	82.10	85.60	84.70	1.44	1.60	85.33	84.70	2.67
P-2	I-2	I-3	1.30	3.40	132.60	0.004072	15 inch	0.013	4.12	82.00	81.46	84.70	86.20	1.45	3.49	84.73	84.36	2.77
P-3	I-3	I-4	1.90	5.30	35.30	0.003966	15 inch	0.013	4.07	81.36	81.22	86.20	86.20	3.59	3.73	84.22	83.98	4.32
P-4	I-4	Outlet	4.20	9.50	250.00	0.003000	18 inch	0.013	5.75	81.12	80.37	86.20	86.00	3.58	4.13	83.75	81.56	5.85
N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A	N/A

Inlet Sizing - 10-Yr Storm

Basin	Q ₁₀	Inlet Type	Location	Size	depth
A	10.4	Curb	Sump	10'	0.47'
B	2.7	Curb	Sump	5'	0.28'
C	10.7	Pond	—	—	—
D	0.4	Curb	Sump	5'	0.18'
E	0.4	Curb	Sump	5'	0.18'
F	1.6	Area	Sump	2'x4'	0.14'
G	2.3	Area	Sump	2'x2'	0.18'
H	5.6	Curb	Sump	10'	0.32' (0.37' overflow)
I	1.8	Area	Sump	2'x4'	0.15'
J	6.5	Curb	Sump	10'	0.35' (0.37' overflow)
K	2.8	Area	Sump	2'x2'	0.15'
L	4.2	Curb	Sump	5'	0.25'
M	1.9	Curb	Sump	5'	0.37'
N	2.1	Area	Sump	2'x4'	0.21'
O	1.3	Area	Sump	2'x2'	0.15'
				2'x4'	0.20'
				2'x4'	0.11'
				2'x2'	0.16'

Curb Inlet were assumed to be City of Wichita Type 1A curb inlets.

Area Inlets were assumed to be standard City of Wichita 2' x 4' area inlets.

2'x4' P=12' A=4 sq. ft (50% open area)
 2'x2' P=8' A=2 sq. ft (50% open area)

Recommend using 2' x 2' area inlets as standard for this project.

Street Capacity

assume slope = 0.5% (Min)

Basin	Q ₁₀	Depth	Q ₁₀₀	depth	Comments
A	10.4	0.47'	18.7	0.58'	OK
B	2.7	0.29'	4.8	0.35'	OK
D	0.4	0.14'	0.8	0.18'	OK
E	0.4	0.14'	0.8	0.18'	OK
H	5.6	0.38'	10.0	0.47'	OK
J	6.5	0.39'	11.6	0.49'	OK
L	4.2	0.34'	7.5	0.42'	OK
M	1.9	0.25'	3.5	0.31'	OK

The combined 100-year flows of adjacent basins does not exceed the maximum street capacity of 53.7 cfs.

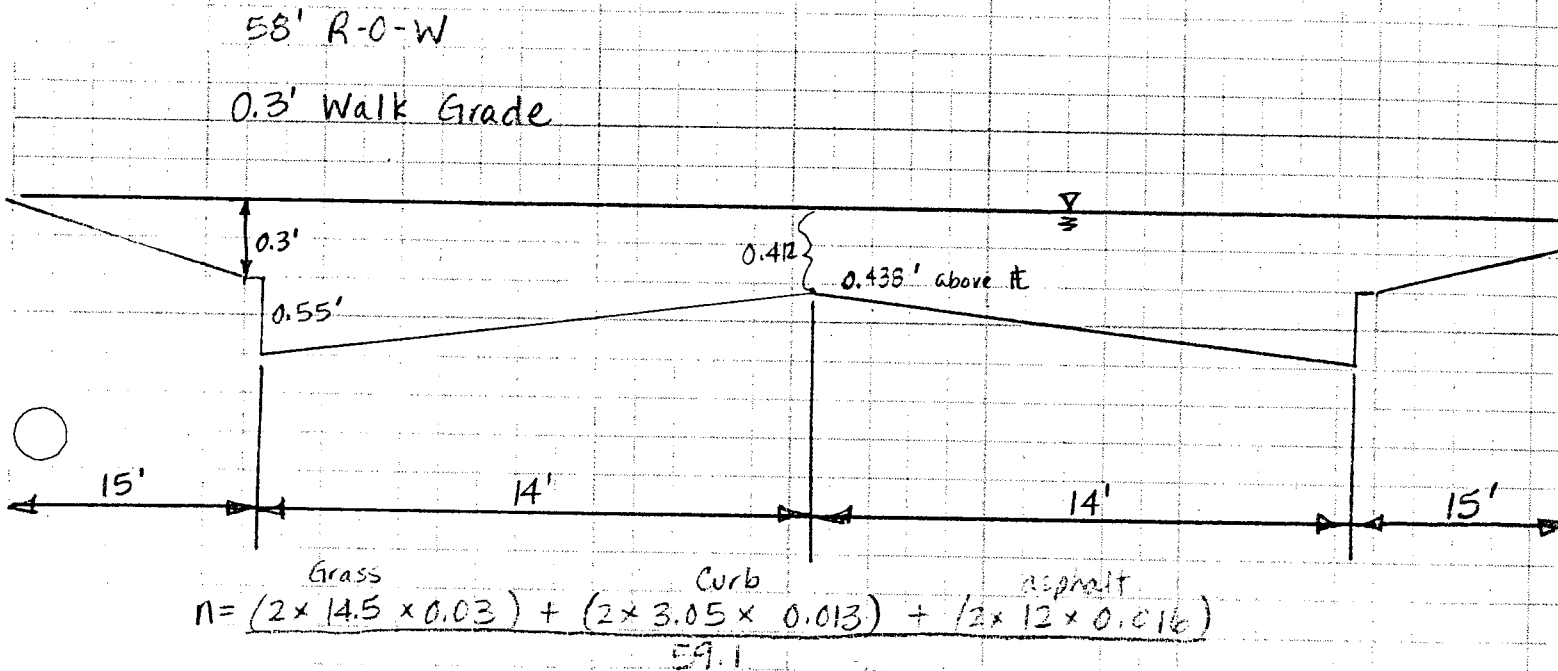
In large storms basin A will overtop the street crown and combine with basin B. In the 100-year storm this flow is still contained in the right-of-way.

Recommend using 0.3' walk grade and standard height curb.



13 S. TOPEKA - WICHITA, KANSAS 67202
 316-262-2691 - FAX 316-262-3003
 www.pec1.com - designers@pec1.com

Project Angel Acres 2nd Addition Date 4/19/00
 Item Street Capacity - 100-yr storm By KER



$$= \frac{0.87 + 0.0793 + 0.384}{59.1} = \frac{1.3333}{59.1} = 0.02256$$

$$A = (2 \times \frac{1}{2} \times 0.3 \times 15) + (28 \times 0.412) + (2 \times \frac{1}{2} \times 0.438 \times 14)$$

$$A = 22.168 \text{ sq. ft}$$

$$P = 59.1 \text{ ft}$$

$$R = A/P = 0.375093062$$

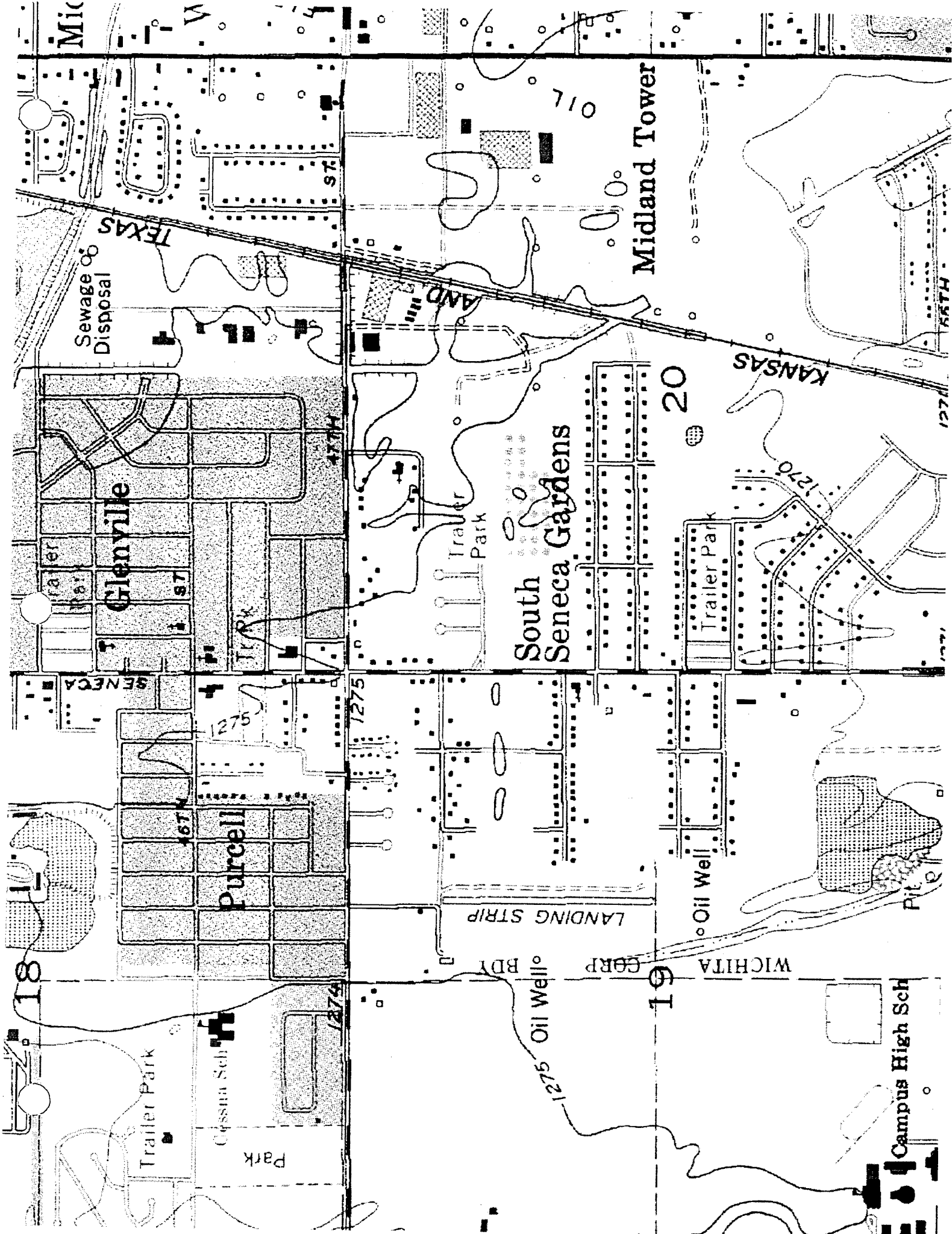
$$R^{2/3} = 0.520106986$$

$$Q = \frac{1.486}{n} \times A \times R^{2/3} \sqrt{S}$$

$$Q_{\text{max}} = 53.7 \text{ cfs}$$

$$Q = 759.45 \sqrt{S}$$

$$S_{\text{min}} = 0.005$$



Mic

TEXAS

Midland Tower

KANSAS

Glenville

South Seneca Gardens

Purcell

Campus High Sch

Sewage Disposal

Trailer Park

Trailer Park

Trailer Park

Park

Oil Well

Oil Well

Pit

LANDING STRIP

BDY

CORP

WICHITA

18

19

20

1275

1275

1270

1270

710

AND

ST

W

4

55TH

SENECA

ST

TRK

Trailer



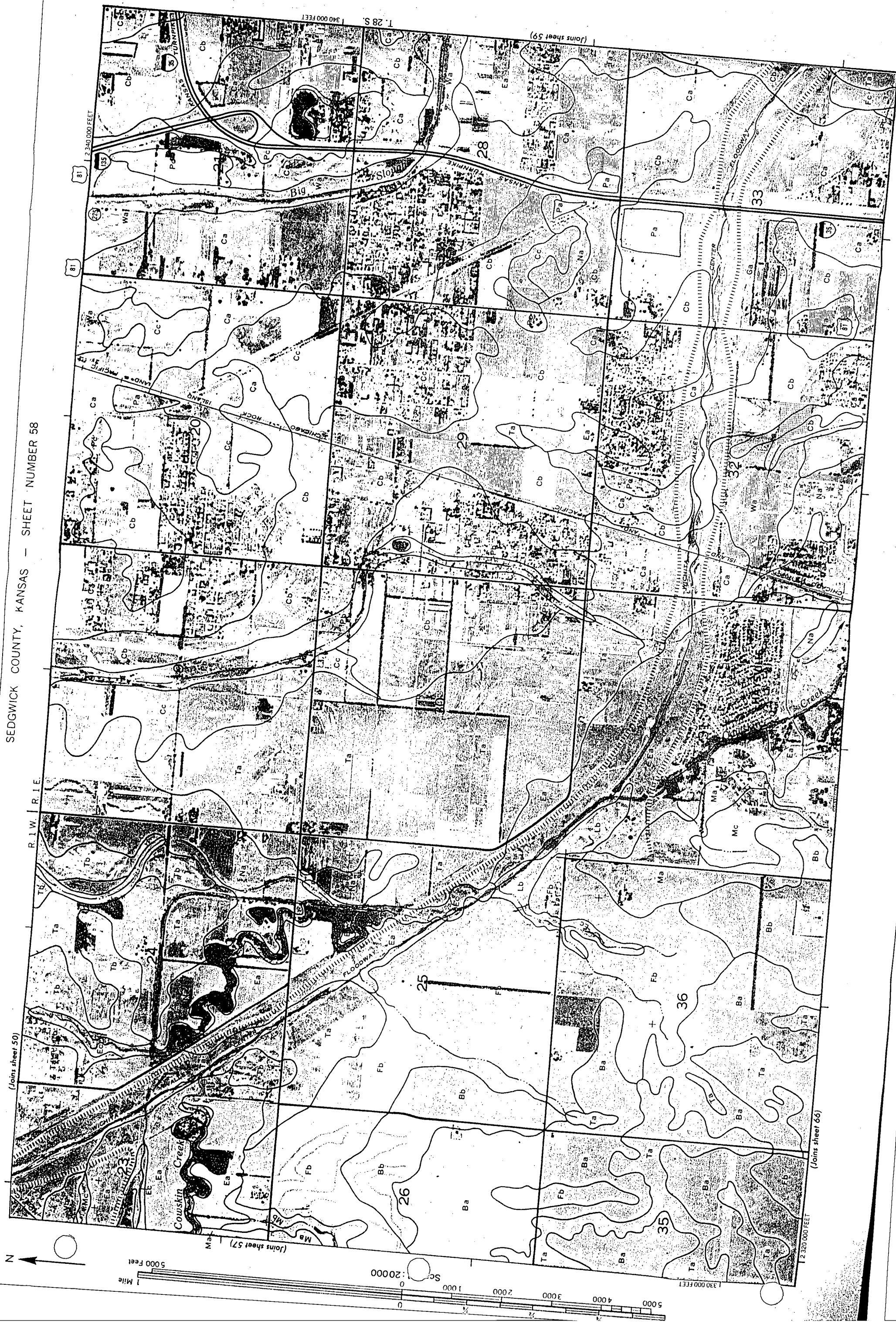


Table 2-2.--Runoff curve numbers for selected agricultural, suburban, and urban land use. (Antecedent moisture condition II, and $I_a = 0.2S$)

LAND USE DESCRIPTION	HYDROLOGIC SOIL GROUP			
	A	B	C	D
Cultivated land ^{1/} : without conservation treatment	72	81	83	91
: with conservation treatment	62	71	76	81
Pasture or range land: poor condition	68	79	66	59
good condition	39	61	74	80
Meadow: good condition	30	58	71	78
Wood or Forest land: thin stand, poor cover, no mulch	45	66	77	83
good cover ^{2/}	25	55	70	77
Open Spaces, lawns, parks, golf courses, cemeteries, etc.				
good condition: grass cover on 75% or more of the area	39	61	74	80
fair condition: grass cover on 50% to 75% of the area	49	69	79	84
Commercial and business areas (85% impervious)	89	92	94	95
Industrial districts (72% impervious).	81	88	91	93
Residential: ^{3/}				
Average lot size	Average % Impervious ^{4/}			
1/8 acre or less	65	77	85	90
1/4 acre	38	61	75	83
1/3 acre	30	57	72	81
1/2 acre	25	54	70	80
1 acre	20	51	68	79
Paved parking lots, roofs, driveways, etc. ^{5/}	98	98	98	98
Streets and roads:				
paved with curbs and storm sewers ^{5/}	98	98	98	98
gravel	76	85	89	91
dirt	72	82	87	89

^{1/} For a more detailed description of agricultural land use curve numbers refer to National Engineering Handbook, Section 4, Hydrology, Chapter 9, Aug. 1972.

^{2/} Good cover is protected from grazing and litter and brush cover soil.

^{3/} Curve numbers are computed assuming the runoff from the house and driveway is directed towards the street with a minimum of roof water directed to lawns where additional infiltration could occur.

^{4/} The remaining pervious areas (lawn) are considered to be in good pasture condition for these curve numbers.

^{5/} In some warmer climates of the country a curve number of 95 may be used.

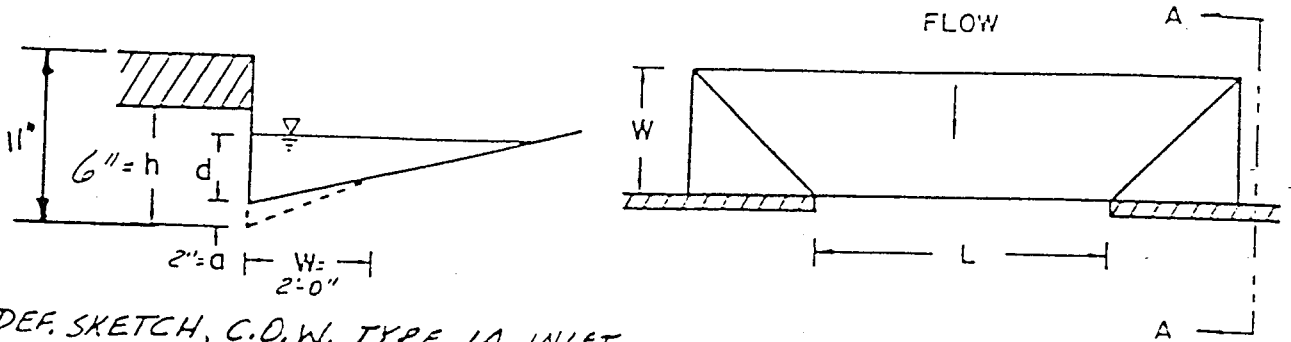
ATTACHMENT E

DRAINAGE CRITERIA

CITY OF WICHITA, KANSAS

AVERAGE OVERLAND FLOW VELOCITY FOR USE WITH URBANIZED AREAS

Surface Type	VELOCITY IN FEET/SECOND FOR SLOPES IN PERCENT SHOWN																			
	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	0.9	1.0	2.0	3.0	4.0	5.0	6.0	7.0	8.0	9.0	10.0	20.0
Forest with Heavy Ground Litter or Meadow	0.08	0.11	0.14	0.16	0.18	0.19	0.20	0.22	0.23	0.25	0.35	0.42	0.50	0.55	0.60	0.66	0.70	0.75	0.80	1.10
Fallow or Minimum Tillage Cultivation	0.15	0.21	0.26	0.29	0.33	0.35	0.39	0.41	0.44	0.46	0.65	0.80	0.92	1.10	1.20	1.30	1.40	1.50	1.60	2.10
Short Grass Pasture or Lawns	0.23	0.32	0.38	0.44	0.50	0.53	0.58	0.62	0.66	0.70	1.00	1.20	1.40	1.60	1.80	1.90	2.00	2.10	2.20	3.20
Almost Bare Ground	0.32	0.44	0.53	0.62	0.69	0.75	0.82	0.87	0.92	0.98	1.40	1.70	1.90	2.10	2.30	2.50	2.70	2.90	3.10	4.40
Grassed Waterway	0.50	0.68	0.83	0.95	1.10	1.20	1.30	1.40	1.50	1.60	2.20	2.60	3.00	3.40	3.70	4.00	4.30	4.60	4.80	7.00
Paved Areas (Sheet Flow) or Shallow Gutter Flow	0.63	0.89	1.10	1.30	1.50	1.60	1.70	1.80	1.90	2.00	2.80	3.40	4.00	4.50	4.90	5.30	5.70	6.00	6.20	9.00



DEF. SKETCH, C.D.W. TYPE 1A INLET

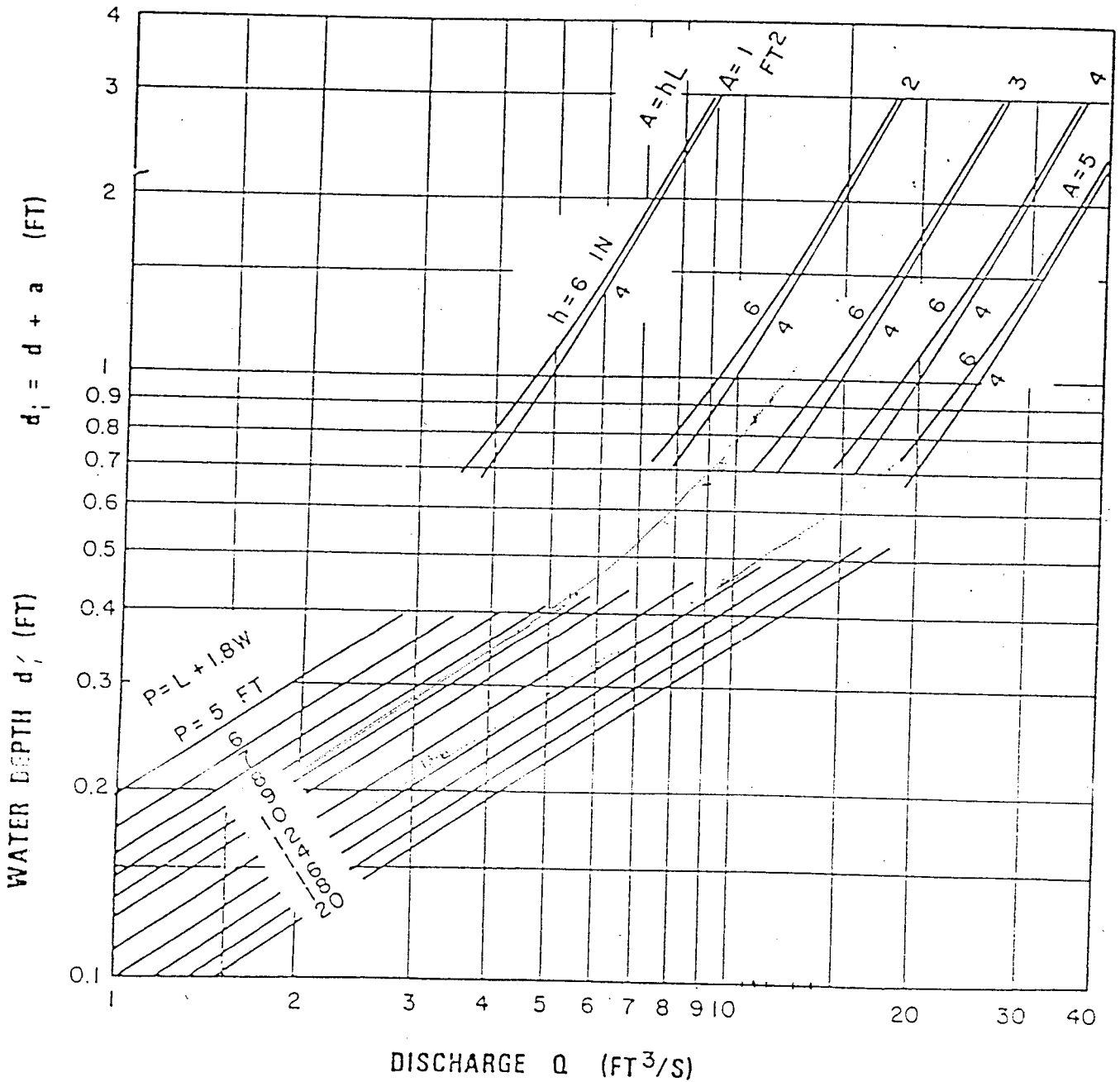
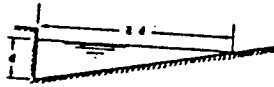


CHART 12. Depressed curb-opening inlet capacity in sump locations.

FROM: HEC-12, DRAINAGE OF HIGHWAY PAVEMENTS, F.H.W.A., MAR., 1984

NOMOGRAPH FOR FLOW IN TRIANGULAR CHANNELS

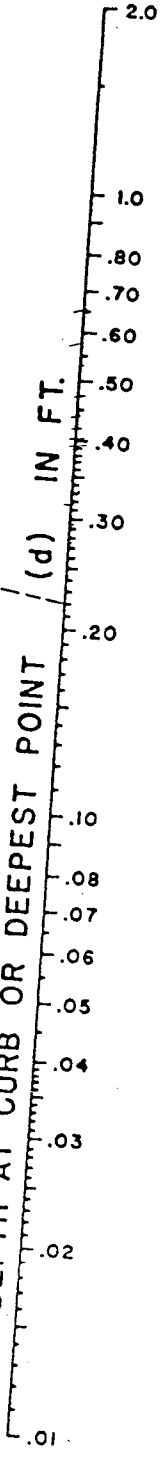
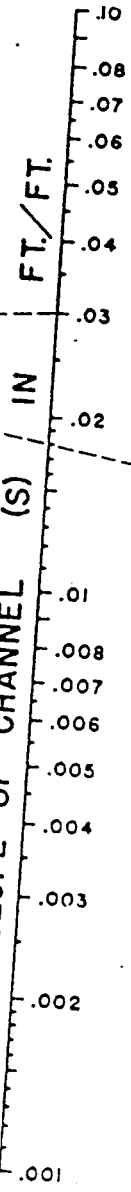
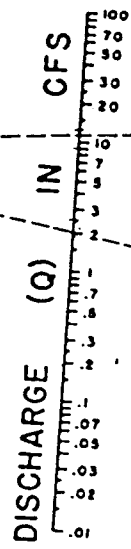
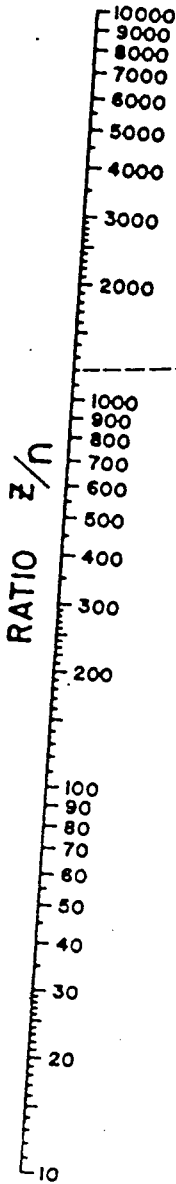


EQUATION: $Q = 0.85 (A) z^{2/3} S^{1/2}$

n IS ROUGHNESS COEFFICIENT IN MANNING
 FORMULA APPROPRIATE TO MATERIAL IN
 BOTTOM OF CHANNEL
 S IS RECIPROCAL OF CROSS SLOPE
 REFERENCE: H. R. E. PROCEEDINGS 1948,
 PAGE 180, EQUATION (14)

EXAMPLE (SEE DASHED LINES)

GIVEN: $n = 0.03$
 $z = 24$
 $n = .02$ $z/n = 1200$
 $S = 0.22$
 FIND: $Q = 2.0$ CFS

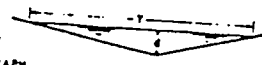


TURNING LINE

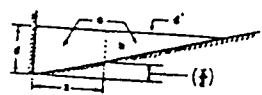
INSTRUCTIONS

1. CONNECT z/n RATIO WITH SLOPE (S) AND CONNECT DISCHARGE (Q) WITH DEPTH (d). THESE TWO LINES MUST INTERSECT AT TURNING LINE FOR COMPLETE SOLUTION.

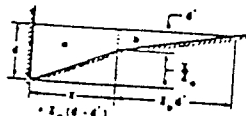
2. FOR SHALLOW V-SHAPED CHANNEL AS SHOWN USE NOMOGRAPH WITH $z = \frac{b}{2}$



3. TO DETERMINE DISCHARGE Q , IN PORTION OF CHANNEL HAVING WIDTH z : DETERMINE DEPTH d FOR TOTAL DISCHARGE IN ENTIRE SECTION b . THEN USE NOMOGRAPH TO DETERMINE z , IN SECTION b FOR DEPTH $d = d - (\frac{b}{2})$



4. TO DETERMINE DISCHARGE IN COMPOSITE SECTION-- FOLLOW INSTRUCTION 3. TO OBTAIN DISCHARGE IN SECTION b AT ASSUMED DEPTH d ; OBTAIN Q_1 FOR SLOPE RATIO S_1 AND DEPTH d' THEN $Q = Q_1 + Q_2$



One foot is 0.3048m
 One cubic foot is 0.0283m³

4-CORNER PLAN ANGEL ACRES 2ND ADDITION

TO WICHITA, SEDGWICK COUNTY, KANSAS

LEGEND

- STORM SEWER AND INLET
- STORM SEWER AND MANHOLE
- SPOT ELEVATION
- ▲— HIGH POINT ELEVATION
- MATCH EXISTING ELEVATION

BENCHMARK

- BM-1 C.O.W. Disc 74' N. and 44' E. of the center intersection of Seneca and 47th St. Elevation=187.43
- BM-2 Railroad Spike in W. face of Power Pole 30'± E. at E of Seneca, 170'± N. of Sunrise. Elevation=184.79
- BM-3 Railroad Spike in N. face of Light Pole on S.E. Cor. of Sunrise and Osage. Elevation=185.32
- BM-4 Railroad Spike in S. face of Light Pole on S.E. Cor. of Sunrise and Sycamore Ave. Elevation=186.16
- BM-5 Railroad Spike in E. face of Light Pole on N.W. Cor. of Sunrise and Gold. Elevation=187.44

SCALE: 1"=100'

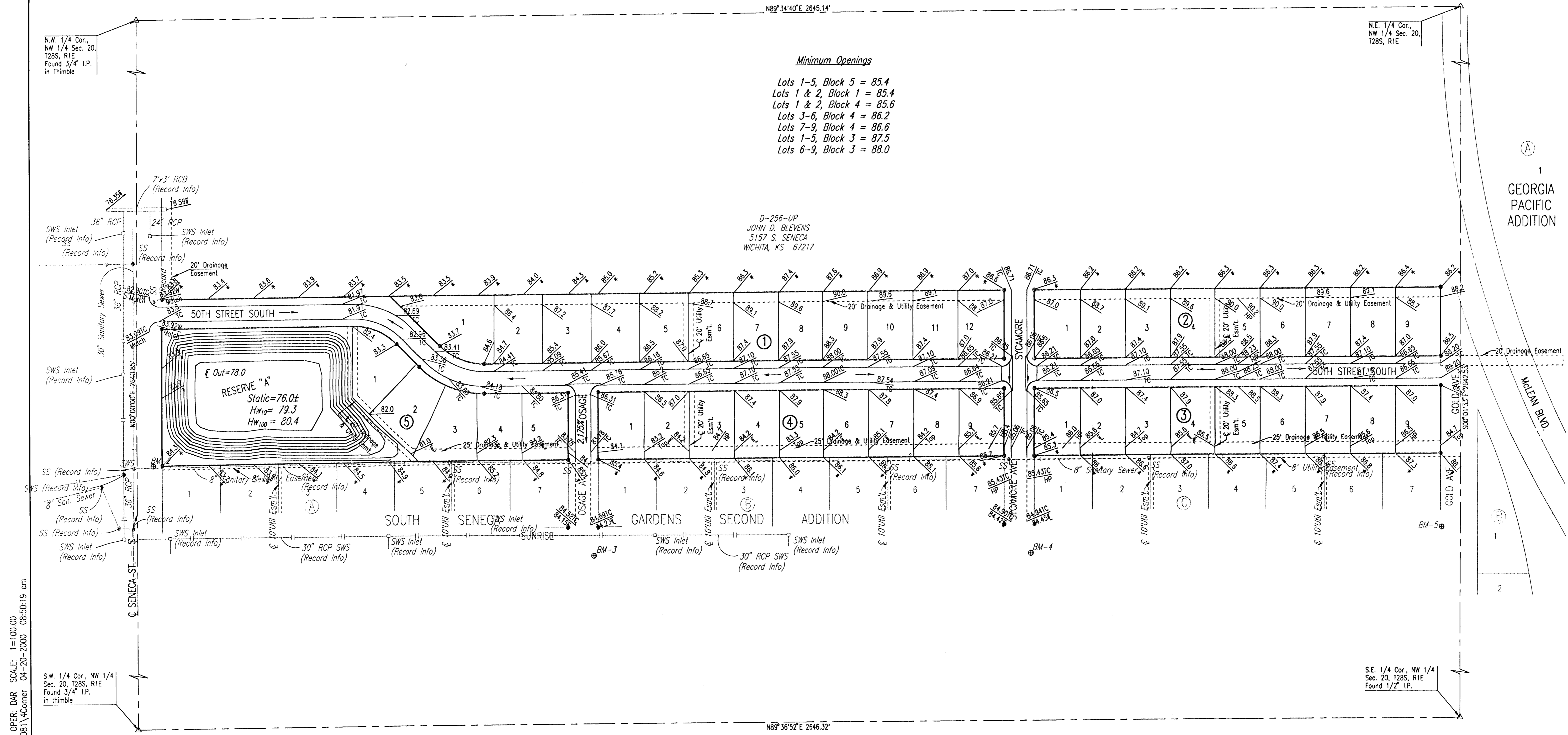
Add 100 to Elevations shown for City of Wichita Datum.

Add 1287.4 to Elevations shown for N.G.V.D.

Minimum Openings

- Lots 1-5, Block 5 = 85.4
- Lots 1 & 2, Block 1 = 85.4
- Lots 1 & 2, Block 4 = 85.6
- Lots 3-6, Block 4 = 86.2
- Lots 7-9, Block 4 = 86.6
- Lots 1-5, Block 3 = 87.5
- Lots 6-9, Block 3 = 88.0

D-256-UP
JOHN D. BLEVENS
5157 S. SENECA
WICHITA, KS 67217



1
GEORGIA
PACIFIC
ADDITION

1
2

DSNR: KER OPER: DAR SCALE: 1"=100.00
C:\2000\00081\4-Corner 04-20-2000 08:50:19 am

S.W. 1/4 Cor., NW 1/4 Sec. 20, T28S, R1E Found 3/4" I.P. in thimble

S.E. 1/4 Cor., NW 1/4 Sec. 20, T28S, R1E Found 1/2" I.P.

N89°34'40"E 2645.14'

N89°36'52"E 2646.32'