

DRAINAGE PLAN
GRACE PARK
AN ADDITION TO
WICHITA, SEDGWICK COUNTY, KANSAS

Prepared By

 **BAUGHMAN COMPANY, P.A.**
ENGINEERING, SURVEYING & PLANNING
316/262-7271 FAX 316/262-0149 WICHITA, KANSAS 67211

December 24, 2002

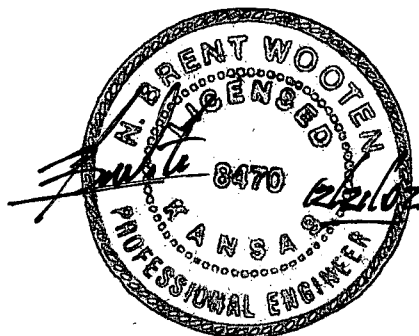


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NARRATIVE

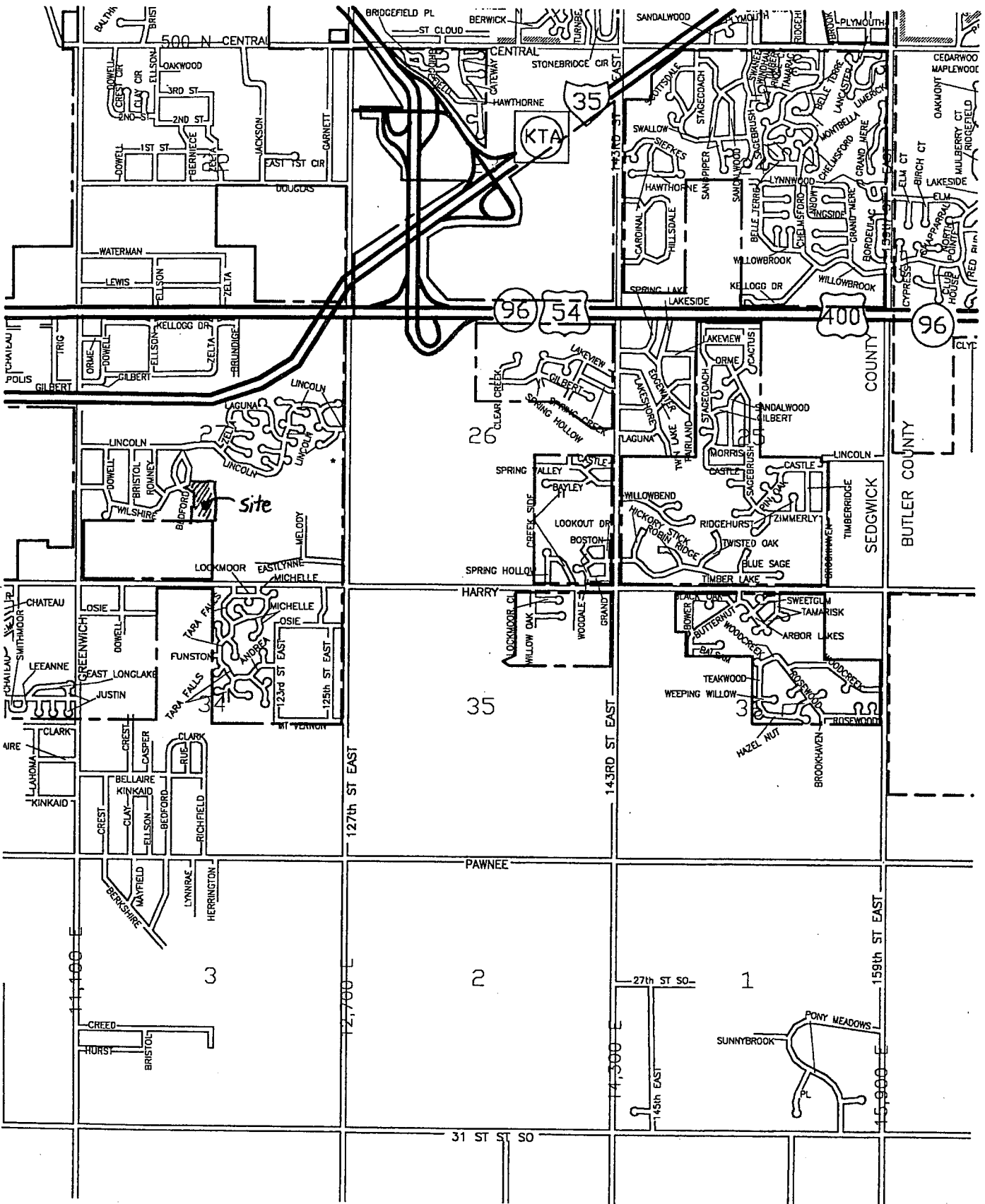
This report provides information and supporting documentation to support the "Drainage Plan" for the property located in the Southwest Quarter of Section 27, T-27-S, R-2-E in Sedgwick County, Kansas.

The "Drainage Plan" being submitted herein is intended to serve as a guide for the design of streets and storm water sewer improvements to the proposed development. Modifications to structures, pipes, etc. may be made as necessary during the final design in order to obtain the most economical design and construction possible.

Storm water sewer systems within this proposed development have been designed to convey the 2-yr rainfall event. Calculations have also been performed to check the design during the 100-yr event. A sub-division grading plan has been prepared in conjunction with the storm water sewer design. This grading plan allows for emergency outflow points during major rainfall events.

A detention facility has been provided on-site to restrict discharge of the development. The proposed discharges are less than the existing calculated discharges.

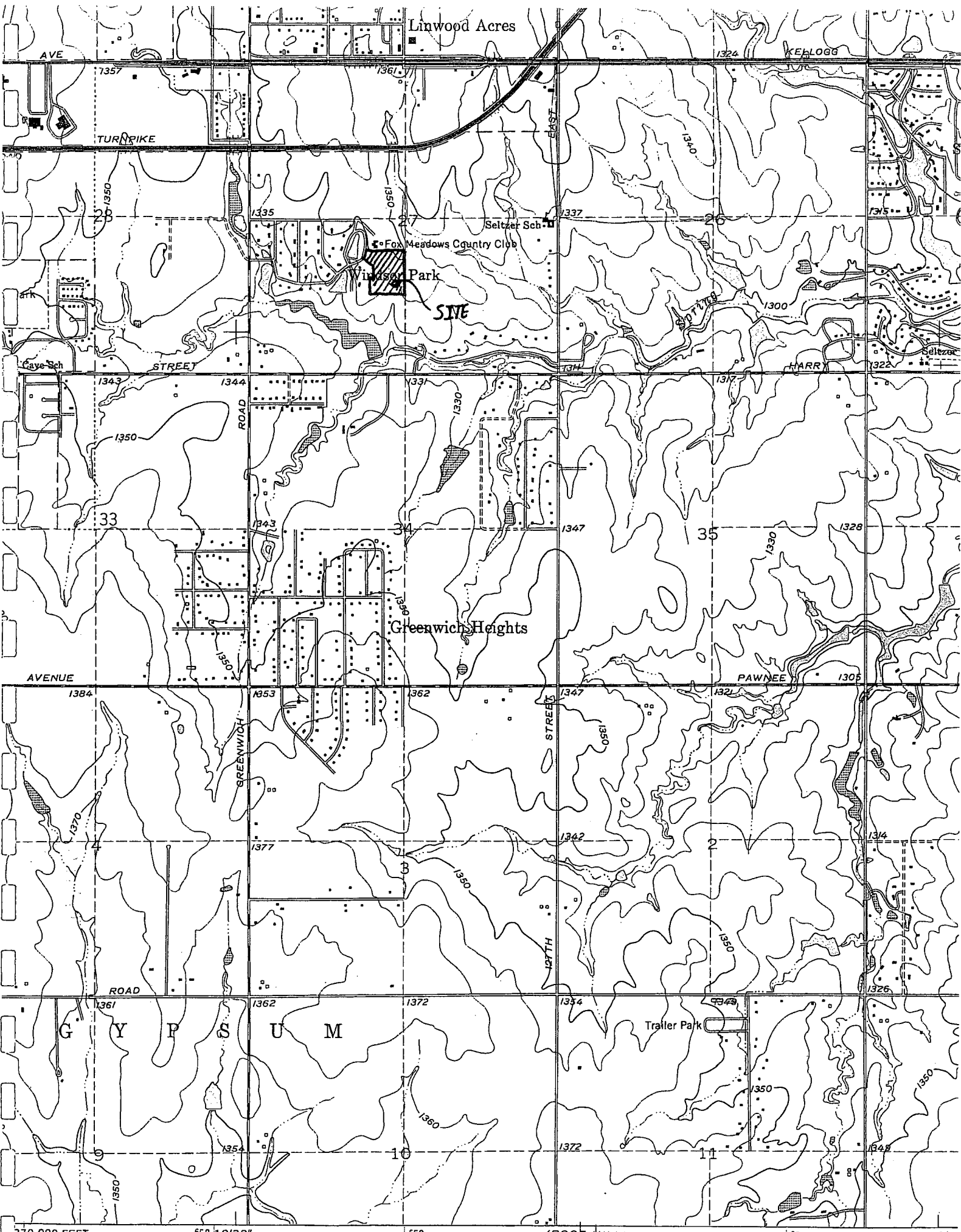
(See Page 25)



(See Page 40)

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Linwood Acres

KELOGG

TURNPIKE

Seltzer Sch

Fox Meadows Country Club

Windsor Park

SIVE

Caye Sch

STREET

HARRY

Greenwich Heights

AVENUE

PAWNEE

GREENWICH

STREETS

ROAD

G Y P U M

Trailer Park

370 000 FEET

658 12'30"

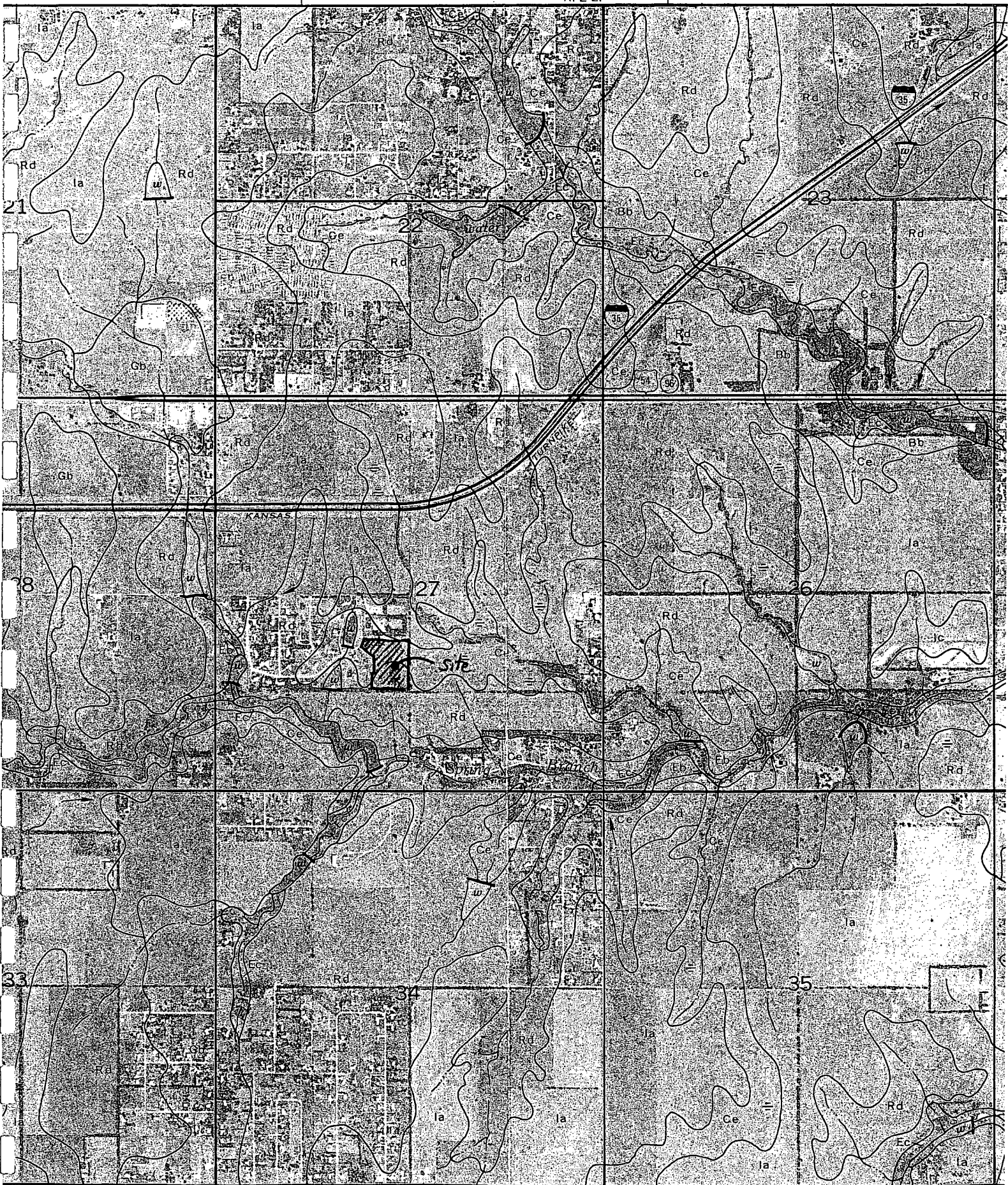
659

(ROSE HILL)
6559 11 SW

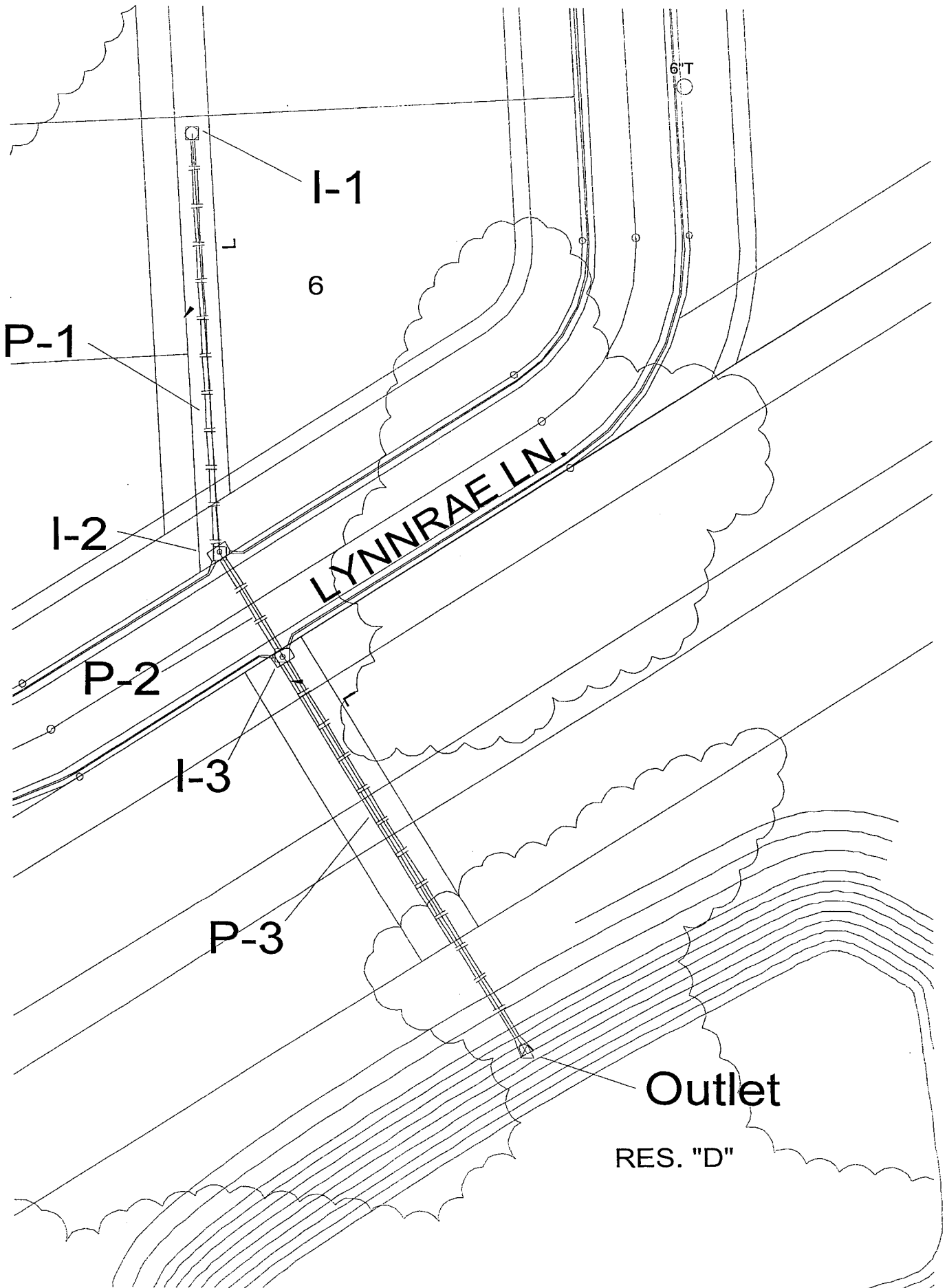
661

10'

R. 2 E.



StormCad Hydraulic Calculations



System Report

Pipe	Additional Flow (cfs)	Total Upstream Added (cfs)	Structure Discharge (cfs)	-Node- Upstream Downstream	-Section- Shape Size	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	-Ground- Upstream Downstream (ft)	-HGL- Upstream Downstream (ft)	-Slope- Energy Constructed (ft/ft)	-Section- Discharge Capacity (cfs)	Length (ft)	Average Velocity (ft/s)	Description
P-1	5.50	0.00	5.50	I-1	Circular 18 inch	150.40	149.90	153.70 153.50	153.84 153.50	0.002742 0.004032	5.50 6.67	124.00	3.11	
P-2	12.00	5.50	17.50	I-2 I-3	Circular 24 inch	149.80	149.60	153.50 153.50	153.44 153.23	0.005985 0.005714	17.50 17.10	35.00	5.57	
P-3	12.00	17.50	29.50	I-3 Outlet	Circular 24 inch	149.50	145.50	153.50 151.00	152.55 150.20	0.017006 0.028986	29.50 38.51	138.00	9.39	

Inlet: I-2
 Rim: 153.50 ft
 Sump: 149.80 ft

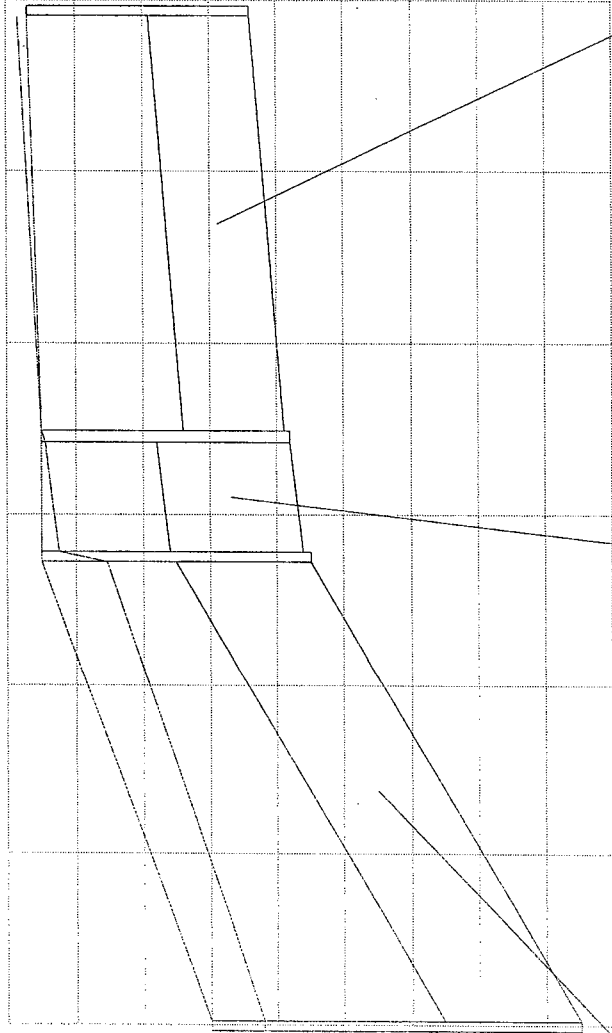
Inlet: I-1
 Rim: 153.70 ft
 Sump: 150.40 ft

Inlet: I-3
 Rim: 153.50 ft
 Sump: 149.50 ft

Outlet: Outlet
 Rim: 151.00 ft
 Sump: 145.50 ft

154.00
 153.00
 152.00
 151.00
 150.00
 149.00
 148.00
 147.00
 146.00
 145.00

Elevation ft



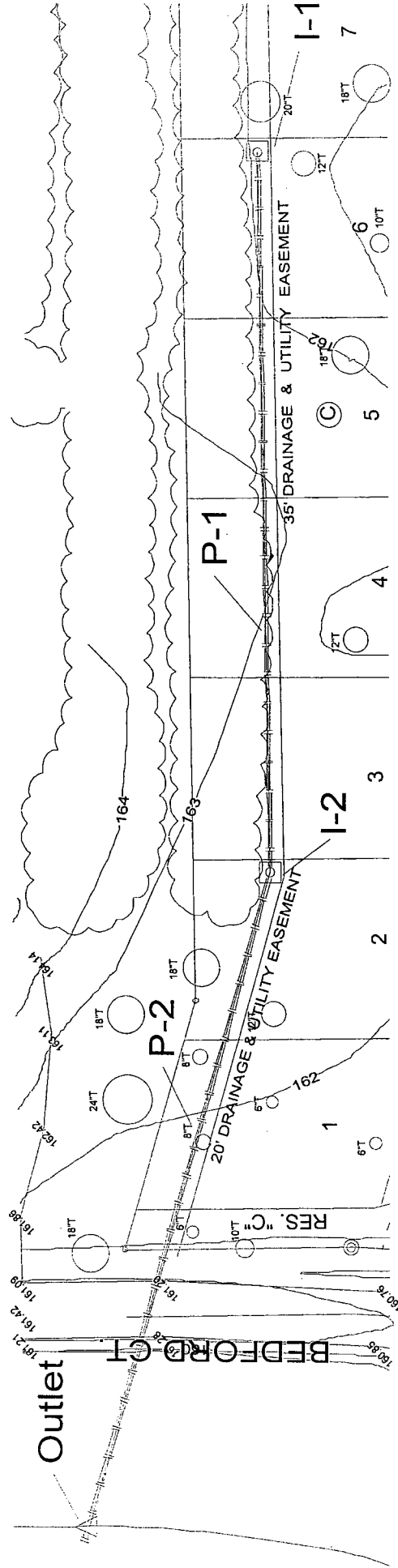
0+00 0+50 1+00 1+50 2+00 2+50 3+00

Station ft

Pipe: P-3
 Up Invert: 149.50 ft
 Dn Invert: 145.50 ft
 Length: 138.00 ft
 Size: 24 inch

Pipe: P-2
 Up Invert: 149.80 ft
 Dn Invert: 149.60 ft
 Length: 35.00 ft
 Size: 24 inch

Pipe: P-1
 Up Invert: 150.40 ft
 Dn Invert: 149.90 ft
 Length: 124.00 ft
 Size: 18 inch



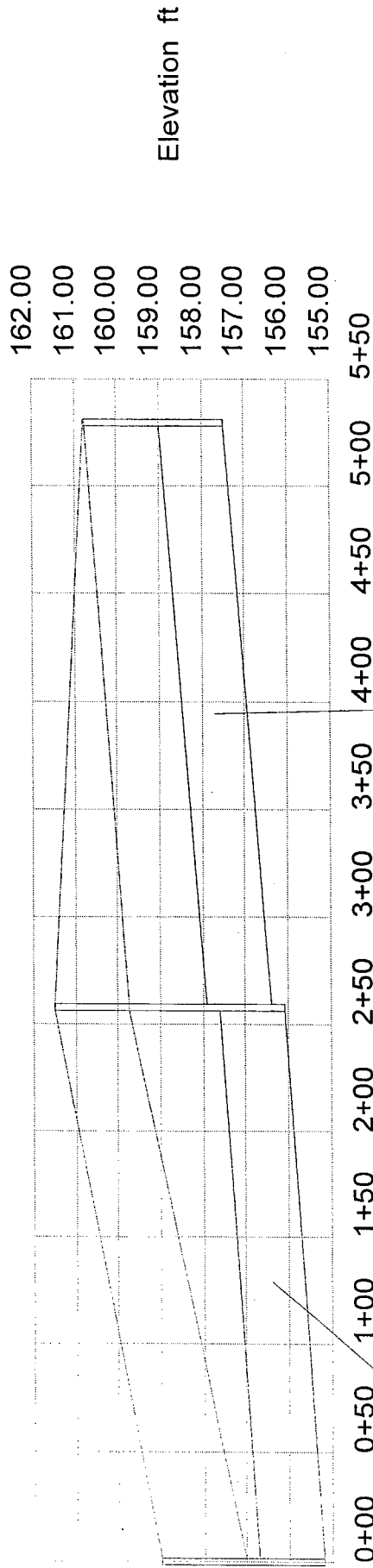
System Report

Pipe	Additional Flow (cfs)	Total Upstream Added (cfs)	Structure Discharge (cfs)	-Node- Upstream Downstream	-Section- Shape Size	Upstream Invert Elevation (ft)	Downstream Invert Elevation (ft)	-Ground- Upstream Downstream (ft)	-HGL- Upstream Downstream (ft)	-Slope- Energy Constructed (ft/ft)	-Section- Discharge Capacity (cfs)	Length (ft)	Average Velocity (ft/s)	Description
P-1	6.50	0.00	6.50	I-1	Circular 18 inch	157.50	156.40	160.80	160.77	0.003829	6.50	272.00	3.68	
P-2	4.30	6.50	10.80	I-2 I-2 Outlet	Circular 18 inch	156.10	155.20	161.50 161.50 159.00	159.73 159.73 157.00	0.010572 0.004044 0.003488	10.80	258.00	6.11	

Outlet: Outlet
Rim: 159.00 ft
Sump: 155.20 ft

Inlet: I-2
Rim: 161.50 ft
Sump: 156.10 ft

Inlet: I-1
Rim: 160.80 ft
Sump: 157.50 ft



Pipe: P-2
Up Invert: 156.10 ft
Dn Invert: 155.20 ft
Length: 258.00 ft
Size: 18 inch

Pipe: P-1
Up Invert: 157.50 ft
Dn Invert: 156.40 ft
Length: 272.00 ft
Size: 18 inch

HEC-1 Storage Routing

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* Lahey F77L-EM/32 version 5.01 *
* Dodson & Associates, Inc. *
* RUN DATE 12/23/02 TIME 11:39:54 *
*****

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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *
*****

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X X X X X X
X X X X X X
X X XXXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID GRACE PARK ADDITION
* ADDITIONAL ROUTING
*
2 IT 5 23DEC02 0000 300 2002
3 IO 3 0
4 JR PREC 1.0000 1.3143 1.5143 1.7714 2.0000 2.2286
*DIAGRAM
*
* EXISTING
5 KK LARK
6 BA .018
7 PB 3.5 35min 24hr
8 IN 60 1.42 3.5
9 PC 0 0.011 0.022 0.035 0.048 0.063 0.080 0.098 0.120 0.147
10 PC 0.181 0.235 0.663 0.772 0.820 0.854 0.880 0.902 0.921 0.937
11 PC 0.952 0.965 0.978 0.989 1.000
12 UD .35
13 LS 0 81 0
*
*
14 KK DEVEL
15 BA .018
16 UD .25
17 LS 0 86 0
*
*
18 KK POND
19 RS 1 ELEV 148.5
20 SA 0.61 0.71 0.81 0.91 1.02
21 SE 148.5 149 150 151 152
22 SS 148.5 5 2.6 1.5
*
*
23 ZZ

```

SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW
5 LARK
.
.
14 . DEVEL
. V
. V
18 . POND

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

(AC-FT) 3. 4. 4. 4.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION DEVEL
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 1.60, TOTAL EXCESS = 4.60

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	MAXIMUM AVERAGE FLOW		
			24-HR	72-HR	24.92-HR
26.	12.00	7.	2.	2.	2.
		(INCHES) 3.626	4.600	4.600	4.600
		(AC-FT) 3.	4.	4.	4.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION DEVEL
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 1.63, TOTAL EXCESS = 5.37

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	MAXIMUM AVERAGE FLOW		
			24-HR	72-HR	24.92-HR
31.	12.00	8.	3.	3.	3.
		(INCHES) 4.210	5.366	5.366	5.366
		(AC-FT) 4.	5.	5.	5.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION DEVEL
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 1.66, TOTAL EXCESS = 6.14

PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)	MAXIMUM AVERAGE FLOW		
			24-HR	72-HR	24.92-HR
35.	12.00	9.	3.	3.	3.
		(INCHES) 4.794	6.138	6.138	6.138
		(AC-FT) 5.	6.	6.	6.

CUMULATIVE AREA = 0.02 SQ MI

*** **

* *
* POND *
* *

HYDROGRAPH ROUTING DATA

19 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 148.50 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

20 SA AREA 0.6 0.7 0.8 0.9 1.0

21 SE ELEVATION 148.50 149.00 150.00 151.00 152.00

22 SS SPILLWAY
CREL 148.50 SPILLWAY CREST ELEVATION
SPWID 5.00 SPILLWAY WIDTH
COGW 2.60 WEIR COEFFICIENT
EXPW 1.50 EXPONENT OF HEAD

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	0.33	1.09	1.95	2.91
ELEVATION	148.50	149.00	150.00	151.00	152.00

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	0.00	0.00	0.01	0.12	0.39	0.93	1.82	3.15	5.01	7.47
ELEVATION	148.50	148.50	148.51	148.54	148.60	148.67	148.77	148.89	149.03	149.19
OUTFLOW	10.64	14.60	19.43	25.22	32.07	40.05	49.26	59.78	71.71	85.12
ELEVATION	149.38	149.58	149.81	150.06	150.33	150.62	150.93	151.27	151.62	152.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.01	0.03	0.06	0.11	0.17	0.25	0.33	0.35	0.47
OUTFLOW	0.00	0.01	0.12	0.39	0.93	1.82	3.15	4.60	5.01	7.47
ELEVATION	148.50	148.51	148.54	148.60	148.67	148.77	148.89	149.00	149.03	149.19
STORAGE	0.60	0.76	0.93	1.09	1.13	1.36	1.61	1.89	1.95	2.19
OUTFLOW	10.64	14.60	19.43	23.88	25.22	32.07	40.05	49.26	51.39	59.78
ELEVATION	149.38	149.58	149.81	150.00	150.06	150.33	150.62	150.93	151.00	151.27

STORAGE	2.54	2.91
OUTFLOW	71.71	85.12
ELEVATION	151.62	152.00

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM	AVERAGE FLOW	
(CFS)	(HR)		6-HR	24-HR	72-HR
9.	12.25	(CFS)			24.92-HR
		(INCHES)	3.	1.	1.
		(AC-FT)	1.643	2.051	2.051
			2.	2.	2.

PEAK STORAGE	TIME		MAXIMUM	AVERAGE STORAGE	
(AC-FT)	(HR)		6-HR	24-HR	72-HR
1.	12.25				24.92-HR
			0.	0.	0.

PEAK STAGE	TIME		MAXIMUM	AVERAGE STAGE	
(FEET)	(HR)		6-HR	24-HR	72-HR
149.26	12.25				24.92-HR
			148.87	148.64	148.63
					148.63

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.31

PEAK FLOW	TIME		MAXIMUM	AVERAGE FLOW	
(CFS)	(HR)		6-HR	24-HR	72-HR
14.	12.25	(CFS)			24.92-HR
		(INCHES)	5.	1.	1.
		(AC-FT)	2.428	3.039	3.039
			2.	3.	3.

PEAK STORAGE	TIME		MAXIMUM	AVERAGE STORAGE	
(AC-FT)	(HR)		6-HR	24-HR	72-HR
1.	12.25				24.92-HR
			0.	0.	0.

PEAK STAGE	TIME		MAXIMUM	AVERAGE STAGE	
(FEET)	(HR)		6-HR	24-HR	72-HR
149.53	12.25				24.92-HR
			148.97	148.68	148.67
					148.67

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.51

PEAK FLOW	TIME		MAXIMUM	AVERAGE FLOW	
(CFS)	(HR)		6-HR	24-HR	72-HR
17.	12.25	(CFS)			24.92-HR
		(INCHES)	6.	2.	2.
		(AC-FT)	2.933	3.686	3.686
			3.	4.	4.

PEAK STORAGE	TIME		MAXIMUM	AVERAGE STORAGE	
--------------	------	--	---------	-----------------	--

			6-HR	24-HR	72-HR	24.92-HR
+	(AC-FT)	(HR)				
	1.	12.25	0.	0.	0.	0.
	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	149.69	12.25	149.04	148.70	148.70	148.70

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.77

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
+	(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	21.	12.25	(CFS)	7.	2.	2.
			(INCHES)	3.585	4.531	4.531
			(AC-FT)	3.	4.	4.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	1.	12.25	0.	0.	0.	0.

	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	149.87	12.25	149.11	148.74	148.73	148.73

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 2.00

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
+	(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	25.	12.25	(CFS)	8.	3.	2.
			(INCHES)	4.168	5.291	5.291
			(AC-FT)	4.	5.	5.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	1.	12.25	0.	0.	0.	0.

	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	150.03	12.25	149.17	148.76	148.75	148.75

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 2.23

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
+	(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	28.	12.25	(CFS)	9.	3.	3.
			(INCHES)	4.752	6.058	6.058
			(AC-FT)	5.	6.	6.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	1.	12.17	1.	0.	0.	0.

	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR
	150.18	12.25	149.23	148.79	148.78	148.78

CUMULATIVE AREA = 0.02 SQ MI

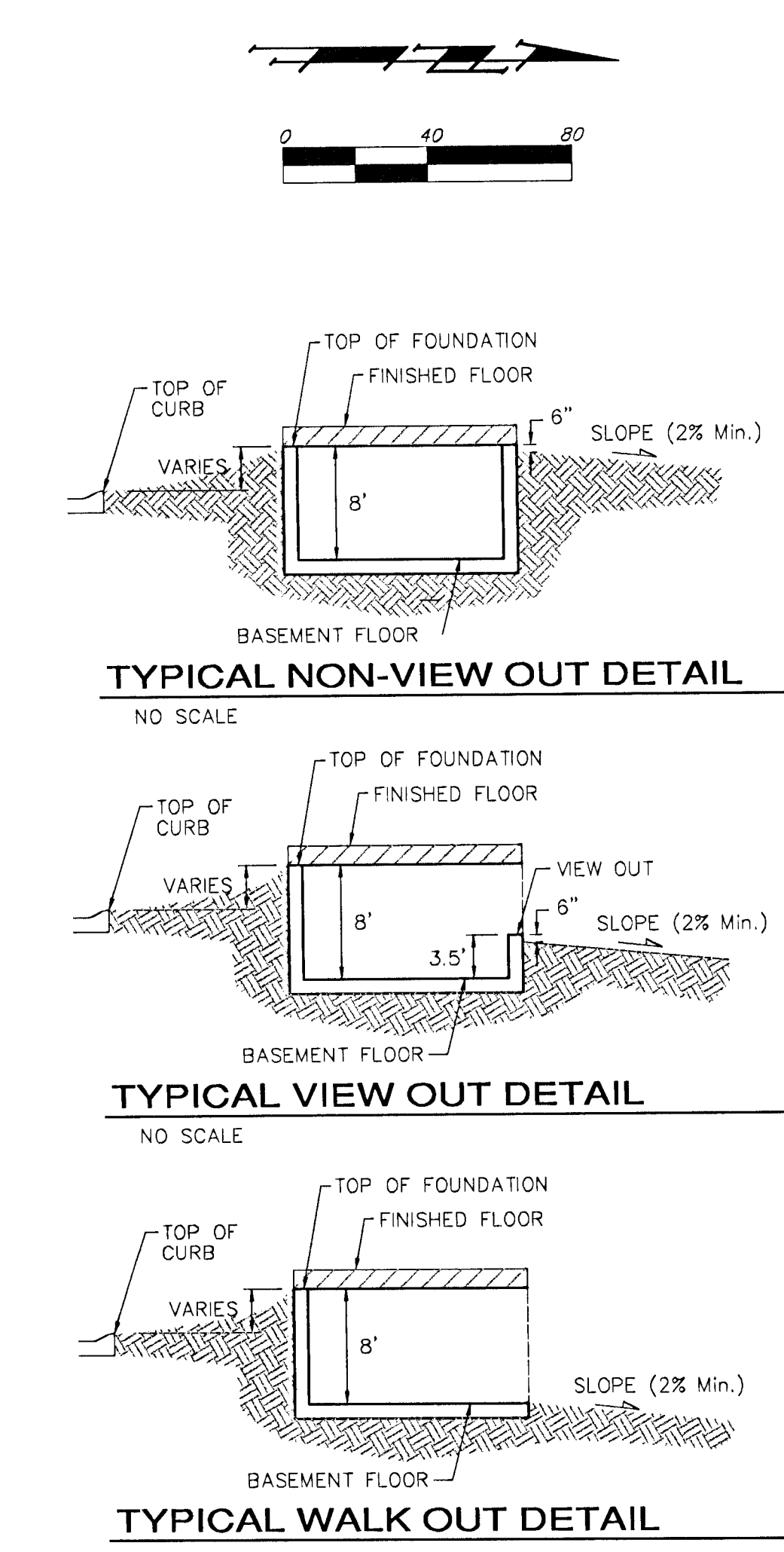
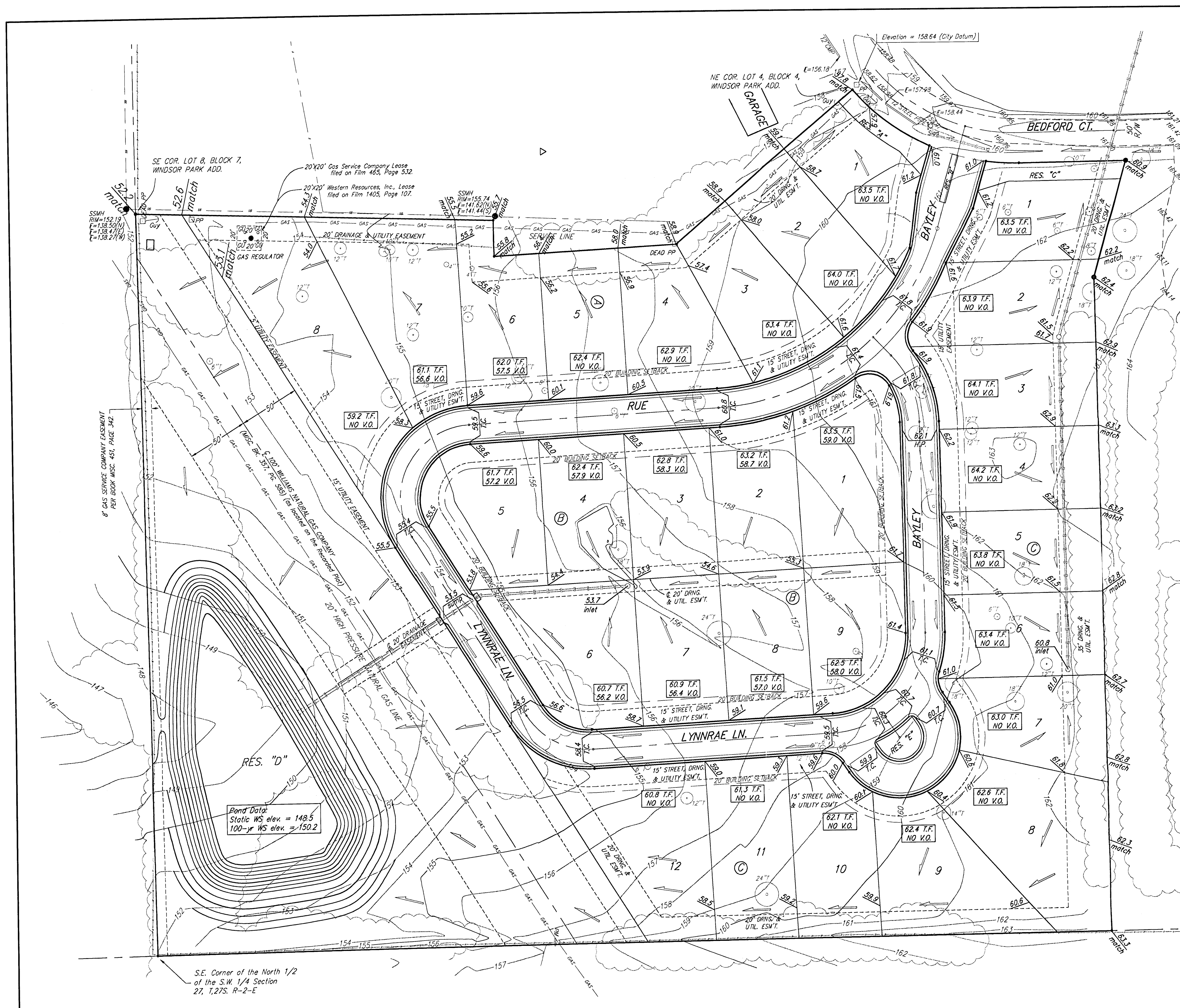
FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION						
				RATIO 1	RATIO 2	RATIO 3	RATIO 4	RATIO 5	RATIO 6	
				1.00	1.31	1.51	1.77	2.00	2.23	
HYDROGRAPH AT	LARK	0.02	1	FLOW TIME	10. 12.08	15. 12.08	18. 12.08	23. 12.08	27. 12.08	31. 12.08
HYDROGRAPH AT	DEVEL	0.02	1	FLOW TIME	12. 12.00	18. 12.00	22. 12.00	26. 12.00	31. 12.00	35. 12.00
ROUTED TO	POND	0.02	1	FLOW TIME	9. 12.25	14. 12.25	17. 12.25	21. 12.25	25. 12.25	28. 12.25
** PEAK STAGES IN FEET **										
	1	STAGE		149.26	149.53	149.69	149.87	150.03	150.18	
		TIME		12.25	12.25	12.25	12.25	12.25	12.25	

*** NORMAL END OF HEC-1 ***

Drainage Plan Sheet

Sub-Division Grading Plan



BENCHMARK:
 □ top of curb southeast corner
 Reserve "G" Cedarview Addition.
 Elev.=161.87(City Datum)
 Elev.=1349.27(NGVD29)

MINIMUM BUILDING PAD ELEVATIONS FOR LOWEST OPENING TO THE STRUCTURES

LOT	BLOCK	ELEVATION	
		CITY DATUM	NGVD29
8	A	153.0	1340.40
12	C	153.0	1340.40

NOTES:
 Proposed Top of Foundation Elevations Are Shown On Plans. Contractor to Set Finished Floor Elevations.

All Street Elevations Shown on Plans Are for Top of Curb (Full-Height).

This Grading Plan is Designed with View-Outs. Elevations Shown at Rear of House (XX.X V.O.)

Lot dimensions have been omitted on this plan, refer to the recorded plat for this information.

PRELIMINARY PLAN
NOT FOR CONSTRUCTION

GRACE PARK ADDITION
GRADING PLAN
 WICHITA, KANSAS

BAUGHMAN COMPANY P.A.
 ENGINEERING, SURVEYING, & PLANNING
 316-282-7277 • 315 ELLIS • WICHITA, KANSAS 67211

PROJECT NUMBER: _____ SHEET **1** OF **1**

DESIGN: BLG DRAWN: BLG APPROVED: _____ DATE: 01/02/2003 SCALE: NOTED