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TRANSMITTAL

November 14, 2000

To: Vicki Huang, P.E.
City of Wichita
455 N. Main, 7th Floor
Wichita, KS 67202

RE: Replat of Part of K.T. Wiedemann Business Park

AM No: 99319

We transmit the following:

<input type="checkbox"/> Bluelines	<input checked="" type="checkbox"/> Photocopies	<input type="checkbox"/> Legal Desc.
<input type="checkbox"/> Originals	<input type="checkbox"/> Shop Drawings	<input type="checkbox"/> Application

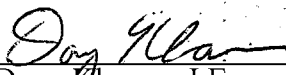
Copies
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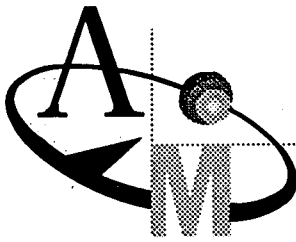
Dated

Description
Drainage Plan & Calcs

Remarks:

AUSTIN MILLER, P.A.


Doug Klassen, I.E.

JOB WIEDERMANN BUSINESS PARK 2ND

SHEET NO. _____ OF _____

CALCULATED BY _____ DATE _____

CHECKED BY _____ DATE _____

SCALE _____

DRAINAGE CALCS:SOIL GROUP D:

DA-1 = 7.17 Ac

DA-2 = 19.84 Ac

DA-3 = 0.55 Ac

DA-1

$C_{SUN} = 0.37$

$C_{5DEV} = 0.69$

$Q_{SUN} = (0.37)(4.56)(7.17) = 12.1 \text{ cfs}$

$C_{100UN} = 0.67$

$C_{100DEV} = 0.80$

$Q_{100UN} = (0.67)(7.37)(7.17) = 35.4 \text{ cfs}$

$Q_{5DEV} = (0.69)(4.56)(7.17) = 22.6 \text{ cfs}$

$Q_{100DEV} = (0.80)(7.37)(7.17) = 42.3 \text{ cfs}$

DA-2

$Q_{SUN} = (0.37)(4.56)(19.84) = 33.5 \text{ cfs}$

$Q_{100UN} = (0.67)(7.37)(19.84) = 98.0 \text{ cfs}$

$Q_{5DEV} = (0.69)(4.56)(19.84) = 62.4 \text{ cfs}$

$Q_{100DEV} = (0.80)(7.37)(19.84) = 117 \text{ cfs}$

TOTAL DRAINING TO POND:

$Q_{SUN} = 33.5 + 12.1 = 45.6 \text{ cfs}$

$Q_{5DEV} = 62.4 + 22.6 = 85 \text{ cfs}$

$Q_{100UN} = 98.0 + 35.4 = 133.4 \text{ cfs} = \text{ALLOWABLE DISCHARGE}$

$Q_{100DEV} = 117.0 + 42.3 = 159.3 \text{ cfs} = \text{INFLOW TO POND}$

DA-3

$Q_{SUN} = (0.37)(4.56)(0.55) = 0.9 \text{ cfs}$

$Q_{100UN} = (0.67)(7.37)(0.55) = 2.7 \text{ cfs}$

$Q_{5DEV} = (0.69)(4.56)(0.55) = 1.7 \text{ cfs}$

$Q_{100DEV} = (0.80)(7.37)(0.55) = 3.2 \text{ cfs}$

Hydrograph Report

Hyd. No. 1

Wiedemann 1

Hydrograph type	= Rational	Peak discharge	= 159.14 cfs
Storm frequency	= 100 yrs	Time interval	= 1 min
Drainage area	= 27.01 ac	Runoff coeff.	= .8
Intensity	= 7.37 in/hr	Time of conc. (Tc)	= 15 min
I-D-F Curve	= WICHITA.IDF	Reced. limb factor	= 1

Total Volume = 143,230 cuft, 3.288 acft

Hydrograph Discharge Table

Time -- Outflow
(hrs cfs)

0.02	10.61
0.03	21.22
0.05	31.83
0.07	42.44
0.08	53.05
0.10	63.66
0.12	74.27
0.13	84.88
0.15	95.49
0.17	106.10
0.18	116.71
0.20	127.32
0.22	137.93
0.23	148.53
0.25	159.14 <<
0.27	148.53
0.28	137.93
0.30	127.32
0.32	116.71
0.33	106.10
0.35	95.49
0.37	84.88
0.38	74.27
0.40	63.66
0.42	53.05
0.43	42.44
0.45	31.83
0.47	21.22
0.48	10.61

...End

Hydrograph Report

Hyd. No. 2

route 1

Hydrograph type = Reservoir
 Storm frequency = 100 yrs
 Inflow hyd. No. = 1
 Max. Elevation = 1263.21 ft

Peak discharge = 89.10 cfs
 Time interval = 1 min
 Reservoir name = Pond 2
 Max. Storage = 53,659 cuft

Storage Indication method used.

Total Volume = 143,230 cuft, 3.288 acft

Hydrograph Discharge Table

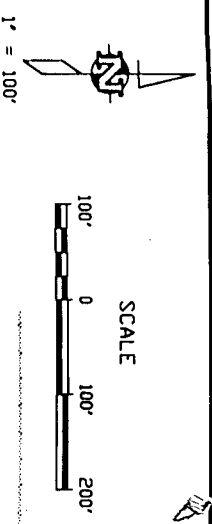
Time (hrs)	Inflow (cfs)	Elevation (ft)	Culv. A (cfs)	Culv. B (cfs)	Culv. C (cfs)	Weir A (cfs)	Weir B (cfs)	Weir C (cfs)	Outflow (cfs)
0.02	10.61	1260.02	2.53	----	----	----	----	----	2.52
0.03	21.22	1260.06	8.90	----	----	----	----	----	8.90
0.05	31.83	1260.12	16.21	----	----	----	----	----	16.21
0.07	42.44	1260.19	21.00	----	----	----	----	----	21.00
0.08	53.05	1260.29	25.94	----	----	----	----	----	25.94
0.10	63.66	1260.41	30.94	----	----	----	----	----	30.94
0.12	74.27	1260.56	35.97	----	----	----	----	----	35.97
0.13	84.88	1260.73	41.05	----	----	----	----	----	41.05
0.15	95.49	1260.92	46.13	----	----	----	----	----	46.13
0.17	106.10	1261.12	50.91	----	----	----	----	----	50.91
0.18	116.71	1261.33	55.53	----	----	----	----	----	55.53
0.20	127.32	1261.57	60.22	----	----	----	----	----	60.22
0.22	137.93	1261.82	64.96	----	----	----	----	----	64.96
0.23	148.53	1262.09	69.57	----	----	----	----	----	69.57
0.25	159.14 <<	1262.36	73.92	----	----	----	----	----	73.92
0.27	148.53	1262.61	77.82	----	----	----	----	----	77.82
0.28	137.93	1262.82	80.88	----	----	----	----	----	80.88
0.30	127.32	1262.99	83.08	----	----	----	----	----	83.08
0.32	116.71	1263.10	86.15	----	----	----	----	----	86.15
0.33	106.10	1263.17	88.06	----	----	----	----	----	88.06
0.35	95.49	1263.21	89.01	----	----	----	----	----	89.01
0.37	84.88	1263.21 <<	89.10	----	----	----	----	----	89.10 <<
0.38	74.27	1263.19	88.39	----	----	----	----	----	88.39
0.40	63.66	1263.13	86.92	----	----	----	----	----	86.92
0.42	53.05	1263.05	84.64	----	----	----	----	----	84.64
0.43	42.44	1262.94	82.43	----	----	----	----	----	82.43
0.45	31.83	1262.79	80.43	----	----	----	----	----	80.43
0.47	21.22	1262.62	77.90	----	----	----	----	----	77.90
0.48	10.61	1262.42	74.89	----	----	----	----	----	74.89
0.50	0.00	1262.20	71.37	----	----	----	----	----	71.37
0.52	0.00	1261.97	67.51	----	----	----	----	----	67.51
0.53	0.00	1261.73	63.27	----	----	----	----	----	63.27
0.55	0.00	1261.51	59.05	----	----	----	----	----	59.05
0.57	0.00	1261.30	54.82	----	----	----	----	----	54.82
0.58	0.00	1261.10	50.58	----	----	----	----	----	50.58
0.60	0.00	1260.92	46.13	----	----	----	----	----	46.13
0.62	0.00	1260.74	41.36	----	----	----	----	----	41.36

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Hydrograph Discharge Table

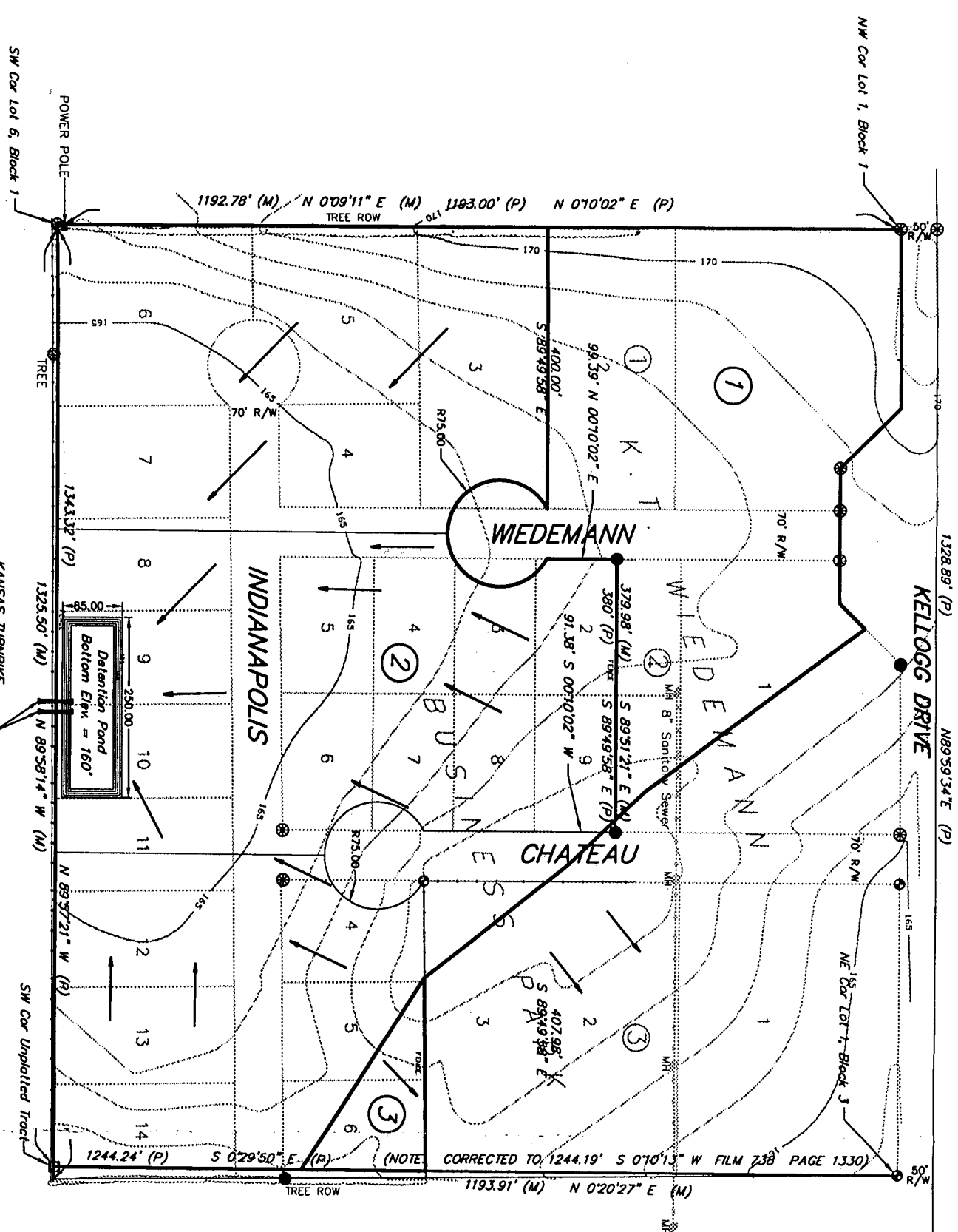
Time (hrs)	Inflow (cfs)	Elevation (ft)	Culv. A (cfs)	Culv. B (cfs)	Culv. C (cfs)	Weir A (cfs)	Weir B (cfs)	Weir C (cfs)	Outflow (cfs)
0.63	0.00	1260.58	36.62	----	----	----	----	----	36.61
0.65	0.00	1260.44	31.85	----	----	----	----	----	31.84
0.67	0.00	1260.32	27.10	----	----	----	----	----	27.10
0.68	0.00	1260.22	22.34	----	----	----	----	----	22.34
0.70	0.00	1260.14	17.44	----	----	----	----	----	17.44
0.72	0.00	1260.08	11.54	----	----	----	----	----	11.54
0.73	0.00	1260.04	6.04	----	----	----	----	----	6.05
0.75	0.00	1260.02	3.18	----	----	----	----	----	3.17
0.77	0.00	1260.01	1.65	----	----	----	----	----	1.66

...End



KELLOGG (US-54)
 1228.89' (P)
 N69°39'34\"/>

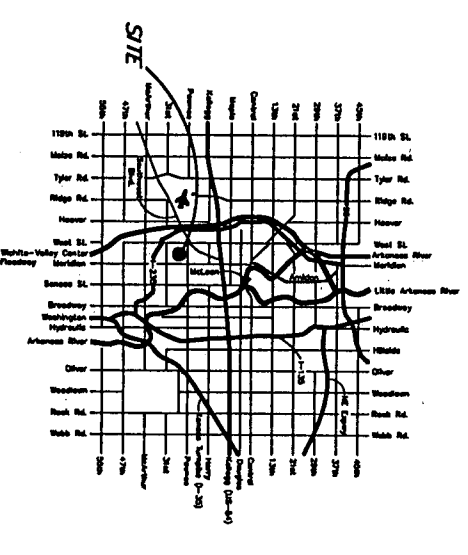
DRAINAGE PLAN FOR:
A REPLAT OF PART OF K.T. WIEDEMANN BUSINESS PARK
WICHITA, SEDGWICK COUNTY, KANSAS



Area #	Area	In	Undeveloped		Developed		
			Q _u	Q _{av}	Q _u	Q _{av}	
1	2.17	4.56	2.37	12.1	35.4	22.6	42.3
2	19.84	4.56	2.37	31.5	98.0	62.4	112.0
3	0.55	4.56	2.37	0.9	2.7	1.7	3.2
TOTAL	21.5	4.56	2.37	48.5	136.1	66.7	162.5

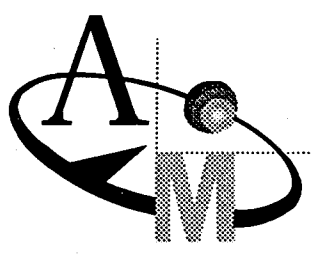
- NOTES:**
1. On-site detention will be required. Max. allowable O100 = 131.4 cfs to South, O100 = 2.7 to North.
 2. Detention may be above or below ground. Final detention pond location and configuration shall be determined at time of development.
 3. Approval for application of outlet pipe(s) shall be obtained by the Kansas Turnpike Assn. prior to installation.

- LEGEND:**
- 1/2" Rubber (Round) SSB
 - 1/2" Iron (Round)
 - 1/2" Iron (Round) FFC
 - ▲ Power Pole
 - (M) Measured
 - (P) Platified



REPORT OF MEDIAN BUSINESS PARK
 DATE PLAT PREPARED: 4/12/2000
 TITLE: REDEVELOPMENT

AUSTIN MILLER & ASSOCIATES
 ENGINEERING SERVICES
 200 N. Meade, Suite 202, Wichita, KS 67202
 Tel: 316-261-1881 Fax: 316-261-5119



DRAINAGE CALCS:

SOIL GROUP D:

- DA-1 = 7.17 AC
- DA-2 = 19.84 AC
- DA-3 = 0.55 AC

DA-1

$$C_{sun} = 0.37 \quad C_{soil} = 0.69 \quad Q_{sun} = (0.37)(4.56)(7.17) = 12.1 \text{ cfs}$$

$$C_{soil} = 0.67 \quad C_{soil} = 0.80 \quad Q_{soil} = (0.67)(7.37)(7.17) = 35.4 \text{ cfs}$$

$$Q_{soil} = (0.69)(4.56)(7.17) = 22.6 \text{ cfs} \quad Q_{soil} = (0.80)(7.37)(7.17) = 42.3 \text{ cfs}$$

DA-2

$$Q_{sun} = (0.37)(4.56)(19.84) = 33.5 \text{ cfs} \quad Q_{soil} = (0.67)(7.37)(19.84) = 98.0 \text{ cfs}$$

$$Q_{soil} = (0.69)(4.56)(19.84) = 62.4 \text{ cfs} \quad Q_{soil} = (0.80)(7.37)(19.84) = 117 \text{ cfs}$$

TOTAL DRAINING TO POND: $Q_{sun} = 33.5 + 12.1 = 45.6 \text{ cfs}$

$$Q_{soil} = 62.4 + 22.6 = 85 \text{ cfs}$$

$$Q_{soil} = 98.0 + 35.4 = 133.4 \text{ cfs} = \text{ALLOWABLE DISCHARGE}$$

$$Q_{soil} = 117.0 + 42.3 = 159.3 \text{ cfs} = \text{INFLOW TO POND}$$

DA-3

$$Q_{sun} = (0.37)(4.56)(0.55) = 0.9 \text{ cfs} \quad Q_{soil} = (0.67)(7.37)(0.55) = 2.7 \text{ cfs}$$

$$Q_{soil} = (0.69)(4.56)(0.55) = 1.7 \text{ cfs} \quad Q_{soil} = (0.80)(7.37)(0.55) = 3.2 \text{ cfs}$$

Hydrograph Report

Hyd. No. 1

Wiedemann 1

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 27.01 ac
Intensity = 7.37 in/hr
I-D-F Curve = WICHITA.IDF

Peak discharge = 159.14 cfs
Time interval = 1 min
Runoff coeff. = .8
Time of conc. (Tc) = 15 min
Reced. limb factor = 1

Total Volume = 143,230 cuft, 3.288 acft

Hydrograph Discharge Table

Time -- Outflow
(hrs) (cfs)

0.02	10.61
0.03	21.22
0.05	31.83
0.07	42.44
0.08	53.05
0.10	63.66
0.12	74.27
0.13	84.88
0.15	95.49
0.17	106.10
0.18	116.71
0.20	127.32
0.22	137.93
0.23	148.53
0.25	159.14 <<
0.27	148.53
0.28	137.93
0.30	127.32
0.32	116.71
0.33	106.10
0.35	95.49
0.37	84.88
0.38	74.27
0.40	63.66
0.42	53.05
0.43	42.44
0.45	31.83
0.47	21.22
0.48	10.61

...End

Hydrograph Report

Hyd. No. 2

route 1

Hydrograph type = Reservoir
 Storm frequency = 100 yrs
 Inflow hyd. No. = 1
 Max. Elevation = 1263.21 ft

Peak discharge = 89.10 cfs
 Time interval = 1 min
 Reservoir name = Pond 2
 Max. Storage = 53,659 cuft

Storage Indication method used.

Total Volume = 143,230 cuft, 3.288 acft

Hydrograph Discharge Table

Time (hrs)	Inflow (cfs)	Elevation (ft)	Culv. A (cfs)	Culv. B (cfs)	Culv. C (cfs)	Weir A (cfs)	Weir B (cfs)	Weir C (cfs)	Outflow (cfs)
0.02	10.61	1260.02	2.53	----	----	----	----	----	2.52
0.03	21.22	1260.06	8.90	----	----	----	----	----	8.90
0.05	31.83	1260.12	16.21	----	----	----	----	----	16.21
0.07	42.44	1260.19	21.00	----	----	----	----	----	21.00
0.08	53.05	1260.29	25.94	----	----	----	----	----	25.94
0.10	63.66	1260.41	30.94	----	----	----	----	----	30.94
0.12	74.27	1260.56	35.97	----	----	----	----	----	35.97
0.13	84.88	1260.73	41.05	----	----	----	----	----	41.05
0.15	95.49	1260.92	46.13	----	----	----	----	----	46.13
0.17	106.10	1261.12	50.91	----	----	----	----	----	50.91
0.18	116.71	1261.33	55.53	----	----	----	----	----	55.53
0.20	127.32	1261.57	60.22	----	----	----	----	----	60.22
0.22	137.93	1261.82	64.96	----	----	----	----	----	64.96
0.23	148.53	1262.09	69.57	----	----	----	----	----	69.57
0.25	159.14 <<	1262.36	73.92	----	----	----	----	----	73.92
0.27	148.53	1262.61	77.82	----	----	----	----	----	77.82
0.28	137.93	1262.82	80.88	----	----	----	----	----	80.88
0.30	127.32	1262.99	83.08	----	----	----	----	----	83.08
0.32	116.71	1263.10	86.15	----	----	----	----	----	86.15
0.33	106.10	1263.17	88.06	----	----	----	----	----	88.06
0.35	95.49	1263.21	89.01	----	----	----	----	----	89.01
0.37	84.88	1263.21 <<	89.10	----	----	----	----	----	89.10 <<
0.38	74.27	1263.19	88.39	----	----	----	----	----	88.39
0.40	63.66	1263.13	86.92	----	----	----	----	----	86.92
0.42	53.05	1263.05	84.64	----	----	----	----	----	84.64
0.43	42.44	1262.94	82.43	----	----	----	----	----	82.43
0.45	31.83	1262.79	80.43	----	----	----	----	----	80.43
0.47	21.22	1262.62	77.90	----	----	----	----	----	77.90
0.48	10.61	1262.42	74.89	----	----	----	----	----	74.89
0.50	0.00	1262.20	71.37	----	----	----	----	----	71.37
0.52	0.00	1261.97	67.51	----	----	----	----	----	67.51
0.53	0.00	1261.73	63.27	----	----	----	----	----	63.27
0.55	0.00	1261.51	59.05	----	----	----	----	----	59.05
0.57	0.00	1261.30	54.82	----	----	----	----	----	54.82
0.58	0.00	1261.10	50.58	----	----	----	----	----	50.58
0.60	0.00	1260.92	46.13	----	----	----	----	----	46.13
0.62	0.00	1260.74	41.36	----	----	----	----	----	41.36

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Hydrograph Discharge Table

Time (hrs)	Inflow (cfs)	Elevation (ft)	Culv. A (cfs)	Culv. B (cfs)	Culv. C (cfs)	Weir A (cfs)	Weir B (cfs)	Weir C (cfs)	Outflow (cfs)
0.63	0.00	1260.58	36.62	----	----	----	----	----	36.61
0.65	0.00	1260.44	31.85	----	----	----	----	----	31.84
0.67	0.00	1260.32	27.10	----	----	----	----	----	27.10
0.68	0.00	1260.22	22.34	----	----	----	----	----	22.34
0.70	0.00	1260.14	17.44	----	----	----	----	----	17.44
0.72	0.00	1260.08	11.54	----	----	----	----	----	11.54
0.73	0.00	1260.04	6.04	----	----	----	----	----	6.05
0.75	0.00	1260.02	3.18	----	----	----	----	----	3.17
0.77	0.00	1260.01	1.65	----	----	----	----	----	1.66

...End