

SUBDIVISION COMMITTEE
METROPOLITAN AREA PLANNING COMMISSION

STAFF REPORT
(Preliminary Plat)

SUB 2001-25 - MEL HAMBELTON ADDITION

Mel Hambelton Real Estate, L.L.C., 3901 N. Broadway, Wichita, KS
67219-3202

SURVEYOR/ENGINEER:

Baughman Company, P.A., 315 Ellis, Wichita, KS 67211

LOCATION:

Southeast corner of 119th St. West and Kellogg

SITE SIZE:

21.15 Acres

NUMBER OF LOTS

Residential:
Office:
Commercial:
Industrial:
Total:

8
8

MINIMUM LOT AREA:

20,000 sq. ft.

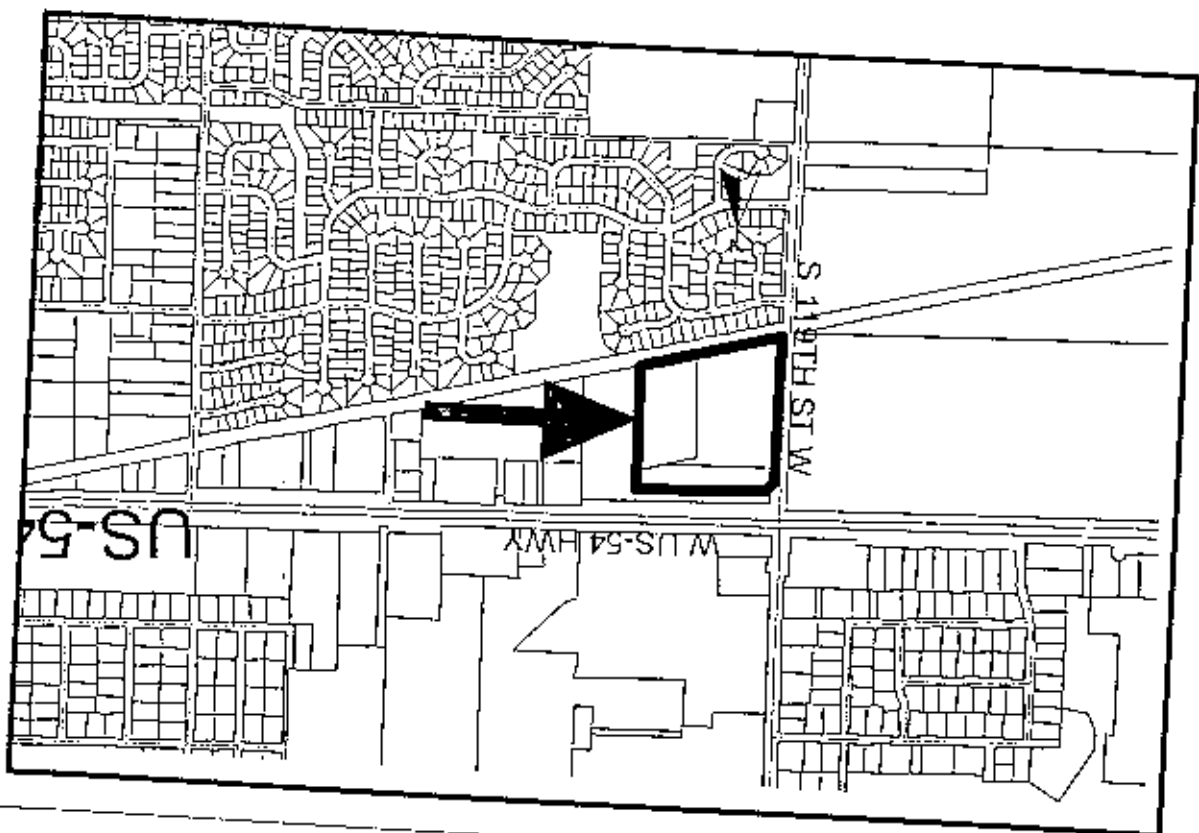
CURRENT ZONING:

L1, Limited Industrial

PROPOSED ZONING:

Same

VICINITY MAP



SCANNED

- L. The platlor's text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- M. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- N. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.
- O. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-729-0102) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.
- P. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- Q. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoffs.
- R. Perimeter closure computations shall be submitted with the final plat tracing.
- S. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- T. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property.
- U. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.

Huang, Vicky

From: Huang, Vicky
Sent: Monday, March 12, 2001 3:52 PM
To: Carrier, Christopher
Cc: Lindebak, Mike ; Lackey, Stephen
Subject: RE: Mel Hambleton Addition Drainage Plan

Sensitivity: Private

This is entirely Baughman's design. But here are some points for your consideration:

1. Although this site naturally drain south to the Railroad ditch, the railroad ditch drain to the east and than will try to drain north to Kellogg again.

2. When we have detention/retention design using a pump system to drain down the ponding water, we make the developer create an association to maintain the pond and the pump. The New Market Square at 21st and Maize Road has one of such system.

3. The West Kellogg project being designed by PEC will be let later this year with storm sewer along the frontage road that lies to the proposed RCB crossing Kellogg to drain to the north. We can advise Baughman to work out the storm sewer design with PEC to make sure the storm sewer is sized to handle the discharge from the pond.

-----Original Message-----

From: Carrier, Christopher
Sent: Monday, March 12, 2001 2:56 PM
To: Huang, Vicky
Cc: Undebak, Mike ; Lackey, Stephen
Subject: Mel Hambleton Addition Drainage Plan

Sensitivity: Private

How much input have you had in this or is this something that Baughman dreamed up? This does not look good to me. The outlet from the pond is a force main into the Kellogg median, where drainage is bad already! The natural flow for this is to the south also.

Even if it's something we've worked with them on, let it be known that we can't operate and maintain the pump station - someone else would have to. I hate to do that when many properties are involved and many of them would just naturally think that the city would run it. But, we have a full plate now. This could create a situation later, years down the line, when we get pressured into taking it cause someone else can't afford to maintain it. We have these situations all the time. Don't need another one.

I would not recommend proceeding until all this is thoroughly reviewed, in detail, including all design calcs, etc. I think this is a bad design.

SCANNED

STAFF REPORT
 (Final Plat, Preliminary Plat Approved 3/15/01)

CASE NUMBER: SUB 2001-25 -- MEL HAMBELTON ADDITION

OWNER/APPLICANT: Mel Hamblton Real Estate, L.L.C., 3901 N. Broadway, Wichita, KS 67219-3202

SURVEYOR/ENGINEER: Baughman Company, P.A., 315 Ellis, Wichita, KS 67211

LOCATION: Southeast corner of 119th St, West and Kellogg

SITE SIZE: 21.15 Acres

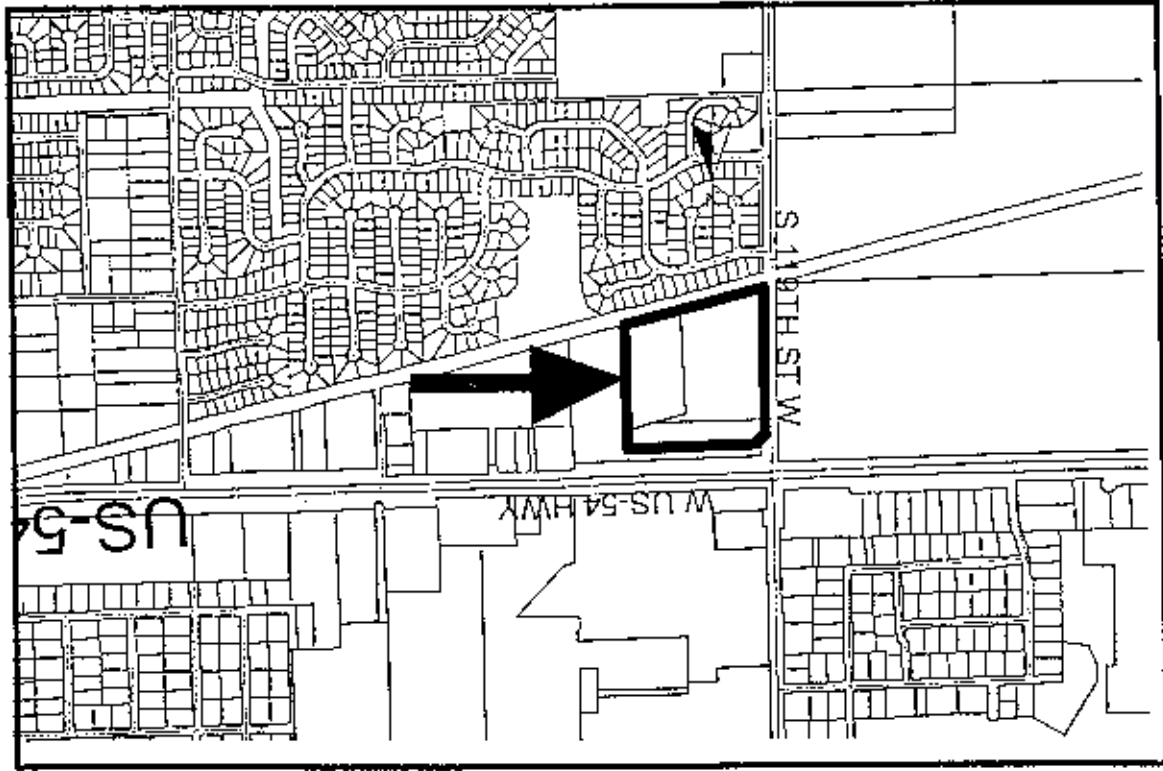
NUMBER OF LOTS

Residential:
 Office:
 Commercial: 8
 Industrial:
 Total: 8

MINIMUM LOT AREA: 20,000 Sq. Ft.

CURRENT ZONING: LI, Limited Industrial

PROPOSED ZONING: Same



VICINITY MAP

SCANNED

SCANNED

- N. On the final plat tracing, the MAPC signature block needs to reference "J.D. Michaelis, Acting Chair".
- O. The plat's text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- P. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- Q. The applicant's engineer is advised that the Registrar of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.
- R. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-729-0102) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.
- S. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- T. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoff.
- U. Perimeter closure computations shall be submitted with the final plat tracing.
- V. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- W. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property. KGE has requested additional easements which have been denoted on the final plat.
- X. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.

BAUGHMAN COMPANY P. A.
ENGINEERING & SURVEYING
316/262-7271 • 315 ELLIS • WICHITA, KANSAS 67211



FACSIMILE COVER

SHEET

TO:

FAX NUMBER: 268-4390

NAME: Neil Strahl

FIRM NAME: Planning Dept.

CITY/STATE: Wichita, KS

FROM:

BAUGHMAN COMPANY, P.A.

FAX NUMBER (316) 262-0149

NAME OF SENDER: Judy Terhune

PROJECT: Mel Hambleton Addition

PROJECT NO:

DATE: May 30, 2001

NUMBER OF PAGES (INCLUDING THIS PAGE): 2

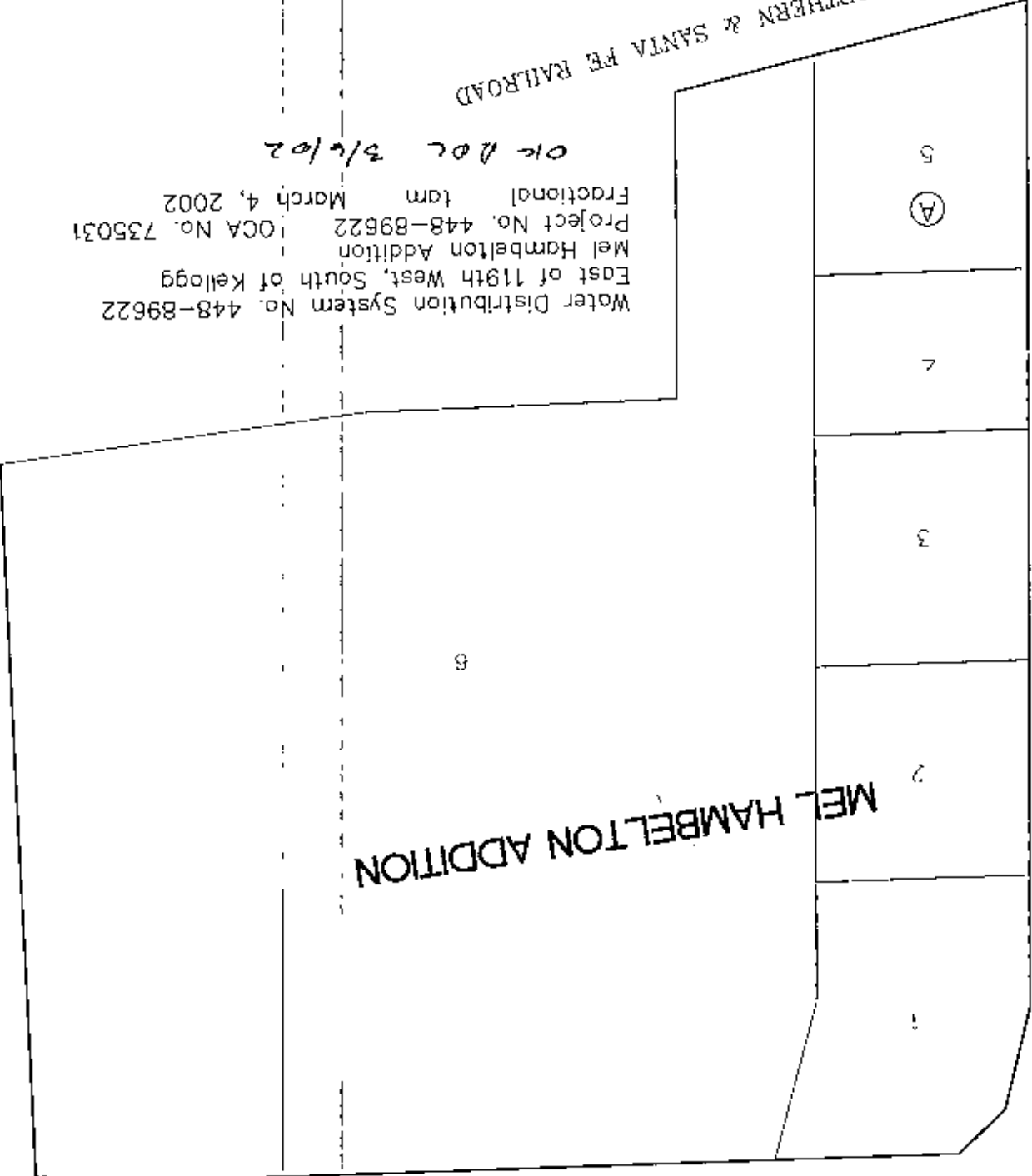
COMMENTS:

Well,
Attached is a copy of the memo we have been forwarded for the above addition.
Will this allow you to now submit this mylar to City Council for approval? Please
advise.
Thanks, Judy
cc: Vicky Huang, Engineering Dept, City of Wichita
(IF YOU DO NOT RECEIVE ALL PAGES, PLEASE CALL (316) 262-7271)

SCANNED

BURLINGTON NORTHERN & SANTA FE RAILROAD

119TH ST. WEST



MEL HAMBELTON ADDITION

KELOGG DRIVE

SCANNED

Water Distribution System No. 448-89622
 East of 119th West, South of Kellogg
 Mel Hambelton Addition
 Project No. 448-89622 OCA No. 735031
 Fractional lam March 4, 2002

OK 1102 3/4/02

CITY OF MICHIGAN Scale 1" = 150'





October 17, 2001

Mr. Christopher Carrico, P.E.
 Storm Water Engineer
 City Hall - 8th Floor
 455 N. Main
 Wichita, KS 67202

RECEIVED
 OCT 18 2001

RE: Mel Hambleton Addition
 Storm water Sewer Project
 Wichita, Kansas

Dear Chris:

As your aware, the owner of the above referenced site has requested us to modify the original drainage plan that was submitted at the time of platting. We have prepared engineered plans that routed the plat's existing pond through a 24" pipe to a proposed offsite detention pond. The system was designed to discharge a peak flow equal to the existing peak runoff that is occurring on the acquired land west of 119th Street. The proposed pond will detain the runoff using a 15" RCP as the pond's outfall control. The west detention pond is located outside of the Callskin's special flood hazard area, as mapped by F.E.M.A., to avoid additional permitting and to preserve the existing trees. We have used HEC-1 to model the stage vs. discharge of the two ponds and have attached the routings for your use. The two tables below summarizes the input variables used in the model and computed peak flow before and after being routed through the detention ponds.

Table 1: Input Variables

Mel Hambleton Addition	West Basin	D.A.	21.2 ac.	Tc	15 min.	CN	95
					15 min.		85

Table 2: Peak Flows & Pond Routings

Mel Hambleton Addition	Fast Pond	West Basin	West Pond
Q ₁	82 cfs	11 cfs	8 cfs
Q ₂	142 cfs	13 cfs	17 cfs
			11 cfs

SCANNED

Sincerely,
 Baughman Company, PA
M.C. J.K.
 Scott C. Lindbak, I.E.
 cc: file

Please call if you have any questions or concerns with the proposed project. Thank you.

Discharge to Callskin after improvements

Control flow from Basin & flow into Callskin now.

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * MAY 1991 *
 * VERSION 4.0.1E *
 * Lahey F77L-EM/32 version 5.01 *
 * Dodson & Associates, Inc. *
 * RUN DATE 10/17/01 TIME 15:37:53 *

 * U.S. ARMY CORPS OF ENGINEERS *
 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 551-1748 *

Mel Hambelton Addition
 Wichita, Sedgwick County, Kansas
 File: MelHam.INI
 Storm Events: 2,5,10,25,50, & 100 YR.

6 IO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

1T HYDROGRAPH TIME DATA
 NMIN 15 MINUTES IN COMPUTATION INTERVAL
 IDATE 1 0 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 4 0 ENDING DATE
 NDTIME 0245 ENDING TIME
 ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.25 HOURS
 TOTAL TIME BASE 74.75 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 1.00 1.29 1.51 1.74 1.97 2.23

SCANNED

*** **

 * 8 KK *
 * 81 *
 * *

East Site

SUBBASIN RUNOFF DATA

10 BA SUBBASIN CHARACTERISTICS
 TAREA 0.03 SUBBASIN AREA

PRECIPITATION DATA

11 PB STORM 3.50 BASIN TOTAL PRECIPITATION

12 PJ INCREMENTAL PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.01 0.01 0.00 0.01 0.00 0.01
 0.01 0.00 0.01 0.01 0.01 0.01 0.01 0.01 0.01 0.01

			6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
+	96.	12.00	(CFS)	13.	5.	2.
			(INCHES)	3.757	5.293	5.300
			(AC-FT)	7.	9.	9.
			CUMULATIVE AREA =	0.03 SQ MI		

*** *** *** *** ***

HYDROGRAPH AT STATION B1
FOR PLAN 1, RATIO = 1.74

TOTAL RAINFALL = 6.10, TOTAL LOSS = 0.00, TOTAL EXCESS = 6.10

PEAK FLOW	TIME		6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
+	111.	12.00	(CFS)	15.	5.	2.
			(INCHES)	4.324	6.093	6.100
			(AC-FT)	8.	11.	11.
			CUMULATIVE AREA =	0.03 SQ MI		

*** *** *** *** ***

HYDROGRAPH AT STATION B1
FOR PLAN 1, RATIO = 1.97

TOTAL RAINFALL = 6.90, TOTAL LOSS = 0.00, TOTAL EXCESS = 6.90

PEAK FLOW	TIME		6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
+	125.	12.00	(CFS)	17.	6.	2.
			(INCHES)	4.891	6.891	6.900
			(AC-FT)	9.	12.	12.
			CUMULATIVE AREA =	0.03 SQ MI		

*** *** *** *** ***

HYDRDGRAPH AT STATION B1
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.00, TOTAL EXCESS = 7.80

PEAK FLOW	TIME		6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
+	142.	12.00	(CFS)	20.	7.	2.
			(INCHES)	5.530	7.790	7.800
			(AC-FT)	10.	14.	14.
			CUMULATIVE AREA =	0.03 SQ MI		

*** **

24 KK

* *
* P1 *
* *

East Pond

HYDROGRAPH ROUTING DATA

SCANNED

+	(AC-FT)	(HR)				
	4.	12.75	3.	2.	1.	1.
	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
			6-HR	24-HR	72-HR	74.75-HR
+	(FEET)	(HR)				
	138.04	12.75	137.59	136.84	136.34	136.33

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.51

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
			6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
	12.	12.75	(CFS)			
			10.	4.	2.	2.
			(INCHES)	4.778	5.290	5.292
			(AC-FT)	8.	9.	9.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
			6-HR	24-HR	72-HR	74.75-HR
+	(AC-FT)	(HR)				
	5.	12.75	4.	2.	1.	1.

	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
			6-HR	24-HR	72-HR	74.75-HR
+	(FEET)	(HR)				
	138.37	12.75	137.89	136.97	136.39	136.38

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.74

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
			6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
	12.	13.00	(CFS)			
			11.	5.	2.	2.
			(INCHES)	5.535	6.089	6.091
			(AC-FT)	10.	11.	11.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
			6-HR	24-HR	72-HR	74.75-HR
+	(AC-FT)	(HR)				
	6.	13.00	5.	2.	1.	1.

	PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
			6-HR	24-HR	72-HR	74.75-HR
+	(FEET)	(HR)				
	138.71	13.00	138.22	137.12	136.45	136.43

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.97

	PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
			6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
	13.	13.00	(CFS)			
			12.	6.	2.	2.
			(INCHES)	6.290	6.888	6.890
			(AC-FT)	11.	12.	12.

	PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
			6-HR	24-HR	72-HR	74.75-HR

SCANNED

UNIT HYDROGRAPH
5 END-OF-PERIOD ORDINATES
0.

8. 3. 1. 0.
TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.48, TOTAL EXCESS = 2.02

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
+	6.	12.00	1.	0.	0.	0.
		(INCHES)	1.632	2.016	2.016	2.016
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.48, TOTAL EXCESS = 2.02

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
+	6.	12.00	1.	0.	0.	0.
		(INCHES)	1.632	2.016	2.016	2.016
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.29

TOTAL RAINFALL = 4.50, TOTAL LOSS = 1.59, TOTAL EXCESS = 2.91

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
+	8.	12.00	1.	0.	0.	0.
		(INCHES)	2.336	2.909	2.909	2.909
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 1.65, TOTAL EXCESS = 3.65

PEAK FLOW (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
+	11.	12.00	1.	0.	0.	0.
		(INCHES)	2.912	3.646	3.646	3.646
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.74

SCANNED

CUMULATIVE AREA = 0.04 SQ MI

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.29

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			74.75-HR
			6-HR	24-HR	72-HR	
16.	12.00	10.	4.	1.	1.	
		(INCHES) 2.398	3.887	4.299	4.301	
		(AC-FT) 5.	8.	9.	9.	

CUMULATIVE AREA = 0.04 SQ MI

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			74.75-HR
			6-HR	24-HR	72-HR	
19.	12.00	11.	5.	2.	2.	
		(INCHES) 2.796	4.640	5.091	5.093	
		(AC-FT) 6.	9.	10.	10.	

CUMULATIVE AREA = 0.04 SQ MI

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.74

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			74.75-HR
			6-HR	24-HR	72-HR	
23.	12.00	13.	5.	2.	2.	
		(INCHES) 3.108	5.395	5.885	5.887	
		(AC-FT) 6.	11.	12.	12.	

CUMULATIVE AREA = 0.04 SQ MI

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.97

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			74.75-HR
			6-HR	24-HR	72-HR	
26.	12.00	13.	6.	2.	2.	
		(INCHES) 3.338	6.150	6.679	6.681	
		(AC-FT) 7.	12.	13.	13.	

CUMULATIVE AREA = 0.04 SQ MI

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 2.23

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			74.75-HR
			6-HR	24-HR	72-HR	
28.	12.00	14.	7.	3.	2.	
		(INCHES) 3.548	6.998	7.574	7.576	

SCANNED

CUMULATIVE AREA = 0.04 SQ MI

HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.29

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	74.75-HR	
+ (CFS)	(HR)					
+ 8.	15.75	8.	4.	1.	1.	
		(INCHES)	1.882	3.837	4.291	4.293
		(AC-FT)	4.	8.	9.	9.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (AC-FT)	(HR)					
+ 2.	15.75	2.	1.	0.	0.	
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (FEET)	(HR)					
+ 136.81	15.75	136.66	135.87	135.37	135.35	

CUMULATIVE AREA = 0.04 SQ MI

HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.51

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	74.75-HR	
+ (CFS)	(HR)					
+ 9.	16.50	9.	5.	2.	2.	
		(INCHES)	2.171	4.588	5.083	5.084
		(AC-FT)	4.	9.	10.	10.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (AC-FT)	(HR)					
+ 2.	16.50	2.	1.	0.	0.	
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (FEET)	(HR)					
+ 137.10	16.50	136.96	136.04	135.43	135.41	

CUMULATIVE AREA = 0.04 SQ MI

HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.74

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW				
		6-HR	24-HR	72-HR	74.75-HR	
+ (CFS)	(HR)					
+ 10.	17.25	10.	5.	2.	2.	
		(INCHES)	2.399	5.338	5.876	5.877
		(AC-FT)	5.	11.	12.	12.
PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (AC-FT)	(HR)					
+ 3.	17.25	2.	1.	1.	1.	
PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE				
		6-HR	24-HR	72-HR	74.75-HR	
+ (FEET)	(HR)					
+ 137.35	17.25	137.22	136.22	135.49	135.47	

SCANNED

MEL HAMBELTON ADDITION - POND ROUTINGS

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION						
				RATIO 1 1.00	RATIO 2 1.29	RATIO 3 1.51	RATIO 4 1.74	RATIO 5 1.97	RATIO 6 2.23	
HYDROGRAPH AT +	B1	0.03	1	FLOW TIME	64. 12.00	82. 12.00	96. 12.00	111. 12.00	125. 12.00	142. 12.00
ROUTED TO +	P1	0.03	1	FLOW TIME	9. 12.75	11. 12.75	12. 12.75	12. 13.00	13. 13.00	13. 13.00
				** PEAK STAGES IN FEET **						
			1	STAGE TIME	137.63 12.75	138.04 12.75	138.37 12.75	138.71 13.00	139.04 13.00	139.39 13.00
HYDROGRAPH AT +	B2	0.00	1	FLOW TIME	6. 12.00	8. 12.00	11. 12.00	13. 12.00	15. 12.00	17. 12.00
2 COMBINED AT +	C1	0.04	1	FLOW TIME	11. 12.25	16. 12.00	19. 12.00	23. 12.00	26. 12.00	28. 12.00
ROUTED TO +	P2	0.04	1	FLOW TIME	6. 15.50	8. 15.75	9. 16.50	10. 17.25	11. 18.00	11. 18.75
				** PEAK STAGES IN FEET **						
			1	STAGE TIME	136.37 15.50	136.81 15.75	137.10 16.50	137.35 17.25	137.56 18.00	137.77 18.75

*** NORMAL END OF HEC-1 ***

SCANNED



Department of Public Works

August 23, 2001

Mr. Stan Richardson
5607 Woodland Road
Hutchinson, KS 67502

RE: Mel Hambleton Addition

RECEIVED

AUG 23 2001

CITY - ENGINEERING

Dear Stan:

At your request, we have completed our preliminary review of your proposal to change the drainage outfall from this subdivision. During the platting process, the original drainage plan showed that the drainage from this site would be collected in a storm water detention facility at the south edge of the property near the Central Kansas Railroad right-of-way, then pumped to the north by a pump station with the outfall being in the Kellogg Drive right-of-way. You have proposed an alternate plan wherein the outfall from the storm water detention area would run westerly along the Central Kansas Railroad right-of-way in a 24-inch underground conduit ultimately entering another pond on property that is owned by Mel Hambleton, west of 119th Street. Ultimately that pond would discharge into the main branch of Calfskin Creek south of Kellogg Drive.

Your proposal, if approved, would result in additional waters flowing through Wheatridge Addition which is located north of Kellogg and west of 119th Street. This concerns us as there are several properties in that subdivision that have repeatedly flooded during heavy rains in the Calfskin Creek Basin. Because of this, before we can give final approval to your proposal, we would need to see an engineering analysis of your proposal and Calfskin Creek demonstrating that what you are proposing to do will not increase flood levels on Calfskin Creek downstream.

In addition to the above, we would like to see you keep the outfall pipe from the storm water detention facility in Reserve A on property that is owned by Mel Hambleton as much as possible. This would, of course, reduce the amount of railroad right-of-way that you would be encroaching upon with the pipe. Also, please understand that the City's approval of your proposal would be dependent upon your receiving the proper permits from the Central Kansas Railroad and any utility companies that have lines within the railroad right-of-way or the 119th Street right-of-way.

If you have any questions concerning the above, please let me know.

Sincerely,

Christopher Carrier
Storm Water Engineer

cc: Steve Lackey, Director of Public Works
Mike Lindebak, City Engineer
Vicky Huang, Subdivision Design

Storm Water Management Division

City Hall • 8th Floor • 455 N. Main • Wichita, Kansas 67202

T 316.268.4498