

ENGINEERING, SURVEYING & LAND PLANNING

S R B

SAVOY, RUGGLES & BOHM, P.A.

**FINAL DRAINAGE PLAN
FOR
TARA FALLS
SEDGWICK COUNTY, KANSAS
AND SUPPLEMENTAL CALCULATIONS**

MAY 2, 2000



924 NORTH MAIN
WICHITA, KANSAS 67203
<http://www.feist.com/~srb>

316-264-8008
FAX 264-4621
srb@feist.com

SAVOY, RUGGLES & BOHM, P.A.
ENGINEERING & SURVEYING

May 2, 2000

Mr. Chris Carrier, P.E.
Storm Water Management
City Hall - 8th Floor
455 N. Main
Wichita, KS 67202

Re: Tara Falls Addition, Sedgwick County, Kansas

Dear Mr. Carrier,

At your request, I have run hydrologic models of the Tara Falls detention pond system for the 5, 10, 25, 50, and 100-year return period storm events in order to demonstrate the discharge characteristics of the developed basin. In each run, the undeveloped model has been calculated in conjunction with the developed model, so that a comparison may be made between the undeveloped and developed discharges from the site. These discharges are tabulated on the following page for your convenience.

Please note that the HEC-1 model used for the original drainage calculations has been modified only with regard to rainfall for each of the design storms. The rainfall totals for each design storm are gleaned from ESSA Weather Bureau Technical Paper 40 Modified to NWS Hydro-35, 1977, using the SCS Type II rainfall distribution.

Also included with this report is a copy of the original drainage plan for your files.

I hope this information proves helpful.

Regards,

Christopher M. Bohm, P.E.

Encl.

Summary of Pond Routing Trials
 Tara Falls Addition
 2-May-00

Shows same
 increases for
 smaller
 storms!

Return Peroid (years)	Rainfall (24 hour) (inches)	Existing Runoff (Undeveloped) (cfs)	Pond Outflow (Developed) (cfs)	Percentage of Undeveloped Flow (percent)
5	4.8"	153	168	109.8%
10	5.52"	200	203	101.5%
25	6.48"	258	250	96.9%
50	7.44"	325	296	91.1%
100	7.8"	351	313	89.2%

CMB
1-9-98

Terr Falls

Upstream Basin (Existing) Exiting @ NE
Corner of Site (Meier Property)

Upstream Drainage Area = 107 Act + 27 Acres
= 134 Acres

(From Site Survey; USGS Quad Sheets)

Hydrological Soil Group "D" (Sedgwick County
Soil Survey)

Time of Concentration: Beg. @ U.S. of Basin;
Existing Subdivision:

Tc1: 300' Grass lawn @ 2%; V = 0.45 f/s
Tc1 = 11.1 min.

Tc2: 900' Shallow Curb flow; V = 0.91 f/s
@ 0.40%; Tc2 = 14.4 min

Tc3: Full Pavement flow; L = 1350'; S = 0.40%
V = 3.5 f/sec (see other calcs).
Tc3 = 6.43 min.

Tc4: Channel flow; Use V = 2.5 f/sec (Natural Chan)
L = 2400'
Tc4 = 14.0 min

(Note: Tc4 @ Dam face).

Ignore Dam for Tc; Continue to Harry

Tc5: L=1500' Use V=3.0 f/s

$$Tc5 = 8.3 \text{ min}$$

Tc Total := 58 min (Velocity Method).

Check w/ other Tc Formulas:

Kirpich: $Tc = 0.00781 L^{0.77} S^{-0.385}$

$$L = 6450' \quad \bar{S} = \frac{1370 - 1318}{6450} = 0.806\%$$

$$Tc = 0.00781 (6450)^{0.77} (0.00806)^{-0.385}$$

$$Tc = 42.9 \text{ minutes (Use 43)}$$

SCS Curve No:

Residential: 29 Acres @ CN=93 (0.216)

Balance Pasture 105 Acres @ CN=80 (0.784)

$$\bar{CN} = 82.8; \text{ Use } 83.0$$

$$\text{SCS Lag} = 0.6 Tc; \quad Tc = \frac{58 + 43}{2} = 50.5$$

$$\text{SCS Lag} = (0.6) \left(\frac{50 \text{ min}}{60 \text{ min/hr}} \right) = 0.50 \text{ hours}$$

$$BA = 134 \text{ Ac} / 640 = 0.209 \text{ sq. miles}$$

To HEC-1 (faraml.hcl)

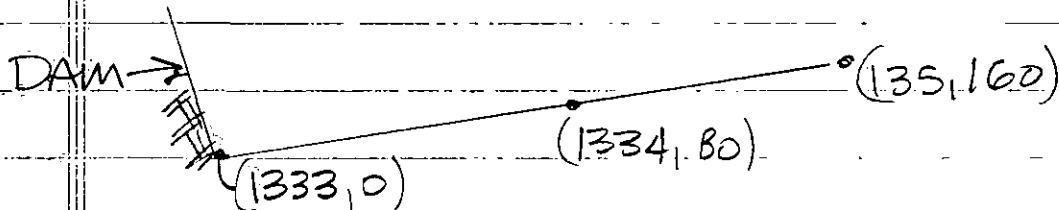
Results w/o Pond: (Existing Condition)

$$Q_{100} = 443 \text{ cfs}$$

Add Pond Into Routing:

Pond has no defined spillway, One small trickle pipe to hold static pool.

East Side of Dam is "Flat" @ 1333 Elevation, Grassed, and Slopes to S.E. As Follows:



Assume Triangular Channel



$$A = 40 \text{ ft}^2$$

$$n = 0.03 \text{ (grass)}$$

$$w_p = 81 \text{ feet}$$

$$S = 0.005 \text{ (Assume)}$$

$$V @ D=1': V = \frac{1.49}{0.03} \left(\frac{40}{81} \right)^{0.67} (0.005)^{1/2} = 2.18 \text{ f/s}$$

$$Q = V \cdot A = (2.18)(40) = 87.2 \text{ cfs}$$

$$V @ D=2': A = (2)(160)/2 = 160 \text{ ft}^2$$

$$w_p = 162 \text{ ft}$$

$$V = (1.49/0.03) \left(\frac{160}{162} \right)^{0.67} (0.005)^{1/2} = 3.48 \text{ cfs}$$

$$Q = V \cdot A = 160 \times 3.48 = 557 \text{ cfs}$$

NEED TO RATE C.O.S' Increments for detail of Outflow vs Storage.

<u>ELEV</u>	<u>Q</u>	
@1333	0	→ Controlling Elevation
@1333.5	D=0.5; A=10ft ² ; WP=40.5	
	V=1.37; Q=14 cfs	
@1334	Q=87 cfs (previous)	
@1334.5	D=1.5; A=(120*1.5/2)=90ft ²	
	WP=121.5	
	V=2.87; Q=258.5	
@1335	Q=557 cfs (previous)	

West Side of Pond Also can discharge across grass at Elev. 1334's Above. To be conservative, this outflow will be ignored to maximize the pond storage.

Existing Pond Stage-Storage:
(From other Calculations - SRB Survey)

<u>Elev</u>	<u>Area</u> (ft ²)	<u>Area</u> (Ac)
1333	217599	5.0
1334	321695	7.4
1335	426678	9.8

To Model Through Pond - Separate U.S. Pond Drainage Area for Routing.

Area to Pond = 107 Acres

$T_{c\text{pond}} = 50$ minutes (from previous calc's - Velocity Method). To Be consistent, subtract 8 minutes (from Dam to Harry) since other T_c was Averaged.
Use $T_c = 42$ minutes

$$\text{SCS Lag} = 0.6T_c = (0.6)(42^m / 60^m/h) = 0.42 \text{ hours}$$

To HEC-1: $BA = 107 / 640 = 0.167$ sq. miles

$CN = 83$ (As Before)

SCS Lag = 0.42 hrs

Stage Storage / Stage Discharge As Calculated.

file = taram2.hcl

To this routed hydrograph; add the additional 27 Acres (HC card).

For this Area, use $T_c = 15$ minutes and

SCS CN of 80 (pasture).

$$BA = 27 / 640 = 0.042 \text{ sq. miles}$$

SCS Lag = 0.15 hours

$CN = 80$

To HEC-1 (taram2.hcl)



Results: (See Output for detailed results)

$$Q_{\text{in pond}} = 388 \text{ cfs}$$

$$Q_{\text{out Pond}} = 317 \text{ cfs (after routing)}$$

$$Q_{\text{downstream area}} = 128 \text{ cfs}$$

$$Q_{\text{combined}} = 351 \text{ cfs (hydrograph addition)}$$

The determined Q_{100} c Harry (Meier Property)
= 351 cfs as modeled.

For A Quick Check; Use Rational Method
(Ignoring Pond Storage)

$$A = 134 \text{ Ac}$$

$$T_c = 50 \text{ min}; i_{100} = 4.13 \text{ "/hr}$$

Use $C = 0.65$ (C of Wichitz; Type D 1-4% slope)

$$Q = C i A = 0.65 (4.13) (134) = 359.7 \text{ cfs}$$

FOR THE DEVELOPED SITE; NO MORE THAN
350 cfs MAY EXIT c HARRY. (100 yr storm).

FOR UPSTREAM (EXISTING) POND:

$$DA = 107 \text{ ACRES}$$

NEW CURVE NUMBER DUE TO DEVELOPMENT:

$$29 + 37 (\text{on site}) = 66 \text{ Acres Residential}$$

$$107 - 66 = 41 \text{ Ac Pasture}$$

$$CN = \left(\frac{66}{107}\right)(93) + \left(\frac{41}{107}\right)(80) = \underline{\underline{88}}$$

Since the main basin drainage is via a grass channel (some natural - some man made) the T_c will not change. A Box Culvert will be necessary Under Bluestem St Near the NW corner of the plot; And detention storage here will be omitted (conservative).

Existing Pond will be controlled with outlet weirs to force the 100 year W.S. Elevation.

Weir Elevation = 1332.50

East weir = 50' length

West weir = 20' length

Concrete; Rip-Rap Bag; or Rip Rap weirs to Be Used.

For such weirs; $Q = CLH^{1.5}$ where $C = 3.0$

Total length = 70 feet (65)

Elev	H	Q	
		(w=70')	(w=65')
1332.5	0	0	0
1332.75	0.25	26	24
1333	0.5	74	69
1333.25	0.75	136	127
1333.5	1.0	210	195
1333.75	1.25	293	272
1334	1.5	385	358
1334.25	1.75	486	451

Use 65' weirs
(Total width)
1-27-98
CMB

8/9

CMB
11/9/98

As Before; Stage-Storage Remains the Same: See sheet # _____ (Existing Pond).

To HEC-1: BA = 0.167 sq. miles
SCS Lag = 0.42 hrs
CN = 88

Route through Modified Pond
file = taram3.hcl

Results:

Q_{in} Pond = 420 cfs

11/27/98
CMB

Q_{out} Pond = ~~353~~ 348 cfs @ Elev = ~~1333.91~~ 1333.97 < 1334
OK ✓

This hydrograph will be added to the D.S. Basin (Improved Condition) and then routed through the new detention pond.

D.S. BASIN = 27 Acres.

CN = 93

T_c = 15 min ; SCS Lag = 0.15 hours.

New Detention Pond:

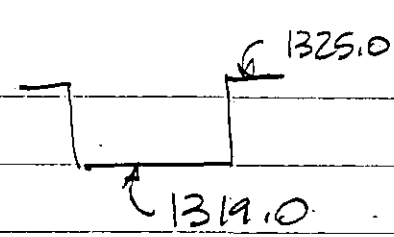
This pond will utilize the existing D.S. Channel, just u.s. of the Meier Property.

To control outflow, a Rip-Rap or concrete weir will be used.

9/9

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1/9/98

Use weir; L=13 feet.

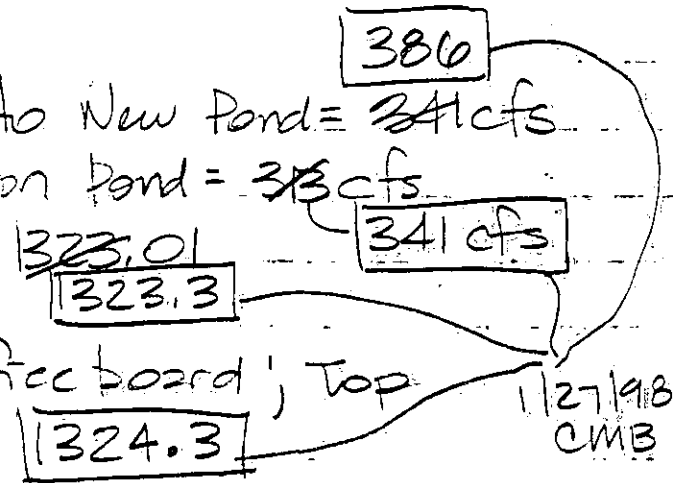


ELEV	Q	STORAGE (ACRES)
1319	—	1.9
1319.5	14	
1320	39	2.1
1320.5	71	
1321.0	110	2.3
1321.5	154	
1322.0	202	2.5
1322.5	255	
1323.0	312	2.7
1323.5	372.3	

ROUTE NEW SYSTEM:

HEC-1 tar2m3.kc1

RESULTS: Comb. Q₁₀₀ to New Pond = 386 cfs
 Through New Detention Pond = 341 cfs
 Elevation Max = 1323.01



Allow 1.0 foot of freeboard; Top
 Weir Elevation = 1324.0

1/27/98
CMB

1-27-98

10/11

CMB

Tre Falls NW Corner Pond "Reserve C"
Pond 3

Outlet from Site = 36" RCP

Flow = 132310; Road Elev = 1329.0

Max Head Avail = $29.0 - 23.1 = 4.9$ feet

Area to N.W. Corner = 28 Acres

1/4 Ac developed $C_{100} = 0.74$

1/4 Ac Pasture $C_{100} = 0.65$

$\bar{C} = 0.71$, $A = 28$ Ac; $T_c = 20$ min

$i_{100} = 6.53$ "/hr.

$$Q_{100} = (0.71)(28)(6.53) = 130 \text{ cfs}$$

Check w/ HEC-1

SCS Lag = 0.20 hours

$$CN = 93 + 80/2 = 87$$

100 yr - 24 hr storm

$$BA = 28/640 = 0.0438 \text{ sq. miles}$$

(over)

Terz Falls 1-27-98

11/11

Route through New Pond in Res "C"

Elev	Area	HW/D (1323.10 #)	Q pipe (Highway Culvert Chet.)
1329	1.05	1.97	73
1328	0.96	1.6	65
1327	-	1.3	52
1326	0.78	1.0	37
1325	-	0.6	16
1324	0.62	0.3	4
1323.5	0.51	-	-

Results

$$Q_{in} = 140 \text{ cfs}$$

$$Q_{out} = 70 \text{ cfs (culvert)}$$

$$100 \text{ yr W.S.} = 1328.58; \text{ Use } 1329.0$$

Check 100 yr W.S. on Spring Branch Four
Mile Creek on N. Side Harry.

$$\text{Elev @ W. Culvert} = 1325.5 < 1329.0 \text{ O.K.}$$

Culvert Controls.

Terz 3.1.1

HEC1 S/N: 1343000364

HMVersion: 6.33

Data File: taram3.hcl

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*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1B *
* RUN DATE 01/20/1998 TIME 14:59:23 *
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*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
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::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
:::
.....
.....

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37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1XW. THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 01/20/1998 TIME 14:59:23 *

* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *

Tara Falls - Modified Existing pond with new DS detention pond

4 IO OUTPUT CONTROL VARIABLES

IPRNT 4 PRINT CONTROL
IPLOT 0 PLOT CONTROL
QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 289 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 2 0 ENDING DATE
NDTIME 0000 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL: 0.08 HOURS
TOTAL TIME BASE 24.00 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRES- FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

26 KK

*
* DPOND *
*

HYDROGRAPH ROUTING DATA

27 RS

STORAGE ROUTING

1 NUMBER OF SUBREACHES
NSTPS TYPE OF INITIAL CONDITION
ITYP INITIAL CONDITION
RSVRC 1319.00
X 0.00 WORKING R AND D COEFFICIENT

28 SA

AREA

1.9 2.1 2.3 2.5 2.7

29 SE

ELEVATION

1319.00 1320.00 1321.00 1322.00 1323.00

30 SQ

DISCHARGE

0. 14. 39. 71. 110. 154. 202. 255. 312.

31 SE

ELEVATION

1319.00 1319.50 1320.00 1320.50 1321.00 1321.50 1322.00 1322.50 1323.00

STORAGE 0.00 2.00 4.20 6.60 9.20
ELEVATION 1319.00 1320.00 1321.00 1322.00 1323.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE 0.00 0.97 2.00 3.07 4.20 5.37 6.60 7.87 9.20
OUTFLOW 0.10 14.00 39.00 71.00 110.00 154.00 202.00 255.00 312.00
ELEVATION 1319.00 1319.50 1320.00 1320.50 1321.00 1321.50 1322.00 1322.50 1323.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

WARNING --- ROUTED OUTFLOW (313.) IS GREATER THAN MAXIMUM OUTFLOW (312.) IN STORAGE-OUTFLOW TABLE

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF	
				6-HOUR	24-HOUR	72-HOUR			MAX STAGE	24-HOUR
HYDROGRAPH AT	BASINA	420.	12.25	89.	28.	28.	0.17			
ROUTED TO	POND	307.	12.50	88.	28.	28.	0.17	1334.30	12.50	
HYDROGRAPH AT	BASINB	151.	12.00	24.	8.	8.	0.04			
2 COMBINED AT	COMB	341.	12.50	112.	36.	36.	0.21			
ROUTED TO	DPOND	313.	12.67	111.	36.	36.	0.21	1323.01	12.67	

*** NORMAL END OF HEC-1 ***

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
*
* RUN DATE 01/20/1998 TIME 15:00:13 *
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
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*****

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Full Microcomputer Implementation
by
Haestad Methods, Inc.

```

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

FLOOD HYDROGRAPH PACKAGE (HEC-1) *
MAY 1991 *
VERSION 4.0.1E *
RUN DATE 01/20/1998 TIME 15:00:13 *

U.S. ARMY CORPS OF ENGINEERS *
HYDROLOGIC ENGINEERING CENTER *
609 SECOND STREET *
DAVIS, CALIFORNIA 95616 *
(916) 756-1104 *

Tara Falls Existing Basin to Harry with existing pond modeled.

4 IO OUTPUT CONTROL VARIABLES

I PRINT 4 PRINT CONTROL
I PLOT 0 PLOT CONTROL
Q SCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA

NMIN 5 MINUTES IN COMPUTATION INTERVAL
IDATE 1 0 STARTING DATE
ITIME 0000 STARTING TIME
NQ 289 NUMBER OF HYDROGRAPH ORDINATES
NDDATE 2 0 ENDING DATE
NDTIME 0000 ENDING TIME
ICENT 19 CENTURY MARK

COMPUTATION INTERVAL 0.08 HOURS
TOTAL TIME BASE 24.00 HOURS

ENGLISH UNITS

DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FOOT
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

5 KK * BASINA *

27 END-OF-PERIOD ORDINATES

15.	46.	96.	147.	173.	173.	155.	129.	94.	68.
50.	38.	28.	21.	16.	12.	9.	7.	5.	4.
3.	2.	2.	1.	1.	1.	0.			

14 KK
* POND *
* *

HYDROGRAPH ROUTING DATA

15 RS
STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 1333.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

16 SA	AREA	5.0	7.4	9.8
17 SE	ELEVATION	1333.00	1334.00	1335.00
18 SQ	DISCHARGE	0.	14.	87.
19 SE	ELEVATION	1333.00	1333.50	1334.00
			1334.00	1334.50
				1335.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	6.16	14.73
ELEVATION	1333.00	1334.00	1335.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	2.78	6.16	10.15	14.73
OUTFLOW	0.10	14.00	87.00	258.50	557.00
ELEVATION	1333.00	1333.50	1334.00	1334.50	1335.00

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SUBBASIN RUNOFF DATA

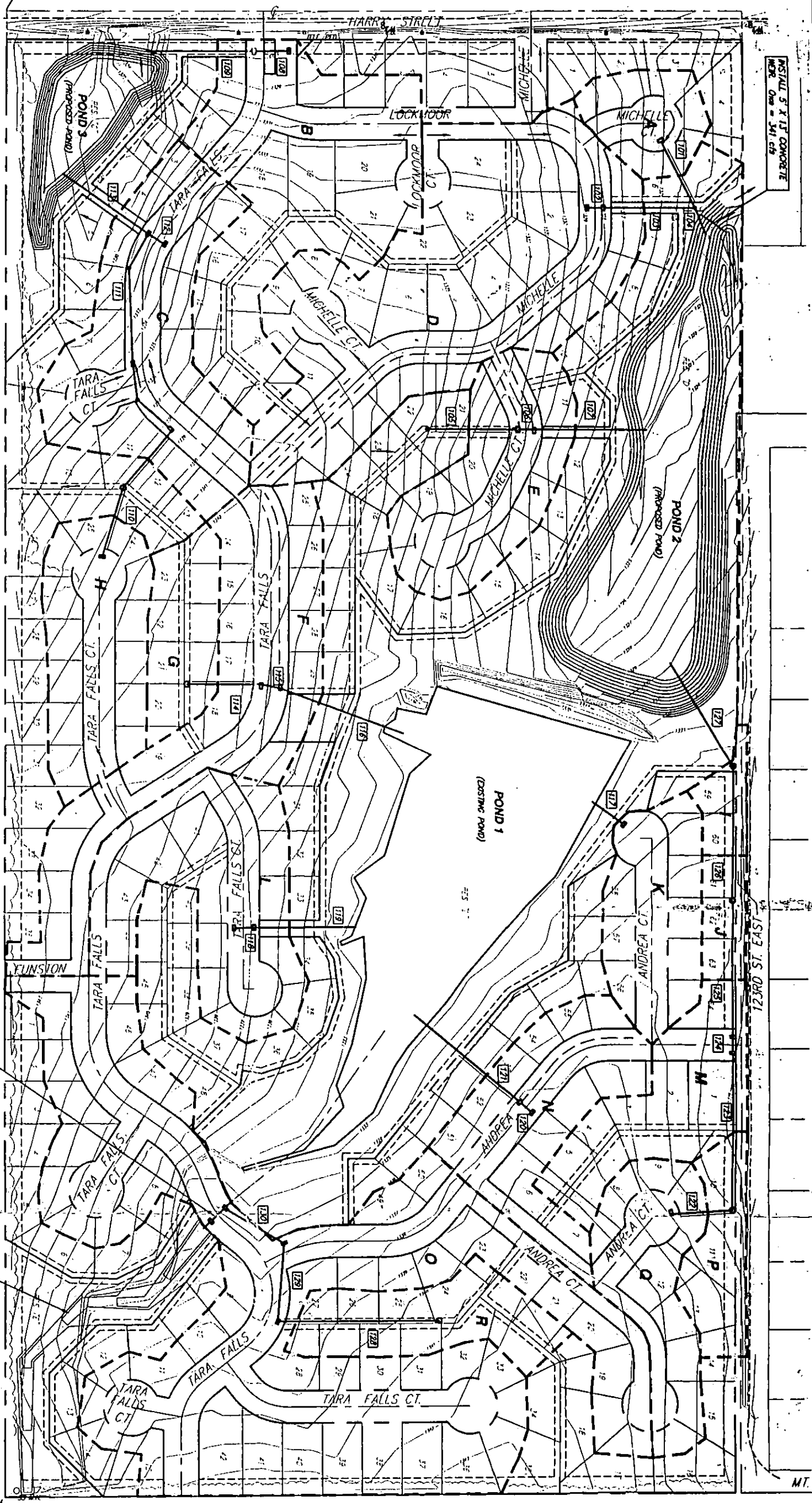
21 BA SUBBASIN CHARACTERISTICS
TAREA 0.04 SUBBASIN AREA

PRECIPITATION DATA

RUNOFF SUMMARY
 FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
				6-HOUR	24-HOUR	72-HOUR			
HYDROGRAPH AT	BASINA	388.	12.25	82.	26.	26.	0.17		
ROUTED TO	POND	317.	12.42	80.	25.	25.	0.17	1334.60	12.42
HYDROGRAPH AT	BASINB	128.	12.00	20.	6.	6.	0.04		
2 COMBINED AT	COMB	351.	12.42	99.	31.	31.	0.21		

*** NORMAL END OF HEC-1 ***



INSTALL 5' X 12" CONCRETE
MANHOLE - 341 cfs

123RD ST. EAST

MT.

POND SUMMARY

- Pond 1: In (100 y) = 420 cfs
Out (100 y) = 348 cfs
Outlet Pipe = 1512.5
100 yr. Water Surface = 1334.0
- Pond 2: In (100 y) = 388 cfs
Out (100 y) = 341 cfs
Outlet = 5' X 12" Matched Man
Stake Pond = 1320.0
100 yr. Water Surface = 1324.3
- Pond 3: In (100 y) = 140 cfs
Out (100 y) = 70 cfs
Stake Pond = 1323.5
100 yr. Water Surface = 1328.0

DRAINAGE BASIN SUMMARY

Basin	Area (Ac)	In	C	Out	Q	Q _{max}		
A	0.79	15	1.61	2.17	0.57	0.79	1.2	4.6
B	3.93	15	3.61	2.17	0.51	0.71	7.2	20.6
C	3.15	15	3.61	2.17	0.57	0.79	6.2	18.3
D	5.79	15	3.61	2.17	0.57	0.79	11.4	30.3
E	2.01	15	3.61	2.17	0.57	0.79	4.4	12.8
F	3.40	15	3.61	2.17	0.57	0.79	7.2	19.8
G	1.97	15	3.61	2.17	0.57	0.79	1.5	4.8
H	1.97	15	3.61	2.17	0.57	0.79	4.2	11.2
I	0.34	15	3.61	2.17	0.41	0.67	0.5	2.0
J	0.41	15	3.61	2.17	0.41	0.67	0.5	2.0
K	1.49	15	3.61	2.17	0.57	0.79	3.3	8.2
L	2.73	15	3.61	2.17	0.57	0.79	6.0	15.9
M	1.65	15	3.61	2.17	0.45	0.71	4.8	6.6
N	1.05	15	3.61	2.17	0.57	0.79	4.5	11.9
O	10.28	15	3.61	2.17	0.71	0.71	20.0	53.7
P	0.84	15	3.61	2.17	0.41	0.67	1.1	4.1
Q	1.58	15	3.61	2.17	0.57	0.79	3.4	9.2
R	1.58	15	3.61	2.17	0.41	0.67	2.8	6.8
S	1.40	15	3.61	2.17	0.41	0.67	2.2	6.0
T	1.41	15	3.61	2.17	0.41	0.67	2.2	6.0
U	2.14	30	2.61	5.40	0.71	0.67	24.2	60.1

PIPE SUMMARY

Pipe	Size	Grade	Length	Q (GPM)
101	18"	0.50%	4.6	1.1
102	18"	0.50%	15.1	3.6
103	24"	2.00%	30.3	7.2
104	30"	1.00%	44.9	11.0
105	18"	0.50%	3.6	0.9
106	18"	0.50%	9.5	2.4
107	18"	2.00%	15.3	3.8
108	18"	1.00%	10.0	2.5
109	24"	0.50%	20.0	5.0
110	24"	0.50%	11.2	2.8
111	24"	0.50%	15.7	4.0
112	18"	1.00%	3.1	0.8
113	24"	1.00%	21.9	5.5
114	18"	0.50%	4.8	1.2
115	18"	1.00%	6.5	1.6
116	24"	0.40%	12.2	3.1
117	18"	1.00%	6.7	1.7
118	18"	0.50%	7.4	1.9
119	24"	0.50%	15.9	4.0
120	24"	0.50%	6.0	1.5
121	24"	0.50%	11.9	3.0
122	24"	1.00%	9.2	2.3
123	24"	0.40%	13.4	3.4
124	24"	0.70%	17.6	4.4
125	24"	1.00%	21.9	5.5
126	24"	1.00%	21.9	5.5
127	24"	1.00%	21.9	5.5
128	24"	1.00%	21.9	5.5
129	24"	1.00%	21.9	5.5
130	24"	1.00%	21.9	5.5

Design channel depth of 10.5
Min. slope = 0.8%

INSTALL 3' X 5' ROBC
Man = 196 cfs

DRAINAGE PLAN
TARA FALLS ADDITION
SEDGWICK COUNTY, KANSAS

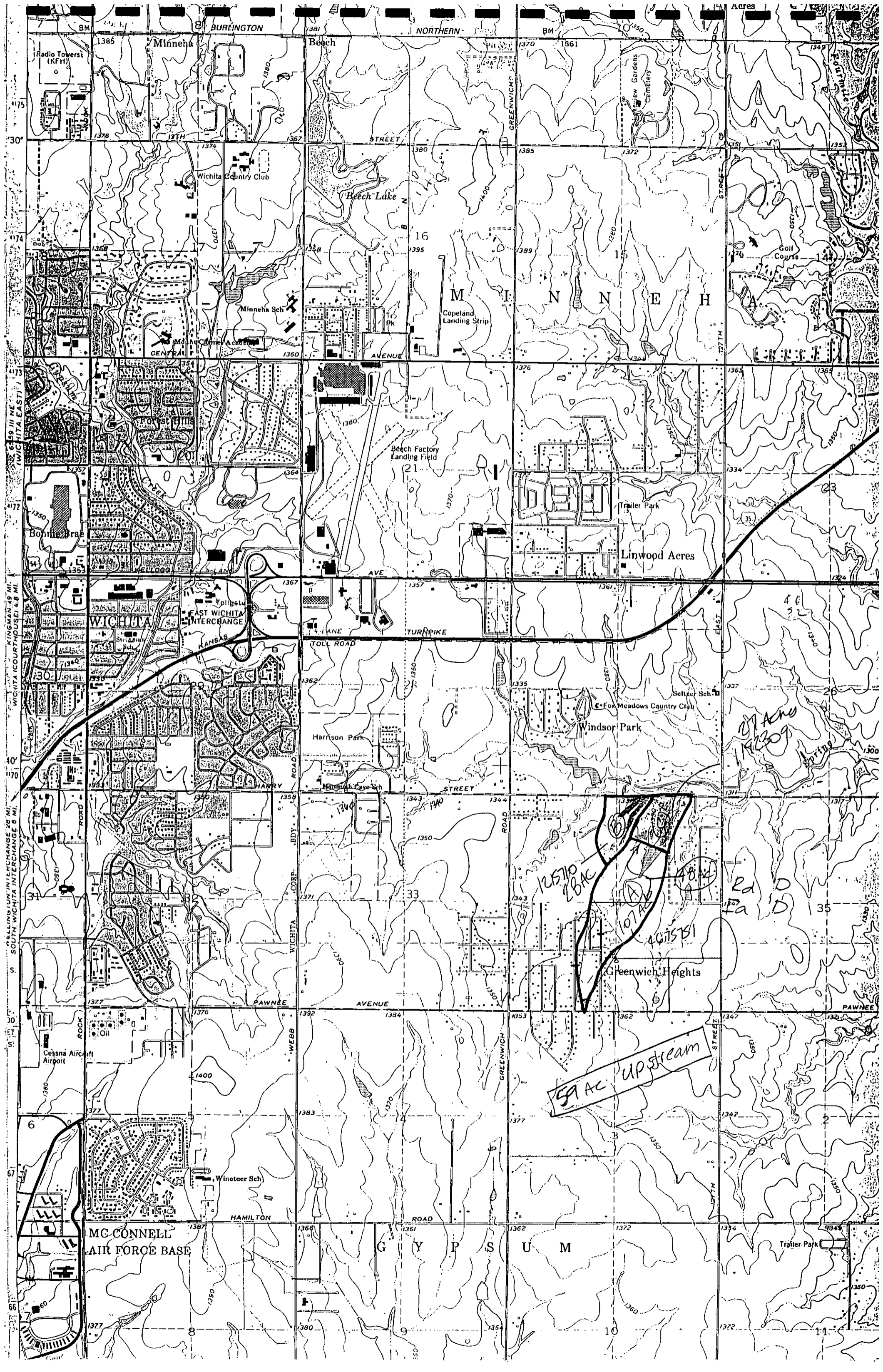
SRB
322 NORTH MAIN
MICHIGAN, KANSAS 67303
316-264-0000
FAX 316-264-0001

SAVOY, RUGGLES & BOHN, P.A.
ENGINEERING & SURVEYING
1001 W. 10TH ST.
WICHITA, KANSAS 67202

DATE: 3/19/97

PROJECT: 1

SCALE: 1" = 40'



Radio Towers (KFH)

Wichita Country Club

Minneha Sch

Bonnie Brae

WICHITA

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

Minneha

Wichita Country Club

Minneha Sch

Bonnie Brae

WICHITA

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

Beech

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

NORTHERN

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

MINNEHA

Beech Lake

Minneha Sch

Beech Factory Landing Field

Wichita Interchange

Wichita Corp

Cessna Airport

MC CONNELL AIR FORCE BASE

Wineteer Sch

6 59 III NE (WICHITA EAST)

12570 28 AC
12571 48 AC
59 AC upstream

```

*****
*****
FLOOD HYDROGRAPH PACKAGE (HEC-1)
MAY 1991
VERSION 4.0.1E
RUN DATE 05/02/2000 TIME 13:32:00
*****
*****

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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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*****
*****
Full Microcomputer Implementation
by
Haestad Methods, Inc.
*****
*****

```

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G5, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND-AMPT INFILTRATION
 KINEMATIC WAVE: NEW:FINITE DIFFERENCE ALGORITHM

EQ

HEC-1 INPUT

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
*** FREE ***
1 ID Tara Falls Existing Basin to Harry with existng pond modeled, 5 year, 24 hour
2 IT 5
3 IN 30 289
4 IO 4
5 KK BASIN
6 EA 0.167
7 PC 0
8 PC 0.30 0.34 0.38 0.43 0.47 0.52 0.58 0.64 0.71 0.78
9 PC 0.87 0.98 1.13 1.36 3.18 3.53 3.71 3.84 3.94 4.02
10 PC 4.09 4.16 4.22 4.28 4.32 4.36 4.41 4.45 4.49 4.53
11 PC 4.57 4.61 4.64 4.67 4.70 4.72 4.75 4.78 4.80
12 UD 0.42
13 LS 0 83
14 KK POND
15 RS 1 ELEV 1333
16 SA 5 7.4 9.8
17 SE 1333 1334 1335
18 SQ 0.1 14 87 258.5 557
19 SE 1333 1333.5 1334 1334.5 1335
20 KK BASINS
21 EA 0.042
22 UD 0.15
23 LS 0 80
24 KK COMB
25 HC 2

```

```

26      KK BASINA
27      BA 0.167
28      UD 0.42
29      LS 0      88

30      KK POND
31      RS 1      ELEV 1332.50
32      SA 4.3    5      7.4    9.8
33      SE 1332.5 1333    1334    1335
34      SQ 0.1    15     45     82     127    177    233    294
35      SE 1332.5 1332.75 1333 1333.25 1333.5 1333.75 1334 1334.25

36      KK BASINB
37      BA 0.042
38      UD 0.15
39      LS 0      93

40      KK COMB
41      HC 2

```

HEC-1 INPUT

PAGE 2

□□

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

42      KK DPOND
43      RS 1      ELEV 1319
44      SA 1.9    2.1    2.3    2.5    2.7
45      SE 1319 1320 1321 1322 1323
46      SQ 0.1    14     39     71     110    154    202    255    312
47      SE 1319 1319.5 1320 1320.5 1321 1321.5 1322 1322.5 1323
48      ZZ

```

HMVersion: 6.33 Data File: tf5.hcl

HEC1 S/N: 134300364

```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
*
* RUN DATE 05/02/2000 TIME 13:32:00 *
*
*
*****

```

```

* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*

```

Tara Falls Existing Basin to Harry with existng pond modeled, 5 year, 24 hour

```

4 IO      OUTPUT CONTROL VARIABLES
          IPRNT 4 PRINT CONTROL
          IPLOT 0 PLOT CONTROL
          QSCAL 0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN 5 MINUTES IN COMPUTATION INTERVAL
          IDATE 1 0 STARTING DATE
          ITIME 0000 STARTING TIME
          NQ 289 NUMBER OF HYDROGRAPH ORDINATES
          NDDATE 2 0 ENDING DATE
          NDTIME 0000 ENDING TIME
          ICENT 19 CENTURY MARK

          COMPUTATION INTERVAL 0.08 HOURS
          TOTAL TIME BASE 24.00 HOURS

ENGLISH UNITS
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-Feet
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

```

```

*****
*
* 5 KK BASINA *
*
*****

```


42 KK

```

*****
*   DPOND   *
*****
  
```

HYDROGRAPH ROUTING DATA

43 RS	STORAGE ROUTING	NSTPS	1	NUMBER OF SUBREACHES							
		ITYP	ELEV	TYPE OF INITIAL CONDITION							
		RSVVIC	1319.00	INITIAL CONDITION							
		X	0.00	WORKING R AND D COEFFICIENT							
44 SA	AREA		1.9	2.1	2.3	2.5	2.7				
45 SE	ELEVATION		1319.00	1320.00	1321.00	1322.00	1323.00				
46 SQ	DISCHARGE		0.	14.	39.	71.	110.	154.	202.	255.	312.
47 SE	ELEVATION		1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	2.00	4.20	6.60	9.20					
ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00					

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.97	2.00	3.07	4.20	5.37	6.60	7.87	9.20		
OUTFLOW	0.10	14.00	39.00	71.00	110.00	154.00	202.00	255.00	312.00		
ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00		

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

6-HOUR	OPERATION	STATION	PEAK	TIME OF	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN	MAXIMUM	TIME OF	
	24-HOUR	72-HOUR	FLOW	PEAK	TIME	PERIOD	AREA	STAGE	MAX STAGE		
	HYDROGRAPH AT				BASINA	203.	12.25	43.	13.	13.	0.17
1334.15	ROUTED TO				POND	138.	12.56	40.	13.	13.	0.17
	12.58										
	HYDROGRAPH AT				BASINB	65.	12.00	10.	3.	3.	0.04
	2 COMBINED AT				COMB	153.	12.50	50.	16.	16.	0.21
	HYDROGRAPH AT				BASINA	234.	12.25	49.	16.	16.	0.17
1333.71	ROUTED TO				POND	168.	12.50	49.	15.	15.	0.17
	12.50										
	HYDROGRAPH AT				BASINB	90.	12.00	14.	5.	5.	0.04
	2 COMBINED AT				COMB	189.	12.50	62.	20.	20.	0.21
1321.65	ROUTED TO				DPOND	168.	12.75	62.	20.	20.	0.21
	12.75										

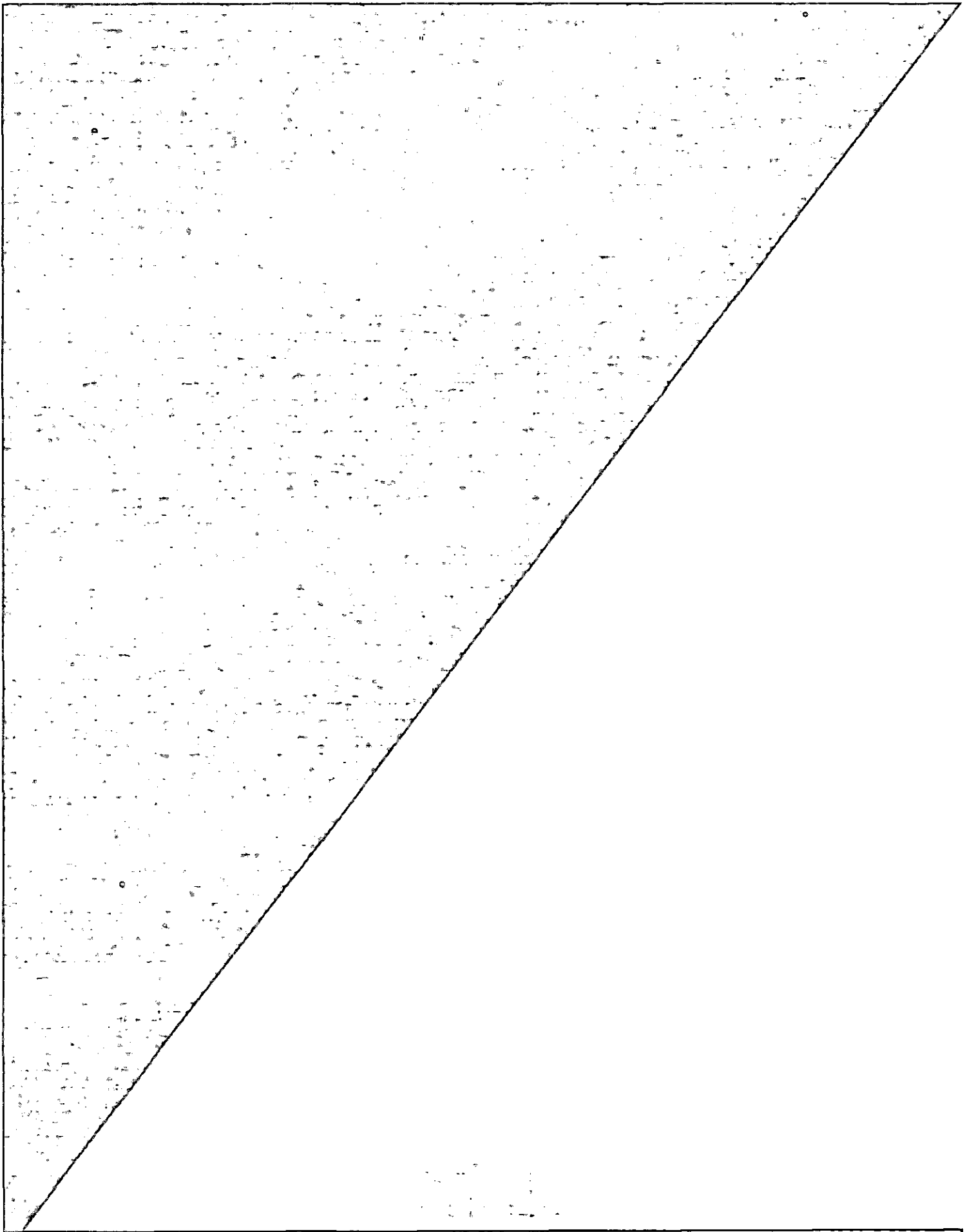
*** NORMAL END OF HEC-1 ***

5 year Storm

SCS TYPE RAINFALL DISTRIBUTION - 24 HOUR STORM
FOR A TOTAL RAINFALL OF 4.8 INCHES

HOUR	% RAINFALL	INCHES
0.00	0.00	0.00
0.50	0.52	0.02
1.00	1.05	0.05
1.50	1.60	0.08
2.00	2.20	0.11
2.50	2.84	0.14
3.00	3.49	0.17
3.50	4.13	0.20
4.00	4.80	0.23
4.50	5.53	0.27
5.00	6.32	0.30
5.50	7.14	0.34
6.00	8.00	0.38
6.50	8.90	0.43
7.00	9.85	0.47
7.50	10.85	0.52
8.00	12.00	0.58
8.50	13.28	0.64
9.00	14.70	0.71
9.50	16.30	0.78
10.00	18.10	0.87
10.50	20.40	0.98
11.00	23.50	1.13
11.50	28.30	1.36
12.00	66.30	3.18
12.50	73.50	3.53
13.00	77.20	3.71
13.50	79.90	3.84
14.00	82.00	3.94
14.50	83.79	4.02
15.00	85.28	4.09
15.50	86.68	4.16
16.00	88.00	4.22
16.50	89.12	4.28
17.00	90.04	4.32
17.50	90.93	4.36
18.00	91.81	4.41
18.50	92.67	4.45
19.00	93.52	4.49
19.50	94.37	4.53
20.00	95.20	4.57
20.50	95.97	4.61
21.00	96.65	4.64
21.50	97.26	4.67
22.00	97.84	4.70
22.50	98.40	4.72
23.00	98.95	4.75
23.50	99.48	4.78
24.00	100.00	4.80

SOURCE: 'HYDROLOGY FOR ENGINEERS AND PLANNERS', HJELMFELT AND CASSIDY
PUBLISHED 1975, TABLE 4.7 - ACCUMULATION OF RAINFALL TO 24 HOURS
U.S. SOIL CONSERVATION SERVICE



10 Year Event - Existing - Developed

SAVOY, RUGGLES & BOHM, P.A.

3 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 30 TIME INTERVAL IN MINUTES
 JXDATE 1 0 STARTING DATE
 JXTIME 0 STARTING TIME

SUBBASIN RUNOFF DATA

6 BA SUBBASIN CHARACTERISTICS
 TAREA 0.17 SUBBASIN AREA

PRECIPITATION DATA

6 PB STORM 5.52 BASIN TOTAL PRECIPITATION

6 PI INCREMENTAL PRECIPITATION PATTERN

0.01	0.01	0.00	0.01	0.00	0.00	0.01	0.00	0.00	0.01	0.01
0.00	0.00	0.00	0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.00	0.01	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.00	0.01	0.00	0.00	0.00	0.01	0.00	0.01	0.01	0.01	0.01
0.01	0.01	0.00	0.00	0.01	0.00	0.00	0.00	0.00	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.02	0.01	0.01	0.01	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.03	0.03	0.03	0.03
0.03	0.03	0.04	0.04	0.04	0.04	0.04	0.04	0.04	0.04	0.04
0.35	0.35	0.35	0.35	0.35	0.07	0.07	0.07	0.07	0.07	0.07
0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.03	0.02	0.03	0.03
0.02	0.03	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02	0.02
0.02	0.02	0.02	0.02	0.02	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.00	0.01	0.01	0.01
0.01	0.00	0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.00	0.01
0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.01	0.00	0.01	0.01
0.00	0.01	0.01	0.01	0.01	0.01	0.00	0.01	0.01	0.01	0.01

13 LS SCS LOSS RATE
 STRTL 0.41 INITIAL ABSTRACTION
 CRVNER 83.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

12 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.42 LAG

UNIT HYDROGRAPH
 27 END-OF-PERIOD ORDINATES

15.	46.	96.	147.	173.	173.	155.	129.	94.	68.
50.	38.	28.	21.	16.	12.	9.	7.	5.	4.
3.	2.	2.	1.	1.	1.	0.			

.....

14 KK

 POND

HYDROGRAPH ROUTING DATA

15 RS STORAGE ROUTING
 NSTPS 1 NUMBER OF SUBREACHES
 ITYP ELEV TYPE OF INITIAL CONDITION
 RSVRIC 1333.00 INITIAL CONDITION
 X 0.00 WORKING R AND D COEFFICIENT

16 SA AREA 5.0 7.4 9.8
 17 SE ELEVATION 1333.00 1334.00 1335.00
 18 SQ DISCHARGE 0. 14. 87. 259. 557.
 19 SE ELEVATION 1333.00 1333.50 1334.00 1334.50 1335.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	6.16	14.73
ELEVATION	1333.00	1334.00	1335.00

42 KK

 * DPOND *

HYDROGRAPH ROUTING DATA

43 RS	STORAGE ROUTING	NSTPS	1	NUMBER OF SUBREACHES						
		ITYP	ELEV	TYPE OF INITIAL CONDITION						
		RSVVIC	1319.00	INITIAL CONDITION						
		X	0.00	WORKING R AND D COEFFICIENT						
44 SA	AREA	1.9	2.1	2.3	2.5	2.7				
45 SE	ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00				
46 SQ	DISCHARGE	0.	14.	39.	71.	110.	154.	202.	255.	312.
47 SE	ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	2.00	4.20	6.60	9.20
ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.97	2.00	3.07	4.20	5.37	6.60	7.87	9.20
OUTFLOW	0.10	14.00	39.00	71.00	110.00	154.00	202.00	255.00	312.00
ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
 TIME IN HOURS, AREA IN SQUARE MILES

6-HOUR	OPERATION 24-HOUR	STATION 72-HOUR	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
	HYDROGRAPH AT			BASINA	248.	12.25	52.	16.	16.	0.17
1334.27	ROUTED TO			POND	180.	12.50	50.	16.	16.	0.17
	HYDROGRAPH AT			BASINB	80.	12.00	12.	4.	4.	0.04
	2 COMBINED AT			COMB	200.	12.50	61.	19.	19.	0.21
	HYDROGRAPH AT			BASINA	279.	12.25	59.	19.	19.	0.17
1333.86	ROUTED TO			POND	203.	12.50	58.	18.	18.	0.17
	HYDROGRAPH AT			BASINB	105.	12.00	16.	5.	5.	0.04
	2 COMBINED AT			COMB	226.	12.50	74.	24.	24.	0.21
1322.01	ROUTED TO			DPOND	203.	12.75	74.	23.	23.	0.21

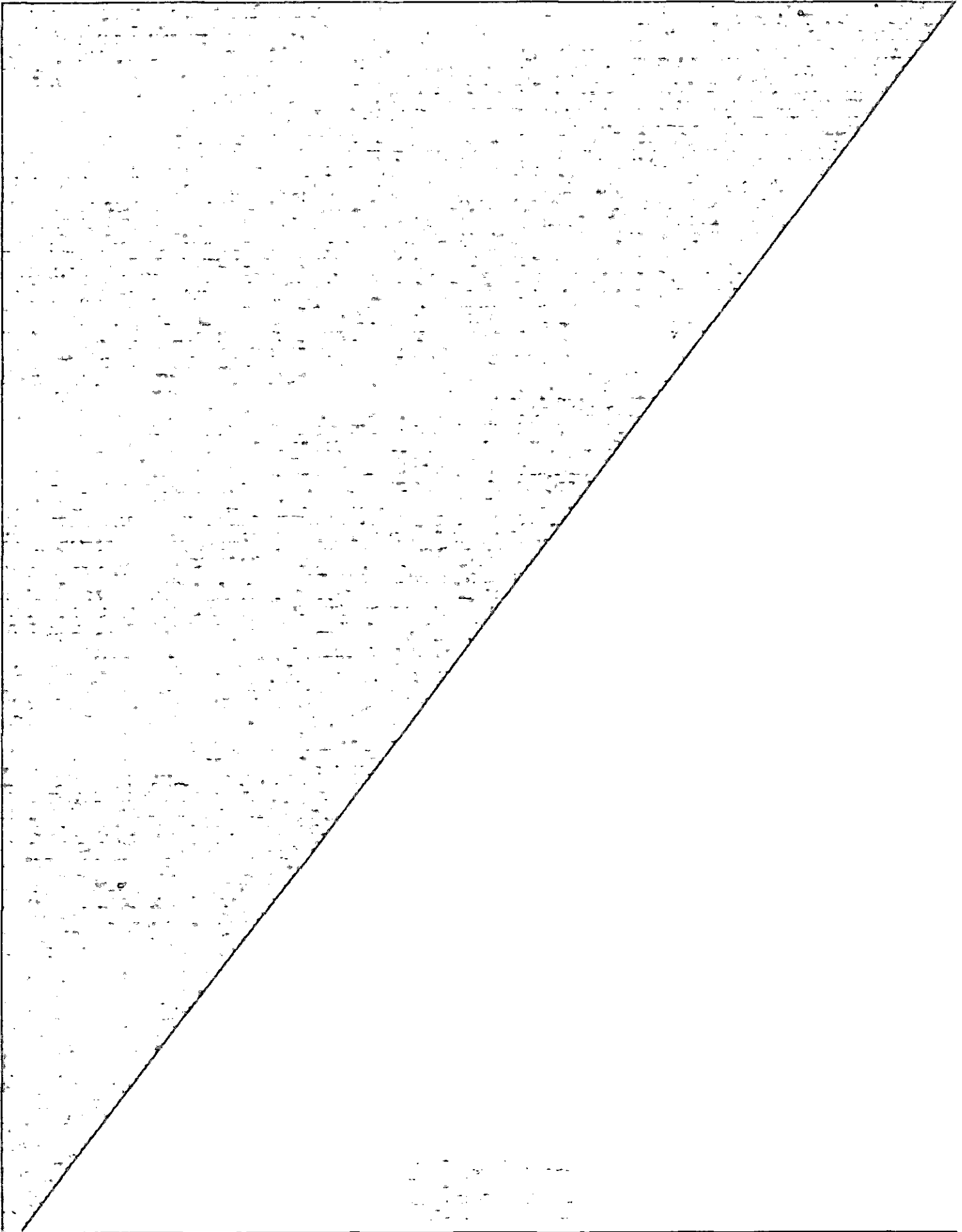
*** NORMAL END OF HEC-1 ***

SCS TYPE RAINFALL DISTRIBUTION - 24 HOUR STORM
 FOR A TOTAL RAINFALL OF 5.52 INCHES

10 year event

HOUR	% RAINFALL	INCHES
0.00	0.00	0.00
0.50	0.52	0.03
1.00	1.05	0.06
1.50	1.60	0.09
2.00	2.20	0.12
2.50	2.84	0.16
3.00	3.49	0.19
3.50	4.13	0.23
4.00	4.80	0.26
4.50	5.53	0.31
5.00	6.32	0.35
5.50	7.14	0.39
6.00	8.00	0.44
6.50	8.90	0.49
7.00	9.85	0.54
7.50	10.85	0.60
8.00	12.00	0.66
8.50	13.28	0.73
9.00	14.70	0.81
9.50	16.30	0.90
10.00	18.10	1.00
10.50	20.40	1.13
11.00	23.50	1.30
11.50	28.30	1.56
12.00	66.30	3.66
12.50	73.50	4.06
13.00	77.20	4.26
13.50	79.90	4.41
14.00	82.00	4.53
14.50	83.79	4.63
15.00	85.28	4.71
15.50	86.68	4.78
16.00	88.00	4.86
16.50	89.12	4.92
17.00	90.04	4.97
17.50	90.93	5.02
18.00	91.81	5.07
18.50	92.67	5.12
19.00	93.52	5.16
19.50	94.37	5.21
20.00	95.20	5.26
20.50	95.97	5.30
21.00	96.65	5.34
21.50	97.26	5.37
22.00	97.84	5.40
22.50	98.40	5.43
23.00	98.95	5.46
23.50	99.48	5.49
24.00	100.00	5.52

SOURCE: 'HYDROLOGY FOR ENGINEERS AND PLANNERS', HJELMFELT AND CASSIDY
 PUBLISHED 1975, TABLE 4.7 - ACCUMULATION OF RAINFALL TO 24 HOURS
 U.S. SOIL CONSERVATION SERVICE



25 year Event - Existing & Developed

SAVOY, RUGGLES & BOHM, P.A.

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*****
*****
FLOOD HYDROGRAPH PACKAGE (HEC-1)
MAY 1991
VERSION 4.0.1E
RUN DATE 05/02/2000 TIME 13:59:45
*****
*****
    
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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104
*
    
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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
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X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX
    
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::::::::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::::::::
Full Microcomputer Implementation
by
Haestad Methods, Inc.
::::::::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::::::::
    
```

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1G5, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT PAGE 1

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Tara Falls Existing Basin to Harry with existng pond modeled, 25 year, 24 hou									
2	IT	5	289								
3	IN	30									
4	IO	4									
5	KK	BASINA									
6	BA	0.167									
7	PC	0	0.03	0.07	0.10	0.14	0.18	0.23	0.27	0.31	0.36
8	PC	0.41	0.46	0.52	0.58	0.64	0.70	0.78	0.86	0.95	1.06
9	PC	1.17	1.32	1.52	1.63	4.30	4.76	5.00	5.18	5.31	5.43
10	PC	5.53	5.62	5.70	5.77	5.83	5.89	5.95	6.01	6.06	6.12
11	PC	6.17	6.22	6.26	6.30	6.34	6.38	6.41	6.45	6.48	
12	UD	0.42									
13	LS	0	83								
14	KK	POND									
15	RS	1	ELEV	1333							
16	SA	5	7.4	9.8							
17	SE	1333	1334	1335							
18	SG	0.1	14	87	258.5	557					
19	SE	1333	1333.5	1334	1334.5	1335					
20	KK	BASINB									
21	BA	0.042									
22	UD	0.15									
23	LS	0	80								
24	KK	COMB									
25	HC	2									

```

26      KK  BASINA
27      BA  0.167
28      UD  0.42
29      LS  0      88

30      KK  POND
31      RS  1      ELEV 1332.50
32      SA  4.3    5      7.4    9.8
33      SE  1332.5 1333    1334    1335
34      SQ  0.1    15     45     82     127    177    233    294
35      SE  1332.5 1332.75 1333    1333.25 1333.5 1333.75 1334    1334.25

36      KK  BASINS
37      BA  0.042
38      UD  0.15
39      LS  0      93

40      KK  COMB
41      HC  2

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HEC-1 INPUT

PAGE 2

☐

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

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42      KK  DPOND
43      RS  1      ELEV 1319
44      SA  1.9    2.1    2.3    2.5    2.7
45      SE  1319    1320    1321    1322    1323
46      SQ  0.1    14     39     71     110    154    202    255    312
47      SE  1319    1319.5 1320    1320.5 1321    1321.5 1322    1322.5 1323
48      ZZ

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HMVersion: 6.33 Data File: tf25.hcl

☐HEC1 S/N: 1343000364

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.....
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*
* MAY 1991 *
*
* VERSION 4.0.1E *
*
* RUN DATE 05/02/2000 TIME 13:59:45 *
*
*
*
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*
* U.S. ARMY CORPS OF ENGINEERS
*
* HYDROLOGIC ENGINEERING CENTER
*
* 609 SECOND STREET
*
* DAVIS, CALIFORNIA 95616
*
* (916) 756-1104
*

```

Tara Falls Existing Basin to Harry with existing pond modeled, 25 year, 24 hou

```

4 IO      OUTPUT CONTROL VARIABLES
          IPRNT      4      PRINT CONTROL
          IPLOT      0      PLOT CONTROL
          OSCAL      0.     HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN      5      MINUTES IN COMPUTATION INTERVAL
          IDATE      1      0      STARTING DATE
          ITIME      0000    STARTING TIME
          NQ        289    NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     2      0      ENDING DATE
          NDTIME     0000    ENDING TIME
          ICENT      19     CENTURY MARK

          COMPUTATION INTERVAL 0.06 HOURS
          TOTAL TIME BASE 24.00 HOURS

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

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.....
*
* 5 KK      BASINA
*
*
.....

```


42 KK

```

*****
*   DPOND   *
*****
  
```

HYDROGRAPH ROUTING DATA

43 RS	STORAGE ROUTING	NSTPS	1	NUMBER OF SUBREACHES						
	ITYP	ELEV	ELEV	TYPE OF INITIAL CONDITION						
	RSVVIC	1319.00		INITIAL CONDITION						
	X	0.00		WORKING R AND D COEFFICIENT						
44 SA	AREA	1.9	2.1	2.3	2.5	2.7				
45 SE	ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00				
46 SQ	DISCHARGE	0.	14.	39.	71.	110.	154.	202.	255.	312.
47 SE	ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	2.00	4.20	6.60	9.20					
ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00					

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.97	2.00	3.07	4.20	5.37	6.60	7.87	9.20		
OUTFLOW	0.10	14.00	39.00	71.00	110.00	154.00	202.00	255.00	312.00		
ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00		

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

6-HOUR	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
	24-HOUR	72-HOUR								
	HYDROGRAPH AT			BASINA	308.	12.25	65.	20.	20.	0.17
1334.43	ROUTED TO			POND	234.	12.50	62.	19.	19.	0.17
	12.50									
	HYDROGRAPH AT			BASINB	101.	12.00	15.	5.	5.	0.04
	2 COMBINED AT			COMB	258.	12.50	77.	24.	24.	0.21
	HYDROGRAPH AT			BASINA	339.	12.25	71.	23.	23.	0.17
1334.06	ROUTED TO			POND	248.	12.50	71.	22.	22.	0.17
	12.50									
	HYDROGRAPH AT			BASINB	125.	12.00	19.	6.	6.	0.04
	2 COMBINED AT			COMB	276.	12.50	90.	29.	29.	0.21
1322.45	ROUTED TO			DPOND	250.	12.67	90.	28.	28.	0.21
	12.67									

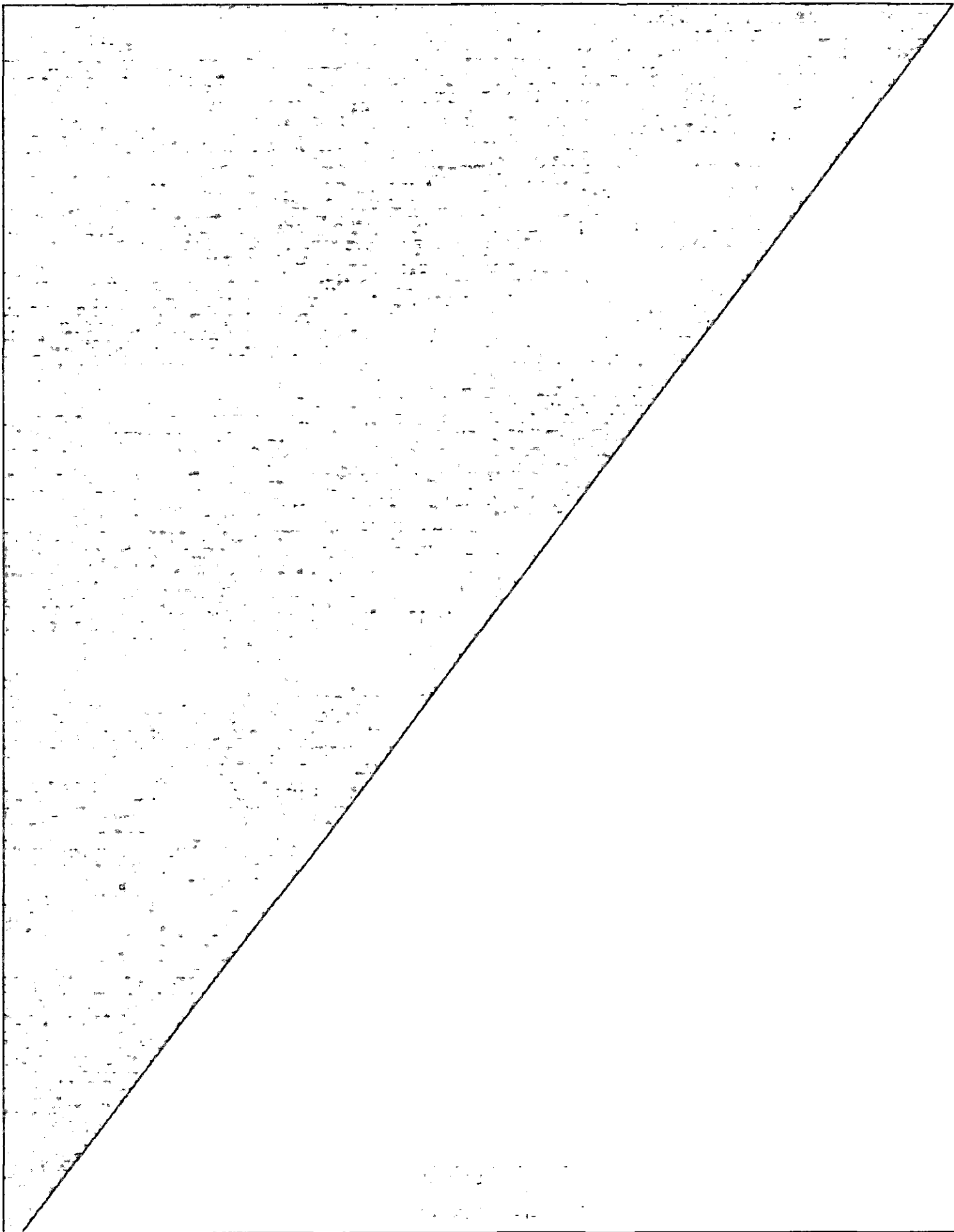
*** NORMAL END OF HEC-1 ***

SCS TYPE RAINFALL DISTRIBUTION - 24 HOUR STORM
 FOR A TOTAL RAINFALL OF 6.48 INCHES

25 year event

HOUR	% RAINFALL	INCHES
0.00	0.00	0.00
0.50	0.52	0.03
1.00	1.05	0.07
1.50	1.60	0.10
2.00	2.20	0.14
2.50	2.84	0.18
3.00	3.49	0.23
3.50	4.13	0.27
4.00	4.80	0.31
4.50	5.53	0.36
5.00	6.32	0.41
5.50	7.14	0.46
6.00	8.00	0.52
6.50	8.90	0.58
7.00	9.85	0.64
7.50	10.85	0.70
8.00	12.00	0.78
8.50	13.28	0.86
9.00	14.70	0.95
9.50	16.30	1.06
10.00	18.10	1.17
10.50	20.40	1.32
11.00	23.50	1.52
11.50	28.30	1.83
12.00	66.30	4.30
12.50	73.50	4.76
13.00	77.20	5.00
13.50	79.90	5.18
14.00	82.00	5.31
14.50	83.79	5.43
15.00	85.28	5.53
15.50	86.68	5.62
16.00	88.00	5.70
16.50	89.12	5.77
17.00	90.04	5.83
17.50	90.93	5.89
18.00	91.81	5.95
18.50	92.67	6.01
19.00	93.52	6.06
19.50	94.37	6.12
20.00	95.20	6.17
20.50	95.97	6.22
21.00	96.65	6.26
21.50	97.26	6.30
22.00	97.84	6.34
22.50	98.40	6.38
23.00	98.95	6.41
23.50	99.48	6.45
24.00	100.00	6.48

SOURCE: 'HYDROLOGY FOR ENGINEERS AND PLANNERS', HJELMFELT AND CASSIDY
 PUBLISHED 1975, TABLE 4.7 - ACCUMULATION OF RAINFALL TO 24 HOURS
 U.S. SOIL CONSERVATION SERVICE



50 year Event - Existing & Developed

SAVOY, RUGGLES & BOHM, P.A.

```

.....
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*
* RUN DATE 05/02/2000 TIME 14:18:32 *
*
*
.....

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*
* U.S. ARMY CORPS OF ENGINEERS
*
* HYDROLOGIC ENGINEERING CENTER
*
* 609 SECOND STREET
*
* DAVIS, CALIFORNIA 95616
*
* (916) 756-1104
*

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X X XXXXXXX XXXXX X
X X X X X XX
X X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X X
X X X X X X
X X XXXXXXX XXXXX XXX

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.....
Full Microcomputer Implementation
by
Haestad Methods, Inc.
.....

```

37 Brookside Road * Waterbury, Connecticut 06708 * (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.
 THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.
 THE DEFINITION OF -AMSCK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Tara Falls Existing Basin to Harry with existng pond modeled, 50 year, 24 hou									
2	IT	5 289									
3	IN	30									
4	IO	4									
5	KK	BASINA									
6	BA	0.167									
7	PC	0	0.04	0.08	0.12	0.16	0.21	0.26	0.31	0.36	0.41
8	PC	0.47	0.53	0.60	0.66	0.73	0.81	0.89	0.99	1.09	1.21
9	PC	1.35	1.52	1.75	2.11	4.93	5.47	5.74	5.94	6.10	6.23
10	PC	6.34	6.45	6.55	6.63	6.70	6.77	6.83	6.89	6.96	7.02
11	PC	7.08	7.14	7.19	7.24	7.28	7.32	7.36	7.40	7.44	
12	UD	0.42									
13	LS	0	83								
14	KK	POND									
15	RS	1	ELEV 1333								
16	SA	5	7.4 9.6								
17	SE	1333	1334 1335								
18	SO	0.1	14	87	258.5	557					
19	SE	1333	1333.5	1334	1334.5	1335					
20	KK	BASINB									
21	BA	0.042									
22	UD	0.15									
23	LS	0	80								
24	KK	COMB									
25	HC	2									

```

26      KK  BASINA
27      BA  0.167
28      UD  0.42
29      LS   0      88

30      KK  POND
31      RS   1      ELEV 1332.50
32      SA  4.3      5      7.4      9.8
33      SE 1332.5    1333    1334    1335
34      SQ  0.1      15      45      82      127      177      233      294
35      SE 1332.5    1332.75  1333  1333.25  1333.5  1333.75  1334  1334.25

36      KK  BASINB
37      BA  0.042
38      UD  0.15
39      LS   0      93

40      KK  COMB
41      HC   2

```

HEC-1 INPUT

PAGE 2

□□

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

42      KK  DPOND
43      RS   1      ELEV 1319
44      SA  1.9      2.1      2.3      2.5      2.7
45      SE 1319    1320    1321    1322    1323
46      SQ  0.1      14      39      71      110      154      202      255      312
47      SE 1319    1319.5  1320    1320.5  1321    1321.5  1322    1322.5  1323
48      ZZ

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□□HEC1 S/N: 1343000364

HMVersion: 6.33 Data File: tf50.hci

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*****
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* RUN DATE 05/02/2000 TIME 14:18:32 *
*****
*****

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```

* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 756-1104

```

Tara Falls Existing Basin to Harry with existng pond modeled, 50 year, 24 hou

```

4 IO OUTPUT CONTROL VARIABLES
      IPRNT 4 PRINT CONTROL
      IPLOT 0 PLOT CONTROL
      QSCAL 0 HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
      NMIN 5 MINUTES IN COMPUTATION INTERVAL
      IDATE 1 0 STARTING DATE
      ITIME 0000 STARTING TIME
      NQ 289 NUMBER OF HYDROGRAPH ORDINATES
      NDATE 2 0 ENDING DATE
      NDTIME 0000 ENDING TIME
      ICENT 19 CENTURY MARK

      COMPUTATION INTERVAL 0.08 HOURS
      TOTAL TIME BASE 24.00 HOURS

```

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ENGLISH UNITS:
DRAINAGE AREA SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW CUBIC FEET PER SECOND
STORAGE VOLUME ACRE-FEET
SURFACE AREA ACRES
TEMPERATURE DEGREES FAHRENHEIT

```

```

*****
* 5 KK BASINA *
*****

```


42 KK

```

*****
*           *
*   DPOND   *
*           *
*****
  
```

HYDROGRAPH ROUTING DATA

43 RS	STORAGE ROUTING	NSTPS	1	NUMBER OF SUBREACHES						
	ITYP	ELEV		TYPE OF INITIAL CONDITION						
	RSVVIC	1319.00		INITIAL CONDITION						
	X	0.00	WORKING R AND D	COEFFICIENT						
44 SA	AREA	1.9	2.1	2.3	2.5	2.7				
45 SE	ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00				
46 SQ	DISCHARGE	0.	14.	39.	71.	110.	154.	202.	255.	312.
47 SE	ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	2.00	4.20	6.60	9.20
ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.97	2.00	3.07	4.20	5.37	6.60	7.87	9.20
OUTFLOW	0.10	14.00	39.00	71.00	110.00	154.00	202.00	255.00	312.00
ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

RUNOFF SUMMARY

FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

6-HOUR	OPERATION 24-HOUR	STATION 72-HOUR	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME OF MAX STAGE
	HYDROGRAPH AT			BASINA	366.	12.25	77.	24.	24.	0.17
1334.56	ROUTED TO			POND	292.	12.42	75.	23.	23.	0.17
	HYDROGRAPH AT			BASINB	120.	12.00	18.	6.	6.	0.04
	2 COMBINED AT			COMB	325.	12.42	93.	29.	29.	0.21
	HYDROGRAPH AT			BASINA	397.	12.25	84.	27.	27.	0.17
1334.24	ROUTED TO			POND	292.	12.50	84.	27.	27.	0.17
	HYDROGRAPH AT			BASINB	144.	12.00	23.	7.	7.	0.04
	2 COMBINED AT			COMB	325.	12.50	106.	34.	34.	0.21
1322.86	ROUTED TO			DPOND	296.	12.67	105.	34.	34.	0.21

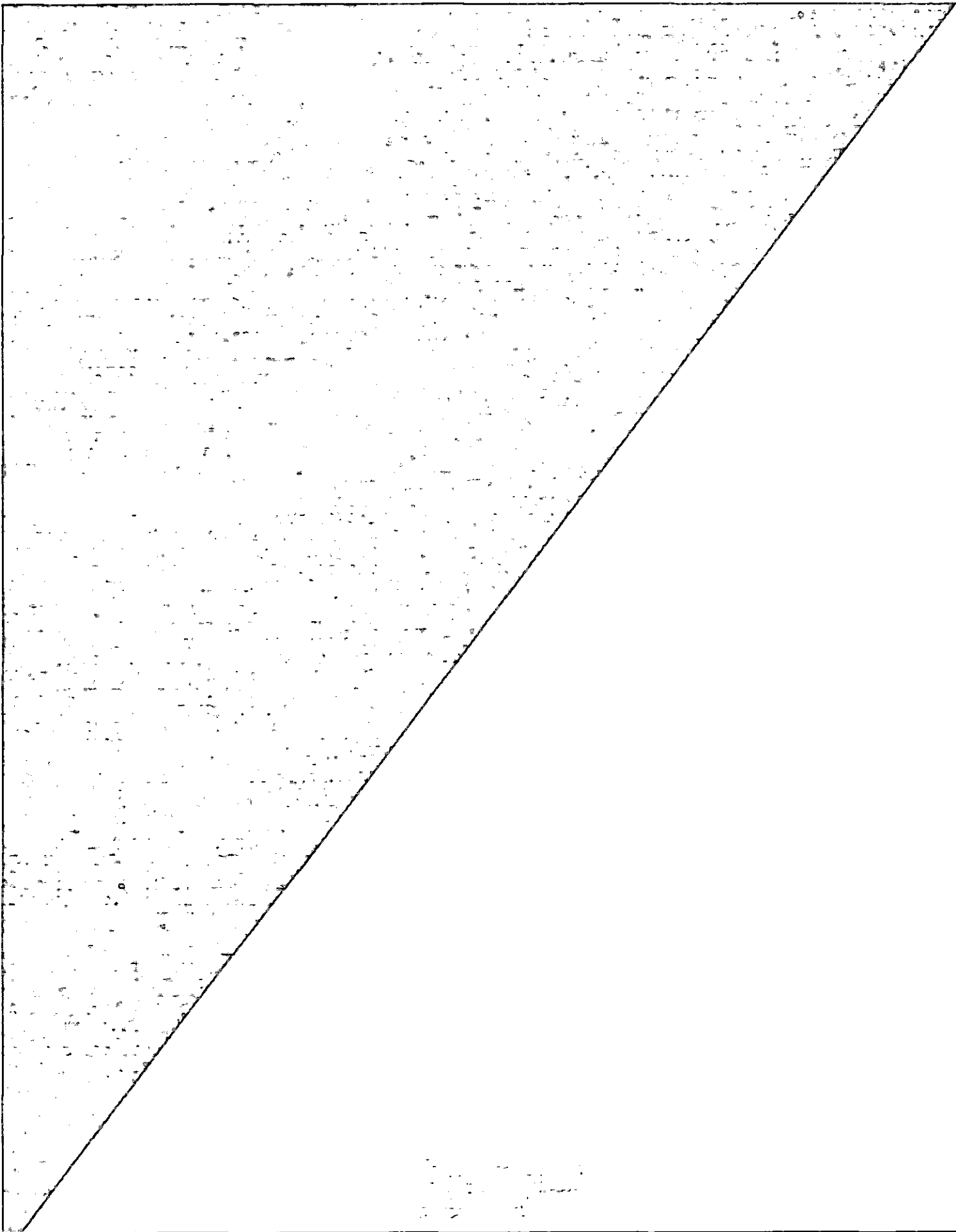
*** NORMAL END OF HEC-1 ***

50 year event

SCS TYPE RAINFALL DISTRIBUTION - 24 HOUR STORM
FOR A TOTAL RAINFALL OF 7.44 INCHES

HOUR	% RAINFALL	INCHES
0.00	0.00	0.00
0.50	0.52	0.04
1.00	1.05	0.08
1.50	1.60	0.12
2.00	2.20	0.16
2.50	2.84	0.21
3.00	3.49	0.26
3.50	4.13	0.31
4.00	4.80	0.36
4.50	5.53	0.41
5.00	6.32	0.47
5.50	7.14	0.53
6.00	8.00	0.60
6.50	8.90	0.66
7.00	9.85	0.73
7.50	10.85	0.81
8.00	12.00	0.89
8.50	13.28	0.99
9.00	14.70	1.09
9.50	16.30	1.21
10.00	18.10	1.35
10.50	20.40	1.52
11.00	23.50	1.75
11.50	28.30	2.11
12.00	66.30	4.93
12.50	73.50	5.47
13.00	77.20	5.74
13.50	79.90	5.94
14.00	82.00	6.10
14.50	83.79	6.23
15.00	85.28	6.34
15.50	86.68	6.45
16.00	88.00	6.55
16.50	89.12	6.63
17.00	90.04	6.70
17.50	90.93	6.77
18.00	91.81	6.83
18.50	92.67	6.89
19.00	93.52	6.96
19.50	94.37	7.02
20.00	95.20	7.08
20.50	95.97	7.14
21.00	96.65	7.19
21.50	97.26	7.24
22.00	97.84	7.28
22.50	98.40	7.32
23.00	98.95	7.36
23.50	99.48	7.40
24.00	100.00	7.44

SOURCE: 'HYDROLOGY FOR ENGINEERS AND PLANNERS', HJELMFELT AND CASSIDY
PUBLISHED 1975, TABLE 4.7 - ACCUMULATION OF RAINFALL TO 24 HOURS
U.S. SOIL CONSERVATION SERVICE



100 year Event - Existing & Developed

SAVOY, RUGGLES & BOHM, P.A.


```

29      LS      0      88
30      KK      POND
31      RS      1      ELEV 1332.50
32      SA      4.3    5      7.4    9.8
33      SE 1332.5 1333    1334    1335
34      SQ      0.1    15     45     82     127    177    233    294
35      SE 1332.5 1332.75 1333 1333.25 1333.5 1333.75 1334 1334.25

36      KK      BASIN
37      EA      0.042
38      UD      0.15
39      LS      0      93

40      KK      COMB
41      HC      2

```

HEC-1 INPUT

PAGE 2

□

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

42      KK      DPOND
43      RS      1      ELEV 1319
44      SA      1.9    2.1    2.3    2.5    2.7
45      SE 1319 1320 1321 1322 1323
46      SQ      0.1    14     39     71     110    154    202    255    312
47      SE 1319 1319.5 1320 1320.5 1321 1321.5 1322 1322.5 1323
48      ZZ

```

1343000364

HMVersion: 6.33 Data File: tf100.hcl

HEC1 S/N:

```

*****
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* ENGINEERS *
* MAY 1991 *
* CENTER *
* VERSION 4.0.1E *
*
* 95616 *
* RUN DATE 05/02/2000 TIME 14:46:42 *
*
*****
*****

```

```

* U.S. ARMY CORPS OF
* HYDROLOGIC ENGINEERING
* 609 SECOND STREET
* DAVIS, CALIFORNIA
* (916) 756-1104

```

Tara Falls Existing Basin to Harry with existng pond modeled, 100 year, 24 ho

```

4 IO      OUTPUT CONTROL VARIABLES
          IPRNT      4      PRINT CONTROL
          IPLOT      0      PLOT CONTROL
          OSCAL      0.    HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA
          NMIN      5      MINUTES IN COMPUTATION INTERVAL
          IDATE      1      0      STARTING DATE
          ITIME      0000   STARTING TIME
          NQ         289   NUMBER OF HYDROGRAPH ORDINATES
          NDDATE     2      0      ENDING DATE
          NDTIME     0000   ENDING TIME
          ICENT      19     CENTURY MARK

          COMPUTATION INTERVAL 0.08 HOURS
          TOTAL TIME BASE 24.00 HOURS

```

```

ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH INCHES
LENGTH, ELEVATION FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME    ACRE-FEET
SURFACE AREA      ACRES
TEMPERATURE        DEGREES FAHRENHEIT

```

5 KK

BASINA

3 IN

```

TIME DATA FOR INPUT TIME SERIES
JXMIN      30      TIME INTERVAL IN MINUTES
JXDATE     1      0      STARTING DATE
JXTIME     0      STARTING TIME

```

SUBBASIN RUNOFF DATA

STORAGE	0.00	2.00	4.20	6.60	9.20
ELEVATION	1319.00	1320.00	1321.00	1322.00	1323.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.97	2.00	3.07	4.20	5.37	6.60	7.87	9.20
OUTFLOW	0.10	14.00	39.00	71.00	110.00	154.00	202.00	255.00	312.00
ELEVATION	1319.00	1319.50	1320.00	1320.50	1321.00	1321.50	1322.00	1322.50	1323.00

WARNING --- ROUTED OUTFLOW (313.) IS GREATER THAN MAXIMUM OUTFLOW (312.) IN STORAGE-OUTFLOW TABLE

RUNOFF SUMMARY
FLOW IN CUBIC FEET PER SECOND
TIME IN HOURS, AREA IN SQUARE MILES

OF STAGE	OPERATION	STATION	PEAK FLOW	TIME OF PEAK	AVERAGE FLOW FOR MAXIMUM PERIOD			BASIN AREA	MAXIMUM STAGE	TIME MAX
					6-HOUR	24-HOUR	72-HOUR			
0.17	HYDROGRAPH AT			BASINA	388.	12.25	82.	26.	26.	
1334.60	ROUTED TO			POND	317.	12.42	80.	25.	25.	0.17
0.04	HYDROGRAPH AT			BASINB	128.	12.00	20.	6.	6.	
0.21	2 COMBINED AT			COMB	351.	12.42	99.	31.	31.	
0.17	HYDROGRAPH AT			BASINA	420.	12.25	89.	28.	28.	
1334.30	ROUTED TO			POND	307.	12.50	88.	28.	28.	0.17
0.04	HYDROGRAPH AT			BASINB	151.	12.00	24.	8.	8.	
0.21	2 COMBINED AT			COMB	341.	12.50	112.	36.	36.	
1323.01	ROUTED TO			DPOND	313.	12.67	111.	36.	36.	0.21

*** NORMAL END OF HEC-1 ***