

PROJECT: Woodland Estates Second & Third  
 PROJECT #: \_\_\_\_\_ DATE: 01/12/82

TO: Carl Gipson  
 Engineering Department  
 455 N. Main, 7th Floor  
 Wichita, KS 67202

MID-KANSAS ENGINEERING  
 CONSULTANTS, P.A.  
 3500 N. Rock Road, #800  
 Wichita, KS 67226

Mr. Frank Nelson has met with you regarding our storm sewer plans for the referenced additions. We have asked for a meeting with Mr. Bill Yung to understand or obtain plans for how Mr. Coulter will convey storm run off through his "platted" lots to tie with the proposed storm water sewer.

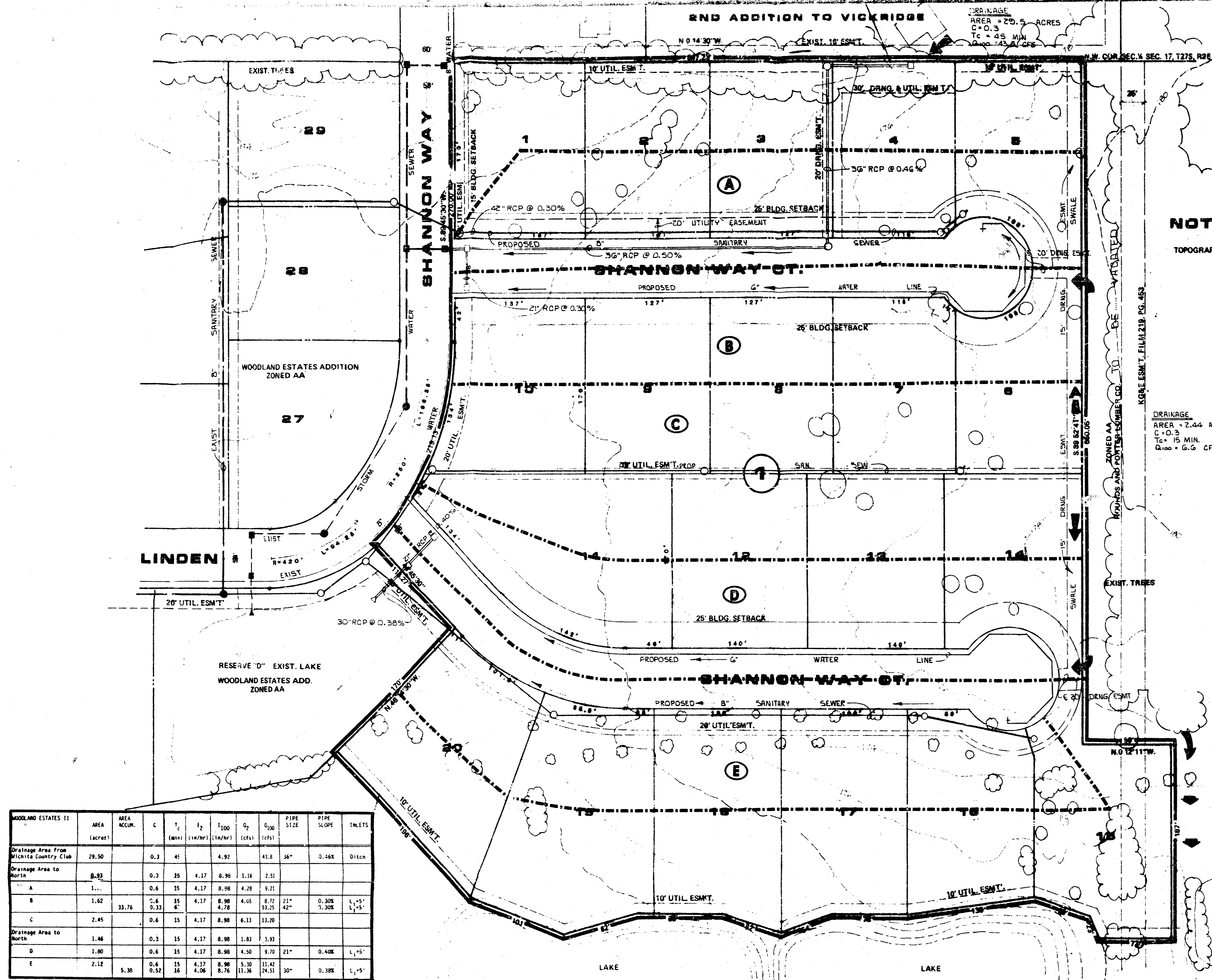
If the plans for Mr. Coulter's lots cannot be solved within the next two weeks we will call a meeting with you to discuss our proposed solution, so as to permit the construction plans for Woodland Estates II to be bid.

MID-KANSAS ENGINEERING CONSULTANTS, P.A.

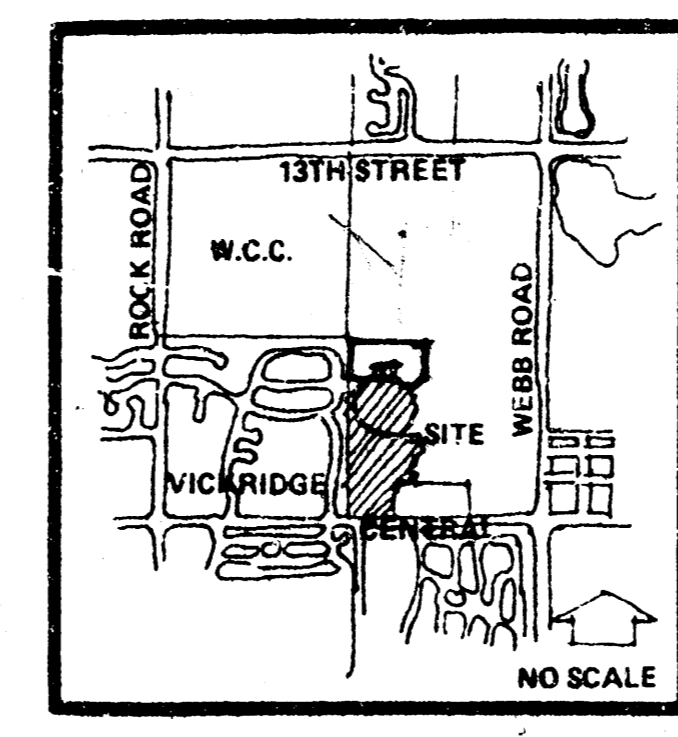
Kenneth H. Bengtson, P.E.

KHB/dm

cc: Vicki Huang  
 Bill Yung  
 Jack Ritchie



NOTES:  
 TOPOGRAPHY IS FROM AERIAL PHOTOGRAPH TAKEN JUNE 1985



LOCATION MAP

WOODLAND ESTATES II	AREA (ACRES)	AREA ACCUR.	C	T <sub>1</sub>	T <sub>2</sub>	T <sub>3</sub>	T <sub>4</sub>	T <sub>5</sub>	T <sub>6</sub>	T <sub>7</sub>	T <sub>8</sub>	T <sub>9</sub>	T <sub>10</sub>	PIPE SIZE	PIPE SLOPE	INLETS
Drainage Area From Wichita Country Club	29.50		0.3	41		4.92				41.8	34"	0.168	0.168			
Drainage Area to North	8.93		0.3	25	4.17	8.96	1.16	2.51								
A	1.11		0.8	35	4.17	8.96	4.28	9.22								
B	1.62	33.76	0.4	35	4.17	8.96	4.05	8.72	21"	0.305	1.5"					
C	2.45		0.6	35	4.17	8.96	6.33	13.20								
Drainage Area to South	1.46		0.3	35	4.17	8.96	1.83	3.93								
D	1.80		0.4	35	4.17	8.96	4.50	9.70	21"	0.408	1.5"					
E	2.12	5.30	0.6	35	4.17	8.96	5.30	11.42								

# WOODLAND ESTATES 2ND

OWNER: RITCHIE ASSOCIATES 8100 E. 22ND STREET NORTH BLDG. 300 WICHITA KANSAS 67226

7" CONTOUR INTERVALS

MID-KANSAS ENGINEERING CONSULTANTS P.A.  
 3500 N. ROCK ROAD  
 WICHITA, KANSAS 67226  
 682-4541

MID-KANSAS ENGINEERING CONSULTANTS P.A.  
 CALCULATIONS & SKETCHES  
 User: Lakes point  
 Review: Lakes point, Woodland Estates 2nd

North Lake	Area (Acres)	Storage (Ac-ft)	Elev.	Storage
152	0.44			
153	0.57	0.06		
154	0.57	0.06		
155	0.52	0.06		
156	0.69	0.08	168	0
157	0.75	0.08	168.5	0.25
158	0.85	0.08	169	0.5
159	1.00	0.08	169.5	1.0
160	1.10	0.08	170	2.0
161	1.10	0.08	170	2.0

South Lake	Area (Acres)	Storage (Ac-ft)	Elev.	Storage
160	0.61			
161	0.70	0.06		
162	0.75	0.06		
163	0.85	0.06	165	0
164	0.87	0.06	165.5	0.07
165	0.87	0.06	166	0.14
166	1.02	0.06	166.5	0.28
167	1.09	0.06	167	0.56

Drainage Area to Lake = 62.5 + 62.2 = 124.7 acres  
 C = 0.76 CN = 87 L = 3300' T = 1.92  
 S = 1000 - 10 = 1.49 L = 3300' (1.49) = 0.58 hrs  
 4-Max. lon. F<sub>max</sub> = 0.85 L<sub>max</sub> = 35 T<sub>max</sub> = 0.85(35)(1.92) = 5.82 hrs  
 16-Max. Q<sub>max</sub> = (0.85)(124.7) = 106.0 cfs (Q = (0.85)(124.7)(1.92) = 202.4)

MID-KANSAS ENGINEERING CONSULTANTS P.A.  
 CALCULATIONS & SKETCHES  
 User: Lakes point  
 Review: Lakes point, Woodland Estates 2nd

Try 20" Circular pipe

H	C	Q
0.5	95.2	
1.0	209.8	
1.5	426.2	
2.0	743.1	
2.5	1060.7	

For South Lake assume constant pipe

Try 8" x 25' x 1.90' x 90' Y = 165' H = 5'

C = (0.85)(0.813)(8) = 5.58 (1.49)(1.92) = 2.82

Q = 5.58(8) = 44.64 cfs

H	C	Q
0.5	61.1	89.7
1.0	137.2	209.8
1.5	209.8	426.2
2.0	281.9	743.1

Check for a constant pipe with H = 5'

For V.I. sp. area for a pipe 8" x 25' x 1.90' H = 5'

Try 5" x 25' x 1.90' H = 5'

COMPUTER PROGRAM FOR PROJECT INFORMATION - HYDRAULIC USER NOTES

THE USER MANUAL FOR THIS PROGRAM IS THE MAY 1980 EDITION OF "7-20" CHANGES FROM THE 2/14/74 VERSION INCLUDE:

SEVEN ROUTING - THE MODIFIED ATTEN PARTIAL PROCEDURE USES THE CONVEY METHOD. INPUT DATA PREPARED FOR PROGRESS FROM ROUTING USING CONVEY ROUTING DIFFERENTIALS WILL NOT RUN ON THIS VERSION.

THE PREPARED TIME OF DATA ENTRY IS CROSS SECTION DATA REPRESENTATIVE OF A REACH. IT IS RECOMMENDED THAT THE SECTION CROSS SECTION DISCHARGE AREA PLOTS BE VIEWED UNDER NEW CROSS SECTION DATA IS ENTERED. THE PLOTS SHOULD BE CHECKED FOR REASONABLENESS AND ADEQUACY OF INPUT DATA FOR THE COMPUTATION OF "M" VALUES USED IN THE ROUTING PROCEDURE.

ROUTING FOR RETENTION OR AMPLIFYING REACH LENGTHS AND COEFFICIENTS (K, B) ARE AVAILABLE IN THE USERS MANUAL. SUMMARY TABLE 2 DISPLAYS REACH ROUTING RESULTS AND ROUTING PARAMETERS FOR COMPARISON AND CHECKING.

HYDROGRAPH REDUCTION - THE PROCEDURE TO CALCULATE THE INTERNAL TIME, PEAK TIME AND PEAK TIME OF THE UNIT HYDROGRAPH HAVE BEEN IMPROVED. PEAK DISCHARGES AND TIMES NOW DIFFER FROM THE PREVIOUS VERSION. OUTPUT HYDROGRAPHS ARE STILL INTERPOLATED, PRINTED, AND RATED AT THE USER SELECTED MAIN TIME INCREMENT.

INTERMEDIATE PEAKS - METHOD ADDED TO PROVIDE DISCHARGES AT INTERMEDIATE POINTS WITHIN REACHES WITHOUT ROUTING.

OTHER - THIS VERSION CONTAINS SOME ADDITIONS TO THE INPUT AND AMPLIFIED NOTIFICATIONS TO THE OUTPUT. USER OPTIONS HAVE BEEN MODIFIED AND ADDED TO THE JOB RECORD, PRINTABLES ADDED. ERROR AND WARNING MESSAGES EXPANDED, AND THE SUMMARY TABLES COMPLETELY REVISED. THE HIGHLIGHT OPTION IS NOT OPERATIONAL AT THIS TIME.

PROGRAM QUESTIONS OR PROBLEMS SHOULD BE DIRECTED TO HYDRAULIC ENGINEERS AT THE 905 NATIONAL TECHNICAL CENTER:

CHESTER, PA (NORTHEAST) - 215-489-2923 FORT WORTH, TX (SOUTH) - 334-5242 (FTS)  
 LINCOLN, NE (WEST) - 541-8316 (FTE), PORTLAND, OR (WEST) - 422-4499 (FTE)  
 OR HYDRAULIC UNIT, ENGINEERING DIVISION, LAMAR, MO - 435-2882 (FTE).

PROGRAM CHANGES SINCE MAY 1982:

12/7/82 - CORRECT PEAK GATE FACTOR FOR USER ENTERED 0.50  
 CORRECT REACH ROUTING PEAK TIME, THE PRINTED WITH PLUMPTON OPTION  
 SIZES - CORRECT COMPUTATION FOR  
 1. DIVISION OF DISCHARGE IN CURVE OPERATION  
 2. HYDROGRAPH VOLUME SPLIT BETWEEN BASEFLOW AND ABOVE BASEFLOW  
 3. CROSS SECTION DATA PLUETING POSITION  
 4. INTERMEDIATE PEAK AREA THRU AREA IS LARGER THAN THRU AREA  
 5. STORAGE RATED REACH TRAVEL TIME FOR MULTIPLE HYDROGRAPH  
 6. SECTION "PEAK-TIME" FILE FROM SUMMARY TABLE NO DATA  
 7. BASEFLOW ENTERED WITH REACH

STRUCTURE DATA, STRUCTURE NO. 1

ELEVATION	DISCHARGE	STORAGE
150.00	0.00	0.00
155.00	52.30	0.00
159.00	239.40	0.00
163.00	424.00	0.00
167.00	751.30	1.92

STRUCTURE DATA, STRUCTURE NO. 2

ELEVATION	DISCHARGE	STORAGE
155.00	0.00	0.00
155.00	52.30	0.00
156.00	239.40	1.97
156.00	424.00	1.92
157.00	751.30	2.27

STANDARD CONTROL OPERATION RANOFF STRUCTURE 1  
 OUTPUT HYDROGRAPH = 5  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 1108 87 0000 2500

STANDARD CONTROL OPERATION RANOFF STRUCTURE 2  
 INPUT HYDROGRAPH = 6  
 OUTPUT HYDROGRAPH = 6  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 100 0000 0000 0000

EXECUTIVE CONTROL OPERATION RANOFF STRUCTURE 1  
 PEAK TIME(S) PEAK DISCHARGE(CFS) PEAK ELEVATION(Feet)

TIME	DISCHARGE	ELEVATION
12.00	466.11	166.11
15.45	11.12	166.11
17.65	11.11	166.11
19.45	11.11	166.11
23.05	8.89	166.11
6.04		

STANDARD CONTROL OPERATION RANOFF STRUCTURE 1  
 INPUT HYDROGRAPH = 5  
 OUTPUT HYDROGRAPH = 5  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 100 0000 0000 0000

STANDARD CONTROL OPERATION RANOFF STRUCTURE 2  
 INPUT HYDROGRAPH = 6  
 OUTPUT HYDROGRAPH = 6  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 100 0000 0000 0000

EXECUTIVE CONTROL OPERATION RANOFF STRUCTURE 2  
 PEAK TIME(S) PEAK DISCHARGE(CFS) PEAK ELEVATION(Feet)

TIME	DISCHARGE	ELEVATION
12.00	466.11	166.11
15.45	11.12	166.11
17.65	11.11	166.11
19.45	11.11	166.11
23.05	8.89	166.11
6.04		

STANDARD CONTROL OPERATION RANOFF STRUCTURE 1  
 INPUT HYDROGRAPH = 5  
 OUTPUT HYDROGRAPH = 5  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 100 0000 0000 0000

STANDARD CONTROL OPERATION RANOFF STRUCTURE 2  
 INPUT HYDROGRAPH = 6  
 OUTPUT HYDROGRAPH = 6  
 OUTPUT OPTIONS IN EFFECT PEAK AND ELEV. VAL. SUM DATA FIELD VALUES = 100 0000 0000 0000

