

RESOLUTION NO. _____

735031

RESOLUTION

RESOLUTION OF FINDINGS OF ADVISABILITY AND RESOLUTION AUTHORIZING IMPROVING OF **WATER DISTRIBUTION SYSTEM NUMBER 448-89622, (EAST OF 119TH STREET WEST, SOUTH OF KELLOGG)** IN THE CITY OF WICHITA, KANSAS, PURSUANT TO FINDINGS OF ADVISABILITY MADE BY THE GOVERNING BODY OF THE CITY OF WICHITA, KANSAS.

BE IT RESOLVED BY THE GOVERNING BODY OF THE CITY OF WICHITA, KANSAS, THAT THE FOLLOWING FINDINGS AS TO THE ADVISABILITY OF IMPROVING **WATER DISTRIBUTION SYSTEM NUMBER 448-89622, (EAST OF 119TH STREET WEST, SOUTH OF KELLOGG)** IN THE CITY OF WICHITA, KANSAS, ARE HEREBY MADE TO-WIT:

SECTION 1. That it is necessary and in the public interest to improve **Water Distribution System Number 448-89622, (east of 119th Street West, south of Kellogg)**.

Said pavement shall be constructed of the material in accordance with plans and specifications provided by the City Engineer.

SECTION 2. That the cost of said improvements provided for in Section 1 hereof is estimated to be **Seventy-Four Thousand Dollars (\$74,000)**, with 100 percent payable by the improvement district. Said estimated cost as above set forth is hereby increased at the pro-rata rate of 1 percent per month from and after **October 1, 2001**, exclusive of the costs of temporary financing.

SECTION 3. That all costs of said improvements attributable to the improvement district, when ascertained, shall be assessed against the land lying within the improvement district described as follows:

MEL HAMBELTON ADDITION

Lots 1 through 6, Block A

SECTION 4. That the method of apportioning all costs of said improvements attributable to the improvement district to the owners of land liable for assessment therefor shall be on a **fractional** basis.

Lot 1, Block A, MEL HAMBELTON ADDITION, shall pay 130/1,000 of the total project costs, Lots 2 through 5, Block A, MEL HAMBELTON ADDITION shall each pay 75/1,000 of the total project costs, and 570/1,000 of the total project costs.

Except when driveways are requested to serve a particular tract, lot, or parcel and shall be in addition to the assessment for other improvements. Where the ownership of a single lot is or may be divided into two or more parcels, the assessment to the lot so divided shall be assessed to each ownership or parcel on a square foot basis.

SECTION 5. That payment of said assessments may indefinitely be deferred as against those property owners eligible for such deferral available through the Special Assessment Deferral Program.

SECTION 6. That the City Engineer shall prepare plans and specifications for said improvement and a preliminary estimate of cost therefor, which plans, specifications, and a preliminary estimate of cost shall be presented to this Body for its approval.

SECTION 7. Whereas, the Governing Body of the City, upon examination thereof, considered, found and determined the Petition to be sufficient, having been signed by the owners of record, whether resident or not, of more than Fifty Percent (50%) of the property liable for assessment for the costs of the improvement requested thereby; the advisability of the improvements set forth above is hereby established as authorized by K.S.A. 1980 Supp. 12-6a01 et seq. as amended and supplemented, and K.S.A. 12-693.

SECTION 8. Be it further resolved that the above described improvement is hereby authorized and declared to be necessary in accordance with the findings of the Governing Body as set out in this resolution.

SECTION 9. That the City Clerk shall make proper publication of this resolution, which shall be published once in the official City paper and which shall be effective from and after said publication.

ADOPTED at Wichita, Kansas, _____.

BOB KNIGHT, MAYOR

ATTEST:

PAT BURNETT, CITY CLERK

(SEAL)

STAFF REPORT
(Preliminary Plat)

CASE NUMBER: SUB 2001-25 – MEL HAMBELTON ADDITION

OWNER/APPLICANT: Mel Hambelton Real Estate, L.L.C., 3901 N. Broadway, Wichita, KS 67219-3202

SURVEYOR/ENGINEER: Baughman Company, P.A., 315 Ellis, Wichita, KS 67211

LOCATION: Southeast corner of 119th St. West and Kellogg

SITE SIZE: 21.15 Acres

NUMBER OF LOTS

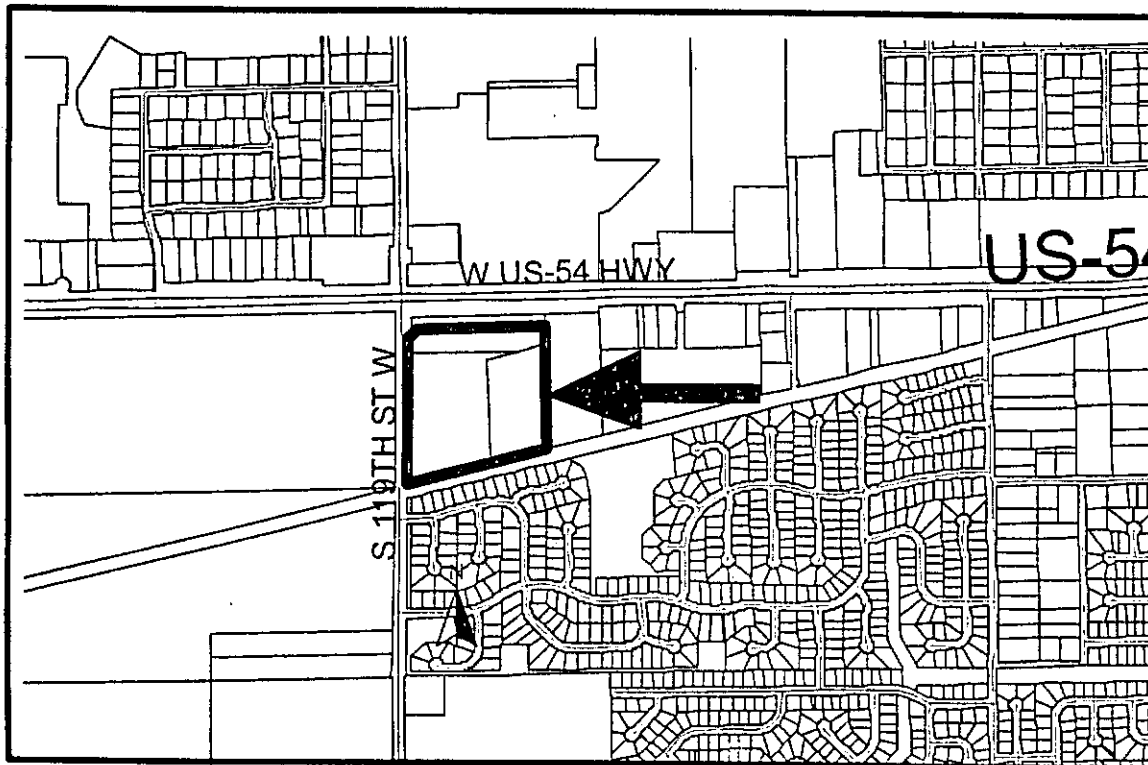
Residential:	
Office:	
Commercial:	8
Industrial:	1
Total:	9

MINIMUM LOT AREA: 20,000 Sq. Ft.

CURRENT ZONING: LI, Limited Industrial

PROPOSED ZONING: Same

VICINITY MAP



SUB 2001-25 -- Preliminary Plat of MEL HAMBELTON ADDITION
March 15, 2001 - Page 2

Note: This is a replat of the A & J Industrial Addition.

STAFF COMMENTS:

- A. The applicant shall guarantee the extension of sanitary sewer and City water services. City Engineering needs to comment on the need for any additional guarantees or easements.
- B. If improvements are guaranteed by petition, a notarized certificate listing the petitions shall be submitted to the Planning department for recording.
- C. City/County Engineering needs to comment on the status of the applicant's drainage concept. County Engineering requests a copy of the drainage plan.
- D. Traffic/County Engineering needs to comment on the access controls, particularly the need for increased distance between access openings along 119th St. West, and the need for additional access control dedicated beyond the railroad. For lots adjacent to railroad tracks, the Subdivision Regulations requires the dedication of 150 feet of complete access control along the street frontage. The plat proposes four access openings along 119th St. West including three joint openings. The final plat shall reference the access controls in the plat's text. County Engineering has approved two access openings. MAPD recommends a cross-lot circulation agreement to assure internal access between the lots.
- E. Traffic Engineering and KDOT need to comment on the need for additional right-of-way for the future Kellogg freeway/interchange, in addition to the medial opening from Kellogg Drive.
- F. County Engineering requests petitions for left and right turn lane improvements to 119th St. West.
- G. The joint access openings shall be established by separate instrument. Initial construction responsibilities and future maintenance of the driveway within the easement should also be addressed by the text of the instrument.
- H. The joint access easement in Lot 2 needs to be labelled as 30 feet.
- I. Provisions shall be made for ownership and maintenance of the proposed reserves. The applicant shall either form a lot owners' association prior to recording the plat or shall submit a covenant stating when the association will be formed, when the reserves will be deeded to the association and who is to own and maintain the reserves prior to the association taking over those responsibilities.
- J. For those reserves being platted for drainage purposes, the required covenant which provides for ownership and maintenance of the reserves shall grant, to the appropriate governing body, the authority to maintain the drainage reserves in the event the owner(s) fail to do so. The covenant shall provide for the cost of such maintenance to be charged back to the owner(s) by the governing body.
- K. The applicant is reminded that a platting binder is required with the final plat. Approval of this plat will be subject to submittal of this binder and any relevant conditions found by such a review.

SUB 2001-25 -- Preliminary Plat of MEL HAMBELTON ADDITION
March 15, 2001 - Page 3

- L. The platlor's text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- M. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- N. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.
- O. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-729-0102) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.
- P. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- Q. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoffs.
- R. Perimeter closure computations shall be submitted with the final plat tracing.
- S. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- T. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property.
- U. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.

Huang, Vicky

From: Huang, Vicky
Sent: Monday, March 12, 2001 3:52 PM
To: Carrier, Christopher
Cc: Lindebak, Mike ; Lackey, Stephen
Subject: RE: Mel Hambleton Addition Drainage Plan

Sensitivity: Private

This is entirely Baughman's design. But here are some points for your consideration:

1. Although this site naturally drain south to the Railroad ditch, the railroad ditch drain to the east and than will try to drain north to Kellogg again.
2. When we have detention/retention design using a pump system to drain down the ponding water, we make the developer create an association to maintain the pond and the pump. The New Market Square at 21st and Maize Road has one of such system.
3. The West Kellogg project being designed by PEC will be let later this year with storm sewer along the frontage road that ties to the proposed RCB crossing Kellogg to drain to the north. We can advise Baughman to work out the storm sewer design with PEC to make sure the storm sewer is sized to handle the discharge from the pond.

-----Original Message-----

From: Carrier, Christopher
Sent: Monday, March 12, 2001 2:56 PM
To: Huang, Vicky
Cc: Lindebak, Mike ; Lackey, Stephen
Subject: Mel Hambleton Addition Drainage Plan
Sensitivity: Private

How much input have you had in this or is this something that Baughman dreamed up? This does not look good to me. The outlet from the pond is a force main into the Kellogg median, where drainage is bad already! The natural flow for this is to the south also.

Even if it's something we've worked with them on, let it be known that we can't operate and maintain the pump station - someone else would have to. I hate to do that when many properties are involved and many of them would just naturally think that the city would run it. But, we have a full plate now. This could create a situation later, years down the line, when we get pressured into taking it cause someone else can't afford to maintain it. We have these situations all the time. Don't need another one.

I would not recommend proceeding until all this is thoroughly reviewed, in detail, including all design calcs, etc. I think this is a bad design.

STAFF REPORT
(Final Plat, Preliminary Plat Approved 3/15/01)

CASE NUMBER: SUB 2001-25 -- MEL HAMBELTON ADDITION

OWNER/APPLICANT: Mel Hambelton Real Estate, L.L.C., 3901 N. Broadway, Wichita, KS 67219-3202

SURVEYOR/ENGINEER: Baughman Company, P.A., 315 Ellis, Wichita, KS 67211

LOCATION: Southeast corner of 119th St. West and Kellogg

SITE SIZE: 21.15 Acres

NUMBER OF LOTS

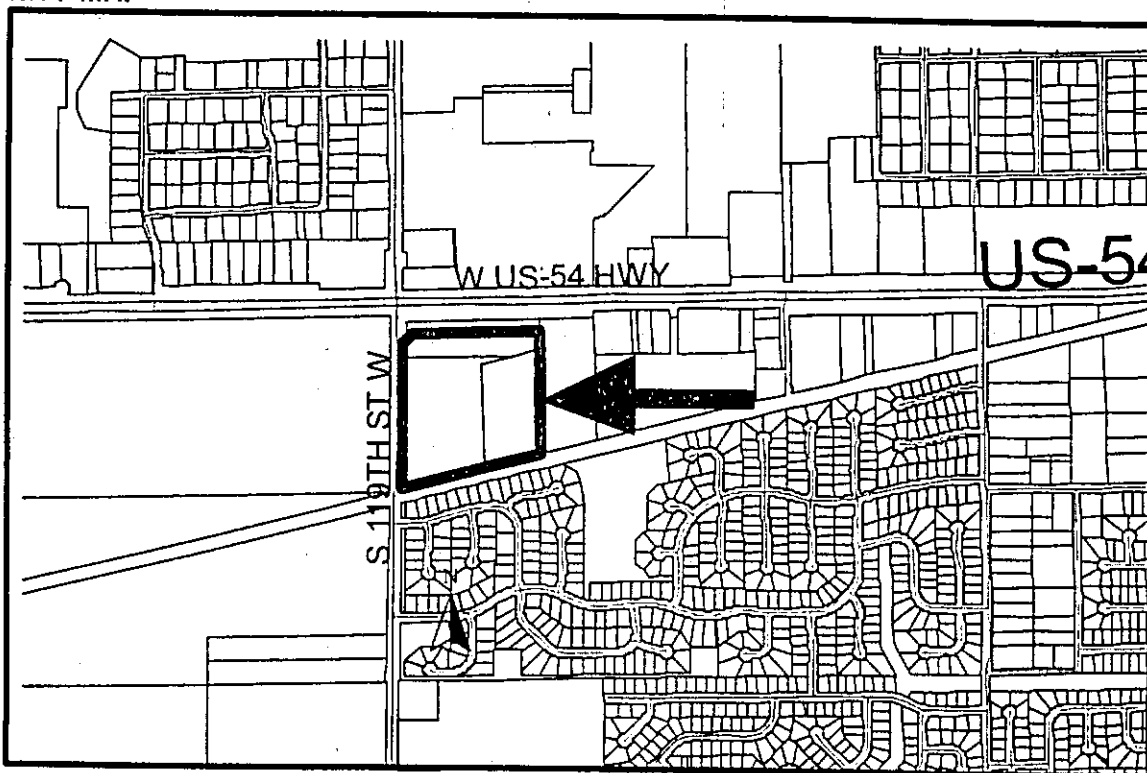
Residential:	
Office:	
Commercial:	8
Industrial:	1
Total:	8

MINIMUM LOT AREA: 20,000 Sq. Ft.

CURRENT ZONING: LI, Limited Industrial

PROPOSED ZONING: Same

VICINITY MAP



Note: This is a replat of the A & J Industrial Addition.

STAFF COMMENTS:

- A. The applicant shall guarantee the extension of sanitary sewer and City water services.
- B. If improvements are guaranteed by petition, a notarized certificate listing the petitions shall be submitted to the Planning department for recording.
- C. City/County Engineering needs to comment on the status of the applicant's drainage plan. A drainage guarantee is required. A letter from KDOT is required regarding their acceptance of drainage directed onto US-54.
- D. For lots adjacent to railroad tracks, the Subdivision Regulations requires the dedication of 150 feet of complete access control along the street frontage. City Engineering has approved three access openings. A cross-lot circulation agreement is required to assure internal access between the lots. The southernmost opening is approved contingent upon the abandonment of the railroad. Prior to its abandonment, 150 feet of complete access control is required from the railroad tracks.
- E. Traffic Engineering and KDOT need to comment on the need for additional right-of-way for the future Kellogg freeway/interchange, in addition to the medial opening from Kellogg Drive. Traffic Engineering has required complete access control from Kellogg Drive to U.S. 54 Highway.
- F. County Engineering requests petitions for left and right turn lane improvements to 119th St. West.
- G. The joint access openings shall be established by separate instrument. Initial construction responsibilities and future maintenance of the driveway within the easement should also be addressed by the text of the instrument.
- H. The joint access easement in Lot 2 needs to be labelled as 30 feet.
- I. The bench mark needs a better described location.
- J. The easement in Misc. Bk. 654, Pg. 229 needs to be located on Lots 1 and 5.
- K. Provisions shall be made for ownership and maintenance of the proposed reserves. The applicant shall either form a lot owners' association prior to recording the plat or shall submit a covenant stating when the association will be formed, when the reserves will be deeded to the association and who is to own and maintain the reserves prior to the association taking over those responsibilities.
- L. For those reserves being platted for drainage purposes, the required covenant which provides for ownership and maintenance of the reserves shall grant, to the appropriate governing body, the authority to maintain the drainage reserves in the event the owner(s) fail to do so. The covenant shall provide for the cost of such maintenance to be charged back to the owner(s) by the governing body.
- M. Based upon the platting binder, property taxes are still outstanding. Before the plat is scheduled for City Council consideration, proof shall be provided indicating that all applicable property taxes have been paid.

- N. On the final plat tracing, the MAPC signature block needs to reference "J.D. Michaelis, Acting Chair".
- O. The platting text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- P. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- Q. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.
- R. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-729-0102) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.
- S. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- T. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoffs.
- U. Perimeter closure computations shall be submitted with the final plat tracing.
- V. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- W. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property. **KGE has requested additional easements which have been denoted on the final plat.**
- X. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.



BAUGHMAN COMPANY P. A.
ENGINEERING & SURVEYING
316/262-7271 • 315 ELLIS • WICHITA, KANSAS 67211

FACSIMILE COVER SHEET

TO:

~~FAX NUMBER: 268 4390~~

~~NAME: Nell Strahl~~

~~FIRM NAME: Planning Dept.~~

~~CITY / STATE: Wichita, KS~~

FROM:

BAUGHMAN COMPANY, P.A.

FAX NUMBER (316) 262-0149

NAME OF SENDER: Judy Terhune

PROJECT: Mel Hambelton Addition

PROJECT NO:

DATE: May 30, 2001

NUMBER OF PAGES (INCLUDING THIS PAGE): 2

COMMENTS:

Nell,

Attached is a copy of the memo we have been forwarded for the above addition. Will this allow you to now submit this mylar to City Council for approval? Please advise.

Thanks, Judy

cc: Vicky Huang, Engineering Dept., City of Wichita

(IF YOU DO NOT RECEIVE ALL PAGES, PLEASE CALL (316) 262-7271)

Kansas Department of Transportation

MEMO TO: Chuck Luedders, P.E.
District V Engineer

ATTENTION: Robert G. Kuhn, R.L.S.
Construction Specialist

FROM: Jim L. Kowach, P.E.
Chief, Bureau of Design

BY: Corky Armstrong, P.E. *CA*
Road Design Engineer

DATE: May 16, 2001

SUBJECT: US-54 and 119th Street
Sedgwick County
Mel Hamblton Addition

Post-it* Fax Note	7671	Date	May 29/01	# of pages	1
To	Brian Glenn	From	Bob Wandel		
Co./Dept.		Co.			
Phone #		(785) #	296-4139		
Fax #	(316) 262-0149	Fax #	296-0746		

We have received additional drainage information from the Baughman Company and letters from the city of Wichita and the engineering firm of Professional Engineering Consultants (PEC). Both the city and PEC indicated that they have reviewed and concur with the proposed drainage improvements for this subdivision. It is our understanding that when US-54 is improved through this area, the outlet pipe from the detention pond will be connected into a new storm sewer system that is designed to handle this flow.

Based on our review and the information contained in these letters, we have no objection to the proposed drainage plan.

Since a portion of the construction will occur adjacent to the existing eastbound lanes of US-54, the contractor should be required to install the appropriate traffic-control signing.

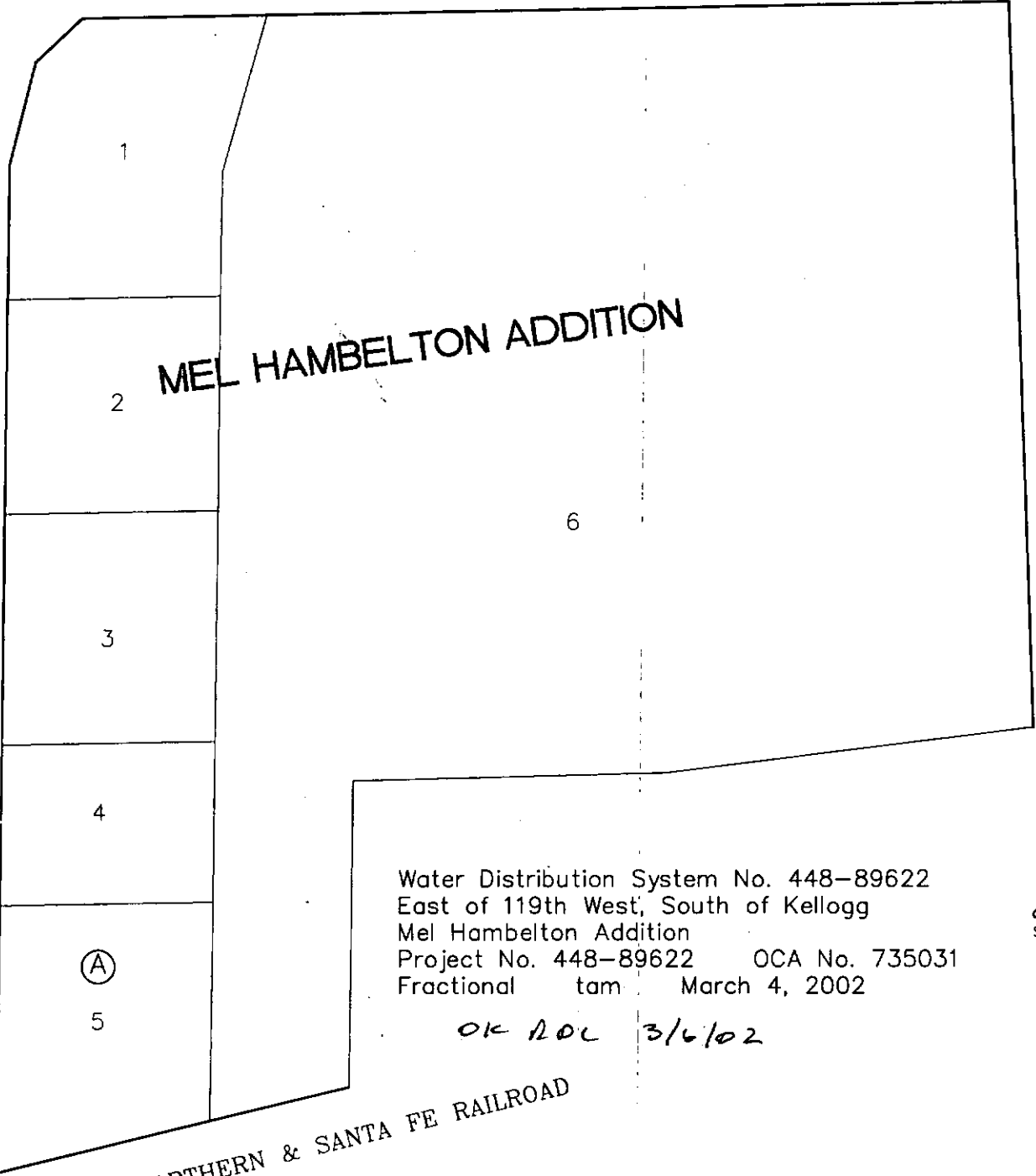
As noted in our memo of April 24, 2001, ingress and egress to the subdivision should be given careful consideration in light of the proposed improvements to US-54 highway and the adjacent access roads.

If you have any questions concerning our review, please contact Bob Wandel at (785) 296-4139.

LCA:js

cc: Robert L. Wandel, Road Design Leader

KELLOGG DRIVE



119TH ST. WEST

MEL HAMBELTON ADDITION

6

3

4

(A)

5

Water Distribution System No. 448-89622
 East of 119th West, South of Kellogg
 Mel Hambelton Addition
 Project No. 448-89622 OCA No. 735031
 Fractional tam March 4, 2002

OK AOL 3/6/02

BURLINGTON NORTHERN & SANTA FE RAILROAD





BAUGHMAN COMPANY, P.A.

ENGINEERING, SURVEYING & PLANNING

316/262-7271 • FAX 316/262-0149 • 315 ELLIS • WICHITA, KANSAS 67211

October 17, 2001

Mr. Christopher Carrier, P.E.
Storm Water Engineer
City Hall - 8th Floor
455 N. Main
Wichita, KS 67202

RECEIVED

OCT 18 2001

RECEIVED
OCT 23 2001
CITY - ENGINEERING

**RE: Mel Hambelton Addition
Storm water Sewer Project
Wichita, Kansas**

Dear Chris:

As your aware, the owner of the above referenced site has requested us to modify the original drainage plan that was submitted at the time of platting. We have prepared engineered plans that routed the plat's existing pond through a 24" pipe to a proposed offsite detention pond. The system was designed to discharge a peak flow equal to the existing peak runoff that is occurring on the acquired land west of 119th Street. The proposed pond will detain the runoff using a 15" RCP as the pond's outfall control. The west detention pond is located outside of the Calfskin's special flood hazard area, as mapped by F.E.M.A, to avoid additional permitting and to preserve the existing trees. We have used HEC-1 to model the stage vs. discharge of the two ponds and have attached the routings for your use. The two tables below summarizes the input variables used in the model and computed peak flows before and after being routed through the detention ponds.

Table 1: Input Variables

	Mel Hambelton Addition	West Basin
D.A.	21.2 ac.	2.9 ac.
Tc	15 min.	15 min.
CN	95	85

Table 2: Peak Flows & Pond Routings

	Mel Hambelton Addition	East Pond	West Basin	West Pond
Q ₅	82 cfs	11 cfs	8 cfs	8 cfs
Q ₁₀₀	142 cfs	13 cfs	17 cfs	11 cfs

Contribution from Area = Flow into Calfskin now.
↑ West of 119th

↓ without Detention

↓ out of On-site Pond

↳ Discharge to Calfskin after improvements

Please call if you have any questions or concerns with the proposed project. Thank you.

Sincerely,
Baughman Company, PA

Scott C. Lindebak, I.E.

cc: file

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
* Lahey F77L-EM/32 version 5.01
* Dodson & Associates, Inc.
* RUN DATE 10/17/01 TIME 15:37:53
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
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X X XXXXXXX XXXXX X
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XXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION. NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY, DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL, LOSS RATE:GREEN AND AMPT INFILTRATION. KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM.

HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
1	ID	Mel Hamblton Addition									
2	ID	Wichita, Sedgwick County, Kansas									
3	ID	File: MelHam.IHI									
4	ID	Storm Events: 2,5,10,25,50, & 100 YR.									
5	IT	15	300								
6	IO	3	0	0							
7	JR	PREC	1.000	1.2857	1.5143	1.7429	1.9714	2.2286			
		*Diagram									
		*									
8	KK	B1									
9	KM	East Site									
10	BA	.03305									
11	PB	3.5									
12	PC	0	.002	.005	.008	.011	.014	.017	.020	.023	.026
13	PC	.029	.032	.035	.038	.041	.044	.048	.052	.056	.060
14	PC	.064	.068	.072	.076	.080	.085	.090	.095	.100	.105
15	PC	.110	.115	.120	.126	.133	.140	.147	.155	.163	.172
16	PC	.181	.191	.203	.218	.236	.257	.283	.387	.663	.707
17	PC	.735	.758	.776	.791	.804	.815	.825	.834	.842	.849
18	PC	.856	.863	.869	.875	.881	.887	.893	.898	.903	.908
19	PC	.913	.918	.922	.926	.930	.934	.938	.942	.946	.950
20	PC	.953	.956	.959	.962	.965	.968	.971	.974	.977	.980
21	PC	.983	.986	.989	.992	.995	.998	1.00			
22	LS	0	95	100							
23	UD	0.15									
		*									
24	KK	P1									
25	KM	East Pond									
26	RS	1	ELEV	136							
27	SA	1.99	2.15	2.34	2.56	2.86	2.9	2.95			
28	SE	136	137	138	139	140	140.5	141			
29	SQ	0	1	2	3	4	5	6	7	8	9
30	SQ	10	11	12	13	13.5	14	15			
31	SE	136.0	136.46	136.65	136.80	136.94	137.07	137.20	137.32	137.45	137.59
32	SE	137.76	138.02	138.70	139.37	139.72	140.08	140.84			
		*									

33 KK B2
 34 KM West Site
 35 BA .00454
 36 LS 0 85 0
 37 UD 0.15
 *

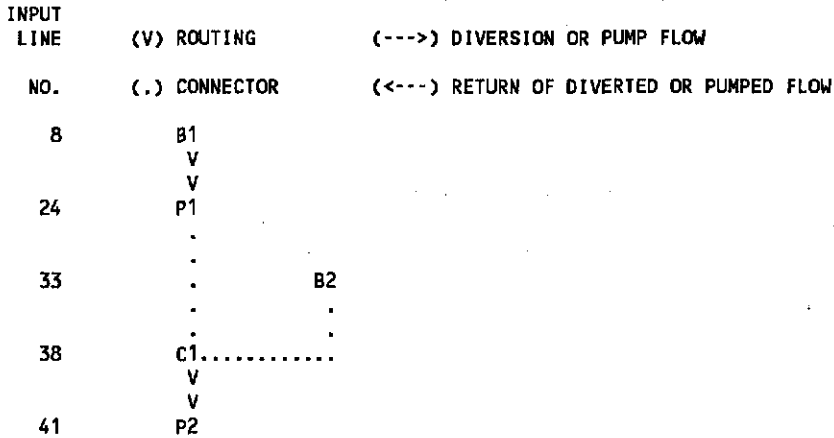
38 KK C1
 39 KM Combo P1 & B2
 40 HC 2 0
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

41 KK P2
 42 KM West Pond
 43 RS 1 ELEV 135
 44 SA 1.0 1.11 1.21 1.32 1.42 2.0
 45 SE 135 136 137 138 139 140
 46 SQ 0 .5 1.91 3.81 5.54 5.67 6.94 8.01 8.96 9.81
 47 SQ 10.6 11.3 12.0 12.7 13.3 13.9 14.4 14.989 15.5 16.0
 48 SE 135.0 135.25 135.50 135.75 136.0 136.25 136.5 136.75 137.0 137.25
 49 SE 137.5 137.75 138.0 138.25 138.5 138.75 139.0 139.25 139.5 139.75
 *
 50 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK



(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

			6-HR	24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
			(CFS)			
+	96.	12.00	13.	5.	2.	2.
			(INCHES)	5.293	5.300	5.300
			(AC-FT)	9.	9.	9.

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B1
FOR PLAN 1, RATIO = 1.74

TOTAL RAINFALL = 6.10, TOTAL LOSS = 0.00, TOTAL EXCESS = 6.10

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW		
				24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
			(CFS)			
+	111.	12.00	15.	5.	2.	2.
			(INCHES)	4.324	6.093	6.100
			(AC-FT)	8.	11.	11.

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B1
FOR PLAN 1, RATIO = 1.97

TOTAL RAINFALL = 6.90, TOTAL LOSS = 0.00, TOTAL EXCESS = 6.90

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW		
				24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
			(CFS)			
+	125.	12.00	17.	6.	2.	2.
			(INCHES)	4.891	6.891	6.900
			(AC-FT)	9.	12.	12.

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION B1
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.00, TOTAL EXCESS = 7.80

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW		
				24-HR	72-HR	74.75-HR
+	(CFS)	(HR)				
			(CFS)			
+	142.	12.00	20.	7.	2.	2.
			(INCHES)	5.530	7.790	7.800
			(AC-FT)	10.	14.	14.

CUMULATIVE AREA = 0.03 SQ MI

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24 KK * P1 *
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East Pond

HYDROGRAPH ROUTING DATA

26 RS	STORAGE ROUTING NSTPS ITYP RSVRC X	1 ELEV 136.00 0.00	NUMBER OF SUBREACHES TYPE OF INITIAL CONDITION INITIAL CONDITION WORKING R AND D COEFFICIENT								
27 SA	AREA	2.0	2.2	2.3	2.6	2.9	2.9	3.0			
28 SE	ELEVATION	136.00	137.00	138.00	139.00	140.00	140.50	141.00			
29 SQ	DISCHARGE	0. 10.	1. 11.	2. 12.	3. 13.	4. 14.	5. 14.	6. 15.	7.	8.	9.
31 SE	ELEVATION	136.00 137.76	136.46 138.02	136.65 138.70	136.80 139.37	136.94 139.72	137.07 140.08	137.20 140.84	137.32	137.45	137.59

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	2.07	4.31	6.76	9.47	10.91	12.37
ELEVATION	136.00	137.00	138.00	139.00	140.00	140.50	141.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.93	1.33	1.64	1.94	2.07	2.22	2.50	2.77	3.06
OUTFLOW	0.00	1.00	2.00	3.00	4.00	4.46	5.00	6.00	7.00	8.00
ELEVATION	136.00	136.46	136.65	136.80	136.94	137.00	137.07	137.20	137.32	137.45
STORAGE	3.37	3.76	4.31	4.36	6.01	6.76	7.73	8.68	9.47	9.70
OUTFLOW	9.00	10.00	10.92	11.00	12.00	12.45	13.00	13.50	13.89	14.00
ELEVATION	137.59	137.76	138.00	138.02	138.70	139.00	139.37	139.72	140.00	140.08
STORAGE	10.91	11.90	12.37							
OUTFLOW	14.55	15.00	15.21							
ELEVATION	140.50	140.84	141.00							

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM	AVERAGE FLOW		
			6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)	(CFS)				
+ 9.	12.75	7.	3.	1.	1.	
		(INCHES)	1.850	3.081	3.491	3.493
		(AC-FT)	3.	5.	6.	6.
PEAK STORAGE	TIME		MAXIMUM	AVERAGE STORAGE		
			6-HR	24-HR	72-HR	74.75-HR
+ (AC-FT)	(HR)					
+ 3.	12.75	3.	1.	1.	1.	
PEAK STAGE	TIME		MAXIMUM	AVERAGE STAGE		
			6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)					
+ 137.63	12.75	137.27	136.70	136.29	136.28	

CUMULATIVE AREA = 0.03 SQ MI

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.29

PEAK FLOW	TIME		MAXIMUM	AVERAGE FLOW		
			6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)	(CFS)				
+ 11.	12.75	9.	4.	1.	1.	
		(INCHES)	2.430	4.023	4.490	4.492
		(AC-FT)	4.	7.	8.	8.
PEAK STORAGE	TIME		MAXIMUM	AVERAGE STORAGE		
			6-HR	24-HR	72-HR	74.75-HR

+ (AC-FT)	(HR)				
4.	12.75	3.	2.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)				
138.04	12.75	137.59	136.84	136.34	136.33

CUMULATIVE AREA = 0.03 SQ MI

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HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.51

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)				
		(CFS)			
+ 12.	12.75	10.	4.	2.	2.
		(INCHES)	2.816	4.778	5.290
		(AC-FT)	5.	8.	9.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (AC-FT)	(HR)				
5.	12.75	4.	2.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)				
138.37	12.75	137.89	136.97	136.39	136.38

CUMULATIVE AREA = 0.03 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.74

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)				
		(CFS)			
+ 12.	13.00	11.	5.	2.	2.
		(INCHES)	3.112	5.535	6.089
		(AC-FT)	5.	10.	11.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (AC-FT)	(HR)				
6.	13.00	5.	2.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)				
138.71	13.00	138.22	137.12	136.45	136.43

CUMULATIVE AREA = 0.03 SQ MI

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HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 1.97

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)				
		(CFS)			
+ 13.	13.00	12.	6.	2.	2.
		(INCHES)	3.314	6.290	6.888
		(AC-FT)	6.	11.	12.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
		6-HR	24-HR	72-HR	74.75-HR

+ (AC-FT)	(HR)				
7.	13.00	6.	3.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)				
139.04	13.00	138.56	137.28	136.51	136.49

CUMULATIVE AREA = 0.03 SQ MI

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HYDROGRAPH AT STATION P1
FOR PLAN 1, RATIO = 2.23

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW		
		6-HR	24-HR	72-HR	74.75-HR
+ (CFS)	(HR)				
13.	13.00	12.	6.	2.	2.
		(INCHES)	7.138	7.787	7.790
		(AC-FT)	13.	14.	14.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (AC-FT)	(HR)				
8.	13.00	7.	3.	1.	1.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
		6-HR	24-HR	72-HR	74.75-HR
+ (FEET)	(HR)				
139.39	13.00	138.94	137.47	136.58	136.56

CUMULATIVE AREA = 0.03 SQ MI

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33 KK * B2 *

West Site

SUBBASIN RUNOFF DATA

35 BA SUBBASIN CHARACTERISTICS
TAREA 0.00 SUBBASIN AREA

PRECIPITATION DATA

11 PB STORM 3.50 BASIN TOTAL PRECIPITATION

12 PI INCREMENTAL PRECIPITATION PATTERN

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.01	0.00	0.01	0.00	0.01	0.01
0.01	0.00	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.02	0.02	0.02	0.03	0.10	0.28	0.04	0.03	0.03
0.02	0.02	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01	0.01
0.01	0.01	0.01	0.01	0.01	0.01	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

36 LS SCS LOSS RATE
STRTL 0.35 INITIAL ABSTRACTION
CRVNR 85.00 CURVE NUMBER
RTIMP 0.00 PERCENT IMPERVIOUS AREA

37 UD SCS DIMENSIONLESS UNITGRAPH
TLAG 0.15 LAG

UNIT HYDROGRAPH
5 END-OF-PERIOD ORDINATES

	8.	3.	1.	0.	0.
TOTAL RAINFALL =	3.50,	TOTAL LOSS =	1.48,	TOTAL EXCESS =	2.02
PEAK FLOW	TIME			MAXIMUM AVERAGE FLOW	
+ (CFS)	(HR)		6-HR	24-HR	72-HR
		(CFS)			74.75-HR
+ 6.	12.00		1.	0.	0.
		(INCHES)	1.632	2.016	2.016
		(AC-FT)	0.	0.	0.
		CUMULATIVE AREA =	0.00 SQ MI		

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.00

	8.	3.	1.	0.	0.
TOTAL RAINFALL =	3.50,	TOTAL LOSS =	1.48,	TOTAL EXCESS =	2.02
PEAK FLOW	TIME			MAXIMUM AVERAGE FLOW	
+ (CFS)	(HR)		6-HR	24-HR	72-HR
		(CFS)			74.75-HR
+ 6.	12.00		1.	0.	0.
		(INCHES)	1.632	2.016	2.016
		(AC-FT)	0.	0.	0.
		CUMULATIVE AREA =	0.00 SQ MI		

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.29

	8.	3.	1.	0.	0.
TOTAL RAINFALL =	4.50,	TOTAL LOSS =	1.59,	TOTAL EXCESS =	2.91
PEAK FLOW	TIME			MAXIMUM AVERAGE FLOW	
+ (CFS)	(HR)		6-HR	24-HR	72-HR
		(CFS)			74.75-HR
+ 8.	12.00		1.	0.	0.
		(INCHES)	2.336	2.909	2.909
		(AC-FT)	1.	1.	1.
		CUMULATIVE AREA =	0.00 SQ MI		

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.51

	8.	3.	1.	0.	0.
TOTAL RAINFALL =	5.30,	TOTAL LOSS =	1.65,	TOTAL EXCESS =	3.65
PEAK FLOW	TIME			MAXIMUM AVERAGE FLOW	
+ (CFS)	(HR)		6-HR	24-HR	72-HR
		(CFS)			74.75-HR
+ 11.	12.00		1.	0.	0.
		(INCHES)	2.912	3.646	3.646
		(AC-FT)	1.	1.	1.
		CUMULATIVE AREA =	0.00 SQ MI		

HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.74

TOTAL RAINFALL = 6.10, TOTAL LOSS = 1.70, TOTAL EXCESS = 4.40

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
13.	12.00	2.	1.	0.	0.	
		(INCHES) 3.491	4.397	4.397	4.397	
		(AC-FT) 1.	1.	1.	1.	

CUMULATIVE AREA = 0.00 SQ MI

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HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 1.97

TOTAL RAINFALL = 6.90, TOTAL LOSS = 1.74, TOTAL EXCESS = 5.16

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
15.	12.00	2.	1.	0.	0.	
		(INCHES) 4.074	5.157	5.157	5.157	
		(AC-FT) 1.	1.	1.	1.	

CUMULATIVE AREA = 0.00 SQ MI

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HYDROGRAPH AT STATION B2
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 1.78, TOTAL EXCESS = 6.02

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
17.	12.00	2.	1.	0.	0.	
		(INCHES) 4.731	6.021	6.021	6.021	
		(AC-FT) 1.	1.	1.	1.	

CUMULATIVE AREA = 0.00 SQ MI

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38 KK * C1 *
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Combo P1 & B2

40 HC HYDROGRAPH COMBINATION
ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** **

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	74.75-HR
11.	12.25	7.	3.	1.	1.	
		(INCHES) 1.812	2.952	3.313	3.314	
		(AC-FT) 4.	6.	7.	7.	

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.29

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			74.75-HR
		6-HR	24-HR	72-HR	
16.	12.00	10.	4.	1.	1.
		(INCHES) 2.398	3.887	4.299	4.301
		(AC-FT) 5.	8.	9.	9.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.51

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			74.75-HR
		6-HR	24-HR	72-HR	
19.	12.00	11.	5.	2.	2.
		(INCHES) 2.796	4.640	5.091	5.093
		(AC-FT) 6.	9.	10.	10.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.74

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			74.75-HR
		6-HR	24-HR	72-HR	
23.	12.00	13.	5.	2.	2.
		(INCHES) 3.108	5.395	5.885	5.887
		(AC-FT) 6.	11.	12.	12.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 1.97

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			74.75-HR
		6-HR	24-HR	72-HR	
26.	12.00	13.	6.	2.	2.
		(INCHES) 3.338	6.150	6.679	6.681
		(AC-FT) 7.	12.	13.	13.

CUMULATIVE AREA = 0.04 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
FOR PLAN 1, RATIO = 2.23

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			74.75-HR
		6-HR	24-HR	72-HR	
28.	12.00	14.	7.	3.	2.
		(INCHES) 3.548	6.998	7.574	7.576

(AC-FT) 7. 14. 15. 15.

CUMULATIVE AREA = 0.04 SQ MI

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* P2 *
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West Pond

HYDROGRAPH ROUTING DATA

43 RS	STORAGE ROUTING											
	NSTPS	1	NUMBER OF SUBREACHES									
	ITYP	ELEV	TYPE OF INITIAL CONDITION									
	RSVVIC	135.00	INITIAL CONDITION									
	X	0.00	WORKING R AND D COEFFICIENT									
44 SA	AREA	1.0	1.1	1.2	1.3	1.4	2.0					
45 SE	ELEVATION	135.00	136.00	137.00	138.00	139.00	140.00					
46 SQ	DISCHARGE	0.	1.	2.	4.	6.	6.	7.	8.	9.	10.	
		11.	11.	12.	13.	13.	14.	14.	15.	16.	16.	
48 SE	ELEVATION	135.00	135.25	135.50	135.75	136.00	136.25	136.50	136.75	137.00	137.25	
		137.50	137.75	138.00	138.25	138.50	138.75	139.00	139.25	139.50	139.75	

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	1.05	2.21	3.48	4.85	6.55
ELEVATION	135.00	136.00	137.00	138.00	139.00	140.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.25	0.51	0.78	1.05	1.34	1.62	1.91	2.21	2.52
OUTFLOW	0.00	0.50	1.91	3.81	5.54	5.67	6.94	8.01	8.96	9.81
ELEVATION	135.00	135.25	135.50	135.75	136.00	136.25	136.50	136.75	137.00	137.25
STORAGE	2.83	3.15	3.48	3.81	4.15	4.50	4.85	5.22	5.63	6.07
OUTFLOW	10.60	11.30	12.00	12.70	13.30	13.90	14.40	14.99	15.50	16.00
ELEVATION	137.50	137.75	138.00	138.25	138.50	138.75	139.00	139.25	139.50	139.75
STORAGE	6.55									
OUTFLOW	16.50									
ELEVATION	140.00									

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HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+	6.	15.50	6.	3.	1.	1.
			(CFS)			
			(INCHES)	1.453	2.902	3.306
			(AC-FT)	3.	6.	7.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+	1.	15.50	1.	1.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+	136.37	15.50	136.25	135.67	135.29	135.28

CUMULATIVE AREA = 0.04 SQ MI

PEAK FLOW		TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
8.	15.75	(CFS)	8.	4.	1.	1.
		(INCHES)	1.882	3.837	4.291	4.293
		(AC-FT)	4.	8.	9.	9.
PEAK STORAGE		TIME	MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
2.	15.75		2.	1.	0.	0.
PEAK STAGE		TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
136.81	15.75		136.66	135.87	135.37	135.35

CUMULATIVE AREA = 0.04 SQ MI

HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.51

PEAK FLOW		TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
9.	16.50	(CFS)	9.	5.	2.	2.
		(INCHES)	2.171	4.588	5.083	5.084
		(AC-FT)	4.	9.	10.	10.
PEAK STORAGE		TIME	MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
2.	16.50		2.	1.	0.	0.
PEAK STAGE		TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
137.10	16.50		136.96	136.04	135.43	135.41

CUMULATIVE AREA = 0.04 SQ MI

HYDROGRAPH AT STATION P2
FOR PLAN 1, RATIO = 1.74

PEAK FLOW		TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
10.	17.25	(CFS)	10.	5.	2.	2.
		(INCHES)	2.399	5.338	5.876	5.877
		(AC-FT)	5.	11.	12.	12.
PEAK STORAGE		TIME	MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
3.	17.25		2.	1.	1.	1.
PEAK STAGE		TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
137.35	17.25		137.22	136.22	135.49	135.47

MEL HAMBELTON ADDITION - POND ROUTINGS

CUMULATIVE AREA = 0.04 SQ MI

		***	***	***	***	***
		HYDROGRAPH AT STATION P2				
		FOR PLAN 1, RATIO = 1.97				
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 11.	18.00	(CFS)	10.	6.	2.	2.
		(INCHES)	2.584	6.084	6.669	6.671
		(AC-FT)	5.	12.	13.	13.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 3.	18.00		3.	2.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 137.56	18.00		137.45	136.40	135.56	135.54
		CUMULATIVE AREA = 0.04 SQ MI				

		***	***	***	***	***
		HYDROGRAPH AT STATION P2				
		FOR PLAN 1, RATIO = 2.23				
PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 11.	18.75	(CFS)	11.	7.	3.	2.
		(INCHES)	2.756	6.917	7.564	7.566
		(AC-FT)	6.	14.	15.	15.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 3.	18.75		3.	2.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	74.75-HR
+ 137.77	18.75		137.69	136.62	135.64	135.61
		CUMULATIVE AREA = 0.04 SQ MI				

MEL HAMBELTON ADDITION - POND ROUTINGS

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION						
				RATIO 1 1.00	RATIO 2 1.29	RATIO 3 1.51	RATIO 4 1.74	RATIO 5 1.97	RATIO 6 2.23	
HYDROGRAPH AT +	B1	0.03	1	FLOW TIME	64. 12.00	82. 12.00	96. 12.00	111. 12.00	125. 12.00	142. 12.00
ROUTED TO +	P1	0.03	1	FLOW TIME	9. 12.75	11. 12.75	12. 12.75	12. 13.00	13. 13.00	13. 13.00
				** PEAK STAGES IN FEET **						
			1	STAGE TIME	137.63 12.75	138.04 12.75	138.37 12.75	138.71 13.00	139.04 13.00	139.39 13.00
HYDROGRAPH AT +	B2	0.00	1	FLOW TIME	6. 12.00	8. 12.00	11. 12.00	13. 12.00	15. 12.00	17. 12.00
2 COMBINED AT +	C1	0.04	1	FLOW TIME	11. 12.25	16. 12.00	19. 12.00	23. 12.00	26. 12.00	28. 12.00
ROUTED TO +	P2	0.04	1	FLOW TIME	6. 15.50	8. 15.75	9. 16.50	10. 17.25	11. 18.00	11. 18.75
				** PEAK STAGES IN FEET **						
			1	STAGE TIME	136.37 15.50	136.81 15.75	137.10 16.50	137.35 17.25	137.56 18.00	137.77 18.75

*** NORMAL END OF HEC-1 ***