

STAFF REPORT
(PRELIMINARY PLAT)

CASE NUMBER: SUB 2002-54 -- SISTERS OF ST. JOSEPH FIFTH ADDITION

OWNER/APPLICANT: Sisters of St. Joseph, Attn: Mark A. Kuhn, 3700 E. Lincoln, Wichita, KS 67218-2099; City of Wichita, Attn: John Philbrick, 455 N. Main, 13th Floor, Wichita, KS 67202

SURVEYOR/ENGINEER: Baughman Company, P.A., Attn: Phil Meyer, 315 Ellis, Wichita, KS 67211

LOCATION: North of Harry, East of Hillside

SITE SIZE: 10.13 Acres

NUMBER OF LOTS

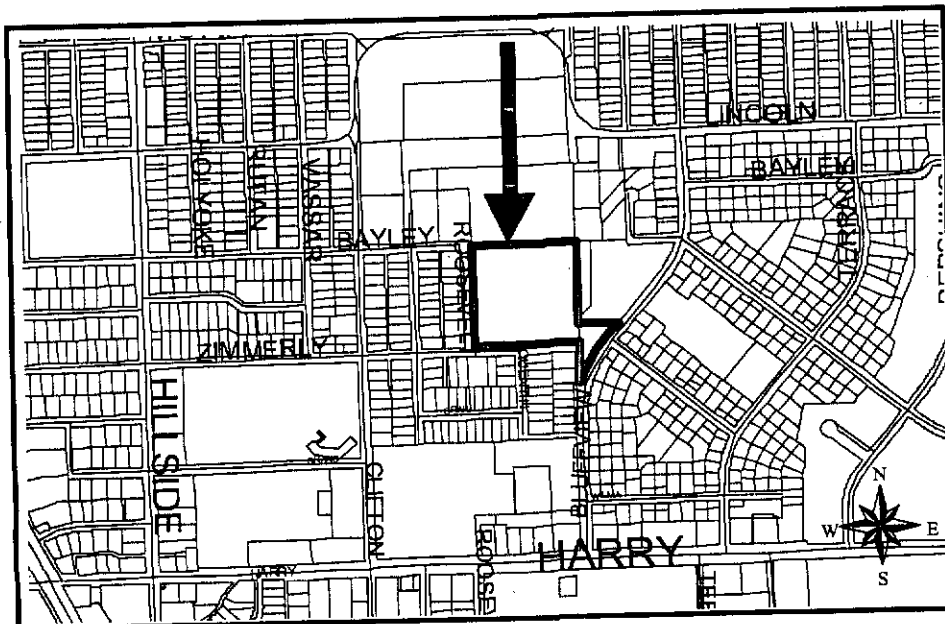
Residential:	1
Office:	
Commercial:	
Industrial:	
Total:	1

MINIMUM LOT AREA: 10.13 Acres

CURRENT ZONING: TF-3, Two-Family Residential; B, Multi-Family Residential

PROPOSED ZONING: Same

VICINITY MAP



DCS # 2 SS # 12

110

NOTE: This is a replat of portions of Hilltop Manor and Hilltop Manor 2nd Addition along with unplatted property. The site has been approved for a zone change (ZON 2002-08) from TF-3, Two-Family Residential to B, Multi-Family Residential for use as an elderly apartment living facility.

Planning Staff recommends approval of the plat.

STAFF COMMENTS:

- A. Municipal services appear to be available to serve the site. **City Engineering** needs to comment on the need for guarantees or easements.
- B. If improvements are guaranteed by petition, a notarized certificate listing the petitions shall be submitted to the Planning Department for recording.
- C. **City Engineering** needs to comment on the status of the applicant's drainage concept.
- D. Roosevelt Avenue is unpaved. **City/Traffic Engineering** needs to comment on the need for any improvements to perimeter streets.
- E. As a lot zoned for multi-family residential abutting non-arterial streets, the Subdivision regulations require a sidewalk along perimeter streets.
- F. A cross-lot access agreement shall be provided with the property to the north.
- G. The Applicant is reminded that a platting binder is required with the final plat. Approval of this plat will be subject to submittal of this binder and any relevant conditions found by such a review.
- H. The plat's text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- I. The applicant shall install or guarantee the installation of all utilities and facilities that are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- J. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.
- K. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-946-4556) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.

- L. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- M. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoffs.
- N. Perimeter closure computations shall be submitted with the final plat tracing.
- O. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- P. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property.
- Q. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.

**STAFF REPORT
(Final Plat, Preliminary Plat Approved 6/27/02)**

CASE NUMBER: SUB 2002-54 -- SISTERS OF ST. JOSEPH FIFTH ADDITION

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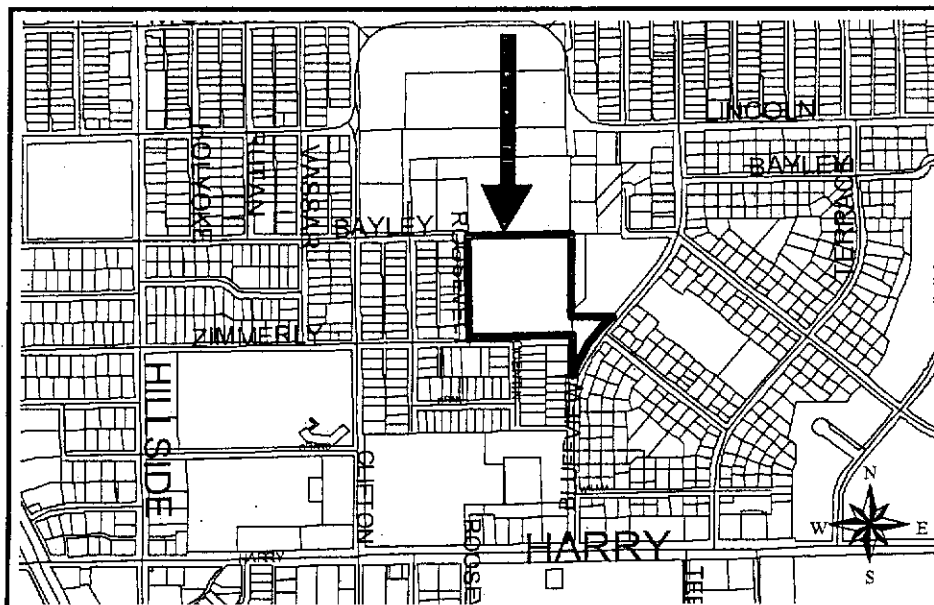
Residential:	1
Office:	
Commercial:	
Industrial:	
Total:	1

MINIMUM LOT AREA: 10.13 Acres

CURRENT ZONING: TF-3, Two-Family Residential; B, Multi-Family Residential

PROPOSED ZONING: Same

VICINITY MAP



SUB 2002-54 -- Final Plat of SISTERS OF ST. JOSEPH FIFTH ADDITION
August 1, 2002 - Page 2

NOTE: This is a replat of portions of Hilltop Manor and Hilltop Manor 2nd Addition along with unplatted property. The site has been approved for a zone change (ZON 2002-08) from TF-3, Two-Family Residential to B, Multi-Family Residential for use as an elderly apartment living facility.

Planning Staff recommends approval of the plat.

STAFF COMMENTS:

- A. Municipal services are available to serve the site. **A drainage reserve is needed along the southeast portion of the plat.**

The drainage reserve has been platted as requested.

- B. If improvements are guaranteed by petition, a notarized certificate listing the petitions shall be submitted to the Planning Department for recording.
- C. **City Engineering** needs to comment on the status of the applicant's drainage plan.
- D. **City Engineering has requested a petition for the paving of Roosevelt. The Applicant is opposed to this condition.**
- E. As a lot zoned for multi-family residential abutting non-arterial streets, the Subdivision regulations require a sidewalk along perimeter streets.
- F. A cross-lot access agreement shall be provided with the property to the north.
- G. Provisions shall be made for ownership and maintenance of the proposed reserves. A covenant shall be submitted regarding ownership and maintenance responsibilities.
- H. For those reserves being platted for drainage purposes, the required covenant which provides for ownership and maintenance of the reserves shall grant, to the City, the authority to maintain the drainage reserves in the event the owner(s) fail to do so. The covenant shall provide for the cost of such maintenance to be charged back to the owner(s) by the governing body.
- I. On the final plat tracing, the MAPC signature block needs to reference "J.D. Michaelis, Chair".
- J. The plat's text shall include language that a drainage plan has been developed for the plat and that all drainage easements, rights-of-way, or reserves shall remain at established grades or as modified with the approval of the applicable City or County Engineer, and unobstructed to allow for the conveyance of stormwater.
- K. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- L. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary's signature.

SUB 2002-54 -- Final Plat of SISTERS OF ST. JOSEPH FIFH ADDITION
August 1, 2002 - Page 3

- M. To receive mail delivery without delay, and to avoid unnecessary expense, the applicant is advised of the necessity to meet with the U.S. Postal Service Growth Management Coordinator (Phone 316-946-4556) prior to development of the plat so that the type of delivery, and the tentative mailbox locations can be determined.
- N. The applicant is advised that various State and Federal requirements (specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 317, Valley Center, KS 67147) for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- O. The owner of the subdivision should be aware of the fact that the development of any subdivision greater than five (5) acres in size may require an NPDES Storm Water Discharge Permit from the Kansas Department of Health and Environment in Topeka. Further, on all construction sites, the City of Wichita requires that best management practices be used to reduce pollutant loadings in storm water runoffs.
- P. Perimeter closure computations shall be submitted with the final plat tracing.
- Q. Recording of the plat within thirty (30) days after approval by the City Council and/or County Commission.
- R. The representatives from the utility companies should be prepared to comment on the need for any additional utility easements to be platted on this property.
- S. The applicant is reminded that a disk shall be submitted with the final plat tracing to the Planning Department detailing this plat in digital format in AutoCAD. This will be used by the City and County GIS Department.

HEC-RAS Version 3.0.1 Mar 2001
 U.S. Army Corp of Engineers
 Hydrologic Engineering Center
 609 Second Street, Suite D
 Davis, California 95616-4687
 (916) 756-1104

```

X      X  XXXXXX  XXXX      XXXX      XX      XXXX
X      X  X      X  X      X  X      X  X      X
X      X  X      X      X  X      X  X      X
XXXXXXXX XXXX  X      XXX XXXX  XXXXXX  XXXX
X      X  X      X      X  X      X  X
X
X      X  X      X  X      X  X      X  X
X
X      X  XXXXXX  XXXX      X  X      X  X      XXXXX
    
```


PROJECT DATA

Project Title: SSTJOE
 Project File : SSTJOE.prj
 Run Date and Time: 7/1/02 10:54:16 AM

Project in English units

PLAN DATA

Plan Title: STJOE
 Plan File : C:\Hec\RAS\STJOE\SSTJOE.p01

Geometry Title: STJOE
 Geometry File : C:\Hec\RAS\STJOE\SSTJOE.g01

Flow Title : STJOE
 Flow File : C:\Hec\RAS\STJOE\SSTJOE.f01

Plan Summary Information:

Number of:	Cross Sections =	5	Multitple Openings =	0
	Culverts =	0	Inline Weirs =	0
	Bridges =	0		

Computational Information

Water surface calculation tolerance = 0.003
 Critical depth calculaton tolerance = 0.003
 Maximum number of interations = 20
 Maximum difference tolerance = 0.1
 Flow tolerance factor = 0.001

Computation Options

Critical depth computed only where necessary
 Conveyance Calculation Method: At breaks in n values only
 Friction Slope Method: Average Conveyance
 Computational Flow Regime: Subcritical Flow

FLOW DATA

Flow Title: STJOE
 Flow File : C:\Hec\RAS\STJOE\SSTJOE.f01

Flow Data (cfs)

 * River Reach RS * 100-YR *
 * STJOE NORTH 792 * 649 *

Boundary Conditions

 * River Reach Profile * Up
 stream Downstream *

 * STJOE NORTH 100-YR *
 Normal S = .002 *

GEOMETRY DATA

Geometry Title: STJOE
 Geometry File : C:\Hec\RAS\STJOE\SSTJOE.g01

CROSS SECTION RIVER: STJOE
 REACH: NORTH RS: 792

INPUT

Description: STA. 7+92.0

Station Elevation Data num= 11

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
20	147	60	145	80	143	100	141.3
110	140.2						
120	137.9	130	140	150	141.8	170	143.2
180	144						
190	145						

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
20	.04	60	.04	190	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff
Contr.						
	60	190		301	301	301
	.1	.3				

CROSS SECTION RIVER: STJOE
 REACH: NORTH RS: 490.8

INPUT

Description: STA. 4+90.8

Station Elevation Data num= 9

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
60	143	100	139	110	138	115	137.5
130	139.7						
140	140.8	160	142.5	170	146.8	180	149

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
60	.04	60	.04	160	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff
Contr.						

.1 60 160 154 190 208
 .3

CROSS SECTION RIVER: STJOE
 REACH: NORTH RS: 294.9

INPUT

Description: STA. 2+94.9

Station Elevation Data num= 10
 Sta Elev Sta Elev Sta Elev Sta Elev
 Sta Elev

 30 143 90 142 130 140 137 138
 150 136.5
 160 135 170 136.5 180 140.5 190 142
 210 144

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val

 30 .04 90 .04 190 .04

Bank Sta: Left Right Lengths: Left Channel Right Coeff
 Contr. Expan.
 90 190 211 209 208
 .1 .3

CROSS SECTION RIVER: STJOE
 REACH: NORTH RS: 76.8

INPUT

Description: STA. 0+76.8

Station Elevation Data num= 10
 Sta Elev Sta Elev Sta Elev Sta Elev
 Sta Elev

 40 143 70 142 100 140 110 138
 115 134
 120 136 140 137 150 139 160 141
 173 142

Manning's n Values num= 3
 Sta n Val Sta n Val Sta n Val

 40 .04 70 .04 173 .04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff
Contr.	Expan.					
	70	173		122	75	24
.1	.3					

CROSS SECTION RIVER: STJOE
 REACH: NORTH RS: 0

INPUT

Description: STA. 0+00

Station Elevation Data	num=	12				
Sta Elev	Sta Elev	Sta Elev	Sta Elev	Sta Elev	Sta Elev	Sta Elev
0 142.3	40 141	60 140	70.5 139			
87 138						
104 134	110 135.2	120 138	130 138.8			
150 140.2						
166 141	188 142					

Manning's n Values	num=	3
Sta n Val	Sta n Val	Sta n Val
0 .04	0 .04	188 .04

Bank Sta:	Left	Right	Coeff	Contr.	Expan.
	0	188		.1	.3

SUMMARY OF MANNING'S N VALUES

River: STJOE

* Reach	* River Sta.	* n1	* n2	* n3
*NORTH	* 792	* .04*	* .04*	* .04*
*NORTH	* 490.8	* .04*	* .04*	* .04*
*NORTH	* 294.9	* .04*	* .04*	* .04*
*NORTH	* 76.8	* .04*	* .04*	* .04*
*NORTH	* 0	* .04*	* .04*	* .04*

SUMMARY OF REACH LENGTHS

River: STJOE

```
*****
* Reach * River Sta. * Left * Channel * Right *
*****
*NORTH * 792 * 301* 301* 301*
*NORTH * 490.8 * 154* 190* 208*
*NORTH * 294.9 * 211* 209* 208*
*NORTH * 76.8 * 122* 75* 24*
*NORTH * 0 * * * *
```


SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

River: STJOE

```
*****
* Reach * River Sta. * Contr. * Expan. *
*****
*NORTH * 792 * .1* .3*
*NORTH * 490.8 * .1* .3*
*NORTH * 294.9 * .1* .3*
*NORTH * 76.8 * .1* .3*
*NORTH * 0 * .1* .3*
```

Profile Output Table - Standard Table 1

```
*****
*****
*****
* Reach * River Sta * Q Total *Min Ch El *W.S. Elev *C
rit W.S. *E.G. Elev *E.G. Slope * Vel Chnl *Flow Area *Top Width *
Froude # Chl *
* * * (cfs) * (ft) * (ft) *
(ft) * (ft) * (ft/ft) * (ft/s) * (sq ft) * (ft) *
*
*****
*****
*****
* NORTH * 792 * 649.00 * 137.90 * 143.11 *
* 143.31 * 0.003650 * 3.57 * 181.81 * 89.89 *
0.44 *
* NORTH * 490.8 * 649.00 * 137.50 * 141.53 *
* 141.86 * 0.006566 * 4.61 * 140.75 * 73.79 *
0.59 *
```

* NORTH	* 294.9	*	649.00 *	135.00 *	141.04 *
	* 141.20 *	0.001875 *	3.14 *	206.85 *	74.51 *
	0.33 *				
* NORTH	* 76.8	*	649.00 *	134.00 *	140.53 *
	* 140.73 *	0.002603 *	3.62 *	179.29 *	65.55 *
	0.39 *				
* NORTH	* 0	*	649.00 *	134.00 *	140.41 *
	138.38 *	0.002003 *	2.83 *	229.66 *	102.49 *
	0.33 *				


```

*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
* Lahey F77L-EM/32 version 5.01
* Dodson & Associates, Inc.
* RUN DATE 07/12/02 TIME 11:38:22
*****

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*****
*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
*****

```

```

X X XXXXXXXX XXXXX X
X X X X X XX
X X X X X
XXXXXXXX XXXX X XXXXX X
X X X X X
X X X X X
X X XXXXXXXX XXXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID SISTERS OF ST. JOSEPH
2 ID EXIST. DRAINAGE CONDITIONS AND DEVELOPED
3 ID 2,5,10,25,50,&100 YEAR STORM EVENTS
4 IT 5 07JUN02 0000 300 2000
5 IO 3 0
6 JR PREC 1.0000 1.3143 1.5143 1.7714 2.0000 2.2286
*Diagram
*
7 KK OFF
8 KM OFFSITE DRAINAGE - 241 ac.
9 BA .3766
10 PB 3.5
11 IN 60
12 PC 0 0.011 0.022 0.035 0.048 0.063 0.080 0.098 0.120 0.147
13 PC 0.181 0.235 0.663 0.772 0.820 0.854 0.880 0.902 0.921 0.937
14 PC 0.952 0.965 0.978 0.989 1.000
15 UD .16
16 LS 0 75
*
17 KK SITE
18 KM EXIST. CONDITIONS - 10.13 ac.
19 BA .0158
20 UD .15
21 LS 0 61
*
22 KK C1
23 KM COMBO
24 HC 2 0
*

```

25 KK OFF2
 26 KM OFFSITE DRAINAGE
 27 BA .3766
 28 UD .16
 29 LS 0 75
 *

30 KK SITE
 31 KM DEVELOPED CONDITIONS
 32 BA .0158
 33 UD .15
 34 LS 0 80
 *

35 KK C2
 36 KM COMBO
 37 HC 2 0
 *

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

38 KK POND
 39 KM DETENTION POND
 40 RS 1 ELEV 134
 41 SA .8 .9 1.0 1.3 1.5
 42 SE 134 135 136 140 141
 43 SL 134.63 2.45 .67 .5
 44 SS 136 30 2.8 1.5
 *
 45 ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
 NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

```

  7 OFF
  .
  .
  17 . SITE
  .
  .
  22 C1.....
  .
  .
  25 . OFF2
  .
  .
  30 . . SITE
  .
  .
  35 . C2.....
  .
  .
  38 . V
  . POND
  
```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

 * FLOOD HYDROGRAPH PACKAGE (HEC-1) *
 * MAY 1991 *
 * VERSION 4.0.1E *
 * Lahey F77L-EM/32 version 5.01 *
 * Dodson & Associates, Inc. *
 * RUN DATE 07/12/02 TIME 11:38:22 *

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 * HYDROLOGIC ENGINEERING CENTER *
 * 609 SECOND STREET *
 * DAVIS, CALIFORNIA 95616 *
 * (916) 551-1748 *

SISTERS OF ST. JOSEPH
 EXIST. DRAINAGE CONDITIONS AND DEVELOPED
 2,5,10,25,50,&100 YEAR STORM EVENTS

5 IO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 7JUN 2 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 8JUN 2 ENDING DATE
 NDTIME 0055 ENDING TIME
 ICENT 20 CENTURY MARK
 COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 1.00 1.31 1.51 1.77 2.00 2.23

*** **

7 KK *****
 * *
 * OFF *
 * *

OFFSITE DRAINAGE - 241 ac.

11 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 7JUN 2 STARTING DATE
 JXTIME 0 STARTING TIME

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS
 TAREA 0.38 SUBBASIN AREA

PRECIPITATION DATA

10 PB STORM 3.50 BASIN TOTAL PRECIPITATION

12 P1 INCREMENTAL PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 2.47, TOTAL EXCESS = 2.13

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)				
+ 286.	12.00	70.	70.	22.	21.	21.
		(INCHES)	1.728	2.129	2.129	2.129
		(AC-FT)	35.	43.	43.	43.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.61, TOTAL EXCESS = 2.69

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)				
+ 360.	12.00	88.	88.	27.	26.	26.
		(INCHES)	2.183	2.695	2.695	2.695
		(AC-FT)	44.	54.	54.	54.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.75, TOTAL EXCESS = 3.45

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)				
+ 456.	12.00	113.	113.	35.	34.	34.
		(INCHES)	2.785	3.453	3.453	3.453
		(AC-FT)	56.	69.	69.	69.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.85, TOTAL EXCESS = 4.15

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)				
+ 543.	12.00	135.	135.	42.	40.	40.
		(INCHES)	3.333	4.149	4.149	4.149
		(AC-FT)	67.	83.	83.	83.

CUMULATIVE AREA = 0.38 SQ MI

21 LS SCS LOSS RATE
 STRTL 1.28 INITIAL ABSTRACTION
 CRVNB 61.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

20 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.15 LAG

UNIT HYDROGRAPH
 11 END-OF-PERIOD ORDINATES

15. 39. 34. 17. 9. 4. 2. 1. 1. 0.
 0.

TOTAL RAINFALL = 3.50, TOTAL LOSS = 2.93, TOTAL EXCESS = 0.57

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 3.	12.08	1.	0.	0.	0.	0.
		(INCHES) 0.433	0.573	0.573	0.573	0.573
		(AC-FT) 0.	0.	0.	0.	0.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.50, TOTAL LOSS = 2.93, TOTAL EXCESS = 0.57

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 3.	12.08	1.	0.	0.	0.	0.
		(INCHES) 0.433	0.573	0.573	0.573	0.573
		(AC-FT) 0.	0.	0.	0.	0.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 3.46, TOTAL EXCESS = 1.14

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 6.	12.00	2.	0.	0.	0.	0.
		(INCHES) 0.890	1.136	1.136	1.136	1.136
		(AC-FT) 1.	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 3.75, TOTAL EXCESS = 1.55

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 9.	12.00		2.	1.	1.	1.
		(INCHES)	1.234	1.553	1.553	1.553
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 4.06, TOTAL EXCESS = 2.14

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 12.	12.00		3.	1.	1.	1.
		(INCHES)	1.723	2.140	2.140	2.140
		(AC-FT)	1.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 4.30, TOTAL EXCESS = 2.70

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 15.	12.00		4.	1.	1.	1.
		(INCHES)	2.188	2.702	2.702	2.702
		(AC-FT)	2.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 4.51, TOTAL EXCESS = 3.29

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 19.	12.00		5.	1.	1.	1.
		(INCHES)	2.673	3.293	3.293	3.293
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

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 * *
 22 KK * C1 *
 * *

COMBO

24 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
179.	12.00	43.	13.	13.	13.	
		(INCHES) 1.027	1.272	1.272	1.272	
		(AC-FT) 21.	27.	27.	27.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.31

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
293.	12.00	71.	22.	21.	21.	
		(INCHES) 1.694	2.089	2.089	2.089	
		(AC-FT) 35.	44.	44.	44.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
369.	12.00	90.	28.	27.	27.	
		(INCHES) 2.144	2.649	2.649	2.649	
		(AC-FT) 45.	55.	55.	55.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
468.	12.00	116.	36.	35.	35.	
		(INCHES) 2.742	3.400	3.400	3.400	

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 286.	12.00	(CFS)	70.	22.	21.	21.
		(INCHES)	1.728	2.129	2.129	2.129
		(AC-FT)	35.	43.	43.	43.
CUMULATIVE AREA =			0.38 SQ MI			

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.61, TOTAL EXCESS = 2.69

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 360.	12.00	(CFS)	88.	27.	26.	26.
		(INCHES)	2.183	2.695	2.695	2.695
		(AC-FT)	44.	54.	54.	54.
CUMULATIVE AREA =			0.38 SQ MI			

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.75, TOTAL EXCESS = 3.45

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 456.	12.00	(CFS)	113.	35.	34.	34.
		(INCHES)	2.785	3.453	3.453	3.453
		(AC-FT)	56.	69.	69.	69.
CUMULATIVE AREA =			0.38 SQ MI			

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.85, TOTAL EXCESS = 4.15

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 543.	12.00	(CFS)	135.	42.	40.	40.
		(INCHES)	3.333	4.149	4.149	4.149
		(AC-FT)	67.	83.	83.	83.
CUMULATIVE AREA =			0.38 SQ MI			

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 2.94, TOTAL EXCESS = 4.86

UNIT HYDROGRAPH
11 END-OF-PERIOD ORDINATES

15. 39. 34. 17. 9. 4. 2. 1. 1. 0.

TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.86, TOTAL EXCESS = 1.64

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
9.	12.00	2.	1.	1.	1.	1.
		(INCHES)	1.328	1.636	1.636	1.636
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.86, TOTAL EXCESS = 1.64

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
9.	12.00	2.	1.	1.	1.	1.
		(INCHES)	1.328	1.636	1.636	1.636
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 2.05, TOTAL EXCESS = 2.55

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
14.	12.00	3.	1.	1.	1.	1.
		(INCHES)	2.055	2.547	2.547	2.547
		(AC-FT)	2.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.14, TOTAL EXCESS = 3.16

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
17.	12.00	4.	1.	1.	1.	1.
		(INCHES)	2.535	3.156	3.156	3.156
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

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HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.24, TOTAL EXCESS = 3.96

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
+ 22.	12.00		5.	2.	2.	2.
		(INCHES)	3.169	3.962	3.962	3.962
		(AC-FT)	3.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

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HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.31, TOTAL EXCESS = 4.69

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
+ 25.	12.00		6.	2.	2.	2.
		(INCHES)	3.742	4.694	4.694	4.694
		(AC-FT)	3.	4.	4.	4.

CUMULATIVE AREA = 0.02 SQ MI

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HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 2.36, TOTAL EXCESS = 5.44

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
+ 29.	12.00		7.	2.	2.	2.
		(INCHES)	4.320	5.438	5.438	5.438
		(AC-FT)	4.	5.	5.	5.

CUMULATIVE AREA = 0.02 SQ MI

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*          *
35 KK    * C2 *
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COMBO

37 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
186.	12.00	45.	14.	13.	13.	
		(INCHES) 1.063	1.315	1.315	1.315	
		(AC-FT) 22.	28.	28.	28.	

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.31

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
301.	12.00	73.	23.	22.	22.	
		(INCHES) 1.741	2.146	2.146	2.146	
		(AC-FT) 36.	45.	45.	45.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
377.	12.00	93.	29.	28.	28.	
		(INCHES) 2.197	2.713	2.713	2.713	
		(AC-FT) 46.	57.	57.	57.	

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
477.	12.00	118.	37.	35.	35.	
		(INCHES) 2.800	3.474	3.474	3.474	
		(AC-FT) 59.	73.	73.	73.	

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION C2

FOR PLAN 1, RATIO = 2.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
568.	12.00	141.	44.	42.	42.	
		(INCHES)	3.349	4.171	4.171	4.171
		(AC-FT)	70.	87.	87.	87.
CUMULATIVE AREA =			0.39 SQ MI			

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HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 2.23

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
659.	12.00	165.	52.	50.	50.	
		(INCHES)	3.911	4.885	4.885	4.885
		(AC-FT)	82.	102.	102.	102.
CUMULATIVE AREA =			0.39 SQ MI			

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38 KK * POND *
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DETENTION POND

HYDROGRAPH ROUTING DATA

40 RS	STORAGE ROUTING					
	NSTPS	1	NUMBER OF SUBREACHES			
	ITYP	ELEV	TYPE OF INITIAL CONDITION			
	RSVRIC	134.00	INITIAL CONDITION			
	X	0.00	WORKING R AND D COEFFICIENT			
41 SA	AREA	0.8	0.9	1.0	1.3	1.5
42 SE	ELEVATION	134.00	135.00	136.00	140.00	141.00
43 SL	LOW-LEVEL OUTLET					
	ELEVL	134.63	ELEVATION AT CENTER OF OUTLET			
	CAREA	2.45	CROSS-SECTIONAL AREA			
	COQL	0.67	COEFFICIENT			
	EXPL	0.50	EXPONENT OF HEAD			
44 SS	SPILLWAY					
	CREL	136.00	SPILLWAY CREST ELEVATION			
	SPWID	30.00	SPILLWAY WIDTH			
	COQW	2.80	WEIR COEFFICIENT			
	EXPW	1.50	EXPONENT OF HEAD			

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	0.85	1.80	6.39	7.78
ELEVATION	134.00	135.00	136.00	140.00	141.00

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	0.00	0.00	11.76	12.17	12.61	13.09	13.60	14.15	14.76	15.41
ELEVATION	134.00	134.63	135.43	135.48	135.55	135.62	135.70	135.79	135.89	136.00
OUTFLOW	17.34	26.13	47.04	85.28	146.08	234.68	356.34	516.32	719.90	972.37
ELEVATION	136.07	136.23	136.49	136.85	137.30	137.85	138.49	139.23	140.07	141.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.52	0.85	1.24	1.30	1.36	1.42	1.50	1.59	1.69
OUTFLOW	0.00	0.00	8.01	11.76	12.17	12.61	13.09	13.60	14.15	14.75
ELEVATION	134.00	134.63	135.00	135.43	135.48	135.55	135.62	135.70	135.79	135.89
STORAGE	1.80	1.87	2.03	2.30	2.67	3.16	3.77	4.52	5.41	6.39
OUTFLOW	15.41	17.34	26.13	47.04	85.28	146.08	234.68	356.34	516.32	702.51
ELEVATION	136.00	136.07	136.23	136.49	136.85	137.30	137.85	138.49	139.23	140.00
STORAGE	6.47	7.78								
OUTFLOW	719.90	972.38								
ELEVATION	140.07	141.00								

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW				
+	(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR	
	179.	12.08	(CFS)	42.	14.	13.	13.
			(INCHES)	1.005	1.287	1.287	1.287
			(AC-FT)	21.	27.	27.	27.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE				
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.92-HR	
	3.	12.08		2.	1.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE				
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR	
	137.51	12.08		136.32	134.97	134.93	134.93

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.31

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW				
+	(CFS)	(HR)	6-HR	24-HR	72-HR	24.92-HR	
	292.	12.08	(CFS)	72.	22.	21.	21.
			(INCHES)	1.694	2.113	2.113	2.113
			(AC-FT)	35.	44.	44.	44.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE				
+	(AC-FT)	(HR)	6-HR	24-HR	72-HR	24.92-HR	
	4.	12.08		2.	1.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE				
+	(FEET)	(HR)	6-HR	24-HR	72-HR	24.92-HR	

138.15 12.08 136.61 135.22 135.18 135.18

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
367.	12.08	(CFS)	91.	28.	27.	27.
		(INCHES)	2.162	2.674	2.674	2.674
		(AC-FT)	45.	56.	56.	56.

PEAK STORAGE + (AC-FT)	TIME (HR)		MAXIMUM AVERAGE STORAGE			24.92-HR
			6-HR	24-HR	72-HR	
5.	12.08		3.	1.	1.	1.

PEAK STAGE + (FEET)	TIME (HR)		MAXIMUM AVERAGE STAGE			24.92-HR
			6-HR	24-HR	72-HR	
138.54	12.08		136.76	135.36	135.31	135.31

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
465.	12.00	(CFS)	118.	36.	35.	35.
		(INCHES)	2.785	3.424	3.424	3.424
		(AC-FT)	58.	72.	72.	72.

PEAK STORAGE + (AC-FT)	TIME (HR)		MAXIMUM AVERAGE STORAGE			24.92-HR
			6-HR	24-HR	72-HR	
5.	12.00		3.	1.	1.	1.

PEAK STAGE + (FEET)	TIME (HR)		MAXIMUM AVERAGE STAGE			24.92-HR
			6-HR	24-HR	72-HR	
138.99	12.00		136.95	135.50	135.45	135.45

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 2.00

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
556.	12.00	(CFS)	141.	43.	42.	42.
		(INCHES)	3.346	4.118	4.118	4.118
		(AC-FT)	70.	86.	86.	86.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
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ROUTED TO

+	POND	0.39	1	FLOW	179.	292.	367.	465.	556.	646.
				TIME	12.08	12.08	12.08	12.00	12.00	12.00
				** PEAK STAGES IN FEET **						
			1	STAGE	137.51	138.15	138.54	138.99	139.39	139.77
				TIME	12.08	12.08	12.08	12.00	12.00	12.00

*** NORMAL END OF HEC-1 ***

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* FLOOD HYDROGRAPH PACKAGE (HEC-1)
* MAY 1991
* VERSION 4.0.1E
* Lahey F77L-EM/32 version 5.01
* Dodson & Associates, Inc.
* RUN DATE 07/12/02 TIME 11:38:22
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*
* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
* 609 SECOND STREET
* DAVIS, CALIFORNIA 95616
* (916) 551-1748
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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1

HEC-1 INPUT

PAGE 1

```

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1 ID SISTERS OF ST. JOSEPH
2 ID EXIST. DRAINAGE CONDITIONS AND DEVELOPED
3 ID 2,5,10,25,50,&100 YEAR STORM EVENTS
4 IT 5 07JUN02 0000 300 2000
5 IO 3 0
6 JR PREC 1.0000 1.3143 1.5143 1.7714 2.0000 2.2286
*Diagram
*
7 KK OFF
8 KM OFFSITE DRAINAGE - 241 ac.
9 BA .3766
10 PB 3.5
11 IN 60
12 PC 0 0.011 0.022 0.035 0.048 0.063 0.080 0.098 0.120 0.147
13 PC 0.181 0.235 0.663 0.772 0.820 0.854 0.880 0.902 0.921 0.937
14 PC 0.952 0.965 0.978 0.989 1.000
15 UD .16
16 LS 0 75
*
17 KK SITE
18 KM EXIST. CONDITIONS - 10.13 ac.
19 BA .0158
20 UD .15
21 LS 0 61
*
22 KK C1
23 KM COMBO
24 HC 2 0
*

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25      KK  OFF2
26      KM  OFFSITE DRAINAGE
27      BA  .3766
28      UD  .16
29      LS  0      75
      *

30      KK  SITE
31      KM  DEVELOPED CONDITIONS
32      BA  .0158
33      UD  .15
34      LS  0      80
      *

35      KK  C2
36      KM  COMBO
37      HC  2      0
      *

```

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

```

38      KK  POND
39      KM  DETENTION POND
40      RS  1  ELEV  134
41      SA  .8  .9  1.0  1.3  1.5
42      SE  134 135 136 140 141
43      SL 134.63 2.45 .67 .5
44      SS  136 30 2.8 1.5
      *
45      ZZ

```

SCHEMATIC DIAGRAM OF STREAM NETWORK

```

INPUT LINE (V) ROUTING (--->) DIVERSION OR PUMP FLOW
NO. (.) CONNECTOR (<---) RETURN OF DIVERTED OR PUMPED FLOW

7      OFF
      .
17     .      SITE
      .
22     C1.....
      .
25     .      OFF2
      .
30     .      .      SITE
      .
35     .      C2.....
      .      V
      .      V
38     .      POND

```

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

```

*****
*      FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*      MAY 1991 *
*      VERSION 4.0.1E *
*      Lahey F77L-EM/32 version 5.01 *
*      Dodson & Associates, Inc. *
*      RUN DATE 07/12/02 TIME 11:38:22 *
*****

```

```

*****
*      U.S. ARMY CORPS OF ENGINEERS *
*      HYDROLOGIC ENGINEERING CENTER *
*      609 SECOND STREET *
*      DAVIS, CALIFORNIA 95616 *
*      (916) 551-1748 *
*****

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SISTERS OF ST. JOSEPH
 EXIST. DRAINAGE CONDITIONS AND DEVELOPED
 2,5,10,25,50,&100 YEAR STORM EVENTS

5 IO OUTPUT CONTROL VARIABLES
 IPRNT 3 PRINT CONTROL
 IPLOT 0 PLOT CONTROL
 QSCAL 0. HYDROGRAPH PLOT SCALE

IT HYDROGRAPH TIME DATA
 NMIN 5 MINUTES IN COMPUTATION INTERVAL
 IDATE 7JUN 2 STARTING DATE
 ITIME 0000 STARTING TIME
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES
 NDDATE 8JUN 2 ENDING DATE
 NDTIME 0055 ENDING TIME
 ICENT 20 CENTURY MARK
 COMPUTATION INTERVAL 0.08 HOURS
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS
 DRAINAGE AREA SQUARE MILES
 PRECIPITATION DEPTH INCHES
 LENGTH, ELEVATION FEET
 FLOW CUBIC FEET PER SECOND
 STORAGE VOLUME ACRE-FEET
 SURFACE AREA ACRES
 TEMPERATURE DEGREES FAHRENHEIT

JP MULTI-PLAN OPTION
 NPLAN 1 NUMBER OF PLANS

JR MULTI-RATIO OPTION
 RATIOS OF PRECIPITATION
 1.00 1.31 1.51 1.77 2.00 2.23

*** **

 * *
 7 KK * OFF *
 * *

OFFSITE DRAINAGE - 241 ac.

11 IN TIME DATA FOR INPUT TIME SERIES
 JXMIN 60 TIME INTERVAL IN MINUTES
 JXDATE 7JUN 2 STARTING DATE
 JXTIME 0 STARTING TIME

SUBBASIN RUNOFF DATA

9 BA SUBBASIN CHARACTERISTICS
 TAREA 0.38 SUBBASIN AREA

PRECIPITATION DATA

10 PB STORM 3.50 BASIN TOTAL PRECIPITATION

12 PI INCREMENTAL PRECIPITATION PATTERN
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00
 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 2.47, TOTAL EXCESS = 2.13

PEAK FLOW	TIME		6-HR	24-HR	72-HR	24.92-HR
(CFS)	(HR)	(CFS)				
+ 286.	12.00		70.	22.	21.	21.
		(INCHES)	1.728	2.129	2.129	2.129
		(AC-FT)	35.	43.	43.	43.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.61, TOTAL EXCESS = 2.69

PEAK FLOW	TIME		6-HR	24-HR	72-HR	24.92-HR
(CFS)	(HR)	(CFS)				
+ 360.	12.00		88.	27.	26.	26.
		(INCHES)	2.183	2.695	2.695	2.695
		(AC-FT)	44.	54.	54.	54.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.75, TOTAL EXCESS = 3.45

PEAK FLOW	TIME		6-HR	24-HR	72-HR	24.92-HR
(CFS)	(HR)	(CFS)				
+ 456.	12.00		113.	35.	34.	34.
		(INCHES)	2.785	3.453	3.453	3.453
		(AC-FT)	56.	69.	69.	69.

CUMULATIVE AREA = 0.38 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION OFF
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.85, TOTAL EXCESS = 4.15

PEAK FLOW	TIME		6-HR	24-HR	72-HR	24.92-HR
(CFS)	(HR)	(CFS)				
+ 543.	12.00		135.	42.	40.	40.
		(INCHES)	3.333	4.149	4.149	4.149
		(AC-FT)	67.	83.	83.	83.

CUMULATIVE AREA = 0.38 SQ MI

21 LS SCS LOSS RATE
 STRTL 1.28 INITIAL ABSTRACTION
 CRVNR 61.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

20 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.15 LAG

UNIT HYDROGRAPH
 11 END-OF-PERIOD ORDINATES

15. 39. 34. 17. 9. 4. 2. 1. 1. 0.
 0.

TOTAL RAINFALL = 3.50, TOTAL LOSS = 2.93, TOTAL EXCESS = 0.57

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 3.	12.08	1.	0.	0.	0.	0.
		(INCHES)	0.433	0.573	0.573	0.573
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.50, TOTAL LOSS = 2.93, TOTAL EXCESS = 0.57

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 3.	12.08	1.	0.	0.	0.	0.
		(INCHES)	0.433	0.573	0.573	0.573
		(AC-FT)	0.	0.	0.	0.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 3.46, TOTAL EXCESS = 1.14

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	24.92-HR
+ 6.	12.00	2.	0.	0.	0.	0.
		(INCHES)	0.890	1.136	1.136	1.136
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
 FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 3.75, TOTAL EXCESS = 1.55

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 9.	12.00	(CFS)	2.	1.	1.	1.
		(INCHES)	1.234	1.553	1.553	1.553
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 4.06, TOTAL EXCESS = 2.14

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 12.	12.00	(CFS)	3.	1.	1.	1.
		(INCHES)	1.723	2.140	2.140	2.140
		(AC-FT)	1.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 4.30, TOTAL EXCESS = 2.70

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 15.	12.00	(CFS)	4.	1.	1.	1.
		(INCHES)	2.188	2.702	2.702	2.702
		(AC-FT)	2.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 4.51, TOTAL EXCESS = 3.29

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 19.	12.00	(CFS)	5.	1.	1.	1.
		(INCHES)	2.673	3.293	3.293	3.293
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

*** **

 * *
 22 KK * C1 *
 * *

COMBO

24 HC HYDROGRAPH COMBINATION
 ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
179.	12.00	43.	13.	13.	13.	
		(INCHES) 1.027	1.272	1.272	1.272	
		(AC-FT) 21.	27.	27.	27.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.31

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
293.	12.00	71.	22.	21.	21.	
		(INCHES) 1.694	2.089	2.089	2.089	
		(AC-FT) 35.	44.	44.	44.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
369.	12.00	90.	28.	27.	27.	
		(INCHES) 2.144	2.649	2.649	2.649	
		(AC-FT) 45.	55.	55.	55.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C1
 FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			24.92-HR
			6-HR	24-HR	72-HR	
468.	12.00	116.	36.	35.	35.	
		(INCHES) 2.742	3.400	3.400	3.400	

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)		24-HR	72-HR		
+ 286.	12.00	70.	22.	21.	21.		21.
		(INCHES) 1.728	2.129	2.129	2.129		2.129
		(AC-FT) 35.	43.	43.	43.		43.
CUMULATIVE AREA =			0.38 SQ MI				

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.61, TOTAL EXCESS = 2.69

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)		24-HR	72-HR		
+ 360.	12.00	88.	27.	26.	26.		26.
		(INCHES) 2.183	2.695	2.695	2.695		2.695
		(AC-FT) 44.	54.	54.	54.		54.
CUMULATIVE AREA =			0.38 SQ MI				

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.75, TOTAL EXCESS = 3.45

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)		24-HR	72-HR		
+ 456.	12.00	113.	35.	34.	34.		34.
		(INCHES) 2.785	3.453	3.453	3.453		3.453
		(AC-FT) 56.	69.	69.	69.		69.
CUMULATIVE AREA =			0.38 SQ MI				

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.85, TOTAL EXCESS = 4.15

PEAK FLOW	TIME		6-HR	MAXIMUM AVERAGE FLOW	24-HR	72-HR	24.92-HR
+ (CFS)	(HR)	(CFS)		24-HR	72-HR		
+ 543.	12.00	135.	42.	40.	40.		40.
		(INCHES) 3.333	4.149	4.149	4.149		4.149
		(AC-FT) 67.	83.	83.	83.		83.
CUMULATIVE AREA =			0.38 SQ MI				

*** *** *** *** ***

HYDROGRAPH AT STATION OFF2
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 2.94, TOTAL EXCESS = 4.86

UNIT HYDROGRAPH
11 END-OF-PERIOD ORDINATES

15. 39. 34. 17. 9. 4. 2. 1. 1. 0.

TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.86, TOTAL EXCESS = 1.64

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
9.	12.00	2.	1.	1.	1.	1.
		(INCHES)	1.328	1.636	1.636	1.636
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 3.50, TOTAL LOSS = 1.86, TOTAL EXCESS = 1.64

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
9.	12.00	2.	1.	1.	1.	1.
		(INCHES)	1.328	1.636	1.636	1.636
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.02 SQ MI

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.31

TOTAL RAINFALL = 4.60, TOTAL LOSS = 2.05, TOTAL EXCESS = 2.55

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
14.	12.00	3.	1.	1.	1.	1.
		(INCHES)	2.055	2.547	2.547	2.547
		(AC-FT)	2.	2.	2.	2.

CUMULATIVE AREA = 0.02 SQ MI

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.51

TOTAL RAINFALL = 5.30, TOTAL LOSS = 2.14, TOTAL EXCESS = 3.16

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	MAXIMUM AVERAGE FLOW 24-HR	72-HR	24.92-HR
17.	12.00	4.	1.	1.	1.	1.
		(INCHES)	2.535	3.156	3.156	3.156
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 1.77

TOTAL RAINFALL = 6.20, TOTAL LOSS = 2.24, TOTAL EXCESS = 3.96

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR (INCHES) (AC-FT)	MAXIMUM AVERAGE FLOW 24-HR 3.962	72-HR 3.962	24.92-HR 3.962
22.	12.00	5.	3.169	2.	2.	2.
			3.	3.	3.	3.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.00

TOTAL RAINFALL = 7.00, TOTAL LOSS = 2.31, TOTAL EXCESS = 4.69

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR (INCHES) (AC-FT)	MAXIMUM AVERAGE FLOW 24-HR 4.694	72-HR 4.694	24.92-HR 4.694
25.	12.00	6.	3.742	2.	2.	2.
			3.	4.	4.	4.

CUMULATIVE AREA = 0.02 SQ MI

*** **

HYDROGRAPH AT STATION SITE
FOR PLAN 1, RATIO = 2.23

TOTAL RAINFALL = 7.80, TOTAL LOSS = 2.36, TOTAL EXCESS = 5.44

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR (INCHES) (AC-FT)	MAXIMUM AVERAGE FLOW 24-HR 5.438	72-HR 5.438	24.92-HR 5.438
29.	12.00	7.	4.320	2.	2.	2.
			4.	5.	5.	5.

CUMULATIVE AREA = 0.02 SQ MI

*** **

* *
35 KK * C2 *
* *

COMBO

37 HC HYDROGRAPH COMBINATION

ICOMP 2 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.00

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
186.	12.00	45.	14.	13.	13.	
		(INCHES) 1.063	1.315	1.315	1.315	
		(AC-FT) 22.	28.	28.	28.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.31

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
301.	12.00	73.	23.	22.	22.	
		(INCHES) 1.741	2.146	2.146	2.146	
		(AC-FT) 36.	45.	45.	45.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
377.	12.00	93.	29.	28.	28.	
		(INCHES) 2.197	2.713	2.713	2.713	
		(AC-FT) 46.	57.	57.	57.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)	(CFS)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
477.	12.00	118.	37.	35.	35.	
		(INCHES) 2.800	3.474	3.474	3.474	
		(AC-FT) 59.	73.	73.	73.	

CUMULATIVE AREA = 0.39 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION C2

FOR PLAN 1, RATIO = 2.00

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
+ 568.	12.00	(CFS)	141.	44.	42.	42.
		(INCHES)	3.349	4.171	4.171	4.171
		(AC-FT)	70.	87.	87.	87.

CUMULATIVE AREA = 0.39 SQ MI

*** **

HYDROGRAPH AT STATION C2
FOR PLAN 1, RATIO = 2.23

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
+ 659.	12.00	(CFS)	165.	52.	50.	50.
		(INCHES)	3.911	4.885	4.885	4.885
		(AC-FT)	82.	102.	102.	102.

CUMULATIVE AREA = 0.39 SQ MI

38 KK *****
* *
* POND *
* *

DETENTION POND

HYDROGRAPH ROUTING DATA

40 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 134.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

41 SA AREA 0.8 0.9 1.0 1.3 1.5

42 SE ELEVATION 134.00 135.00 136.00 140.00 141.00

43 SL LOW-LEVEL OUTLET
ELEV 134.63 ELEVATION AT CENTER OF OUTLET
CAREA 2.45 CROSS-SECTIONAL AREA
COQL 0.67 COEFFICIENT
EXPL 0.50 EXPONENT OF HEAD

44 SS SPILLWAY
CREL 136.00 SPILLWAY CREST ELEVATION
SPWID 30.00 SPILLWAY WIDTH
COQW 2.80 WEIR COEFFICIENT
EXPW 1.50 EXPONENT OF HEAD

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	0.85	1.80	6.39	7.78
ELEVATION	134.00	135.00	136.00	140.00	141.00

COMPUTED OUTFLOW-ELEVATION DATA

OUTFLOW	0.00	0.00	11.76	12.17	12.61	13.09	13.60	14.15	14.76	15.41
ELEVATION	134.00	134.63	135.43	135.48	135.55	135.62	135.70	135.79	135.89	136.00
OUTFLOW	17.34	26.13	47.04	85.28	146.08	234.68	356.34	516.32	719.90	972.37
ELEVATION	136.07	136.23	136.49	136.85	137.30	137.85	138.49	139.23	140.07	141.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.52	0.85	1.24	1.30	1.36	1.42	1.50	1.59	1.69
OUTFLOW	0.00	0.00	8.01	11.76	12.17	12.61	13.09	13.60	14.15	14.75
ELEVATION	134.00	134.63	135.00	135.43	135.48	135.55	135.62	135.70	135.79	135.89
STORAGE	1.80	1.87	2.03	2.30	2.67	3.16	3.77	4.52	5.41	6.39
OUTFLOW	15.41	17.34	26.13	47.04	85.28	146.08	234.68	356.34	516.32	702.51
ELEVATION	136.00	136.07	136.23	136.49	136.85	137.30	137.85	138.49	139.23	140.00
STORAGE	6.47	7.78								
OUTFLOW	719.90	972.38								
ELEVATION	140.07	141.00								

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.00

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 179.	12.08	(CFS)	42.	14.	13.	13.
		(INCHES)	1.005	1.287	1.287	1.287
		(AC-FT)	21.	27.	27.	27.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 3.	12.08		2.	1.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 137.51	12.08		136.32	134.97	134.93	134.93

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.31

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
+ (CFS)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 292.	12.08	(CFS)	72.	22.	21.	21.
		(INCHES)	1.694	2.113	2.113	2.113
		(AC-FT)	35.	44.	44.	44.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
+ (AC-FT)	(HR)		6-HR	24-HR	72-HR	24.92-HR
+ 4.	12.08		2.	1.	1.	1.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
+ (FEET)	(HR)		6-HR	24-HR	72-HR	24.92-HR

138.15 12.08 136.61 135.22 135.18 135.18

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.51

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
367.	12.08	(CFS)	91.	28.	27.	27.
		(INCHES)	2.162	2.674	2.674	2.674
		(AC-FT)	45.	56.	56.	56.

PEAK STORAGE + (AC-FT)	TIME (HR)		MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	24.92-HR
5.	12.08		3.	1.	1.	1.

PEAK STAGE + (FEET)	TIME (HR)		MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	24.92-HR
138.54	12.08		136.76	135.36	135.31	135.31

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 1.77

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
465.	12.00	(CFS)	118.	36.	35.	35.
		(INCHES)	2.785	3.424	3.424	3.424
		(AC-FT)	58.	72.	72.	72.

PEAK STORAGE + (AC-FT)	TIME (HR)		MAXIMUM AVERAGE STORAGE			
			6-HR	24-HR	72-HR	24.92-HR
5.	12.00		3.	1.	1.	1.

PEAK STAGE + (FEET)	TIME (HR)		MAXIMUM AVERAGE STAGE			
			6-HR	24-HR	72-HR	24.92-HR
138.99	12.00		136.95	135.50	135.45	135.45

CUMULATIVE AREA = 0.39 SQ MI

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HYDROGRAPH AT STATION POND
FOR PLAN 1, RATIO = 2.00

PEAK FLOW + (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	24.92-HR
556.	12.00	(CFS)	141.	43.	42.	42.
		(INCHES)	3.346	4.118	4.118	4.118
		(AC-FT)	70.	86.	86.	86.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
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ROUTED TO

+	POND	0.39	1	FLOW	179.	292.	367.	465.	556.	646.
				TIME	12.08	12.08	12.08	12.00	12.00	12.00
				** PEAK STAGES IN FEET **						
			1	STAGE	137.51	138.15	138.54	138.99	139.39	139.77
				TIME	12.08	12.08	12.08	12.00	12.00	12.00

*** NORMAL END OF HEC-1 ***

Sisclose

CLOSURE - SISTERS OF ST. JOSEPH 5TH ADDITION

PT 01 North: 5804.5702 East : 7181.2616
Line Course: N 00-02-00 E Length: 632.1600
PT 02 North: 6436.7301 East : 7181.6294
Line Course: N 89-52-26 E Length: 513.9800
PT 03 North: 6437.8614 East : 7695.6081
Line Course: S 00-00-56 W Length: 512.1700
PT 04 North: 5925.6914 East : 7695.4691
Line Course: N 89-52-31 E Length: 250.3100
PT 05 North: 5926.2363 East : 7945.7785
Line Course: S 40-07-05 W Length: 174.7400
PT 06 North: 5792.6094 East : 7833.1822
Curve Length: 286.6953 Radius: 735.0000
Delta: 22-20-56 Tangent: 145.1926
Chord: 284.8800 Course: S 28-56-37 W
Course In: S 49-52-55 E Course Out: N 72-13-51 W
RP North: 5319.0014 East : 8395.2502
PT 07 End North: 5543.3108 East : 7695.3143
Line Course: N 00-01-50 E Length: 31.3800
PT 08 North: 5574.6908 East : 7695.3311
Line Course: N 00-01-50 E Length: 201.0000
PT 09 North: 5775.6907 East : 7695.4382
Line Course: N 00-00-00 E Length: 30.0000
PT 10 North: 5805.6907 East : 7695.4382
Line Course: S 89-52-31 W Length: 514.1700
PT 01 North: 5804.5715 East : 7181.2695