

DRAINAGE PLAN AND SUPPORTING CALCULATIONS

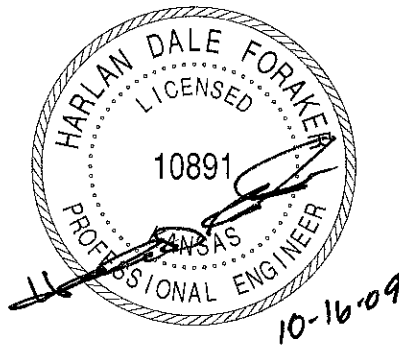
**FOR
DAN SCHMIDT 2nd ADDITION
AN ADDITION TO
WICHITA, KANSAS**

**PREPARED FOR:
SAVOY COMPANY, P.A.
433 SOUTH HYDRAULIC
WICHITA, KS 67211**

OCTOBER 16, 2009

PREPARED BY:

**CERTIFIED ENGINEERING DESIGN, P.A.
810 WEST DOUGLAS, SUITE C
WICHITA, KANSAS 67203-6105
(316)262-8808 PHONE
(316)262-1669 FAX**





**Public Works, Engineering Division
Final Drainage Plan Submittal Checklist**

Reviewer: _____	Date: <u>Oct. 20, 2009</u>
Subdivision Name: <u>Dan Schmidt 2nd Add.</u>	Location: <u>5439 S Hydraulic</u>
Total Land Area Of Ownership: <u>5.06</u> Acres	
Type: _____ Residential _____ Commercial _____ Industrial _____ Recreation _____ Municipal <u>X</u> Other <u>PUD #32</u>	
Applicant: <u>Savoy Company</u>	Contact: <u>Mark Savoy, LS</u> Phone #: <u>265-0005</u>
Engineer: <u>Certified Engineering Design</u>	Contact: <u>Harlan Foreber, PE</u> Phone #: <u>262-8808</u>

Please check the appropriate box:

I = Included; NA = Non-Applicable; R= Required prior to development
(If "NA" is checked, an explanation must be entered)

Tab 1. Project Narrative	Applicant			Engr	
	I	NA	Explanation / Location in Plan	I	NA
A. Site Location Map, using USGS Map					
B. Discussion of development, existing conditions, and proposed impacts on stormwater, wetland, riparian, and flood plain	X				
C. Discussion of offsite conditions	X				
D. Summary of runoff calculations (pre/post development) No increase in peak discharge for all storm series	X				
E. Narrative description of the type and function of the permanent best management practices that are incorporated into the site design		X			
F. Copy of the plat	X				
G. Preliminary grading plan (The final grading plan shall be sealed, signed and dated prior to Engineering receiving the final sanitary sewer plans. One plan sheet and PDF shall be submitted to the Subdivision Engineer.)		X			
H. Professional Engineer seal, signature and date on cover of report	X				
I. CD of drainage plan in PDF format (one file) and one paper copy bound with this checklist included behind the cover					

Tab 2. Existing Conditions Runoff Calculations	Applicant			Engr	
	I	NA	Explanation / Location in Plan	I	NA
A. Copy of applicable orthophoto showing proposed project boundaries (preferable in color)	X				
B. Runoff Method (Rational, Hydrograph Method, or other approved methods by Engineering)	X				
C. Existing topography (no greater than 2-foot contours, 1-foot recommend)	X				
D. Total Site Area and Total Impervious Area (acres)	X				
E. Benchmarks used for site control	X				
F. Streams, creeks, and waterway labeled		X			
G. Predominant soils from USDA soil surveys, and/or on site soil borings	X				
H. Location and boundaries of natural features such as wetlands, lakes, and ponds with the normal water elevation noted		X			
I. Location of existing roads, buildings, parking lots and other impervious areas.	X				



J. Location of existing utilities (e.g., water, sewer, gas, electric) and easements	X				
K. Location of existing conveyance systems such as storm drains, inlets, catch basins, channels, swales, and areas of overland flow	X				
L. Flow paths	X				
M. Location and dimensions of existing channels, bridges or culvert crossings	X				
N. Existing conditions hydrologic analysis for runoff rates, volumes and velocities showing methodologies used and supporting calculations (2, 5, 10, 25 & 100 year, 24-hour storm events) or Critical Duration	X				
O. Assumed pre-developed runoff curve numbers	X				
P. Existing time of concentrations used in calculations	X				
Q. Evaluate immediate downstream drainage capacity, not to exceed more than 0.25 miles downstream of site		X			
R. Existing structural elevations (e.g., invert of pipes, manholes, etc.)		X			
S. Cross-section data for open channels		X			
T. Ground water elevations, if applicable		X			

Tab 3. Post-Development Hydrologic Analysis	Applicant			Engr	
	I	NA	Explanation / Location in Plan	I	NA
A. Proposed (post-development) conditions hydrologic and hydraulic analysis for runoff rates, volumes, HGL, and velocities showing the methodologies used and supporting calculations for all applicable design storms (2, 5, 10, 25 & 100 year, 24-hour storm events)	X				
B. Proposed time of concentrations used in calculations	X				
C. Assumed post-developed runoff curve numbers	X				
D. Proposed contours for detention facilities (to equal area used in outlet rating curves)		X			
E. Preliminary sizing calculations for stormwater controls including contributing drainage area, storage, and outlet configuration		X			
F. Stage-storage-discharge or outlet rating curves and inflow and outflow hydrographs for storage facilities		X			
G. Final analysis of potential upstream/downstream impact/effects of project, where necessary		X			
H. Existing and proposed structural elevations (e.g., invert of pipes, manholes, etc.)		X			
I. Design water surface elevations and normal pool elevation for ponds.		X			
J. Typical detail for outlet structures, embankments, spillways, grade control structures, conveyance channels, etc. To include height, width, elevation, and/or diameter.		X			
K. Proposed limits of clearing and grading		X			
L. Location of existing and proposed roads, buildings, parking lots and other impervious areas.	X				
M. Location of existing and proposed utilities (e.g., water, sewer) and easements	X				
N. Location of existing and proposed conveyance systems such as storm drains, inlets, catch basins, channels, swales, and areas of overland flow	X				
O. Preliminary location and dimensions of proposed channel modifications, such as bridge or culvert crossings		X			



P. Preliminary selection and location of stormwater controls		X		
Q. Emergency overflow structure's flow path		X		
R. Detention facility provides one-foot of freeboard above the HWL and emergency outfall shown (top of berm elevation shown)		X		
S. The 100-year 24-hour HWL delineated on the plan for detention pond		X		
T. Lowest opening elevations table on the plat for structures located adjacent to channels or ponds		X		
U. Stormwater Management Facilities located within a Reserve		X		
V. Maintenance responsibility of stormwater management facility shall be specified in the plat's text. (e.g. HOA, Lot Owners Association, or lot)		X		
W. Off-site drainage easements or agreements required, where necessary		X		

Tab 4. Floodplain Submittal	I	NA	Applicant Explanation / Location in Plan	Engr	
				I	NA
A. Provide source of flood profile		X			
B. Nearest base flood elevations		X			
C. Delineation of pre-developed regulatory floodplain/floodway limits		X			
D. Delineation of post-developed regulatory floodplain and floodway limits		^			
E. Floodplain boundary determination per elevation (project limits shown)		X			
F. Provide source of floodway data table and discharges		X			
G. Provide all hydrologic and hydraulic study information for site-specific floodplain studies, unnumbered Zone A area elevation determinations and flood plain map revisions or required permits		X			
H. Provide regulatory floodway and four natural profile models (10,50,100, and 500-yr) for existing and future watershed conditions		X			
I. Location of floodplain/floodway limits and relationship of site to upstream/downstream properties (floodplain limits to be per elevation and scaled location)		X			
J. Flood plains and floodways located within a Reserve, where necessary		X			

Tab 5. Federal, State and Local Permits (to be provided prior to construction unless otherwise specified)	I/R	NA	Applicant Explanation / Location in Plan	Engr	
				I/R	NA
A. US Army Corps of Engineers - Regulatory program permits (404 water quality certification)		X			
B. Kansas Department of Agriculture - Division of Water Resources Permits (Stream Obstruction, Channel Change, Flood Plain Fill, Levee, Water Appropriations, Dam safety permit, etc.)		X			
C. Federal Emergency Management Agency (FEMA) Letter of Map Changes (LOMA, LOMR, LOMR-f, CLOMR, etc.) Shall be included and approved when project modifies the limits of the floodway.		X			
D. Kansas Department of Transportation		X			
E. Sedgwick County Right-of-way Permit		X			

Dan Schmidt 2nd Addition Drainage Plan(Con't)
Mr. Scott Lindebak
October 16, 2009

CERTIFIED ENGINEERING DESIGN, P.A
810 West Douglas, Suite C
Wichita, KS 67203-6105
(316)262-8808 Office
(316)262-1669 Fax

LETTER OF TRANSMITTAL

DATE: October 16, 2009

TO: Mr. Scott Lindebak, P.E.
Division of Public Works
1155 South Seneca
Wichita, KS 67202

RE: Drainage Plan
Dan Schmidt 2nd Addition
Wichita, KS

FROM: Harlan D. Foraker, P.E.

I. PROJECT NARRATIVE

The site is located on the west side of Hydraulic Avenue approximately a quarter mile north of 55th Street South. The address of the existing business is 5439 S. Hydraulic. The site currently consists of several greenhouses and nursery structures with extensive parking in the east portion of the parcel. The SCS soil present is Canadian-Waldeck fine sandy loam which is a SCS Class B Soil. The site has residential development to the north and west with undeveloped ground to the south. There is a trailer park east across Hydraulic from the site.

II. EXISTING AND PROPOSED CONDITIONS RUNOFF CALCULATIONS

The rational method will be used to determine the peak discharges from the study area. Rational 'C' Factors were assigned to the existing site and proposed improvements from "Interim Drainage and Storm Sewer Policy for Design Criteria and Documentation" for the City of Wichita, Kansas. Rainfall Intensity tables from the same policy were utilized to determine the rainfall intensity for the 2, 5, 10, 25, and 100 year design storms.

The Soil Conservation Service TR-55 manual was used to compute the Time of Concentration for the drainage subareas. A design assumption was made as follows: that the minimum subarea time of concentration is 15 minutes.

Dan Schmidt 2nd Addition Drainage Plan(Con't)
 Mr. Scott Lindebak
 October 16, 2009

Soil types were determined from the Natural Resources Conservation Service's Soil Survey web site. The SCS soil present is Canadian-Waldeck fine sandy loam (0 to 1 percent) which is a SCS Class B Soil.

The developed drainage subareas have been delineated on the 1" = 50' site and topographic mapping survey performed for this site.

Design Storm Events Evaluated: 2, 5, 10, 25, and 100 yr. storm events

The runoff calculations for the property have been completed utilizing the 5 storm events.

The existing site was a nursery with green houses. The proposed 6 lot addition contains several green houses, out buildings, and parking areas. The parcel has 2 undefined ridgelines. The north south ridgeline is approximately 160 feet east of the west property line. The east west ridgeline starts at the north south ridgeline and runs west. This divides the property into 3 drainage subbasins. The northwest subbasin flows to the west where the flow is intercepted by a 2'x2' drop inlet. The southwest subbasin flows to the south where it is intercepted by a slotted top manhole. The east subbasin flows to the east where it is intercepted by Hydraulic and flows immediately into a curb inlet.

NORTHWEST DRAINAGE SUBBASIN

The following table summarizes the peak discharge for the northwest drainage subbasin which includes the west portions of Lots 4 and 5, and the northwest portion of Lot 6; as this area is already developed, the existing condition is the proposed condition also. The drainage sub-basin includes 1.02 acres within the proposed Dan Schmidt 2nd Addition.

EXISTING AND PROPOSED PEAK RUNOFF FOR THE NORTHWEST DRAINAGE SUBBASIN

Description	C	Tc	I	Area	Q(cfs)
Existing Drainage Area(2 yr.)	.24	31	2.59	1.02	0.6
Existing Drainage Area(5 yr.)	.26	31	3.26	1.02	0.9
Existing Drainage Area(10 yr.)	.32	31	3.72	1.02	1.2
Existing Drainage Area(25 yr.)	.40	31	4.39	1.02	1.8
Existing Drainage Area(100 yr.)	.44	31	5.43	1.02	2.5

This sub-basin continues to flow to the west where it is intercepted by a 2'x2' drop inlet connected to the local street storm water system. There are no proposed changes within this subbasin.

SOUTHWEST DRAINAGE SUBBASIN

The following table summarizes the peak discharge for the southwest drainage subbasin which includes the southwest portion of Lot 6; as this area is already developed, the existing condition is the proposed condition also. The drainage sub-basin includes 0.39 acres within the proposed Dan Schmidt 2nd Addition.

**EXISTING AND PROPOSED PEAK RUNOFF
 FOR THE SOUTHWEST DRAINAGE SUBBASIN**

Description	C	Tc	I	Area	Q(cfs)
Existing Drainage Area(2 yr.)	.46	16	3.69	0.39	0.7
Existing Drainage Area(5 yr.)	.49	16	4.49	0.39	0.9
Existing Drainage Area(10 yr.)	.55	16	5.07	0.39	1.1
Existing Drainage Area(25 yr.)	.61	16	5.91	0.39	1.4
Existing Drainage Area(100 yr.)	.64	16	7.21	0.39	1.8

This sub-basin continues to flow to the south where it is intercepted by a slotted covered manhole connected to the local street storm water system. There are no proposed changes within this subbasin.

EAST DRAINAGE SUBBASIN

The following table summarizes the peak discharge for the east drainage subbasin which includes all of Lots 1, 2, and 3, and the eastern section of Lots 4, 5, and 6; as this area is already developed, the existing condition is the proposed condition also. The drainage sub-basin includes 3.65 acres within the proposed Dan Schmidt 2nd Addition.

**EXISTING AND PROPOSED PEAK RUNOFF
 FOR THE EAST DRAINAGE SUBBASIN**

Description	C	Tc	I	Area	Q(cfs)
Existing Drainage Area(2 yr.)	.58	26	2.87	3.65	6.1
Existing Drainage Area(5 yr.)	.60	26	3.57	3.65	7.8
Existing Drainage Area(10 yr.)	.64	26	4.07	3.65	9.6
Existing Drainage Area(25 yr.)	.68	26	4.79	3.65	11.9
Existing Drainage Area(100 yr.)	.70	26	5.90	3.65	15.2

The east drainage subbasin drains to the east to Hydraulic Avenue. The discharge flows to a curb inlet in front of the proposed addition.

Dan Schmidt 2nd Addition Drainage Plan(Con't)
Mr. Scott Lindebak
October 16, 2009

OFF SITE DRAINAGE

There is no offsite drainage to consider for this site.

III. FLOODPLAIN SUBMITTAL – No FEMA floodplain is located on this plat.

IV. FEDERAL, STATE AND LOCAL PERMITS

- A. US Army Corp of Engineers-Not Applicable
- B. Kansas Dept. of Agriculture-Not Applicable
- C. FEMA- Not Applicable
- D. Kansas Department of Transportation-Not Applicable
- E. Sedgwick County Right-of-Way Permit-Not Applicable

V. APPENDIX

All charts, graphs, tables including a 1"=50' scale drainage plan map are included for review.

Dan Schmidt 2nd Addition



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Legend

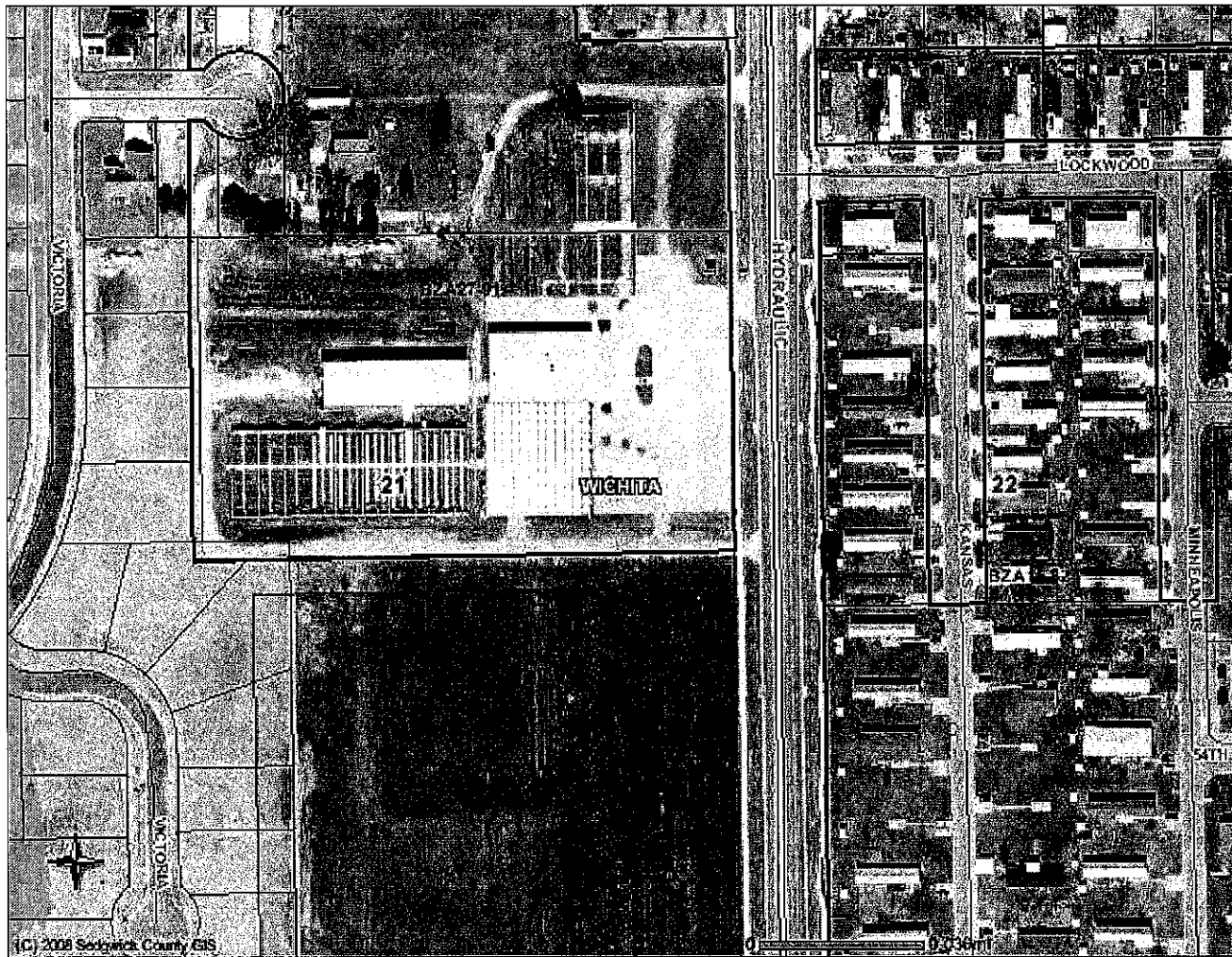
Historic Site Buffers		Zoning Districts	
	1000' National Historic Site Buffers		Rural Residential
	500' Local Historic Site Buffers		Single Family 20,000
	Historic Districts		Single Family 10,000
	Nationally Registered Historic Sites		Single Family 5,000
	Locally Registered Historic Sites		Two-Family
	Special Use Cases		Multi-Family 18 d.u./ac
			Multi-Family 29 d.u./ac
			Multi-Family 75 d.u./ac
			Manufactured Housing
			Neighborhood Office
			General Office
			Neighborhood Retail
			Limited Commercial
			Office Warehouse
			General Commercial
			Industrial Park
			Industrial Park - Airport
			Central Business District
			Limited Industrial
			General Industrial
			University
			Planned Unit Development
			Air Force Base
			Unknown
			Not Zoned



Geographic Information Services
 Division of Information & Operations
www.sedgwickcounty.org/gis
 525 N. Main, Suite 212, Wichita, KS 67203
 Tel: 316.660.9290 Fax: 316.262.1174


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Dan Schmidt 2nd Addition





















Legend

Historic Site Buffers

-  1000' National Historic Site Buffers
-  500' Local Historic Site Buffers
-  Historic Districts
-  Nationally Registered Historic Sites
-  Locally Registered Historic Sites
-  Special Use Cases

Zoning Districts

-  Rural Residential
-  Single Family 20,000
-  Single Family 10,000
-  Single Family 5,000
-  Two-Family
-  Multi-Family 18 d.u./ac
-  Multi-Family 29 d.u./ac
-  Multi-Family 75 d.u./ac
-  Manufactured Housing
-  Neighborhood Office
-  General Office
-  Neighborhood Retail
-  Limited Commercial
-  Office Warehouse
-  General Commercial
-  Industrial Park
-  Industrial Park - Airport
-  Central Business District

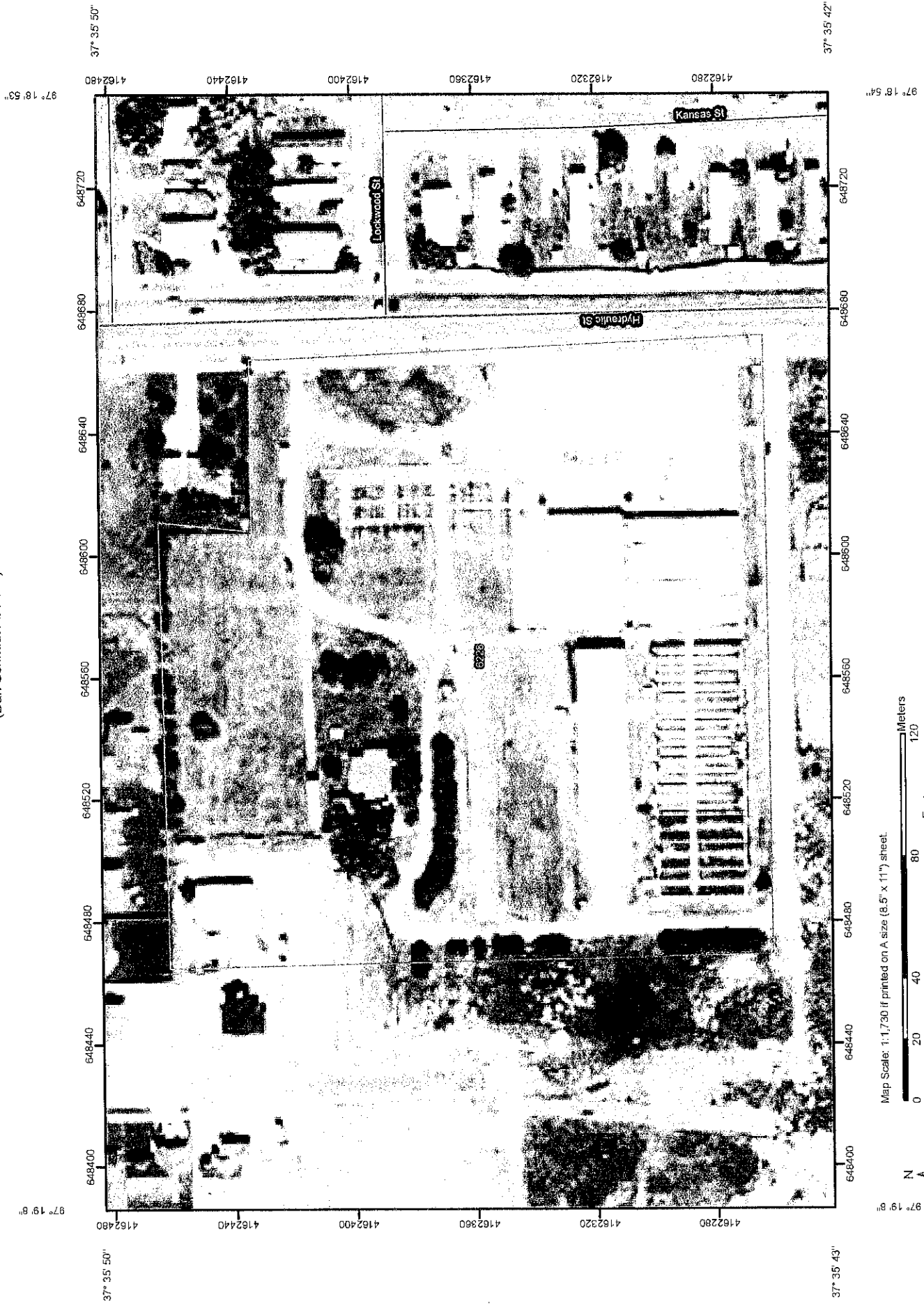
-  Limited Industrial
-  General Industrial
-  University
-  Planned Unit Development
-  Air Force Base
-  Unknown
-  Not Zoned



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Soil Map—Sedgewick County, Kansas
(Dan Schmidt Addition)



MAP INFORMATION

Map Scale: 1:1,730 if printed on A size (8.5" x 11") sheet.
The soil surveys that comprise your AOI were mapped at 1:24,000.
Please rely on the bar scale on each map sheet for accurate map measurements.

Source of Map: Natural Resources Conservation Service
Web Soil Survey URL: <http://websoilsurvey.nrcs.usda.gov>
Coordinate System: UTM Zone 14N NAD83

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Sedgwick County, Kansas
Survey Area Data: Version 5, Dec 3, 2008

Date(s) aerial images were photographed: 6/20/2006

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background imagery displayed on these maps. As a result, some minor shifting of map unit boundaries may be evident.

MAP LEGEND

	Area of Interest (AOI)		Very Stony Spot
	Soils		Wet Spot
	Soil Map Units		Other
Special Point Features			
	Blowout	Special Line Features	
	Borrow Pit		Gully
	Clay Spot		Short Steep Slope
	Closed Depression		Other
	Gravel Pit	Political Features	
	Gravelly Spot		Cities
	Landfill	Water Features	
	Lava Flow		Oceans
	Marsh or swamp		Streams and Canals
	Mine or Quarry	Transportation	
	Miscellaneous Water		Rails
	Perennial Water		Interstate Highways
	Rock Outcrop		US Routes
	Saline Spot		Major Roads
	Sandy Spot		Local Roads
	Severely Eroded Spot		
	Sinkhole		
	Slide or Slip		
	Sodic Spot		
	Spoil Area		
	Stony Spot		

Map Unit Legend

Sedgwick County, Kansas (KS173)			
Map Unit Symbol	Map Unit Name	Acres In AOI	Percent of AOI
6228	Canadian-Waldeck fine sandy loams, rarely flooded	9.5	100.0%
Totals for Area of Interest		9.5	100.0%

WORKSHEET 3: TIME OF CONCENTRATION

Project: Dan Schmidt 2nd By Date

Section: _____ SE 1/4, Sec 21, T28S, R1E, Wichita Checked Date

Parcel Description: _____ Greenhouses, out buildings, and parking lot

Existing and Developed Northwest Sub-basin

Sheet Flow

1	Surface Description (Table 3-1): _____ mix	
2	Manning's Roughness Coefficient, n (Table 3-1)	0.19
3	Flow Length, L<=300 ft	227 ft.
4	Two-yr 24-hr rainfall, P2	3.5 in
5	Land Slope, s	0.008 ft./ft.
6	$T = \frac{0.007 (nL)^{0.8}}{(P2^{0.5})(s^{0.4})}$	0.52439 hr

Shallow Concentrated Flow

7	Surface Description (Paved or Unpaved)	0 ft.
8	Flow Length, L	0.004 ft./ft.
9	Watercourse Slope, s	0.9 ft./s
10	Average Velocity, V (Figure 3-1)	
11	$T = L / 3600 V$	0 hr

Channel Flow

12	Cross sectional Flow Area, a	0 ft ²
13	Wetted Perimeter, P	0 ft.
14	Hydraulic Radius $r = a / P$	0 ft.
15	Channel Slope, S	0 ft./ft.
16	Mannin's Roughness Coeff., n	0
17	$V = 1.49 (r^{2/3})(s^{1/2}) / n$	0 ft./s
18	Flow Length, L	0 ft.
19	$T = L / 3600 V$	0 hr
20	$T = T + T + T$	0.52 hr

31 minutes

Dan Schmidt 2nd Addition
 SE 1/4, Sec 21, T 28 S, R1E

Site
 Existing North West Sub-Basin
 Drainage Area = 1.02 Acres
 Time of Concentratio 31 Minutes

Soil	Canadian-Waldeck Sandy Loam Type B Lawn 77%
Soil	Roofs 11%
Soil	Gravel Drive 12%
Soil	

C	Weighted			Total Average
	B,Lawn 77%	Roofs 11%	Gravel 12%	
2	0.16	0.80	0.24	0.24
5	0.18	0.85	0.26	0.26
10	0.24	0.90	0.33	0.32
25	0.32	0.92	0.42	0.40
100	0.37	0.93	0.48	0.44

I	
2	2.59
5	3.26
10	3.72
25	4.39
100	5.43

Peak Flow					
Year	C	I	A	=	Q
					cfs
2	0.24	2.59	1.02		0.63
5	0.26	3.26	1.02		0.88
10	0.32	3.72	1.02		1.23
25	0.40	4.39	1.02		1.78
100	0.44	5.43	1.02		2.46

WORKSHEET 3: TIME OF CONCENTRATION

Project: Dan Schmidt 2nd By Date

Section: _____ SE 1/4, Sec 21, T28S, R1E, Wichita Checked Date

Parcel Description: _____ Greenhouses, out buildings, and parking lot

Existing and Developed
SW Area

Sheet Flow

1	Surface Description (Table 3-1): _____ mix	
2	Manning's Roughness Coefficient, n (Table 3-1)	0.19
3	Flow Length, L<=300 ft	121 ft.
4	Two-yr 24-hr rainfall, P2	3.5 in
5	Land Slope, s	0.012 ft./ft.
6	$T = \frac{0.007 (nL)^{0.8}}{(P^{0.5})(s^{0.4})}$	0.26954 hr

Shallow Concentrated Flow

7	Surface Description (Paved or Unpaved)	
8	Flow Length, L	0 ft.
9	Watercourse Slope, s	0.004 ft./ft.
10	Average Velocity, V (Figure 3-1)	0.9 ft./s
11	$T = L / 3600 V$	0 hr

Channel Flow

12	Cross sectional Flow Area, a	0 ft ²
13	Wetted Perimeter, P	0 ft.
14	Hydraulic Radius $r = a / P$	0 ft.
15	Channel Slope, S	0 ft./ft.
16	Mannin's Roughness Coeff., n	0
17	$V = 1.49 (r^{2/3})(s^{1/2}) / n$	0 ft./s
18	Flow Length, L	0 ft.
19	$T = L / 3600 V$	0 hr
20	$T = T + T + T$	0.27 hr

16 minutes

Dan Schmidt 2nd Addition
 SE 1/4, Sec 21, T 28 S, R1E

Site
 Existing South West Sub-Basin
 Drainage Area = 0.39 Acres
 Time of Concentratio 16 Minutes

Soil	Canadian-Waldeck Sandy Loam Type B Lawn 34%
Soil	Roofs 44%
Soil	Gravel Drive 22%
Soil	

C	Weighted			Total Average
	B, .5 SF 34%	Roofs 44%	Gravel 22%	
2	0.16	0.80	0.24	0.46
5	0.18	0.85	0.26	0.49
10	0.24	0.90	0.33	0.55
25	0.32	0.92	0.42	0.61
100	0.37	0.93	0.48	0.64

I	
2	3.69
5	4.49
10	5.07
25	5.91
100	7.21

Peak Flow					
Year	C	I	A	=	Q
					cfs
2	0.46	3.69	0.39		0.66
5	0.49	4.49	0.39		0.86
10	0.55	5.07	0.39		1.09
25	0.61	5.91	0.39		1.40
100	0.64	7.21	0.39		1.80

WORKSHEET 3: TIME OF CONCENTRATION

Project: Dan Schmidt 2nd By Date

Section: _____ SE 1/4, Sec 21, T28S, R1E, Wichita Checked Date

Parcel Description: _____ Greenhouses, out buildings, and parking lot

Existing and Developed

East Area

Sheet Flow

- | | | |
|---|--|---------------|
| 1 | Surface Description (Table 3-1): _____ mix | |
| 2 | Manning's Roughness Coefficient, n (Table 3-1) | 0.19 |
| 3 | Flow Length, L<=300 ft | 122 ft. |
| 4 | Two-yr 24-hr rainfall, P2 | 3.5 in |
| 5 | Land Slope, s | 0.008 ft./ft. |
| 6 | $T = \frac{0.007 (nL)^{0.8}}{(P2^{0.5})(s^{0.4})}$ | 0.31909 hr |

Shallow Concentrated Flow

- | | | |
|----|--|----------------|
| 7 | Surface Description (Paved or Unpaved) | |
| 8 | Flow Length, L | 350 ft. |
| 9 | Watercourse Slope, s | 0.0034 ft./ft. |
| 10 | Average Velocity, V (Figure 3-1) | 0.9 ft./s |
| 11 | $T = L / 3600 V$ | 0.10802 hr |

Channel Flow

- | | | |
|----|-----------------------------------|-------------------|
| 12 | Cross sectional Flow Area, a | 0 ft ² |
| 13 | Wetted Perimeter, P | 0 ft. |
| 14 | Hydraulic Radius $r = a / P$ | 0 ft. |
| 15 | Channel Slope, S | 0 ft./ft. |
| 16 | Mannin's Roughness Coeff., n | 0 |
| 17 | $V = 1.49 (r^{2/3})(s^{1/2}) / n$ | 0 ft./s |
| 18 | Flow Length, L | 0 ft. |
| 19 | $T = L / 3600 V$ | 0 hr |
| 20 | $T = T + T + T$ | 0.43 hr |

26 minutes

Dan Schmidt 2nd Addition
 SE 1/4, Sec 21, T 28 S, R1E

Site
 Existing and Proposed East Sub-Basin
 Drainage Area = 3.65 Acres

Time of Concentratio 26 Minutes

Soil	Canadian-Waldeck Sandy Loam Type B Lawn 38%
Soil	Roofs 29%
Soil	Concrete Parking 33%
Soil	

C	Weighted			Total Average
	B, Lawn 38%	Roofs 29%	Concrete 33%	
2	0.16	0.80	0.87	0.58
5	0.18	0.85	0.87	0.60
10	0.24	0.90	0.88	0.64
25	0.32	0.92	0.88	0.68
100	0.37	0.93	0.89	0.70

I	
2	2.87
5	3.57
10	4.07
25	4.79
100	5.9

Peak Flow					
Year	C	I	A	=	Q
					cfs
2	0.58	2.87	3.65		6.07
5	0.60	3.57	3.65		7.84
10	0.64	4.07	3.65		9.55
25	0.68	4.79	3.65		11.87
100	0.70	5.9	3.65		15.16

RAINFALL INTENSITY TABLE
 SEDGWICK COUNTY
 KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
 IN INCHES PER HOUR.

Time of Conc. DURATION, HR:MIN	RETURN PERIOD						
	1 YR	2 YR	5 YR	10 YR	25 YR	50 YR	100 YR
0:05	4.91	5.64	6.64	7.38	8.48	9.34	10.20
0:06	4.62	5.34	6.33	7.07	8.15	9.00	9.84
0:07	4.38	5.09	6.08	6.80	7.86	8.69	9.52
0:08	4.17	4.87	5.85	6.56	7.60	8.41	9.22
0:09	4.00	4.68	5.63	6.33	7.34	8.14	8.93
0:10	3.84	4.50	5.43	6.11	7.10	7.87	8.64
0:11	3.70	4.34	5.25	5.90	6.86	7.61	8.36
0:12	3.56	4.19	5.07	5.71	6.64	7.36	8.09
0:13	3.44	4.05	4.91	5.53	6.43	7.14	7.84
0:14	3.33	3.92	4.76	5.36	6.24	6.92	7.61
0:15	3.22	3.80	4.62	5.21	6.06	6.73	7.40
0:16	3.12	3.69	4.49	5.07	5.91	6.56	7.21
0:17	3.03	3.58	4.37	4.94	5.76	6.40	7.04
0:18	2.94	3.48	4.26	4.82	5.63	6.26	6.88
0:19	2.85	3.39	4.16	4.71	5.50	6.12	6.74
0:20	2.77	3.30	4.06	4.60	5.38	5.99	6.60
0:21	2.70	3.22	3.97	4.50	5.27	5.87	6.47
0:22	2.63	3.14	3.88	4.41	5.17	5.76	6.35
0:23	2.56	3.07	3.80	4.32	5.07	5.65	6.23
0:24	2.50	3.00	3.72	4.23	4.97	5.54	6.12
0:25	2.44	2.93	3.64	4.15	4.88	5.44	6.01
0:26	2.38	2.87	3.57	4.07	4.79	5.35	5.90
0:27	2.33	2.81	3.50	4.00	4.70	5.26	5.80
0:28	2.27	2.75	3.44	3.92	4.62	5.17	5.71
0:29	2.23	2.69	3.37	3.86	4.54	5.08	5.61
0:30	2.18	2.64	3.31	3.79	4.47	4.99	5.52
0:31	2.14	2.59	3.26	3.72	4.39	4.91	5.43
0:32	2.09	2.54	3.20	3.66	4.32	4.83	5.34
0:33	2.05	2.50	3.14	3.60	4.25	4.76	5.26
0:34	2.02	2.45	3.09	3.54	4.18	4.68	5.18
0:35	1.98	2.41	3.04	3.48	4.12	4.61	5.10
0:36	1.94	2.37	2.99	3.43	4.05	4.54	5.02
0:37	1.91	2.33	2.94	3.38	3.99	4.47	4.95
0:38	1.88	2.29	2.90	3.32	3.93	4.40	4.87
0:39	1.85	2.25	2.85	3.27	3.87	4.34	4.80
0:40	1.82	2.22	2.81	3.23	3.82	4.28	4.73
0:41	1.79	2.18	2.77	3.18	3.76	4.22	4.67
0:42	1.76	2.15	2.73	3.13	3.71	4.16	4.60
0:43	1.73	2.12	2.69	3.09	3.66	4.10	4.54
0:44	1.71	2.09	2.65	3.05	3.61	4.04	4.48
0:45	1.68	2.06	2.62	3.01	3.56	3.99	4.42

RAINFALL INTENSITY TABLE
 SEDGWICK COUNTY
 KANSAS

THIS TABLE CONTAINS AVERAGE RAINFALL INTENSITIES
 IN INCHES PER HOUR.

DURATION, HOURS	RETURN PERIOD						
	1 YR.	2 YR.	5 YR.	10 YR.	25 YR.	50 YR.	100 YR.
0:45	1.68	1.63	2.58	2.95	3.81	3.94	4.36
0:47	1.63	1.60	2.55	2.93	3.47	3.89	4.30
0:48	1.61	1.57	2.51	2.89	3.41	3.84	4.25
0:49	1.59	1.55	2.48	2.85	3.38	3.79	4.20
0:50	1.57	1.52	2.45	2.81	3.34	3.74	4.15
0:51	1.55	1.50	2.42	2.78	3.30	3.70	4.10
0:52	1.53	1.47	2.39	2.75	3.26	3.65	4.05
0:53	1.51	1.45	2.36	2.71	3.22	3.61	4.00
0:54	1.49	1.43	2.33	2.68	3.18	3.57	3.95
0:55	1.47	1.40	2.30	2.65	3.14	3.53	3.91
0:56	1.45	1.38	2.28	2.62	3.11	3.49	3.86
0:57	1.43	1.36	2.25	2.59	3.07	3.45	3.82
0:58	1.41	1.34	2.22	2.56	3.04	3.41	3.78
0:59	1.40	1.32	2.20	2.53	3.01	3.37	3.74
1:00	1.38	1.30	2.17	2.50	2.97	3.34	3.70
1:05	1.30	1.21	2.06	2.38	2.82	3.17	3.52
1:10	1.23	1.13	1.96	2.26	2.69	3.02	3.35
1:15	1.17	1.06	1.87	2.16	2.57	2.89	3.20
1:20	1.11	1.00	1.79	2.06	2.46	2.77	3.07
1:25	1.06	0.94	1.71	1.98	2.35	2.65	2.95
1:30	1.01	0.89	1.64	1.90	2.27	2.55	2.83
1:35	0.97	0.84	1.58	1.83	2.18	2.46	2.73
1:40	0.93	0.80	1.52	1.76	2.10	2.37	2.63
1:45	0.89	0.76	1.46	1.70	2.03	2.29	2.54
1:50	0.86	0.72	1.41	1.64	1.96	2.21	2.46
1:55	0.82	0.68	1.36	1.58	1.89	2.13	2.38
2:00	0.79	0.64	1.31	1.53	1.83	2.07	2.30
2:05	0.76	0.61	1.27	1.48	1.77	2.00	2.23
2:10	0.74	0.58	1.23	1.43	1.72	1.94	2.16
2:15	0.71	0.55	1.19	1.39	1.67	1.88	2.10
2:20	0.69	0.52	1.15	1.35	1.62	1.83	2.04
2:25	0.66	0.49	1.11	1.31	1.57	1.78	1.98
2:30	0.64	0.46	1.09	1.27	1.53	1.73	1.93
2:35	0.62	0.43	1.06	1.24	1.49	1.68	1.88
2:40	0.61	0.41	1.03	1.21	1.45	1.64	1.83
2:45	0.59	0.38	1.01	1.18	1.42	1.60	1.79
2:50	0.57	0.36	0.98	1.15	1.38	1.55	1.74
2:55	0.55	0.34	0.94	1.12	1.35	1.53	1.70
3:00	0.55	0.32	0.94	1.10	1.33	1.49	1.67

ATTACHMENT D

DRAINAGE CRITERIA

CITY OF WICHITA, KANSAS

RECOMMENDED RUNOFF COEFFICIENTS FOR RATIONAL METHOD
AND PERCENT IMPERVIOUS FOR UNIT HYDROGRAPH METHOD

Land Use or Surface Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
1. Business:					
Downtown Areas	95	0.84	0.85	0.87	0.91
Neighborhood Areas	70	0.68	0.69	0.73	0.80
2. Residential:					
Single Family (Soil Group D)					
1/8 Acre	50	0.57	0.61	0.66	0.79
1/4 Acre	38	0.50	0.54	0.62	0.76
1/3 Acre	30	0.46	0.50	0.59	0.73
1/2 Acre	25	0.42	0.48	0.56	0.72
3/4 Acre	22	0.42	0.46	0.55	0.71
1 Acre	20	0.41	0.45	0.54	0.71
Multi-Family (Soil Group D)					
Multi-Unit (detached)	60	0.62	0.66	0.72	0.82
Multi-Unit (attached)	65	0.64	0.68	0.73	0.83
Apartments	75	0.70	0.73	0.79	0.86
Single Family (Soil Group C)					
1/8 Acre	50	0.55	0.58	0.64	0.73
1/4 Acre	38	0.48	0.51	0.57	0.68
1/3 Acre	30	0.43	0.46	0.53	0.65
1/2 Acre	25	0.40	0.43	0.50	0.63
3/4 Acre	22	0.39	0.42	0.49	0.62
1 Acre	20	0.37	0.40	0.48	0.61
Multi-Family (Soil Group C)					
Multi-Unit (detached)	60	0.60	0.63	0.69	0.77
Multi-Unit (attached)	65	0.63	0.66	0.71	0.79
Apartments	75	0.68	0.72	0.77	0.83
Single-Family (Soil Group B)					
1/8 Acre	50	0.52	0.54	0.59	0.67
1/4 Acre	38	0.44	0.46	0.52	0.61
1/3 Acre	30	0.39	0.41	0.47	0.57
1/2 Acre	25	0.36	0.38	0.44	0.54
3/4 Acre	22	0.34	0.36	0.42	0.52
1 Acre	20	0.33	0.35	0.40	0.51
Multi-Family (Soil Group B)					
Multi-Unit (detached)	60	0.58	0.60	0.65	0.72
Multi-Unit (attached)	65	0.61	0.64	0.68	0.75
Apartments	75	0.67	0.70	0.74	0.80

Land Use or face Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
<u>Single Family (Soil Group A)</u>					
1/8 Acre	50	0.47	0.50	0.54	0.60
1/4 Acre	38	0.39	0.41	0.45	0.52
1/3 Acre	30	0.33	0.35	0.39	0.47
1/2 Acre	25	0.30	0.31	0.35	0.44
3/4 Acre	22	0.28	0.29	0.33	0.42
1 Acre	20	0.26	0.28	0.32	0.40
<u>Multi-Family (Soil Group A)</u>					
Multi-Unit (detached)	60	0.55	0.57	0.61	0.67
Multi-Unit (attached)	65	0.58	0.60	0.64	0.70
Apartments	75	0.65	0.68	0.72	0.77
3. Industrial:					
Light Areas	70	0.68	0.69	0.73	0.80
Heavy Areas	80	0.74	0.76	0.79	0.84
4. Playgrounds:					
	15	0.33	0.35	0.42	0.55
5. Schools:					
	40	0.49	0.51	0.56	0.66
6. Railroad Yard Areas:					
	30	0.43	0.45	0.50	0.62
Undeveloped Urban Areas: Offsite Flow Analysis (when land use not defined)					
	45	0.52	0.54	0.59	0.68
8. Streets:					
Paved	99	0.87	0.88	0.90	0.93
Gravel	00	0.24	0.26	0.33	0.48
9. Drive, Parking Lots and Walks:					
	96	0.87	0.87	0.88	0.89
10. Roofs:					
	90	0.80	0.85	0.90	0.93
11. Urban Lawn Areas (See Note No. 1 below):					
<u>Soil Group A</u>					
Slope less than 1%	00	0.08	0.09	0.13	0.23
Slope 1% to 4%	00	0.12	0.13	0.17	0.27
Slope more than 4%	00	0.16	0.17	0.21	0.31
<u>Soil Group B</u>					
Slope less than 1%	00	0.16	0.18	0.24	0.37
Slope 1% to 4%	00	0.20	0.22	0.28	0.41
Slope more than 4%	00	0.24	0.26	0.32	0.45
<u>Soil Group C</u>					
Slope less than 1%	00	0.24	0.27	0.35	0.51
Slope 1% to 4%	00	0.26	0.29	0.37	0.53
Slope more than 4%	00	0.28	0.31	0.39	0.55

Land Use or Face Characteristics	Percent Impervious	Frequency			
		<u>2</u>	<u>5</u>	<u>10</u>	<u>100</u>
Soil Group D					
Slope less than 1%	00	0.28	0.33	0.43	0.63
Slope 1% to 4%	00	0.30	0.35	0.45	0.65
Slope more than 4%	00	0.32	0.37	0.47	0.67

Note No. 1: Coefficients shown in the above table are for pervious open space areas with thick turf which includes pervious areas in parks and cemeteries. Coefficients shown above must be increased 0.02 for use with agricultural pasture areas. Coefficients shown above must be reduced by 0.04 for use with agricultural cultivated areas. Group A soils are well-drained, coarse textured sands with high infiltration rates. Group B soils are moderately well-drained, moderately coarse textured soils with moderate infiltration rates. Group C soils are moderately poor-drained, moderately fine textured soils with slow infiltration rates. Group D soils are poor-drained, fine textured soils with very slow infiltration rates.

GENERAL NOTE: These Rational Formula Coefficients may not be valid for basins 320 acres or larger.

EXHIBIT NO. 1

SOIL LEGEND

<u>SYMBOL</u>	<u>HYDROLOGIC GROUP</u>	<u>NAME</u>
Aa	B	Albion-Shellabarger sandy loams, 1 to 4 percent slopes
Ab	B	Albion and Shellabarger sandy loams, 7 to 15 percent slopes
Ba	C	Blanket silt loam, 0 to 1 percent slopes
Bb	C	Blanket silt loam, 1 to 3 percent slopes
Ca	B	Canadian fine sandy loam
Cb	B	Canadian-Waldeck fine sandy loams
Cc	D	Carwile fine sandy loam
Cd	B	Clark-Ost clay loams, 1 to 4 percent slopes
Ce	C	Clime silty clay, 3 to 6 percent slopes
Ea	B	Elandco silt loam
Eb	B	Elandco silt loam, occasionally flooded
Ec	B	Elandco silt loam, frequently flooded
Fa	B	Farnum loam, 0 to 1 percent slopes
Fb	B	Farnum loam, 1 to 3 percent slopes
Fc	B	Farnum loam, sandy substratum, 0 to 1 percent slopes
Ga	D	Goessel silty clay, 0 to 1 percent slopes
Gb	D	Goessel silty clay, 1 to 2 percent slopes
Ia	D	Irwin silty clay loam, 1 to 3 percent slopes
Ib	D	Irwin silty clay loam, 3 to 6 percent slopes
Ic	D	Irwin silty clay loam, 2 to 6 percent slopes, eroded
La	C	Lesho loam
Lb	A	Lincoln soils
Ma	B	Milan loam, 1 to 3 percent slopes
Mb	B	Milan form, 3 to 6 percent slopes
Mc	B	Milan clay loam, 2 to 6 percent slopes, eroded
Na	B	Naron fine sandy loam
Oc	D	Owens clay loam, 1 to 3 percent slopes
Od	D	Owens-Rock outcrop complex, 3 to 10 percent slopes
Pa		Pits
Pb	D	Plevna fine sandy loam
Pc	A	Pratt loamy fine sand, undulating
Pd	A	Pratt-Tivoli complex, rolling
Ra	D	Renfrow silty clay loam, 1 to 3 percent slopes
Rb	D	Renfrow silty clay loam, 3 to 6 percent slopes
Rc	D	Renfrow-Owens clay loams, 1 to 4 percent slopes
Rd	D	Rosehill silty clay, 1 to 3 percent slopes
Sa	B	Shellabarger sandy loam, 1 to 3 percent slopes
Sb	B	Shellabarger sandy loam, 3 to 6 percent slopes
Sc	B	Shellabarger sandy loam, 3 to 6 percent slopes, eroded
Ta	D	Tabler silty clay loam
Tb	D	Tabler-Drummond complex
Ua	B	Urban land-Canadian complex
Ub	B	Urban land-Elandco complex
Uc	B	Urban land-Farnum complex, 0 to 3 percent slopes
Ud	D	Urban land-Irwin complex, 1 to 3 percent slopes
Ue	D	Urban land-Tabler complex
Va	B	Vanoss silt loam, 0 to 1 percent slopes
Vb	B	Vanoss silt loam, 1 to 3 percent slopes
Vc	B	Vanoss silt loam, 3 to 6 percent slopes
Vd	B	Vanoss silt loam, 3 to 6 percent slopes, eroded
Ve	D	Vernon sandy loam, 1 to 3 percent slopes
Vf	D	Vernon sandy loam, 3 to 6 percent slopes
Wa	C	Waldeck sandy loam
Wb	D	Waurika silt loam

