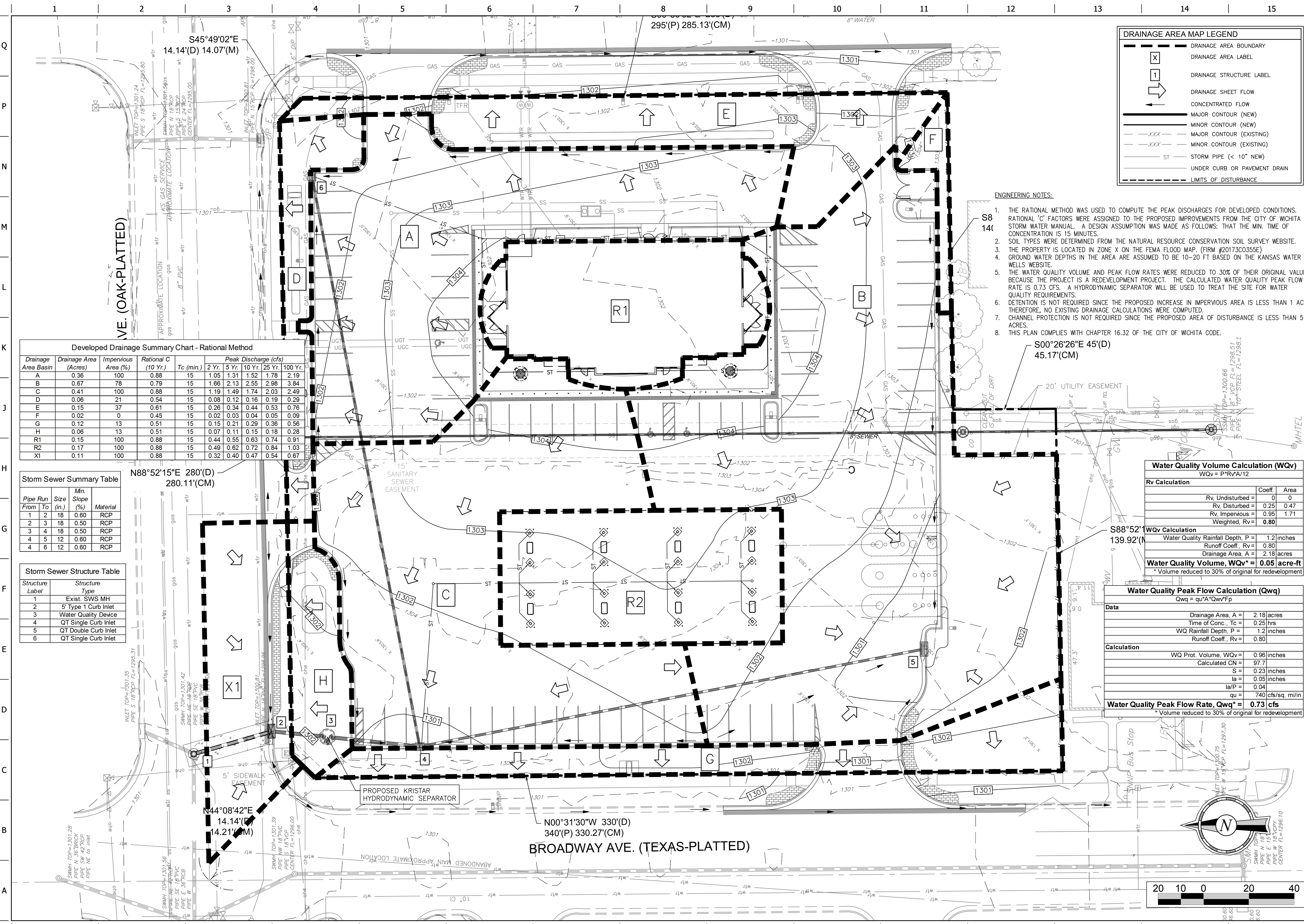


FILE LOCATION: \\Drawing Files\Project AMG 9-16-13\VT #0391R\DWG\03-0391 Drainage Plan.dwg USER:eng2 SAVED:10/27/2014 11:28 AM PLOTTED:10/27/2014 11:34 AM



DRAINAGE AREA MAP LEGEND

- DRAINAGE AREA BOUNDARY
- DRAINAGE AREA LABEL
- DRAINAGE STRUCTURE LABEL
- DRAINAGE SHEET FLOW
- CONCENTRATED FLOW
- MAJOR CONTOUR (NEW)
- MINOR CONTOUR (NEW)
- MAJOR CONTOUR (EXISTING)
- MINOR CONTOUR (EXISTING)
- STORM PIPE (< 10" NEW)
- UNDER CURB OR PAVEMENT DRAIN
- LIMITS OF DISTURBANCE

- ENGINEERING NOTES:**
1. THE RATIONAL METHOD WAS USED TO COMPUTE THE PEAK DISCHARGES FOR DEVELOPED CONDITIONS. RATIONAL 'C' FACTORS WERE ASSIGNED TO THE PROPOSED IMPROVEMENTS FROM THE CITY OF WICHITA STORM WATER MANUAL. A DESIGN ASSUMPTION WAS MADE AS FOLLOWS: THAT THE MIN. TIME OF CONCENTRATION IS 15 MINUTES.
 2. SOIL TYPES WERE DETERMINED FROM THE NATURAL RESOURCE CONSERVATION SOIL SURVEY WEBSITE.
 3. THE PROPERTY IS LOCATED IN ZONE X ON THE FEMA FLOOD MAP. (FIRM #2017300355E)
 4. GROUND WATER DEPTHS IN THE AREA ARE ASSUMED TO BE 10-20 FT BASED ON THE KANSAS WATER WELLS WEBSITE.
 5. THE WATER QUALITY VOLUME AND PEAK FLOW RATES WERE REDUCED TO 30% OF THEIR ORIGINAL VALUES BECAUSE THE PROJECT IS A REDEVELOPMENT PROJECT. THE CALCULATED WATER QUALITY PEAK FLOW RATE IS 0.73 CFS. A HYDRODYNAMIC SEPARATOR WILL BE USED TO TREAT THE SITE FOR WATER QUALITY REQUIREMENTS.
 6. DETENTION IS NOT REQUIRED SINCE THE PROPOSED INCREASE IN IMPERVIOUS AREA IS LESS THAN 1 ACRE. THEREFORE, NO EXISTING DRAINAGE CALCULATIONS WERE COMPUTED.
 7. CHANNEL PROTECTION IS NOT REQUIRED SINCE THE PROPOSED AREA OF DISTURBANCE IS LESS THAN 5 ACRES.
 8. THIS PLAN COMPLIES WITH CHAPTER 16.32 OF THE CITY OF WICHITA CODE.

Developed Drainage Summary Chart - Rational Method

Drainage Area Basin	Drainage Area (Acres)	Impervious Area (%)	Rational C (10 Yr.)	Tc (min.)	Peak Discharge (cfs)				
					2 Yr.	5 Yr.	10 Yr.	25 Yr.	100 Yr.
A	0.36	100	0.88	15	1.05	1.31	1.52	1.78	2.19
B	0.67	78	0.79	15	1.66	2.13	2.55	2.98	3.84
C	0.41	100	0.88	15	1.19	1.49	1.74	2.03	2.49
D	0.66	21	0.54	15	0.08	0.12	0.16	0.19	0.28
E	0.15	37	0.61	15	0.26	0.34	0.44	0.53	0.76
F	0.02	0	0.45	15	0.02	0.03	0.04	0.05	0.09
G	0.12	13	0.51	15	0.15	0.21	0.28	0.36	0.56
H	0.06	13	0.51	15	0.07	0.11	0.15	0.18	0.28
R1	0.15	100	0.88	15	0.44	0.55	0.63	0.74	0.91
R2	0.17	100	0.88	15	0.49	0.62	0.72	0.84	1.03
X1	0.11	100	0.88	15	0.32	0.40	0.47	0.54	0.67

Storm Sewer Summary Table

Pipe Run From	Pipe Run To	Size (in.)	Min. Slope (%)	Material
1	2	18	0.60	RCP
2	3	18	0.50	RCP
3	4	18	0.50	RCP
4	5	12	0.60	RCP
4	6	12	0.60	RCP

Storm Sewer Structure Table

Structure Label	Structure Type
1	Exist. SWS MH
2	5' Type 1 Curb Inlet
3	Water Quality Device
4	QT Single Curb Inlet
5	QT Double Curb Inlet
6	QT Single Curb Inlet

Water Quality Volume Calculation (WQv)
WQv = P*RV/A*12

Rv Calculation	Coeff.	Area
Rv, Undisturbed	0	0
Rv, Disturbed	0.25	0.47
Rv, Impervious	0.95	1.71
Weighted, Rv	0.80	

WQv Calculation
S88°52'15"E 280'(D) 280.11'(CM)
139.92'(N)

Water Quality Rainfall Depth, P	= 1.2 inches
Runoff Coeff., Rv	= 0.80
Drainage Area, A	= 2.18 acres
Water Quality Volume, WQv*	= 0.05 acre-ft

* Volume reduced to 30% of original for redevelopment

Water Quality Peak Flow Calculation (Qwq)
Qwq = qu*A*Qwv*Fp

Data	Value
Drainage Area, A	= 2.18 acres
Time of Conc., Tc	= 0.25 hrs
WQ Rainfall Depth, P	= 1.2 inches
Runoff Coeff., Rv	= 0.80

Water Quality Peak Flow Rate, Qwq*
* Volume reduced to 30% of original for redevelopment

WQ Prot. Volume, WQv	= 0.96 inches
Calculated CN	= 97.7
S	= 0.23 inches
la	= 0.05 inches
la/P	= 0.04
qu	= 740 cfs/sq. mi/in
Water Quality Peak Flow Rate, Qwq*	= 0.73 cfs

HARRIS DALE FORBES
LICENSED PROFESSIONAL ENGINEER
10891 10.27.14
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PROJECT NO.: 20142192

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QuikTrip 18th Addition
730 NORTH BROADWAY STREET
WICHITA, KS

QT

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REV	DATE	DESCRIPTION

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DRAINAGE PLAN

SHEET NUMBER:
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ORIGINAL ISSUE DATE: