

SUBDIVISION COMMITTEE
METROPOLITAN AREA PLANNING COMMISSION

AGENDA ITEM NO. 5

February 6, 1997

STAFF REPORT
(Final Plat)

CASE NUMBER: S/D 97-5 - BRADLEY FAIR 3RD ADDITION

OWNER/APPLICANT: (Contract Purchaser): Vantage Point Properties, Inc., 8110 E. 32nd Street North, Wichita, KS 67226

and (Contract Purchaser): Laham Holding Co., L.L.C., 150 N. Market, Wichita, KS 67202

SURVEYOR/ENGINEER: Rob Hartman, c/o P.E.C., P.A., 303 S. Topeka, Wichita, KS 67202

LOCATION: South of 21st Street North and east of Rock Road

SITE SIZE: 22.7 ± Acres

NUMBER OF LOTS

Residential:	
Office:	
Commercial:	2
Industrial:	
Total:	<u>2</u>

MINIMUM LOT AREA: 1.3 Acres

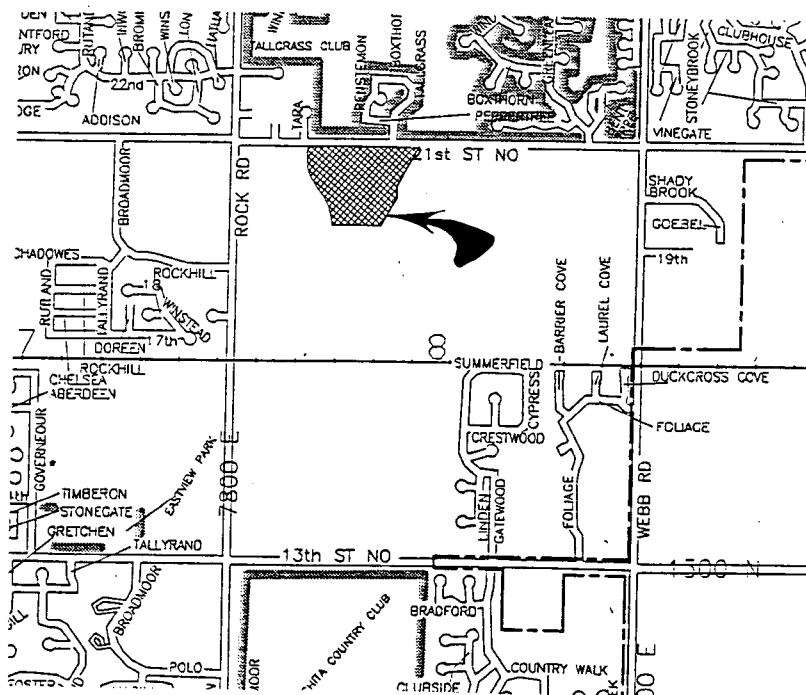
CURRENT ZONING: "L.C." and "AA"

PROPOSED ZONING: "LC" and "GO" (Z-3177)

Suggest lot 1 drain thru lot 2 to creek instead to 21st with cross lot drain agreement

SS ext

VICINITY MAP:



- I. Prior to submitting the final plat tracing, the applicant shall record the 50' Access Easement being shown along the west line of Lot 1 and indicate the recording information in the tracing. A recording copy of the easement shall be submitted to Planning for the plat file.
- J. The applicant shall submit an avigational easement covering all of subject plat and a restrictive covenant assuring that adequate construction methods will be used to minimize the effects of noise pollution in the habitable structures constructed on subject property.
- K. The applicant shall install or guarantee the installation of all utilities and facilities which are applicable and described in Article 8 of the MAPC Subdivision Regulations. (Water service and fire hydrants required by Article 8 for fire protection shall be as per the direction and approval of the Chief of the Fire Department.)
- L. The applicant's engineer is advised that the Register of Deeds is requiring the name(s) of the notary public, who acknowledges the signatures on this plat, to be printed beneath the notary signature.
- M. The applicant is advised that various State and Federal requirements [specifically but not limited to the Army Corps of Engineers, Kanopolis Project Office, Rt. 1, Box 30, Marquette, KS 674 (913-546-2294) or Kansas Department of Wildlife and Parks, P. O. Box 317, Valley Center, 67147] for the control of soil and wind erosion and the protection of wetlands may impact how this site can be developed. It is the applicant's responsibility to contact all appropriate agencies to determine any such requirements.
- N. In accordance with Section 16.04.040 - Permit Fees Item 2a,b (Ord. 42-583) and Section 17.12.065 - payment for connection to water system for properties not included in benefit districts (Ord. 42-584), this property may or may not have been included in a benefit district. The applicant/agent should contact Betty Roark, with OCI at 268-4341 and Chuck Steven, Water & Sewer at 268-4555, to determine whether assessments are due.
- O. Perimeter closure computations shall be submitted with the final plat tracing. Section 5-101
- P. Recording of the plat within 30 days after approval by the City Council and/or County Commission.
- Q. The representatives from the utility companies should be prepared to comment on the need for utility easements to be platted on this property.
- R. The representatives from **City Engineering** should be prepared to comment on the status of the applicant's drainage plan. Engineering also needs to indicate if the minimum building pad elevation, as described on this plat, is acceptable.

MEMO



PROFESSIONAL
ENGINEERING
CONSULTANTS
PROFESSIONAL ASSOCIATION
303 S. TOPEKA
WICHITA, KANSAS
ZIP CODE / 67202

TO: City Engineer's Office

455 N. Main

Wichita, KS 67202

ATTN: V.R. Huang, P.E.

PROJECT NO. 36-97012-1-3917

PROJECT: Bradley Fair 3rd Addition

and SWD #119

DATE: January 17, 1997

COPIES TO:

36-96109-042 thru BER

FROM: Michael W. Berry

REFERENCE: Drainage Plan

PLEASE ADVISE IMMEDIATELY OF ANY MISCONCEPTIONS OR OMISSIONS YOU BELIEVE TO BE CONTAINED HEREIN.

Transmitted herewith is the drainage plan for the referenced project.

The computations for the pond system made for Bradley Fair 2nd Addition, originally submitted December 18, 1995, have been revised herein to reflect the actual design information for Bradley Fair Parkway Phase 2 (City Proj. 472-82732) and for Storm Water Drain No. 119 (City Proj. 468-82602). These changes are as follows:

1. Drainage Area for Pond No. 1 has been reduced.
 - a. Bradley Fair 3rd Addition Lot 1 has been diverted to 21st Street.
 - b. Nearly all of Bradley Fair 3rd Addition Lot 2 has been diverted to Middle Branch Gypsum Creek.
 - c. Street drainage for Bradley Fair Parkway has been diverted from Pond No. 1 to Pond No. 2.
2. Drainage outlet pipe for Pond No. 1 has been reduced to 48-inch in accordance with reduction in drainage area.
3. Reserve area adjacent to Pond Nos. 2 and 3 is proposed to be increased in size, and the contributing area to these ponds has been increased accordingly.

The proposed ponds and control structures for Pond Nos. 2, 3 and 4 are based on preliminary information only. These are subject to change as design development by the platting engineer for those properties takes place. The contributing areas for each pond from Bradley Fair 3rd Addition are tabulated on the Drainage Plan drawing (attached).

A revised hydrologic model (HEC-1) reflecting the design of SWD 119 and Bradley Fair Parkway Phase 2 is attached.

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E &100 (s16.6H &18D &12A &181F*****
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*
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
*   MAY 1991 *
*   VERSION 4.0.1E *
*   Lahey F77L-EM/32 version 5.01 *
*   Dodson & Associates, Inc. *
* RUN DATE 01/16/97 TIME 12:18:35 *
*****

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*
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 551-1748 *
*
*****

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X X XXXXXX XXXX X
X X X X X XX
X X X X X
XXXXXX XXXX X XXXX X
X X X X X
X X X X X
X X XXXXXX XXXX XXX

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THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE. THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

HEC-1 INPUT

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

1 ID Bradley Fair Drainage Plan
 2 ID 5-, 10-, 25-, & 100-Year Storms
 3 ID Professional Engineering Consultants
 4 ID Wichita, Ks
 5 ID DRC 11/20/95
 6 ID REVISED MWB 12/02/96
 7 ID REVISED PDM 1/17/97
 8 ID File: T:\DAR\HEC1\BRADLEY8.IH1
 9 ID PEC PROJ NO 32-96109-042
 10 IT 6 01FEB95 0600 0 02FEB95 1154
 11 IN 30 01FEB95 0600
 12 IO 3 0
 13 JR PREC 0.5902 0.6875 0.8125 1.000

*DIAGRAM

*
 * REVISED CONTRIBUTING DRAINAGE AREA TO POND NO. 1 FROM 57 AC TO 40 AC AS A
 * RESULT OF CHANGES MADE IN DESIGN OF BRADLEY FAIR PARKWAY
 * REVISED POND #2 & #3 STATIC ELEVATION TO CONCUR WITH FUTURE ROAD TO SOUTH
 * OUTLET FROM POND #1 CHANGED TO 48" RCP AS A RESULT OF REVISED DRAINAGE AREA
 *

14 KK UNDEV UNDEVELOPED CONDITIONS
 15 BA .0625
 16 PB 7.8
 17 PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
 18 PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
 19 PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
 20 PC .964 .970 .976 .982 .988 .994 1.000
 21 LS 0 76 0
 22 UD 0.20

23 KK BASIN1 COMMERCIAL DEVELOPED CONDITIONS
 24 BA .0625
 25 PB 7.8
 26 PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
 27 PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
 28 PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
 29 PC .964 .970 .976 .982 .988 .994 1.000
 30 LS 0 92 0
 31 UD 0.15

*
 * 48" OUTLET PIPE FOR POND #1
 * REVISED 1/17/97 U/S F/L = 195.0
 * 6' X 4' CONTROL STRUCTURE
 * WEIR ELEV 200.00 TARGET DWS 100 YR = 202.0

32 KK POND1
 * PLAN 3 BIG ISLAND, NO ENCROACMENT ON LOT 3
 33 RS 1 ELEV 200.
 34 SA 4.20 4.67
 35 SE 200. 202.
 36 SQ 0 20 60 110 145 150 155 165 200

HEC-1 INPUT

LINE	ID.....	1.....	2.....	3.....	4.....	5.....	6.....	7.....	8.....	9.....	10
37	SE	200	200.4	200.8	201.4	201.6	201.8	202.2	203.	206	
	*										
	*										
	*										
	*	REVISED -- ADDED BASINS 2A AND 2B TO TOTAL DRAINAGE AREA FOR BASIN 2									
38	KK	BASIN2 COMMERCIAL DEVELOPED CONDITIONS									
39	BA	0.004									
40	PB	7.8									
41	PC	0.08	.09	.10	.11	.12	.133	.147	.163	.181	.204
42	PC	.235	.283	.663	.735	.772	.799	.820	.835	.850	.865
43	PC	.880	.890	.900	.910	.916	.925	.934	.943	.952	.958
44	PC	.964	.970	.976	.982	.988	.994	1.000			
45	LS	0	92	0							
46	UD	0.15									
	*										
47	KK	2A COMMERCIAL DEVELOPED CONDITIONS									
48	BA	.0066									
49	PB	7.8									
50	PC	0.08	.09	.10	.11	.12	.133	.147	.163	.181	.204
51	PC	.235	.283	.663	.735	.772	.799	.820	.835	.850	.865
52	PC	.880	.890	.900	.910	.916	.925	.934	.943	.952	.958
53	PC	.964	.970	.976	.982	.988	.994	1.000			
54	LS	0	96	0							
55	UD	0.15									
	*										
56	KK	2B COMMERCIAL DEVELOPED CONDITIONS									
57	BA	.001									
58	PB	7.8									
59	PC	0.08	.09	.10	.11	.12	.133	.147	.163	.181	.204
60	PC	.235	.283	.663	.735	.772	.799	.820	.835	.850	.865
61	PC	.880	.890	.900	.910	.916	.925	.934	.943	.952	.958
62	PC	.964	.970	.976	.982	.988	.994	1.000			
63	LS	0	98	0							
64	UD	.09									
	*										
	*										
65	KK	INTO2 COMBINE HYDROGRAPHS FOR BASIN 2, 2A, 2B AND OUT OF POND #1									
66	HC	4									
	*										
	*										
	*	5' RIPRAP CHANNEL SPILLWAY FOR POND #2									
67	KK	POND2									
68	RS	1	ELEV	192							
69	SA	0.5	0.8								
70	SE	192	194								
71	SQ	0	40	60	80	90	100	110	120	130	140
72	SQ	150	160	170	180	190	200				
73	SE	192	192.95	193.18	193.35	193.42	193.5	193.56	193.61	193.67	193.72
74	SE	193.78	193.84	193.89	193.93	193.98	194.02				
	*										
	*										

HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
75	KK BASIN3 COMMERCIAL DEVELOPED CONDITIONS
76	BA .00456
77	PB 7.8
78	PC 0.08 .09 .10 .11 .12 .133 .147 .163 .181 .204
79	PC .235 .283 .663 .735 .772 .799 .820 .835 .850 .865
80	PC .880 .890 .900 .910 .916 .925 .934 .943 .952 .958
81	PC .964 .970 .976 .982 .988 .994 1.000
82	LS 0 92 0
83	UD 0.15
	* *
84	KK INTO3 COMBINE HYDROGRAPHS FOR BASIN 3 AND OUT OF POND #2
85	HC 2
	* *
	* 5' RIPRAP CHANNEL SPILLWAY FOR POND #3
86	KK POND3
87	RS 1 ELEV 184.0
88	SA 0.5 0.8
89	SE 184 187
90	SQ 0 40 60 80 90 100 110 120 130 140
91	SQ 150 160 170 180 190 200
92	SE 184 184.95 185.18 185.35 185.42 185.5 185.56 185.61 185.67 185.72
93	SE 185.78 185.84 185.89 185.93 185.98 186.02
	* *
94	ZZ

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE NO.	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
14	UNDEV	
23	BASIN1	
	V	
	V	
32	POND1	
	.	
38	BASIN2	
	.	
47	.	2A
	.	.
56	.	2B
	.	.
65	INTO2.....	
	V	
	V	
67	POND2	
	.	
75	BASIN3	
	.	
84	INTO3.....	
	V	
	V	
86	POND3	

(***) RUNOFF ALSO COMPUTED AT THIS LOCATION

BRADLEY FAIR 2ND ADDITION - REVISED

01-16-1997

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   MAY 1991
*   VERSION 4.0.1E
*   Lahey F77L-EM/32 version 5.01
*   Dodson & Associates, Inc.
*   RUN DATE 01/16/97 TIME 12:18:35
*****
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* U.S. ARMY CORPS OF ENGINEERS
* HYDROLOGIC ENGINEERING CENTER
*   609 SECOND STREET
*   DAVIS, CALIFORNIA 95616
*   (916) 551-1748
*****
```

Bradley Fair Drainage Plan
 5-, 10-, 25-, & 100-Year Storms
 Professional Engineering Consultants
 Wichita, Ks
 DRC 11/20/95
 REVISED MWB 12/02/96
 REVISED PDM 1/17/97
 File: T:\DAR\HEC1\BRADLEY8.IH1
 PEC PROJ NO 32-96109-042

```
12 IO      OUTPUT CONTROL VARIABLES
           IPRNT      3  PRINT CONTROL
           IPLOT      0  PLOT CONTROL
           QSCAL      0. HYDROGRAPH PLOT SCALE
```

```
IT         HYDROGRAPH TIME DATA
           NMIN      6  MINUTES IN COMPUTATION INTERVAL
           IDATE     1FEB95  STARTING DATE
           ITIME     0600  STARTING TIME
           NQ        300  NUMBER OF HYDROGRAPH ORDINATES
           NDDATE    2FEB95  ENDING DATE
           NDTIME    1154  ENDING TIME
           ICENT     19  CENTURY MARK

           COMPUTATION INTERVAL 0.10 HOURS
           TOTAL TIME BASE     29.90 HOURS
```

```
ENGLISH UNITS
DRAINAGE AREA      SQUARE MILES
PRECIPITATION DEPTH  INCHES
LENGTH, ELEVATION  FEET
FLOW               CUBIC FEET PER SECOND
STORAGE VOLUME     ACRE-FEET
SURFACE AREA       ACRES
TEMPERATURE        DEGREES FAHRENHEIT
```

```
JP         MULTI-PLAN OPTION
           NPLAN      1  NUMBER OF PLANS
```

```
JR         MULTI-RATIO OPTION
           RATIOS OF PRECIPITATION
           0.59      0.69      0.81      1.00
```

```
14 KK      *****
           *
           * UNDEV * UNDEVELOPED CONDITIONS
           *
           *****
```

```
11 IN      TIME DATA FOR INPUT TIME SERIES
           JXMIN      30  TIME INTERVAL IN MINUTES
           JXDATE     1FEB95  STARTING DATE
           JXTIME     600  STARTING TIME
```

SUBBASIN RUNOFF DATA

```
15 BA      SUBBASIN CHARACTERISTICS
           TAREA      0.06  SUBBASIN AREA
```

PRECIPITATION DATA

30 LS . SCS LOSS RATE
 STRTL 0.17 INITIAL ABSTRACTION
 CRVNR 92.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

31 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.15 LAG

UNIT HYDROGRAPH
 10 END-OF-PERIOD ORDINATES

71. 152. 103. 43. 19. 8. 4. 2. 1. 0.

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.95, TOTAL EXCESS = 6.85

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	29.90-HR
241.	6.00	(CFS)	37.	12.	9.	9.
		(INCHES)	5.534	6.846	6.846	6.846
		(AC-FT)	18.	23.	23.	23.

CUMULATIVE AREA = 0.06 SQ MI

HYDROGRAPH AT STATION BASIN1
 FOR PLAN 1, RATIO = 0.59

TOTAL RAINFALL = 4.60, TOTAL LOSS = 0.90, TOTAL EXCESS = 3.70

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	29.90-HR
134.	6.00	(CFS)	20.	6.	5.	5.
		(INCHES)	3.026	3.703	3.703	3.703
		(AC-FT)	10.	12.	12.	12.

CUMULATIVE AREA = 0.06 SQ MI

HYDROGRAPH AT STATION BASIN1
 FOR PLAN 1, RATIO = 0.69

TOTAL RAINFALL = 5.36, TOTAL LOSS = 0.92, TOTAL EXCESS = 4.44

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	29.90-HR
159.	6.00	(CFS)	24.	7.	6.	6.
		(INCHES)	3.622	4.444	4.444	4.444
		(AC-FT)	12.	15.	15.	15.

CUMULATIVE AREA = 0.06 SQ MI

HYDROGRAPH AT STATION BASIN1
 FOR PLAN 1, RATIO = 0.81

TOTAL RAINFALL = 6.34, TOTAL LOSS = 0.94, TOTAL EXCESS = 5.40

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	29.90-HR
192.	6.00	(CFS)	29.	9.	7.	7.
		(INCHES)	4.388	5.402	5.402	5.402
		(AC-FT)	15.	18.	18.	18.

CUMULATIVE AREA = 0.06 SQ MI

HYDROGRAPH AT STATION BASIN1
 FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.95, TOTAL EXCESS = 6.85

PEAK FLOW (CFS) 241.	TIME (HR) 6.00		MAXIMUM AVERAGE FLOW			
		(CFS)	6-HR 37.	24-HR 12.	72-HR 9.	29.90-HR 9.
		(INCHES)	5.534	6.846	6.846	6.846
		(AC-FT)	18.	23.	23.	23.

CUMULATIVE AREA = 0.06 SQ MI

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*****
*
32 KK * POND1 *
*
*****
    
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HYDROGRAPH ROUTING DATA

33 RS	STORAGE ROUTING									
	NSTPS	1	NUMBER OF SUBREACHES							
	ITYP	ELEV	TYPE OF INITIAL CONDITION							
	RSVRC	200.00	INITIAL CONDITION							
	X	0.00	WORKING R AND D COEFFICIENT							
34 SA	AREA	4.2	4.7							
35 SE	ELEVATION	200.00	202.00							
36 SQ	DISCHARGE	0.	20.	60.	110.	145.	150.	155.	165.	200.
37 SE	ELEVATION	200.00	200.40	200.80	201.40	201.60	201.80	202.20	203.00	206.00

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	8.87
ELEVATION	200.00	202.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	1.70	3.43	6.11	7.02	7.94	8.87	9.80	13.66	29.54
OUTFLOW	0.00	20.00	60.00	110.00	145.00	150.00	152.50	155.00	165.00	200.00
ELEVATION	200.00	200.40	200.80	201.40	201.60	201.80	202.00	202.20	203.00	206.00

*** **

HYDROGRAPH AT STATION POND1
FOR PLAN 1, RATIO = 0.59

PEAK FLOW (CFS) 69.	TIME (HR) 6.20		MAXIMUM AVERAGE FLOW			
		(CFS)	6-HR 20.	24-HR 6.	72-HR 5.	29.90-HR 5.
		(INCHES)	2.964	3.703	3.703	3.703
		(AC-FT)	10.	12.	12.	12.

PEAK STORAGE (AC-FT) 4.	TIME (HR) 6.20		MAXIMUM AVERAGE STORAGE			
		(AC-FT)	6-HR 1.	24-HR 0.	72-HR 0.	29.90-HR 0.

PEAK STAGE (FEET) 200.91	TIME (HR) 6.20		MAXIMUM AVERAGE STAGE			
		(FEET)	6-HR 200.33	24-HR 200.11	72-HR 200.09	29.90-HR 200.09

CUMULATIVE AREA = 0.06 SQ MI

*** **

HYDROGRAPH AT STATION POND1
FOR PLAN 1, RATIO = 0.69

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
-----------	------	--	----------------------	--	--	--

(CFS)	(HR)	(CFS)	6-HR	24-HR	72-HR	29.90-HR
83.	6.20	24.	24.	7.	6.	6.
		(INCHES)	3.550	4.444	4.444	4.444
		(AC-FT)	12.	15.	15.	15.
PEAK STORAGE (AC-FT)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
5.	6.20		2.	1.	0.	0.
PEAK STAGE (FEET)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
201.07	6.20		200.38	200.12	200.10	200.10

CUMULATIVE AREA = 0.06 SQ MI

*** **

HYDROGRAPH AT STATION POND1
FOR PLAN 1, RATIO = 0.81

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	29.90-HR
101.	6.20	29.	29.	9.	7.	7.
		(INCHES)	4.305	5.401	5.402	5.402
		(AC-FT)	14.	18.	18.	18.
PEAK STORAGE (AC-FT)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
6.	6.20		2.	1.	1.	1.
PEAK STAGE (FEET)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
201.29	6.20		200.45	200.15	200.12	200.12

CUMULATIVE AREA = 0.06 SQ MI

*** **

HYDROGRAPH AT STATION POND1
FOR PLAN 1, RATIO = 1.00

PEAK FLOW (CFS)	TIME (HR)	(CFS)	6-HR	24-HR	72-HR	29.90-HR
140.	6.20	37.	37.	12.	9.	9.
		(INCHES)	5.438	6.845	6.846	6.846
		(AC-FT)	18.	23.	23.	23.
PEAK STORAGE (AC-FT)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
7.	6.20		2.	1.	1.	1.
PEAK STAGE (FEET)	TIME (HR)		6-HR	24-HR	72-HR	29.90-HR
201.57	6.20		200.54	200.18	200.15	200.15

CUMULATIVE AREA = 0.06 SQ MI

* *
38 KK * BASIN2 * COMMERCIAL DEVELOPED CONDITIONS
* *

11 IN TIME DATA FOR INPUT TIME SERIES
JXMIN 30 TIME INTERVAL IN MINUTES
JXDATE 1FEB95 STARTING DATE
JXTIME 600 STARTING TIME

SUBBASIN RUNOFF DATA

39 BA SUBBASIN CHARACTERISTICS

0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.00

54 LS SCS LOSS RATE
 STRTL 0.08 INITIAL ABSTRACTION
 CRVNBR 96.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

55 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.15 LAG

UNIT HYDROGRAPH
 10 END-OF-PERIOD ORDINATES

8. 16. 11. 4. 2. 1. 0. 0. 0. 0.

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.48, TOTAL EXCESS = 7.32

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
26.	6.00	6-HR	24-HR	72-HR	29.90-HR	
		(CFS)	4.	1.	1.	1.
		(INCHES)	5.808	7.321	7.321	7.321
		(AC-FT)	2.	3.	3.	3.

CUMULATIVE AREA = 0.01 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2A
 FOR PLAN 1, RATIO = 0.59

TOTAL RAINFALL = 4.60, TOTAL LOSS = 0.46, TOTAL EXCESS = 4.14

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
15.	6.00	6-HR	24-HR	72-HR	29.90-HR	
		(CFS)	2.	1.	1.	1.
		(INCHES)	3.328	4.139	4.139	4.139
		(AC-FT)	1.	1.	1.	1.

CUMULATIVE AREA = 0.01 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2A
 FOR PLAN 1, RATIO = 0.69

TOTAL RAINFALL = 5.36, TOTAL LOSS = 0.47, TOTAL EXCESS = 4.89

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
18.	6.00	6-HR	24-HR	72-HR	29.90-HR	
		(CFS)	3.	1.	1.	1.
		(INCHES)	3.919	4.893	4.893	4.893
		(AC-FT)	1.	2.	2.	2.

CUMULATIVE AREA = 0.01 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2A
 FOR PLAN 1, RATIO = 0.81

TOTAL RAINFALL = 6.34, TOTAL LOSS = 0.47, TOTAL EXCESS = 5.86

PEAK FLOW (CFS)	TIME (HR)		MAXIMUM AVERAGE FLOW			
21.	6.00	6-HR	24-HR	72-HR	29.90-HR	
		(CFS)	3.	1.	1.	1.
		(INCHES)	4.676	5.864	5.864	5.864
		(AC-FT)	2.	2.	2.	2.

CUMULATIVE AREA = 0.01 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2A
 FOR PLAN 1, RATIO = 1.00

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2B
FOR PLAN 1, RATIO = 0.59

TOTAL RAINFALL = 4.60, TOTAL LOSS = 0.24, TOTAL EXCESS = 4.37

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
2.	6.00	(CFS) 0.	0.	0.	0.
		(INCHES) 3.450	4.367	4.367	4.367
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2B
FOR PLAN 1, RATIO = 0.69

TOTAL RAINFALL = 5.36, TOTAL LOSS = 0.24, TOTAL EXCESS = 5.13

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
3.	6.00	(CFS) 0.	0.	0.	0.
		(INCHES) 4.036	5.125	5.125	5.125
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2B
FOR PLAN 1, RATIO = 0.81

TOTAL RAINFALL = 6.34, TOTAL LOSS = 0.24, TOTAL EXCESS = 6.10

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
3.	6.00	(CFS) 1.	0.	0.	0.
		(INCHES) 4.786	6.099	6.099	6.099
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION 2B
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.24, TOTAL EXCESS = 7.56

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
4.	6.00	(CFS) 1.	0.	0.	0.
		(INCHES) 5.910	7.560	7.560	7.560
		(AC-FT) 0.	0.	0.	0.

CUMULATIVE AREA = 0.00 SQ MI

*** **

* *
65 KK * INTO2 * COMBINE HYDROGRAPHS FOR BASIN 2, 2A, 2B AND OUT OF POND #1
* *

66 HC HYDROGRAPH COMBINATION

ICOMP 4 NUMBER OF HYDROGRAPHS TO COMBINE

*** *** *** *** ***

HYDROGRAPH AT STATION INTO2
FOR PLAN 1, RATIO = 0.59

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
87.	6.10	(CFS)	24.	7.	6.
		(INCHES)	2.992	3.750	3.751
		(AC-FT)	12.	15.	15.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION INTO2
FOR PLAN 1, RATIO = 0.69

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
104.	6.10	(CFS)	29.	9.	7.
		(INCHES)	3.578	4.493	4.493
		(AC-FT)	14.	18.	18.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION INTO2
FOR PLAN 1, RATIO = 0.81

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
126.	6.10	(CFS)	35.	11.	9.
		(INCHES)	4.332	5.452	5.452
		(AC-FT)	17.	22.	22.

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION INTO2
FOR PLAN 1, RATIO = 1.00

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
167.	6.10	(CFS)	44.	14.	11.
		(INCHES)	5.465	6.897	6.898
		(AC-FT)	22.	27.	27.

CUMULATIVE AREA = 0.07 SQ MI

*** **

* *
67 KK * POND2 *
* *

HYDROGRAPH ROUTING DATA

68 RS STORAGE ROUTING
NSTPS 1 NUMBER OF SUBREACHES
ITYP ELEV TYPE OF INITIAL CONDITION
RSVRIC 192.00 INITIAL CONDITION
X 0.00 WORKING R AND D COEFFICIENT

69 SA	AREA	0.5	0.8								
70 SE	ELEVATION	192.00	194.00								
71 SQ	DISCHARGE	0.	40.	60.	80.	90.	100.	110.	120.	130.	140.
		150.	160.	170.	180.	190.	200.				
73 SE	ELEVATION	192.00	192.95	193.18	193.35	193.42	193.50	193.56	193.61	193.67	193.72
		193.78	193.84	193.89	193.93	193.98	194.02				

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	1.29
ELEVATION	192.00	194.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.54	0.69	0.80	0.85	0.91	0.95	0.99	1.03	1.07
OUTFLOW	0.00	40.00	60.00	80.00	90.00	100.00	110.00	120.00	130.00	140.00
ELEVATION	192.00	192.95	193.18	193.35	193.42	193.50	193.56	193.61	193.67	193.72
STORAGE	1.12	1.16	1.20	1.23	1.27	1.29	1.30			
OUTFLOW	150.00	160.00	170.00	180.00	190.00	195.00	200.00			
ELEVATION	193.78	193.84	193.89	193.93	193.98	194.00	194.02			

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 110. TO 200.
 THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
 THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

*** *** *** *** ***

HYDROGRAPH AT STATION POND2
 FOR PLAN 1, RATIO = 0.59

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
85.	6.20	(CFS)	24.	7.	6.	6.
		(INCHES)	2.990	3.750	3.751	3.751
		(AC-FT)	12.	15.	15.	15.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	29.90-HR
1.	6.20		0.	0.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	29.90-HR
193.38	6.20		192.51	192.16	192.13	192.13

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND2
 FOR PLAN 1, RATIO = 0.69

PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	29.90-HR
101.	6.20	(CFS)	29.	9.	7.	7.
		(INCHES)	3.576	4.493	4.493	4.493
		(AC-FT)	14.	18.	18.	18.
PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	29.90-HR
1.	6.20		0.	0.	0.	0.
PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	29.90-HR
193.51	6.20		192.57	192.19	192.15	192.15

CUMULATIVE AREA = 0.07 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND2
 FOR PLAN 1, RATIO = 0.81

0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00

82 LS SCS LOSS RATE
 STRTL 0.17 INITIAL ABSTRACTION
 CRVNER 92.00 CURVE NUMBER
 RTIMP 0.00 PERCENT IMPERVIOUS AREA

83 UD SCS DIMENSIONLESS UNITGRAPH
 TLAG 0.15 LAG

UNIT HYDROGRAPH
 10 END-OF-PERIOD ORDINATES

5.	11.	8.	3.	1.	1.	0.	0.	0.	0.
----	-----	----	----	----	----	----	----	----	----

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.95, TOTAL EXCESS = 6.85

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
18.	6.00	(CFS) 3.	1.	1.	1.
		(INCHES) 5.534	6.846	6.846	6.846
		(AC-FT) 1.	2.	2.	2.

CUMULATIVE AREA = 0.00 SQ MI

HYDROGRAPH AT STATION BASIN3
 FOR PLAN 1, RATIO = 0.59

TOTAL RAINFALL = 4.60, TOTAL LOSS = 0.90, TOTAL EXCESS = 3.70

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
10.	6.00	(CFS) 1.	0.	0.	0.
		(INCHES) 3.026	3.703	3.703	3.703
		(AC-FT) 1.	1.	1.	1.

CUMULATIVE AREA = 0.00 SQ MI

HYDROGRAPH AT STATION BASIN3
 FOR PLAN 1, RATIO = 0.69

TOTAL RAINFALL = 5.36, TOTAL LOSS = 0.92, TOTAL EXCESS = 4.44

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
12.	6.00	(CFS) 2.	1.	0.	0.
		(INCHES) 3.622	4.444	4.444	4.444
		(AC-FT) 1.	1.	1.	1.

CUMULATIVE AREA = 0.00 SQ MI

HYDROGRAPH AT STATION BASIN3
 FOR PLAN 1, RATIO = 0.81

TOTAL RAINFALL = 6.34, TOTAL LOSS = 0.94, TOTAL EXCESS = 5.40

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
14.	6.00	(CFS) 2.	1.	1.	1.
		(INCHES) 4.388	5.402	5.402	5.402
		(AC-FT) 1.	1.	1.	1.

CUMULATIVE AREA = 0.00 SQ MI

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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  BASIN3
FOR PLAN 1, RATIO = 1.00

TOTAL RAINFALL = 7.80, TOTAL LOSS = 0.95, TOTAL EXCESS = 6.85

PEAK FLOW      TIME      MAXIMUM AVERAGE FLOW
(CFS)          (HR)          6-HR      24-HR      72-HR      29.90-HR
18.           6.00          (CFS)      3.         1.         1.         1.
              (INCHES)    5.534     6.846     6.846     6.846
              (AC-FT)    1.         2.         2.         2.

CUMULATIVE AREA = 0.00 SQ MI
    
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*          *
84 KK      *   INTO3 *          COMBINE HYDROGRAPHS FOR BASIN 3 AND OUT OF POND #2
*          *   *
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85 HC      HYDROGRAPH COMBINATION
           ICOMP          2 NUMBER OF HYDROGRAPHS TO COMBINE
    
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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  INTO3
FOR PLAN 1, RATIO = 0.59

PEAK FLOW      TIME      MAXIMUM AVERAGE FLOW
(CFS)          (HR)          6-HR      24-HR      72-HR      29.90-HR
90.           6.20          (CFS)      25.        8.         6.         6.
              (INCHES)    2.990     3.748     3.748     3.748
              (AC-FT)    13.       16.       16.       16.

CUMULATIVE AREA = 0.08 SQ MI
    
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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  INTO3
FOR PLAN 1, RATIO = 0.69

PEAK FLOW      TIME      MAXIMUM AVERAGE FLOW
(CFS)          (HR)          6-HR      24-HR      72-HR      29.90-HR
108.          6.20          (CFS)      30.        9.         8.         8.
              (INCHES)    3.575     4.490     4.490     4.490
              (AC-FT)    15.       19.       19.       19.

CUMULATIVE AREA = 0.08 SQ MI
    
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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  INTO3
FOR PLAN 1, RATIO = 0.81

PEAK FLOW      TIME      MAXIMUM AVERAGE FLOW
(CFS)          (HR)          6-HR      24-HR      72-HR      29.90-HR
134.          6.10          (CFS)      37.        12.        9.         9.
              (INCHES)    4.329     5.449     5.449     5.449
              (AC-FT)    18.       23.       23.       23.

CUMULATIVE AREA = 0.08 SQ MI
    
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***          ***          ***          ***          ***
HYDROGRAPH AT STATION  INTO3
FOR PLAN 1, RATIO = 1.00
    
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PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
176.	6.20	(CFS) 46.	15.	12.	12.
		(INCHES) 5.461	6.894	6.895	6.895
		(AC-FT) 23.	29.	29.	29.

CUMULATIVE AREA = 0.08 SQ MI

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* *
86 KK * POND3 *
* *

HYDROGRAPH ROUTING DATA

87 RS	STORAGE ROUTING	1 NUMBER OF SUBREACHES									
		NSTPS	ELEV	TYPE OF INITIAL CONDITION							
	ITYP	184.00	INITIAL CONDITION								
	RSVRIC	0.00	WORKING R AND D COEFFICIENT								
	X										
88 SA	AREA	0.5	0.8								
89 SE	ELEVATION	184.00	187.00								
90 SQ	DISCHARGE	0.	40.	60.	80.	90.	100.	110.	120.	130.	140.
		150.	160.	170.	180.	190.	200.				
92 SE	ELEVATION	184.00	184.95	185.18	185.35	185.42	185.50	185.56	185.61	185.67	185.72
		185.78	185.84	185.89	185.93	185.98	186.02				

COMPUTED STORAGE-ELEVATION DATA

STORAGE	0.00	1.93
ELEVATION	184.00	187.00

COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

STORAGE	0.00	0.52	0.65	0.76	0.80	0.85	0.89	0.92	0.96	1.00
OUTFLOW	0.00	40.00	60.00	80.00	90.00	100.00	110.00	120.00	130.00	140.00
ELEVATION	184.00	184.95	185.18	185.35	185.42	185.50	185.56	185.61	185.67	185.72

STORAGE	1.04	1.08	1.11	1.14	1.17	1.20	1.93
OUTFLOW	150.00	160.00	170.00	180.00	190.00	200.00	444.95
ELEVATION	185.78	185.84	185.89	185.93	185.98	186.02	187.00

*** WARNING *** MODIFIED PULS ROUTING MAY BE NUMERICALLY UNSTABLE FOR OUTFLOWS BETWEEN 100. TO 445.
THE ROUTED HYDROGRAPH SHOULD BE EXAMINED FOR OSCILLATIONS OR OUTFLOWS GREATER THAN PEAK INFLOWS.
THIS CAN BE CORRECTED BY DECREASING THE TIME INTERVAL OR INCREASING STORAGE (USE A LONGER REACH.)

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HYDROGRAPH AT STATION POND3
FOR PLAN 1, RATIO = 0.59

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW			
		6-HR	24-HR	72-HR	29.90-HR
87.	6.20	(CFS) 25.	8.	6.	6.
		(INCHES) 2.988	3.748	3.748	3.748
		(AC-FT) 13.	16.	16.	16.

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	29.90-HR
1.	6.20	0.	0.	0.	0.

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	29.90-HR
185.40	6.20	184.53	184.17	184.14	184.14

CUMULATIVE AREA = 0.08 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND3
FOR PLAN 1, RATIO = 0.69

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				29.90-HR
		6-HR	24-HR	72-HR		
108.	6.20	(CFS) 30.	9.	8.	8.	
		(INCHES) 3.573	4.490	4.490	4.490	
		(AC-FT) 15.	19.	19.	19.	

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE				29.90-HR
		6-HR	24-HR	72-HR		
1.	6.20	0.	0.	0.	0.	

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE				29.90-HR
		6-HR	24-HR	72-HR		
185.55	6.20	184.60	184.20	184.16	184.16	

CUMULATIVE AREA = 0.08 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND3
FOR PLAN 1, RATIO = 0.81

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				29.90-HR
		6-HR	24-HR	72-HR		
133.	6.20	(CFS) 37.	12.	9.	9.	
		(INCHES) 4.327	5.449	5.449	5.449	
		(AC-FT) 18.	23.	23.	23.	

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE				29.90-HR
		6-HR	24-HR	72-HR		
1.	6.20	0.	0.	0.	0.	

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE				29.90-HR
		6-HR	24-HR	72-HR		
185.68	6.20	184.68	184.23	184.18	184.18	

CUMULATIVE AREA = 0.08 SQ MI

*** *** *** *** ***

HYDROGRAPH AT STATION POND3
FOR PLAN 1, RATIO = 1.00

PEAK FLOW (CFS)	TIME (HR)	MAXIMUM AVERAGE FLOW				29.90-HR
		6-HR	24-HR	72-HR		
175.	6.20	(CFS) 46.	15.	12.	12.	
		(INCHES) 5.459	6.894	6.895	6.895	
		(AC-FT) 23.	29.	29.	29.	

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE				29.90-HR
		6-HR	24-HR	72-HR		
1.	6.20	0.	0.	0.	0.	

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE				29.90-HR
		6-HR	24-HR	72-HR		
185.91	6.20	184.79	184.27	184.22	184.22	

CUMULATIVE AREA = 0.08 SQ MI

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION			
				RATIO 1	RATIO 2	RATIO 3	RATIO 4
				0.59	0.69	0.81	1.00
HYDROGRAPH AT	UNDEV	0.06	1 FLOW	75.	97.	125.	170.
			TIME	6.10	6.10	6.10	6.10
HYDROGRAPH AT	BASIN1	0.06	1 FLOW	134.	159.	192.	241.
			TIME	6.00	6.00	6.00	6.00
ROUTED TO	POND1	0.06	1 FLOW	69.	83.	101.	140.
			TIME	6.20	6.20	6.20	6.20
** PEAK STAGES IN FEET **							
			1 STAGE	200.91	201.07	201.29	201.57
			TIME	6.20	6.20	6.20	6.20
HYDROGRAPH AT	BASIN2	0.00	1 FLOW	9.	10.	12.	15.
			TIME	6.00	6.00	6.00	6.00
HYDROGRAPH AT	2A	0.01	1 FLOW	15.	18.	21.	26.
			TIME	6.00	6.00	6.00	6.00
HYDROGRAPH AT	2B	0.00	1 FLOW	2.	3.	3.	4.
			TIME	6.00	6.00	6.00	6.00
4 COMBINED AT	INTO2	0.07	1 FLOW	87.	104.	126.	167.
			TIME	6.10	6.10	6.10	6.10
ROUTED TO	POND2	0.07	1 FLOW	85.	101.	123.	166.
			TIME	6.20	6.20	6.20	6.20
** PEAK STAGES IN FEET **							
			1 STAGE	193.38	193.51	193.63	193.87
			TIME	6.20	6.20	6.20	6.20
HYDROGRAPH AT	BASIN3	0.00	1 FLOW	10.	12.	14.	18.
			TIME	6.00	6.00	6.00	6.00
2 COMBINED AT	INTO3	0.08	1 FLOW	90.	108.	134.	176.
			TIME	6.20	6.20	6.10	6.20
ROUTED TO	POND3	0.08	1 FLOW	87.	108.	133.	175.
			TIME	6.20	6.20	6.20	6.20
** PEAK STAGES IN FEET **							
			1 STAGE	185.40	185.55	185.68	185.91
			TIME	6.20	6.20	6.20	6.20

*** NORMAL END OF HEC-1 ***