



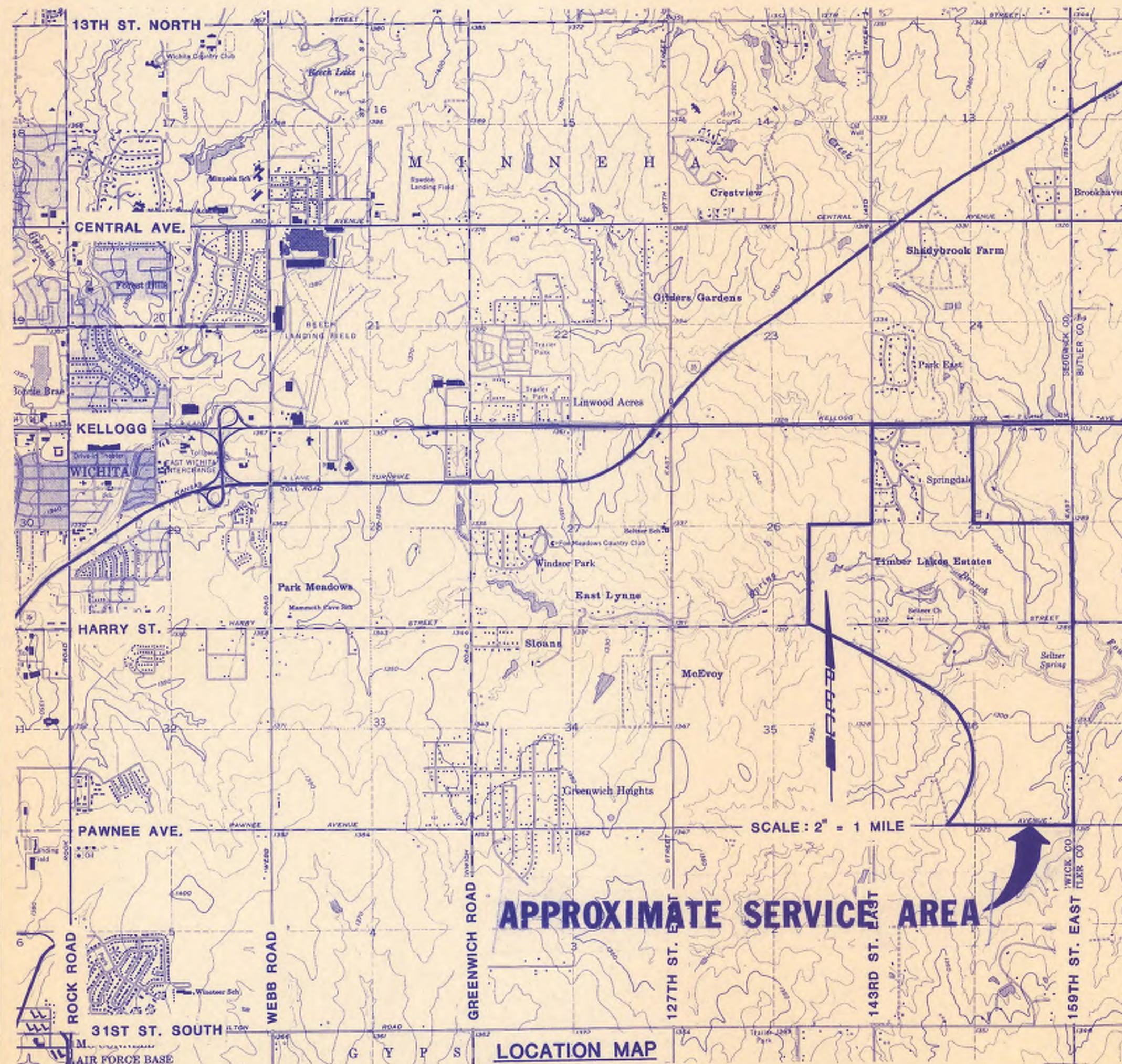
FOUR MILE CREEK ^{4MC-002}

CONSTRUCTION PLANS FOR A SANITARY SEWER INTERCEPTOR

IN THE

FOUR MILE CREEK JOINT SEWER DISTRICT

Project No. 34-81051-2-024
JANUARY, 1983



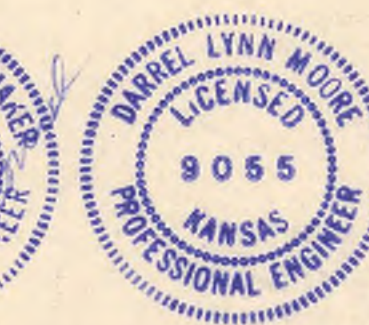
INDEX OF SHEETS	
SHEET NO. 1	TITLE SHEET
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SHEET NO. 4-9	PLAN/PROFILE
SHEET NO. 10	MISCELLANEOUS DETAILS
SHEET NO. 11	PRECAST MH DETAILS
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SHEET NO. 13	REINFORCED CONCRETE MH DETAILS
SHEET NO. 14	BACKFILL DETAILS
SHEET NO. S1 OF 1	EXISTING DAM REPAIR

APPROVED BY: SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES	
DIRECTOR _____	DATE _____

APPROVED BY: BOARD OF COUNTY COMMISSIONERS	
CHAIR _____	DATE _____

FILED IN THE OFFICE OF: THE SEDGWICK COUNTY CLERK	
COUNTY CLERK _____	DATE _____

RECORD DRAWING
MDK PROJ. ENG. 4/27/94 DATE
GRG RES. ENG. 4/15/94 DATE



UPDATED PER CURRENT STANDARDS 5/92

EPA Project No. C200691-02



PROFESSIONAL ENGINEERING CONSULTANTS, P.A.
WICHITA, KANSAS



SCHEDULE OF SHEETS	
STATION TO STATION	SHEET NO.
0+00.00 to 9+00.00	4
9+00.00 to 15+58.00	5
15+58.00 to 20+83.91 back 17+92.05 ahead	6
20+83.91 back to 26+54.75 17+92.05 ahead	7
26+54.75 to 37+23.59	8
37+23.59 to 47+68.10	9

CONSTRUCTION SEQUENCING

THE CONTRACTOR SHALL SUBMIT HIS PROPOSED CONSTRUCTION SCHEDULE TO THE ENGINEER FOR APPROVAL AT LEAST 10 DAYS PRIOR TO THE NOTICE TO PROCEED. THE CONTRACTORS PROPOSED SCHEDULE SHALL INCLUDE, BUT SHALL NOT NECESSARILY BE LIMITED TO THE FOLLOWING ITEMS:

1. DRAIN THE LAKE BY OPENING VALVES ON THE TWO DRAIN PIPES AT THE DAM. SEE DRAINING LAKE AND DAM MODIFICATIONS NOTE, SHT. 10.
2. CONSTRUCT SANITARY SEWER BETWEEN MH TA-8 AND MH TA-14. ALSO, COMPLETE IMPROVEMENTS TO THE DRAIN PIPES AND VALVES AT THE DAM. THE SANITARY SEWER BETWEEN MH TA-8 AND TA-14 SHALL BE TELEVIEWED AND ACCEPTED BY THE OWNER PRIOR TO ALLOWING THE LAKE TO REFILL. THE CONTRACTOR SHALL SUBSTANTIALLY COMPLETE THIS LINE, INCLUDING ALL BACKFILL, SEEDING, MULCHING, AND TESTING PRIOR TO BEGINNING WORK ON ANY OTHER PORTION OF THE PROJECT. UPON COMPLETION OF THE ABOVE LISTED CONSTRUCTION, THE CONTRACTOR SHALL CLOSE THE VALVES AT THE DAM AND ALLOW THE LAKE TO REFILL. PROPOSED MH TA-10 SHALL BE CONSTRUCTED AROUND THE EXISTING 18" PIPE BETWEEN EXISTING MH TLS-9 AND MH TLS-10 AND IT SHALL REMAIN IN SERVICE.
3. CONSTRUCT SANITARY SEWER BETWEEN EXISTING MH TA-4 AND PROPOSED MH TA-8.
4. REMOVE EXISTING SPIROLITE MH TA-4 AND REPLACE WITH A 5 FT. DIAMETER TYPE C MH. INSTALL TEMPORARY 36" PLUG IN THE 36" PIPE ENTERING MH TA-4.

5. CONSTRUCT SANITARY SEWER BETWEEN PROPOSED MH TA-14 AND EXISTING MH TA-22. MH TA-15 AND TA-18 SHALL BE CONSTRUCTED AROUND THE EXISTING LINES. THE EXISTING LINE BETWEEN EXISTING MH TLS-27 AND PROPOSED MH TA-15 SHALL REMAIN IN SERVICE UNTIL THE REMAINDER OF THE PROJECT IS ACCEPTED BY THE OWNER. THE EXISTING 21" PIPE THAT PROPOSED MH TA-18 WILL BE BUILT AROUND SHALL REMAIN IN SERVICE.
6. BREAK INTO THE EXISTING LINE IN PROPOSED MH TA-10. THE EXISTING PIPE SHALL BE CUT AT THE MANHOLE WALL ENTERING AND EXITING THE MANHOLE AND REMOVED. THE REMAINING PIPE BETWEEN EXISTING MH TLS-9 AND PROPOSED MA TA-10 SHALL BE ABANDONED IN PLACE BY PLUGGING BOTH ENDS WITH A 12" THICK CONCRETE PLUG, AS SHOWN IN THE PLANS.
7. BREAK INTO THE EXISTING LINE IN PROPOSED MH TA-15. THE EXISTING PIPE SHALL BE CUT AT THE MANHOLE WALL ENTERING AND EXITING THE MH. THE PIPE BETWEEN EXISTING MH TLS-27 AND PROPOSED MH TA-15 SHALL BE ABANDONED IN PLACE BY PLUGGING BOTH ENDS WITH A 12" THICK CONCRETE PLUG, AS SHOWN IN THE PLANS.
8. BREAK INTO THE EXISTING LINE IN PROPOSED MH TA-18. THE EXISTING PIPE FROM THE NORTH SHALL BE CUT AT THE MH WALL. THE PIPE BETWEEN EXISTING MH SHI-2 AND PROPOSED MH TA-18 SHALL BE REMOVED AND RESET, AS SHOWN IN THE PLANS. THE CONTRACTOR SHALL PROVIDE TEMPORARY PUMPING OF WASTEWATER FLOWS TO PERFORM THIS WORK. UPON COMPLETION OF THIS WORK, THE CONTRACTOR SHALL PLUG THE EXISTING SANITARY SEWER BETWEEN EXISTING MH SHI-1 AND MH SHI-2 WITH A 12" THICK CONCRETE PLUG AT EACH MANHOLE.

9. PLUG AND ABANDON THE EXISTING 12" SANITARY SEWER BETWEEN EXISTING MH SHI-7 AND EXISTING MH TA-22 WITH A 12" THICK CONCRETE PLUG AT EACH MANHOLE.

10. FOR REHABILITATION OF EXISTING MH TA-22, TLS-9, TLS-27, SHI-1, SHI-2, AND SHI-7, TLS-25 SEE GENERAL NOTES.

11. THE FOLLOWING MH'S WERE RAISED: TA-4, MH EAST OF TA-8

SCALE: 1" = 200'

LEGEND

- Existing Sanitary Sewer
- Proposed Interceptor

SEE SHEET NO.10
FOR GENERAL NOTES

AERIAL PHOTO PROVIDED BY M.J. HARDEN ASSOCIATES, INC.
FLOWN MARCH, 1981

RECORD DRAWING

DATE 4/27/94
PROJ. ENG. DAVID C. SPEARS, P.E.
RES. ENG. GREGORY J. GARGAN, P.E.



Updated per current standards		RJ	5/02
No.	Revision	By	Date
SEDCWICK COUNTY BUREAU OF PUBLIC SERVICES DAVID C. SPEARS, P.E. DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER			
KEY MAP			
SANITARY SEWER INTERCEPTOR FOUR MILE CREEK JOINT SEWER DISTRICT			
PROFESSIONAL ENGINEERING CONSULTANTS, P.A.			
ENGINEERS WICHITA, KANSAS			
Designed by	MDS	Job No.	81051-2
Drawn by	TLS	Date	Jan. 1983
		Sht.	2 of 14

FOUR MILE CREEK
SECTION CORNER TIES

SECTION 25

CENTER 568,990.0631 N 387,931.4494 E
(CALCULATED)

SOUTH 1/4 CORNER 566,334.7546 N 387,974.1210 E
1/2" IRON PIPE IN THIMBLE
123.64' SE, NAIL AND SHINER IN NORTHWEST FACE UTILITY POLE
59.91' N, NAIL AND SHINER IN WEST FACE FENCE POST
88.13' NW, NAIL AND SHINER IN WEST FACE FENCE POST

SOUTHWEST CORNER 566,288.8954 N 385,318.8960 E
3/4" IRON PIPE IN THIMBLE
56.70' NE, NAIL AND SHINER IN SOUTHEAST FACE UTILITY POLE
58.57' NW, NAIL AND SHINER IN EAST FACE UTILITY POLE
41.29' SE, NORTHWEST CORNER OF STONE WALL

WEST 1/4 CORNER 568,944.8998 N 385,284.1660 E
3/4" IRON PIPE IN THIMBLE
53.95' NE, NAIL AND SHINER IN NORTHWEST FACE UTILITY POLE
53.94' SE, NAIL AND SHINER IN SOUTHWEST FACE 18" ELM
38.15' W, NAIL AND SHINER IN NORTH FACE UTILITY POLE

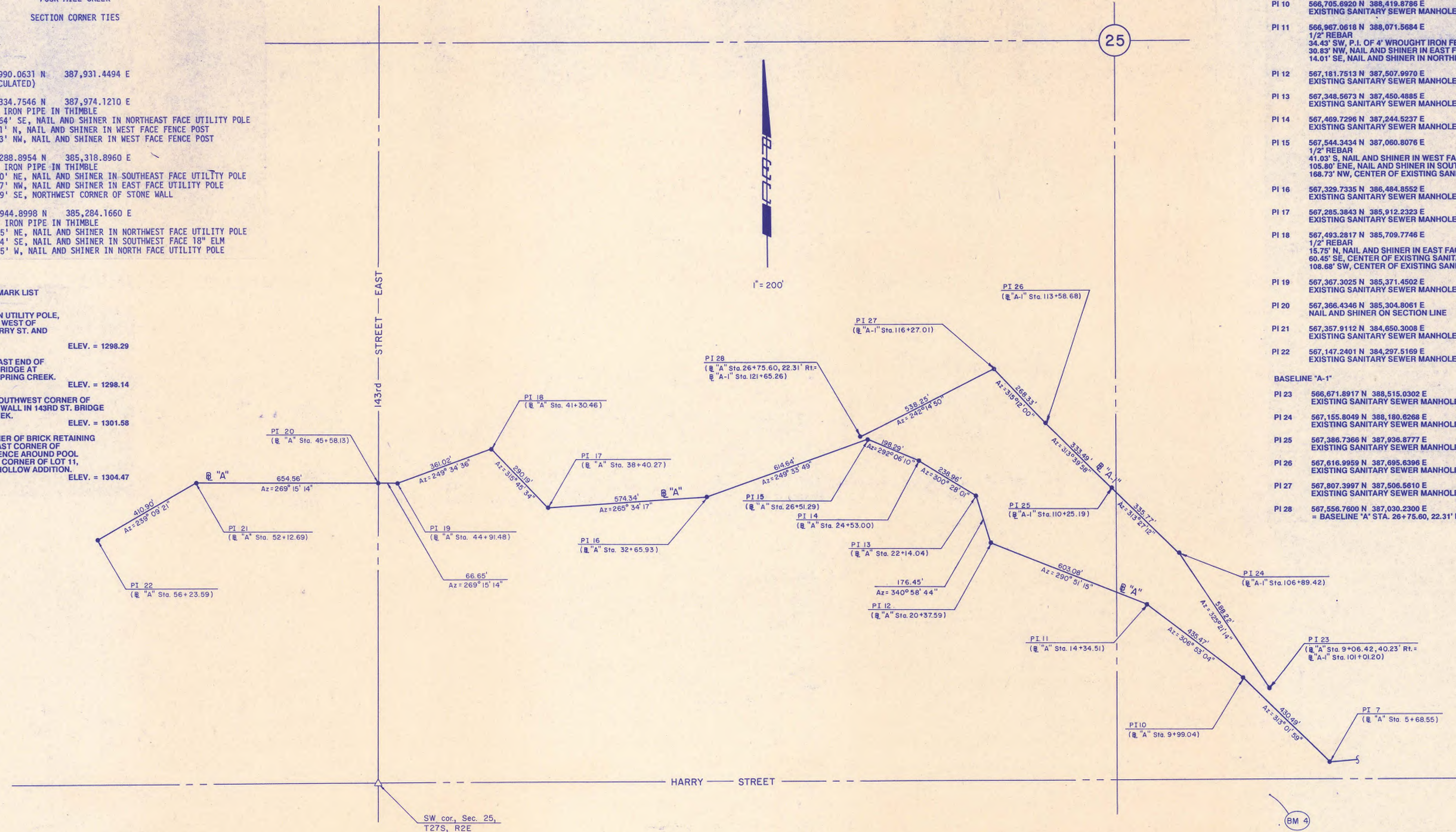
BENCH MARK LIST

BM 4 RAILROAD SPIKE IN UTILITY POLE,
60' SOUTH AND 25' WEST OF
INTERSECTION HARRY ST. AND
ARBOR LAKES DR. ELEV. = 1298.29

BM 101 CHISELED "C" IN EAST END OF
NORTH CURB OF BRIDGE AT
HARRY ST. OVER SPRING CREEK. ELEV. = 1298.14

BM 108 CHISELED "C" IN SOUTHWEST CORNER OF
SOUTHWEST WINGWALL IN 143RD ST. BRIDGE
OVER SPRING CREEK. ELEV. = 1301.58

BM 109 NORTHEAST CORNER OF BRICK RETAINING
WALL AT NORTHEAST CORNER OF
WROUGHT IRON FENCE AROUND POOL
NEAR NORTHEAST CORNER OF LOT 11,
BLOCK 7, SPRINGHOLLOW ADDITION. ELEV. = 1304.47



P.I. TIES	
BASELINE 'A'	
PI 7	566,411.9159 N 388,734.5491 E 1/2" REBAR 58.45' SE, CHISELED "X" IN NORTHWEST END OF NORTHWEST WINGWALL TO CONCRETE BRIDGE AT HARRY ST. AND SPRING CREEK 165.05' WSW, CENTER OF EXISTING SANITARY SEWER MANHOLE 40.49' ENE, NAIL AND SHINER IN NORTH FACE 6" OSAGE ORANGE
PI 10	566,705.6920 N 388,419.8786 E EXISTING SANITARY SEWER MANHOLE
PI 11	566,967.0618 N 388,071.5684 E 1/2" REBAR 34.43' SW, P.I. OF 4' WROUGHT IRON FENCE 30.83' NW, NAIL AND SHINER IN EAST FACE 18" HACKBERRY 14.01' SE, NAIL AND SHINER IN NORTH FACE 24" HACKBERRY
PI 12	567,181.7513 N 387,507.9970 E EXISTING SANITARY SEWER MANHOLE
PI 13	567,348.5673 N 387,450.4885 E EXISTING SANITARY SEWER MANHOLE
PI 14	567,469.7296 N 387,244.5237 E EXISTING SANITARY SEWER MANHOLE
PI 15	567,544.3434 N 387,060.8076 E 1/2" REBAR 41.03' S, NAIL AND SHINER IN WEST FACE 14" WALNUT 105.80' ENE, NAIL AND SHINER IN SOUTH FACE 15' ELM 168.73' NW, CENTER OF EXISTING SANITARY SEWER MANHOLE
PI 16	567,329.7335 N 386,484.8552 E EXISTING SANITARY SEWER MANHOLE
PI 17	567,285.3843 N 385,912.2323 E EXISTING SANITARY SEWER MANHOLE
PI 18	567,493.2817 N 385,709.7746 E 1/2" REBAR 15.75' N, NAIL AND SHINER IN EAST FACE 10" COTTONWOOD 60.45' SE, CENTER OF EXISTING SANITARY SEWER MANHOLE 108.68' SW, CENTER OF EXISTING SANITARY SEWER MANHOLE
PI 19	567,367.3025 N 385,371.4502 E EXISTING SANITARY SEWER MANHOLE
PI 20	567,366.4346 N 385,304.8061 E NAIL AND SHINER ON SECTION LINE
PI 21	567,357.9112 N 384,650.3008 E EXISTING SANITARY SEWER MANHOLE
PI 22	567,147.2401 N 384,297.5169 E EXISTING SANITARY SEWER MANHOLE
BASELINE 'A-1'	
PI 23	566,671.8917 N 388,515.0302 E EXISTING SANITARY SEWER MANHOLE
PI 24	567,155.8049 N 388,180.6268 E EXISTING SANITARY SEWER MANHOLE
PI 25	567,386.7366 N 387,936.8777 E EXISTING SANITARY SEWER MANHOLE
PI 26	567,616.9959 N 387,695.6396 E EXISTING SANITARY SEWER MANHOLE
PI 27	567,907.3997 N 387,506.5610 E EXISTING SANITARY SEWER MANHOLE
PI 28	567,556.7600 N 387,030.2300 E = BASELINE 'A' STA. 26+75.60, 22.31' RIGHT

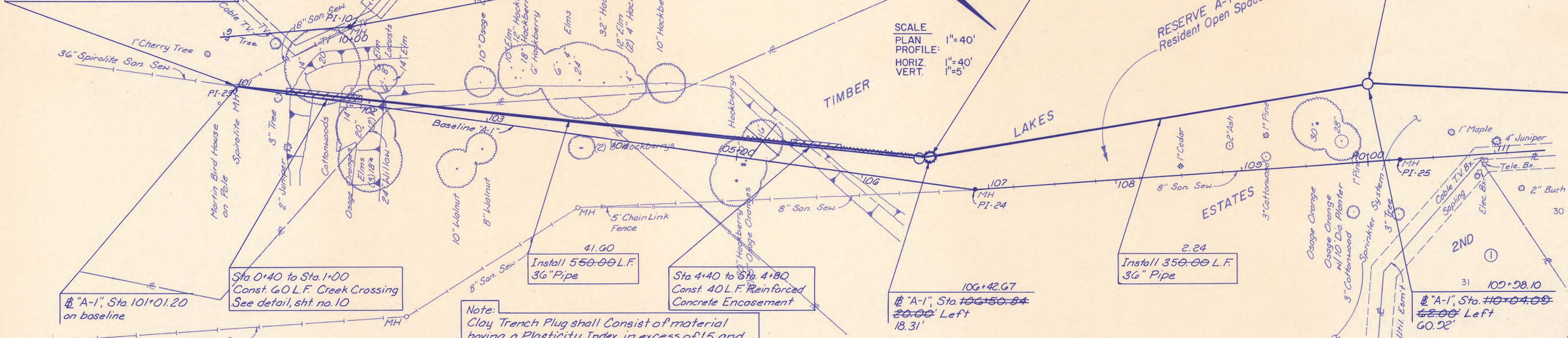


1 Updated per current standards		RJ	5/92
No.	Revision	By	Date
SEDGWICK COUNTY-BUREAU OF PUBLIC SERVICES DIRECTOR: DAVID C. SPEARS, P.E. BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER			
BASELINE KEY MAP			
SANITARY SEWER INTERCEPTOR FOUR MILE CREEK JOINT SEWER DISTRICT			
PROFESSIONAL ENGINEERING CONSULTANTS, P.A.			
ENGINEERS WICHITA, KANSAS			
Designed by	DLM, MDS	Job No.	31051-E
Drawn by	GM	Date	Jan. 1983
		Sht.	3 of 14

PLAN	SURVEYED	DATE
NO.	BY	
NOTE BOOK	PLOTTED	
NO.	GRADES CHECKED	
NO.	STRUCTURE NOTATIONS CHECKED	

PROFILE	SURVEYED	DATE
NO.	BY	
NOTE BOOK	PLOTTED	
NO.	GRADES CHECKED	
NO.	STRUCTURE NOTATIONS CHECKED	

Sta. 0+00.00
Remove Existing Spirolite MHTA-4.
Construct Type C, 5' Dia. MH
w/self-sealing MH frame.
The contractor shall maintain
wastewater flow by pumping
or piping during removal and
replacement of this MH.
Cost for this work shall be considered
subsidiary to the Lump Sum Price Bid
for "MH TA-4 Removal and Replacement".



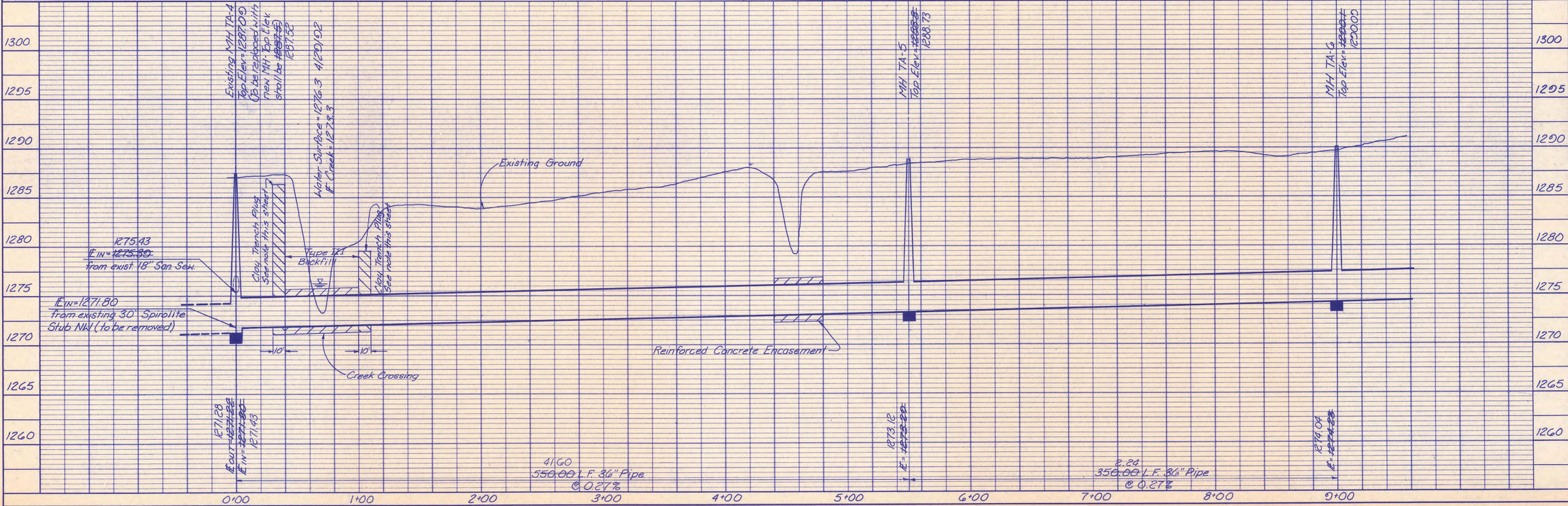
ALL PIPE INSTALLED
IS VCP PIPE UNLESS
OTHERWISE NOTED.

SANITARY SEWER PIPE SLOPES AND DEFLECTION
ANGLES AT MANHOLES HAVE NOT BEEN REVISED
TO REFLECT AS CONSTRUCTED CONDITIONS.

Note:
Clay Trench Plug shall consist of material
having a Plasticity Index in excess of 15 and
Liquid Limit in excess of 35. Clay Plug shall
extend from 1' below finish grade to the
bottom of trench. (Cost of Clay Plugs
shall be subsidiary to Trench Backfill.)

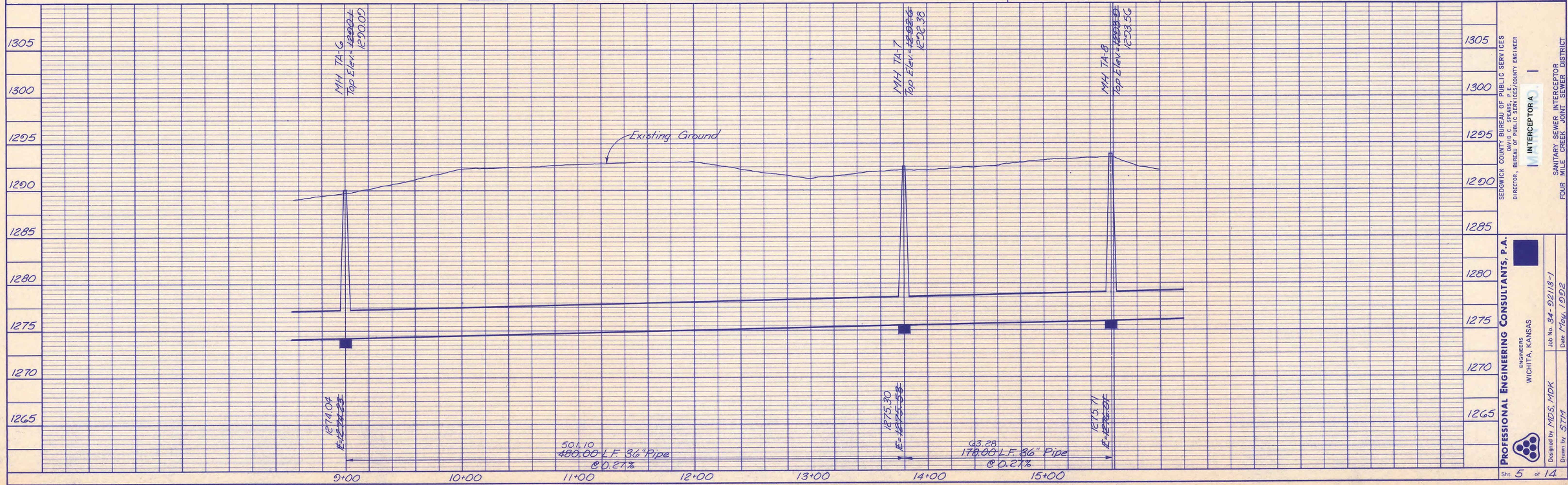
See Sheet No. 10 for table of deflection
angles at MH's.

RECORD DRAWING
MDK PROJ. ENG. 4/21/94
GRG RES. ENG. 4/15/94



PROFESSIONAL ENGINEERING CONSULTANTS, P.A.
ENGINEERS
WICHITA, KANSAS
Joo No. 34-52113-1
Date May, 1992
Designed by MDK
Drawn by STM
SANTARY SEWER INTERCEPTOR
FOUR MILE CREEK JOINT SEWER DISTRICT
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES
DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER
DAVID C. SPEARS, P.E.
INTERCEPTOR A
MICHAEL D. SCHOMMER
LICENSED PROFESSIONAL ENGINEER
KANSAS
6778

NO. _____	BY _____	DATE _____
STRUCTURE NOTATIONS CHECKED		
B. M. 3 NOTED		
GRADES CHECKED		
PLOTTED		
SURVEYED		
NOTE BOOK		
PROFILE		

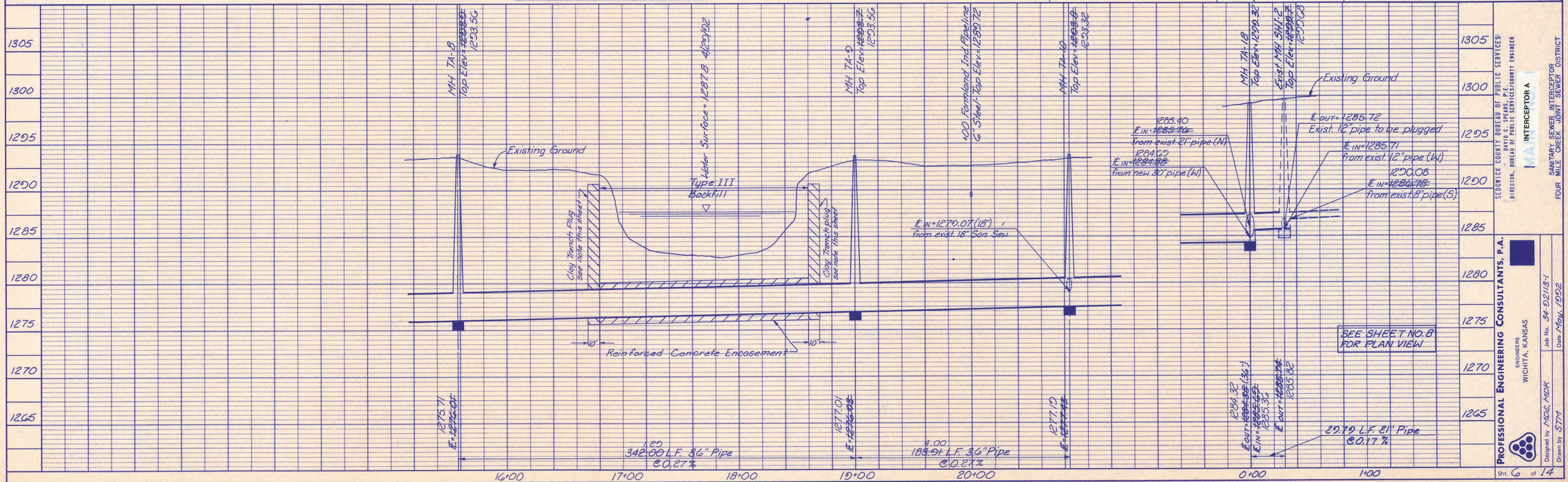


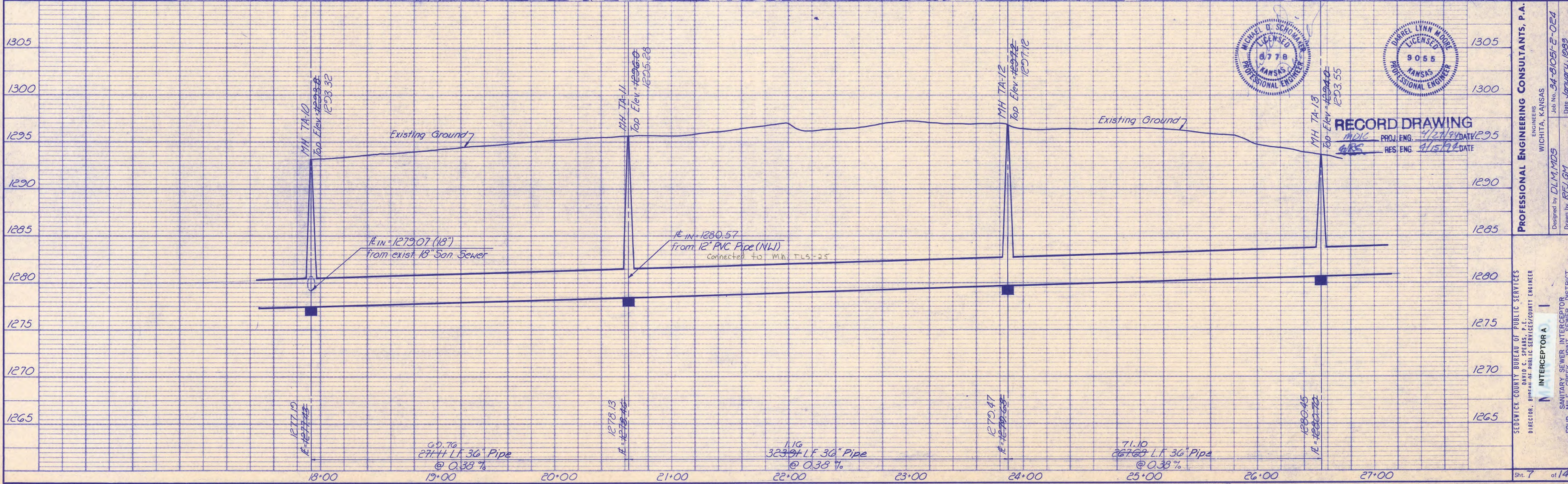
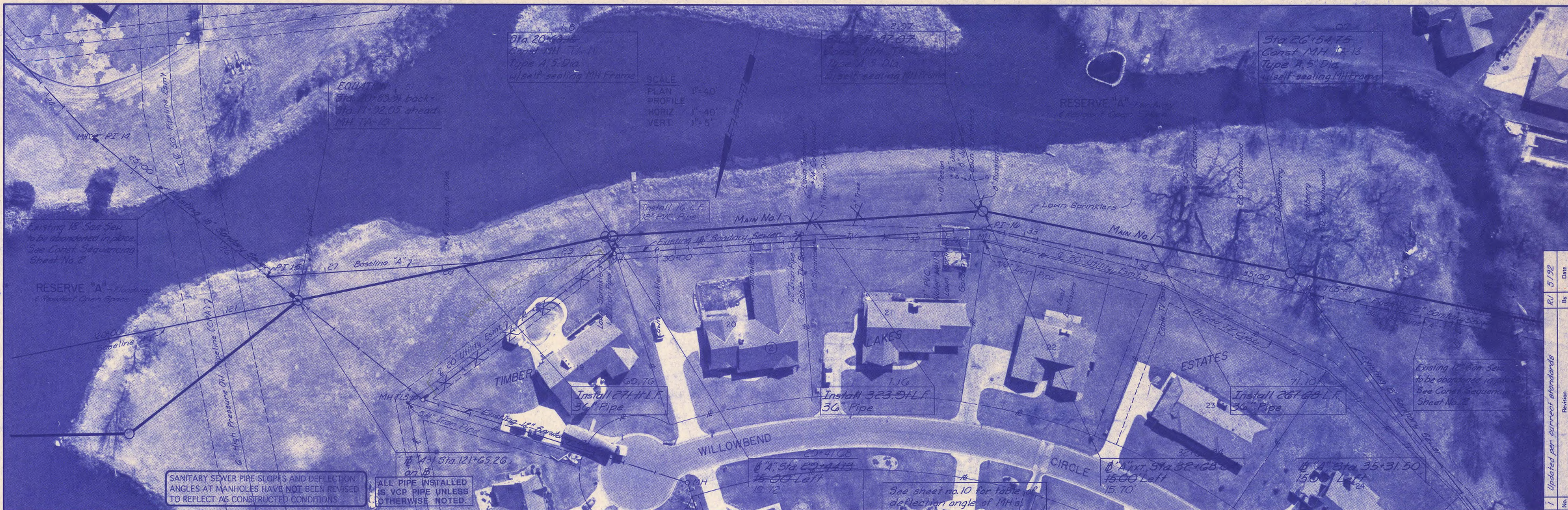
A circular blue ink seal for a Professional Engineer in the State of Kansas. The outer ring contains the text "MICHAEL D. SCHOMAKER" at the top and "PROFESSIONAL ENGINEER" at the bottom. The inner circle contains the word "LICENSED" at the top, the license number "8778" in the center, and the word "KANSAS" at the bottom. A signature is written across the seal.

RECORD DRAWING
MDK PROJ. ENG. 4/27/94 DATE
GRG RES. ENG. 4/15/94 DATE

Sht. <u>5</u> of <u>14</u>		Job No. <u>34-02113-1</u> Date <u>May, 1992</u>	SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES P. E. D. J. D. C. SPANNS DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER
PROFESSIONAL ENGINEERING CONSULTANTS, P.A. ENGINEERS WICHITA, KANSAS		INTERCEPTOR A	
		SANITARY SEWER INTERCEPTOR FOUR MILE CREEK JOINT SEWER DISTRICT	

PROFILE		BY	DATE
SURVEYED			
PLOTTED			
GRADES CHECKED			
B. M. S. NOTED			
NO.	STRUCTURE NOTATIONS CHECKED		

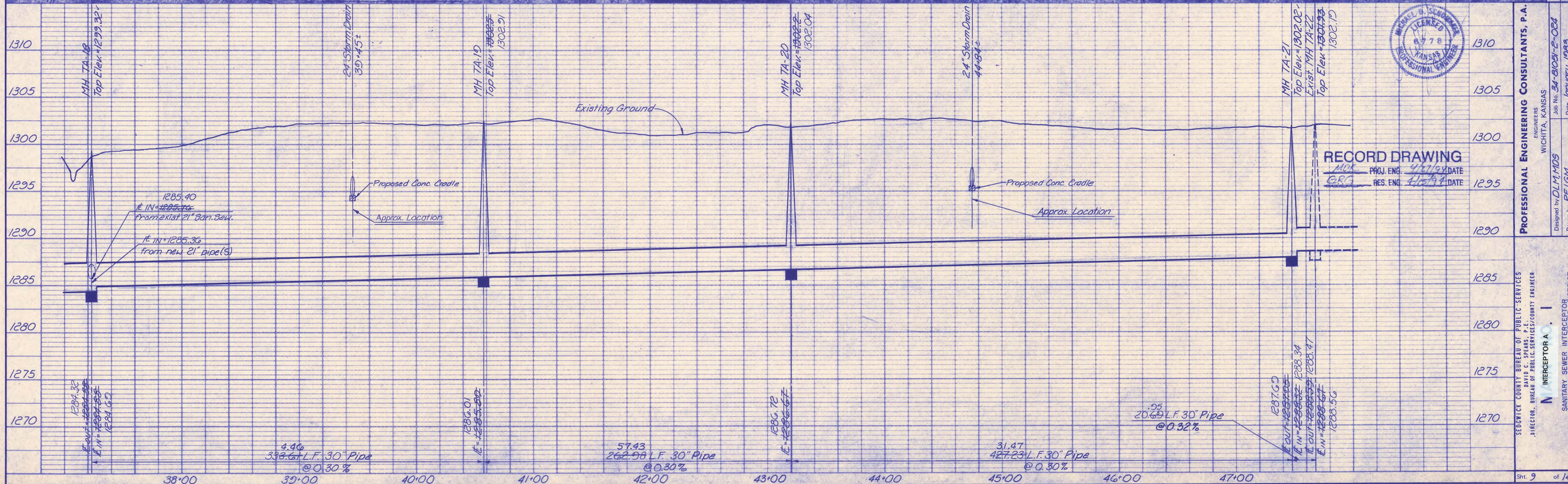
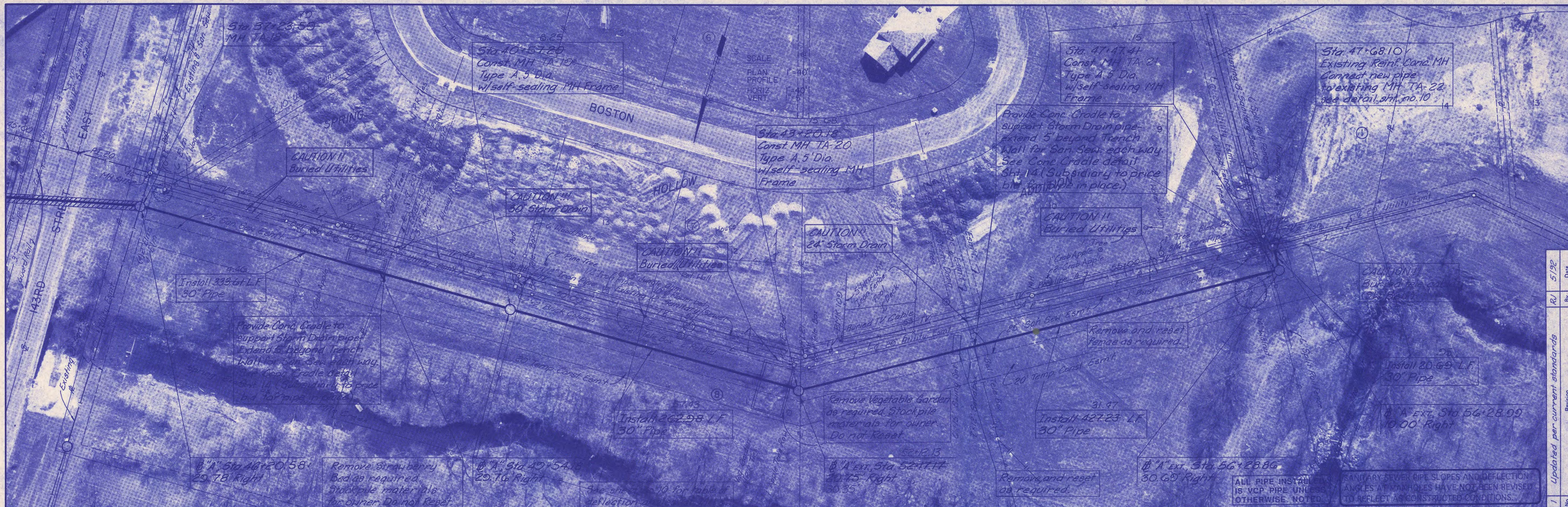




By	Date
RU	5/92

Updated per current standards

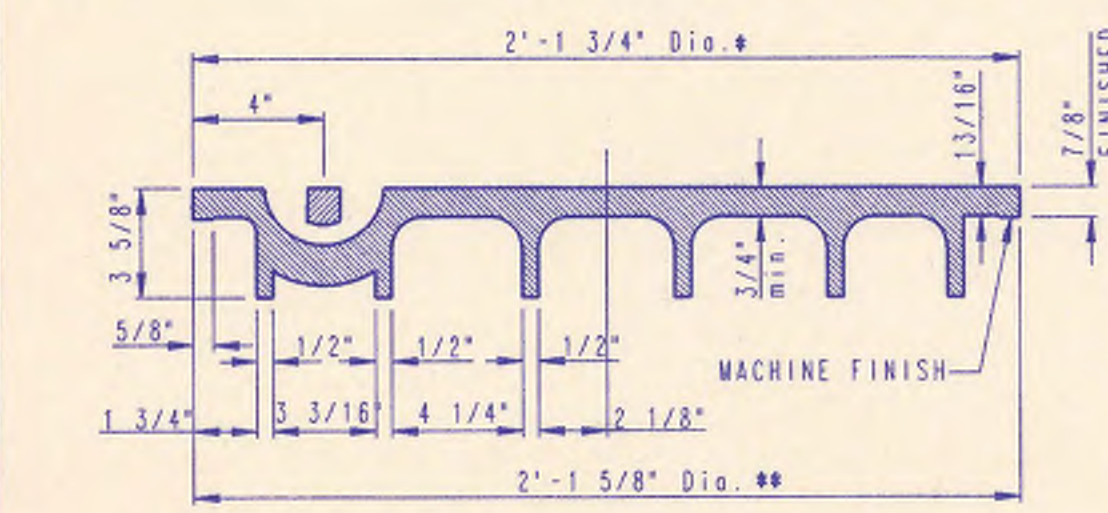
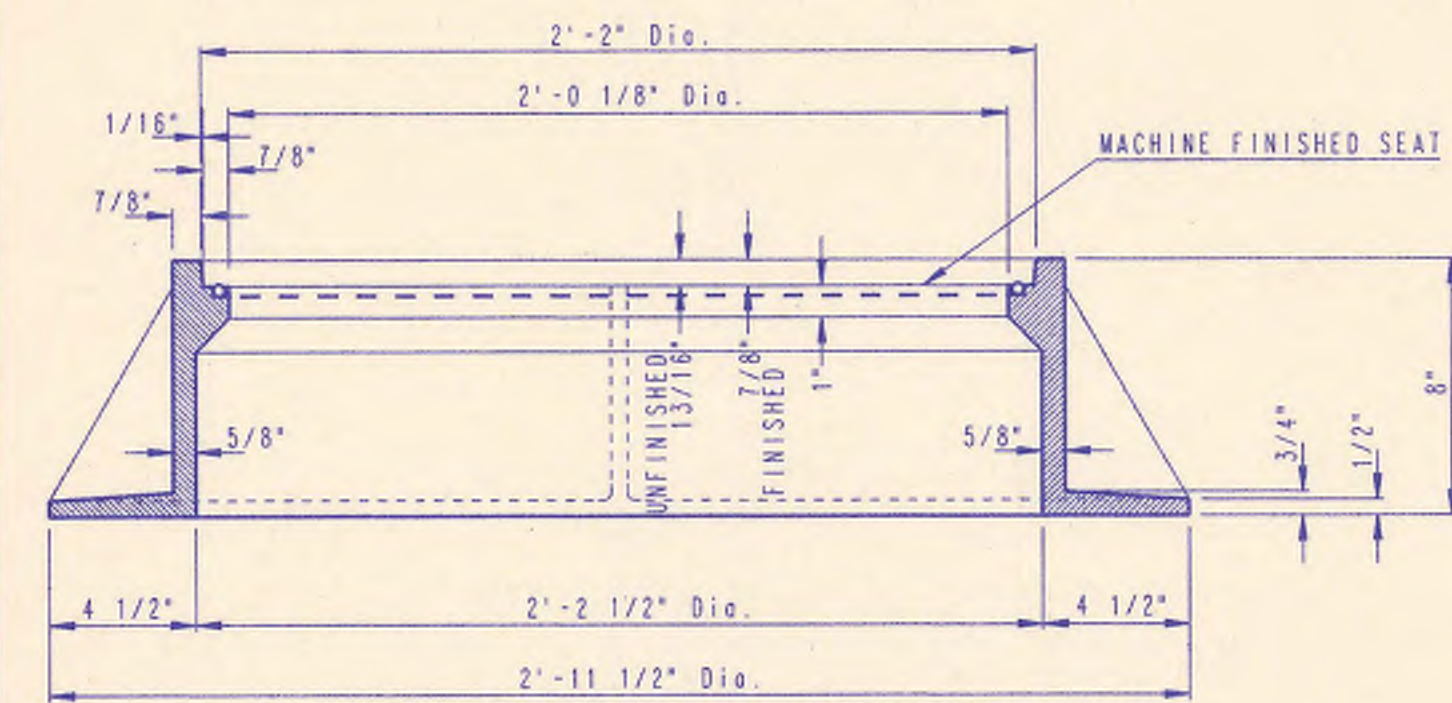
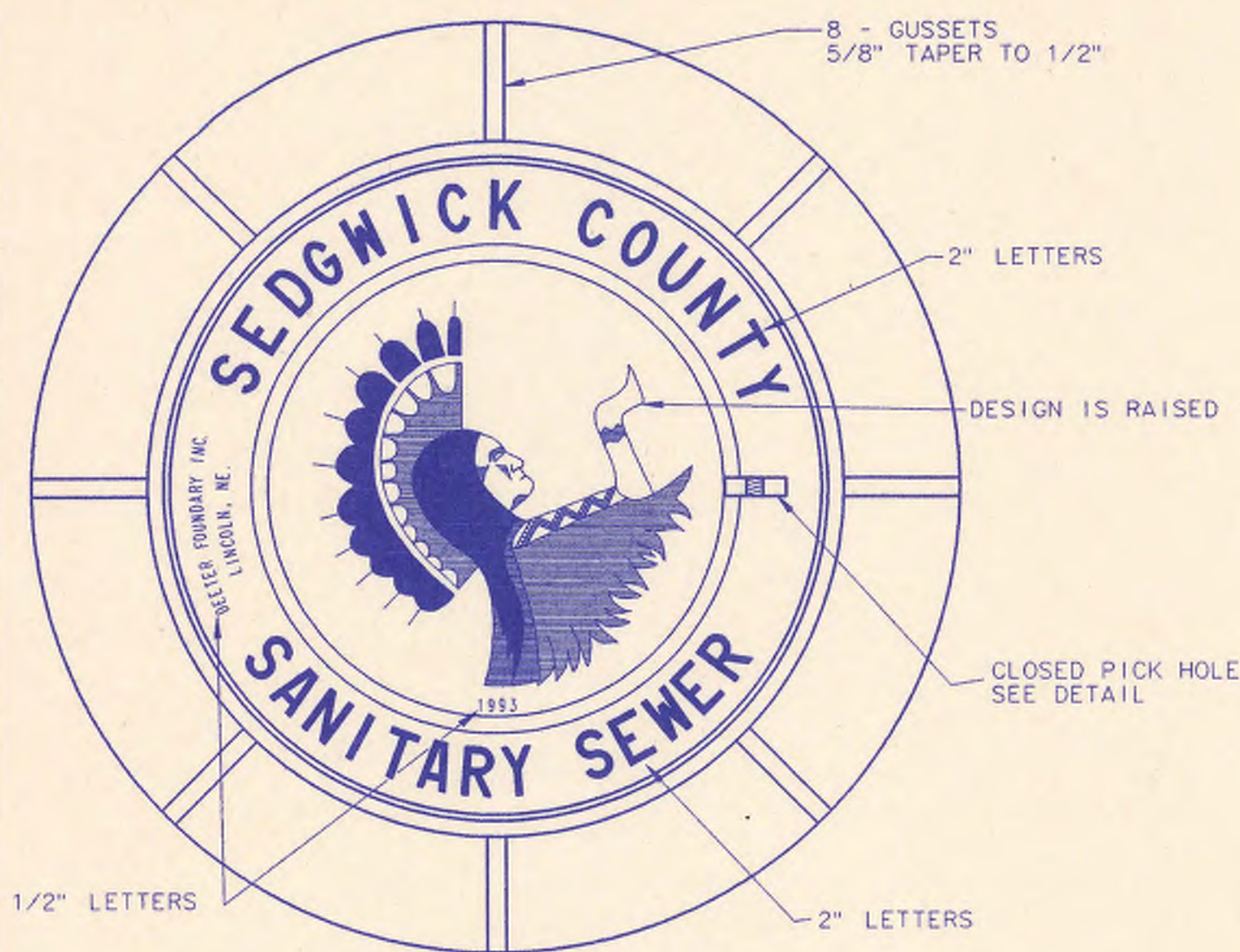
Professional Engineering Consultants, P.A.
WICHITA, KANSAS
Job No. 34-81051-2-024
Date January 4, 1989
Designed by D.L.M./MDS
Drawn by R.F.J./GM
INTERCEPTOR A
SANITARY SEWER INTERCEPTOR
FOUR MILE CREEK JOINT SEWER DISTRICT
Sedgwick County Bureau of Public Services
David C. Sparks, P.E.
Director, Bureau of Public Services/County Engineer
Sht. 7 of 14



Updated per current standards	By	Date
1	RU	5/92

PROFESSIONAL ENGINEERING CONSULTANTS, P.A.
 ENGINEERS
 WICHITA, KANSAS
 Job No. 34-81021-2-024
 Date January, 1993
 Designed by DLM:MD3
 Drawn by RFL:GM

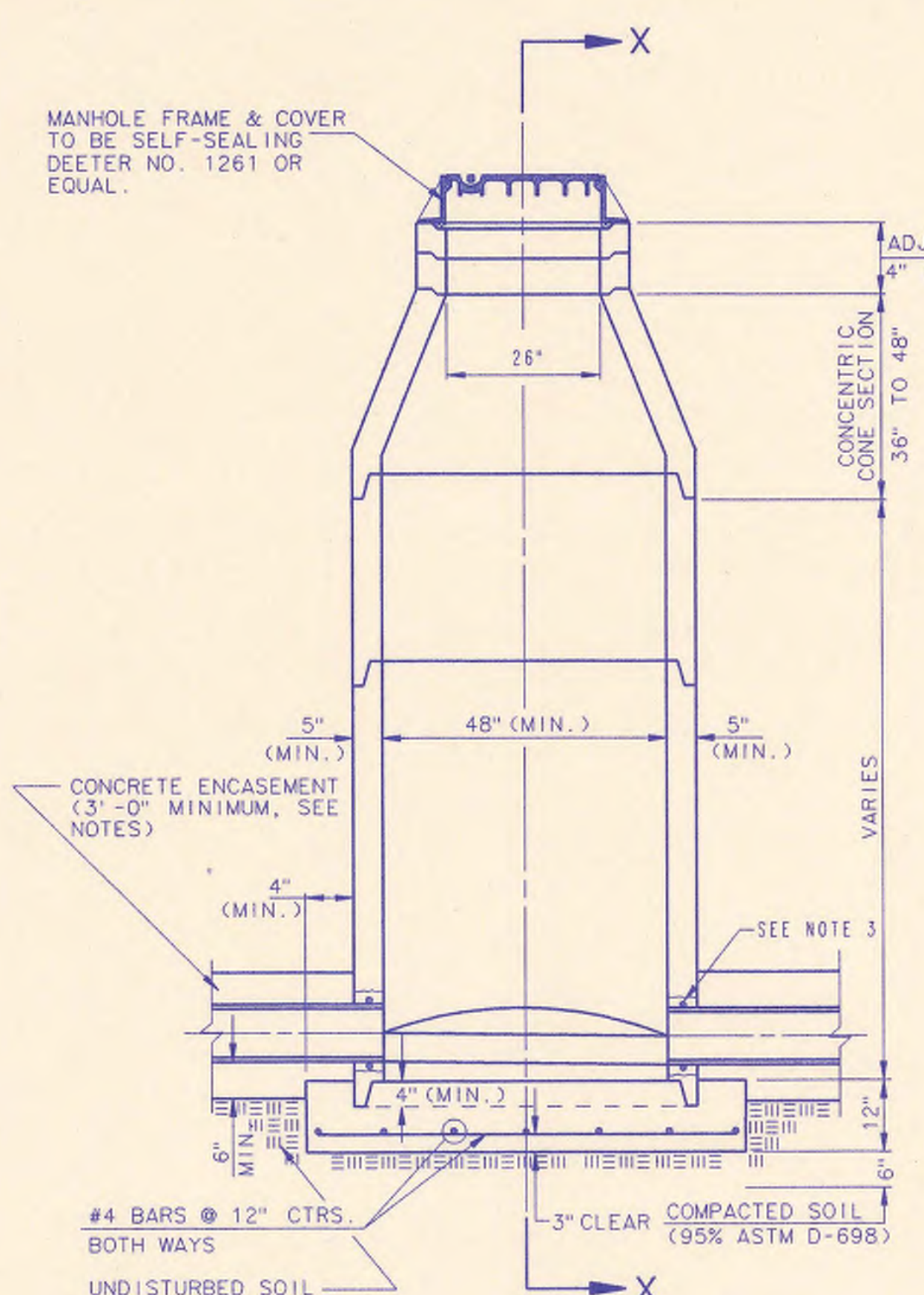
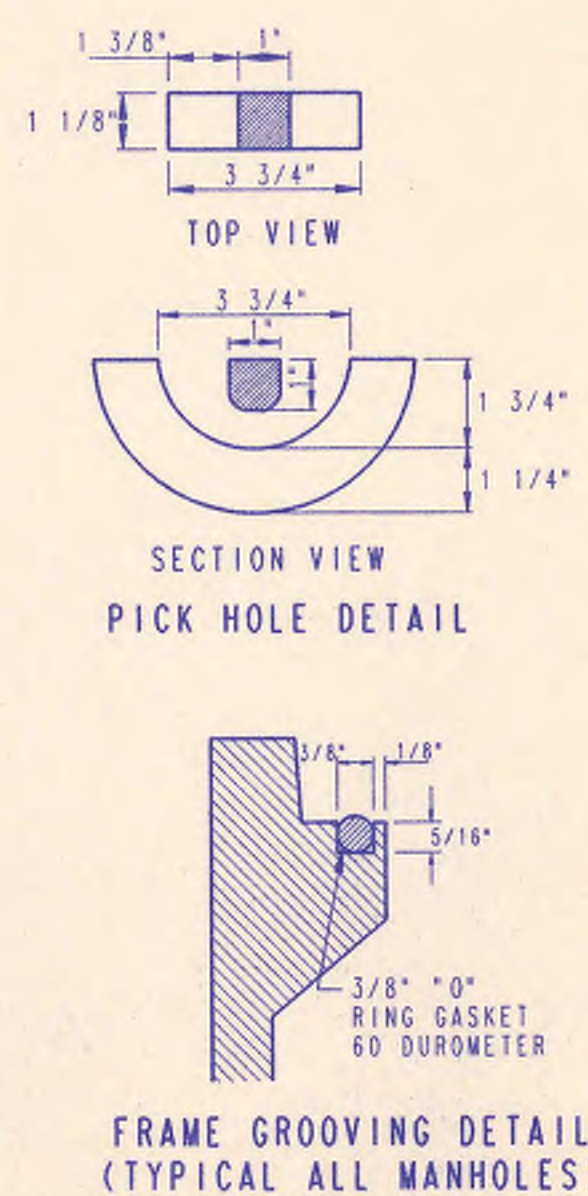
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES
 DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER
 INTERCEPTOR A
 SANITARY SEWER INTERCEPTOR
 FOUR MILE CREEK JOINT SEWER DISTRICT
 Sht. 9 of 14



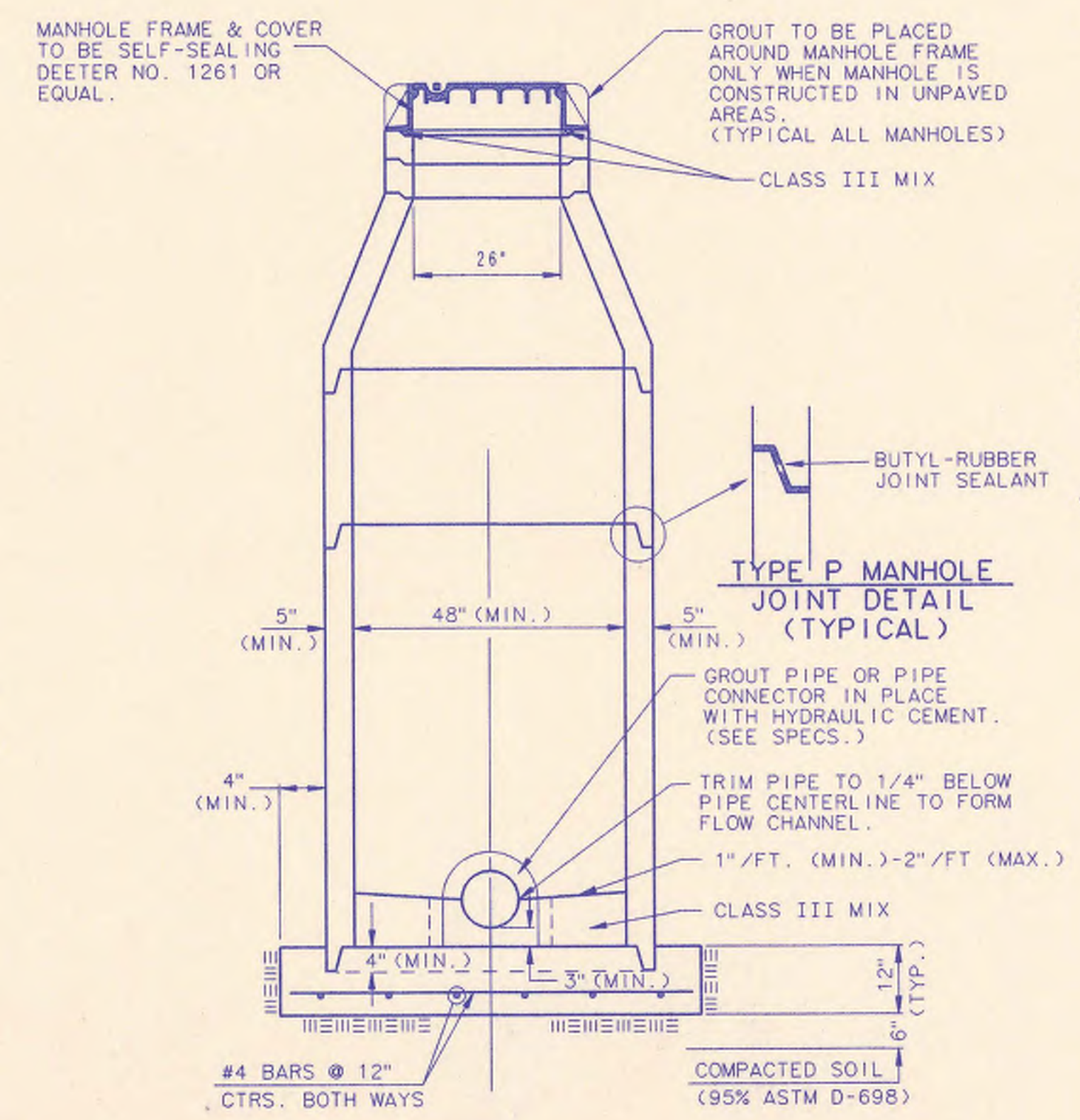
MANHOLE FRAME AND COVER
(TOTAL WEIGHT = 430 LBS.)

MANHOLE FRAME AND COVER NOTES

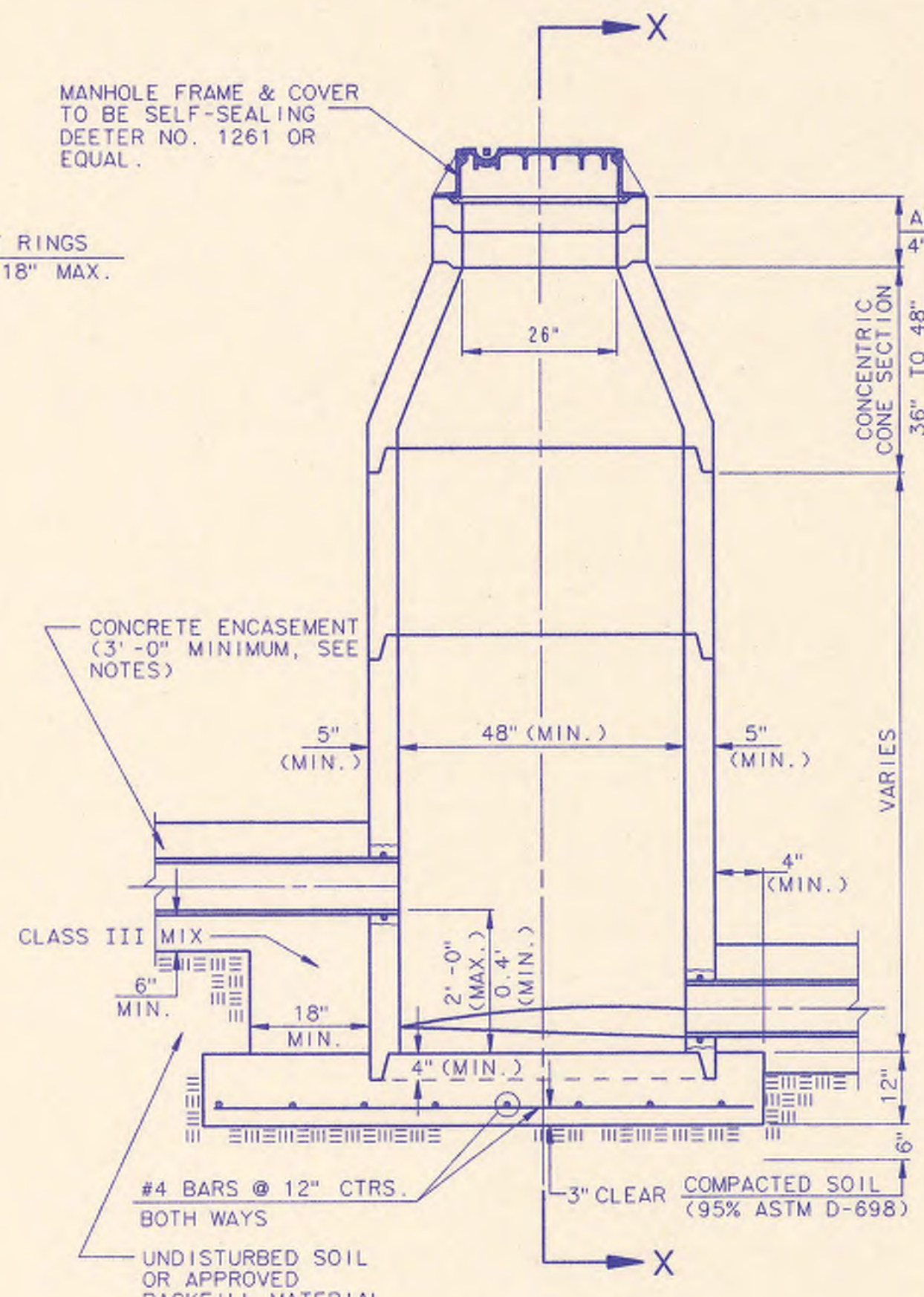
1. CAST IRON MANHOLE FRAME AND COVER SHALL CONFORM TO ASTM A-48, CLASS 35B, OR BETTER.
2. CASTINGS ARE TO BE MANUFACTURED TRUE TO PATTERN AND WITH SATISFACTORY FIT OF COMPONENT PARTS. CASTINGS SHALL BE FREE OF DEFECTS. DIMENSIONS AS DETAILED ON PLAN SHALL NOT DEVIATE BY $\pm 1/16$ " PER FOOT.
3. NO OTHER LETTERING OR MARKINGS OTHER THAN THOSE DETAILED ON PLAN WILL BE PERMITTED ON CASTINGS.
4. CASTINGS MUST BE DOMESTICALLY MANUFACTURED IN THE UNITED STATES OF AMERICA.
5. THE FRAMES AND COVERS SHALL BE FURNISHED WITH MACHINED HORIZONTAL BEARING SURFACES SO FITTING PARTS WILL NOT RATTLE OR ROCK UNDER TRAFFIC.
6. MANHOLE CASTINGS SHALL BE SELF-SEALING DEETER FOUNDRY, INC. NO. 1261 OR APPROVED EQUAL, UNLESS OTHERWISE SPECIFIED IN THE SPECIAL CONDITIONS. (MINIMUM WT. 430 LBS.) ALL MANHOLE CASTINGS SHALL BE CONSIDERED SUBSIDIARY TO THE UNIT PRICES BID FOR THE VARIOUS MANHOLE TYPES.
7. GRIND ALL BURRS SMOOTH, CLEAN THOROUGHLY, THEN APPLY SHOP COAT OF ASPHALT BASE PAINT.
8. THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO MANUFACTURE. THE ENGINEER SHALL RETAIN THE RIGHT TO REJECT CASTINGS NOT CONFORMING TO THE SPECIFICATIONS OR THE APPROVED SHOP DRAWINGS.
9. THE MANHOLE FRAME SHALL BE FURNISHED WITH AN APPROVED "O" RING GASKET GROOVED INTO THE BEARING SURFACE OF THE MANHOLE FRAME (PER DETAIL). THE "O" RING GASKET SHALL NOT BE INSTALLED IN THE MANHOLE FRAME UNTIL AFTER FINAL INSPECTION AND ACCEPTANCE OF THE PROJECT BY THE ENGINEER.



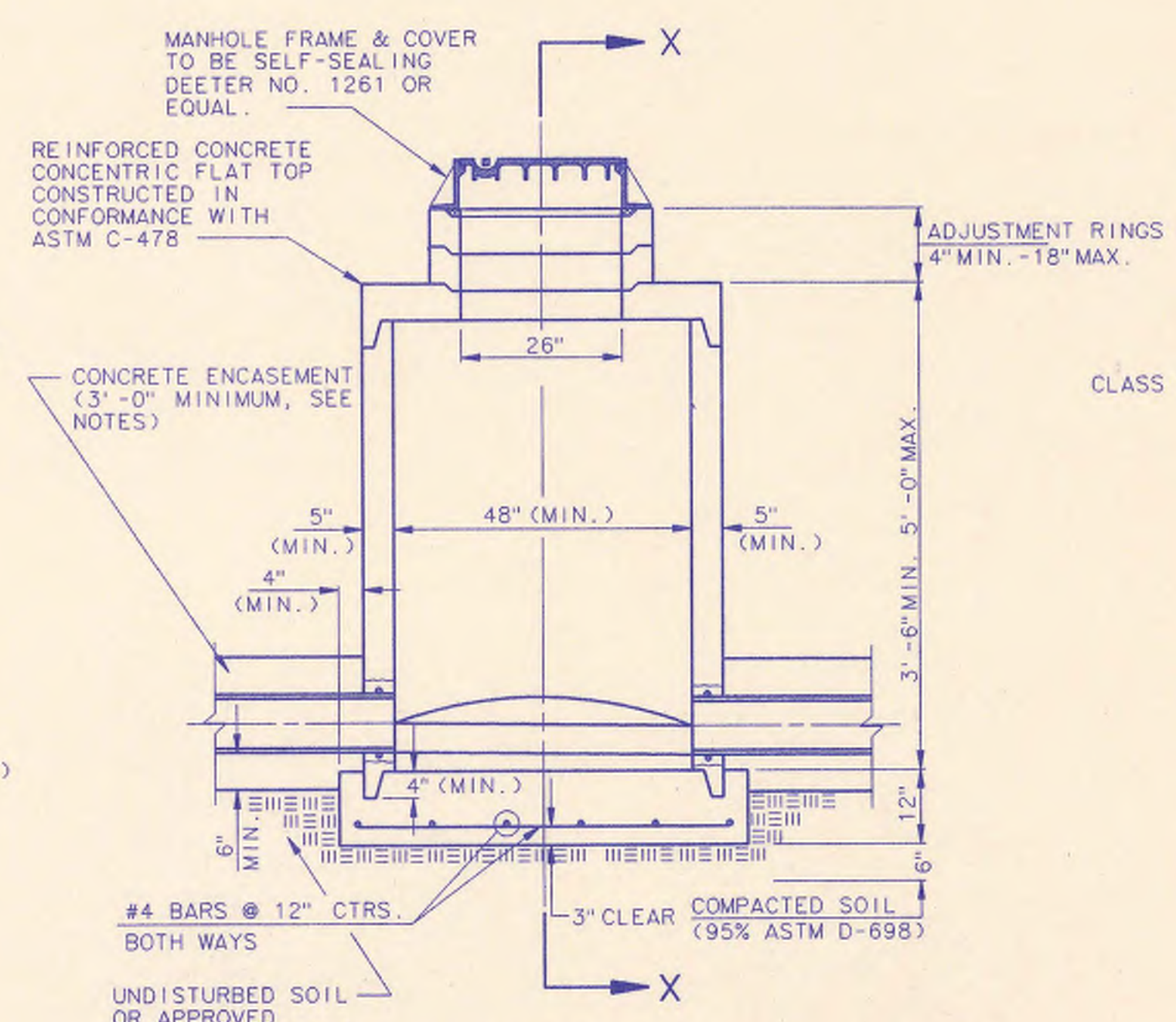
PRECAST STANDARD MANHOLE TYPE "A"



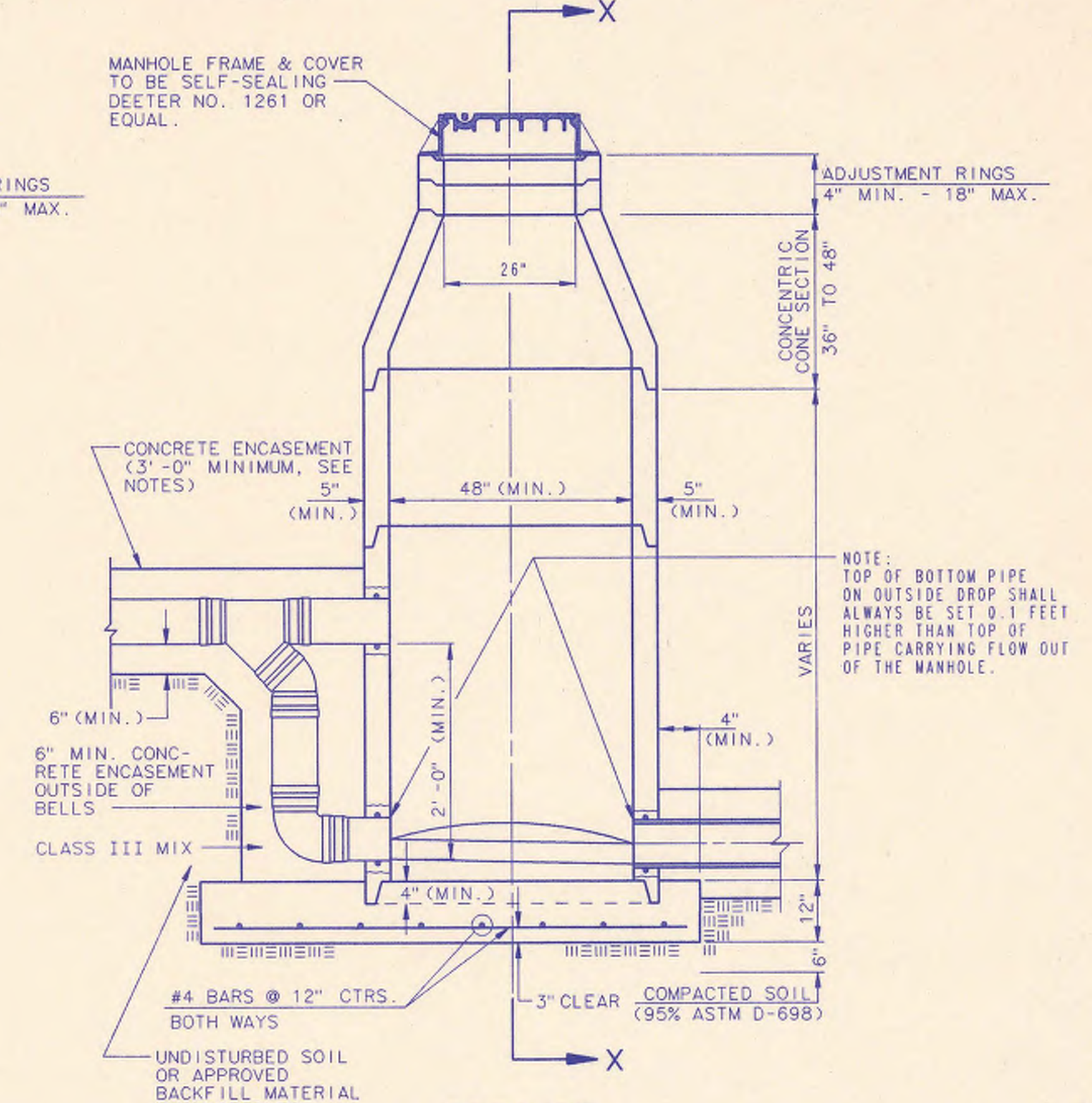
SECTION X (TYPICAL)



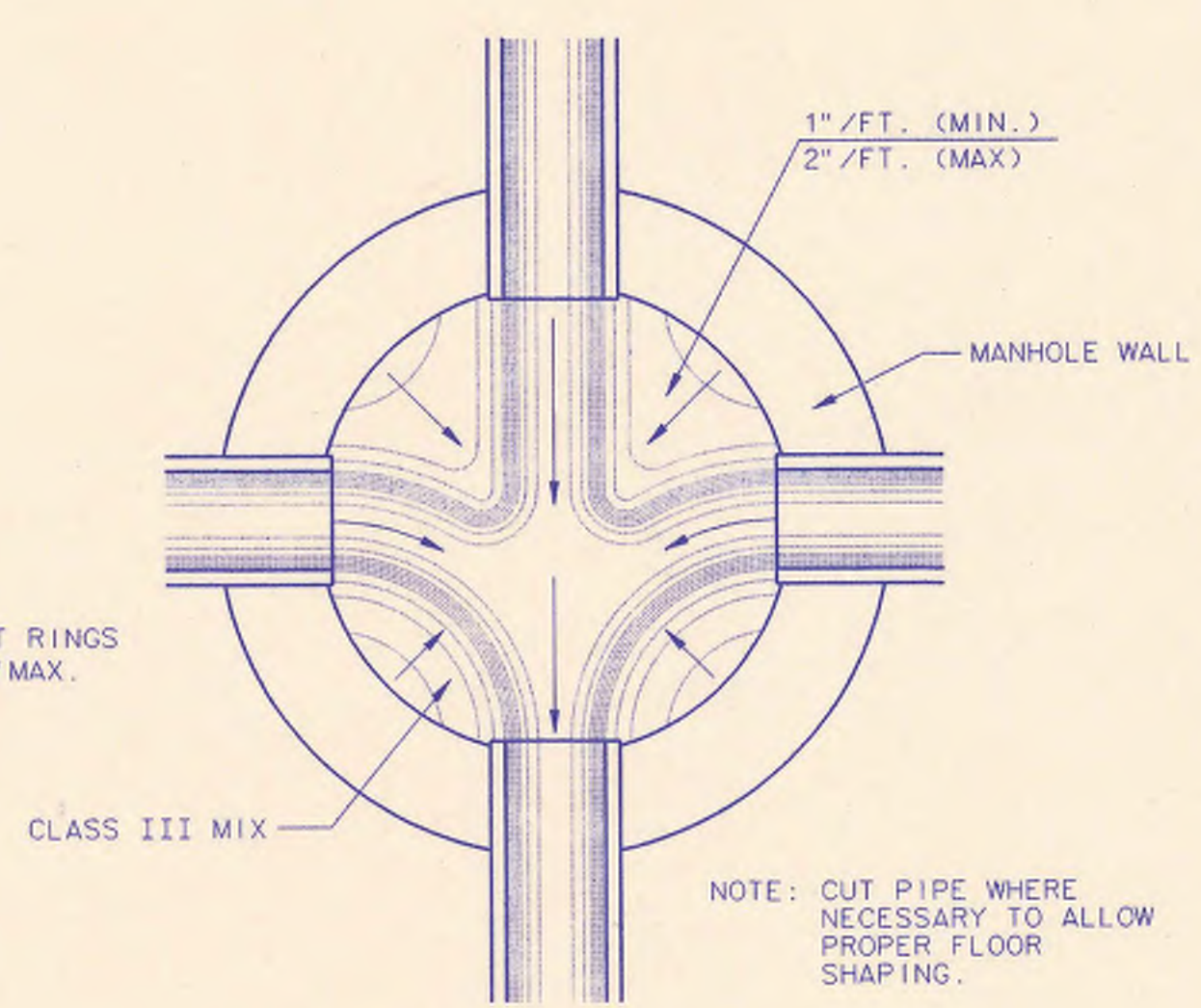
PRECAST INSIDE DROP MANHOLE TYPE "B"



PRECAST SHALLOW MANHOLE TYPE "D"



PRECAST OUTSIDE DROP MANHOLE TYPE "C"

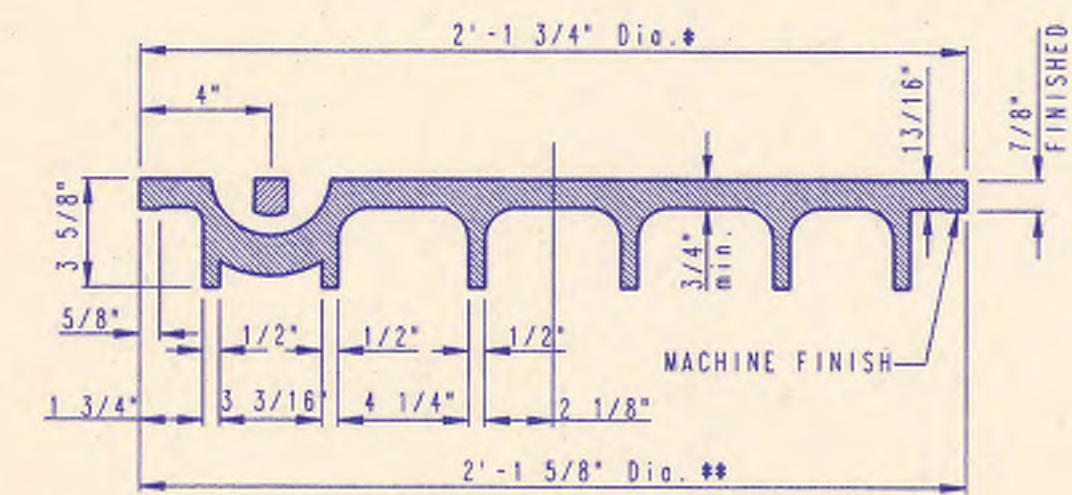
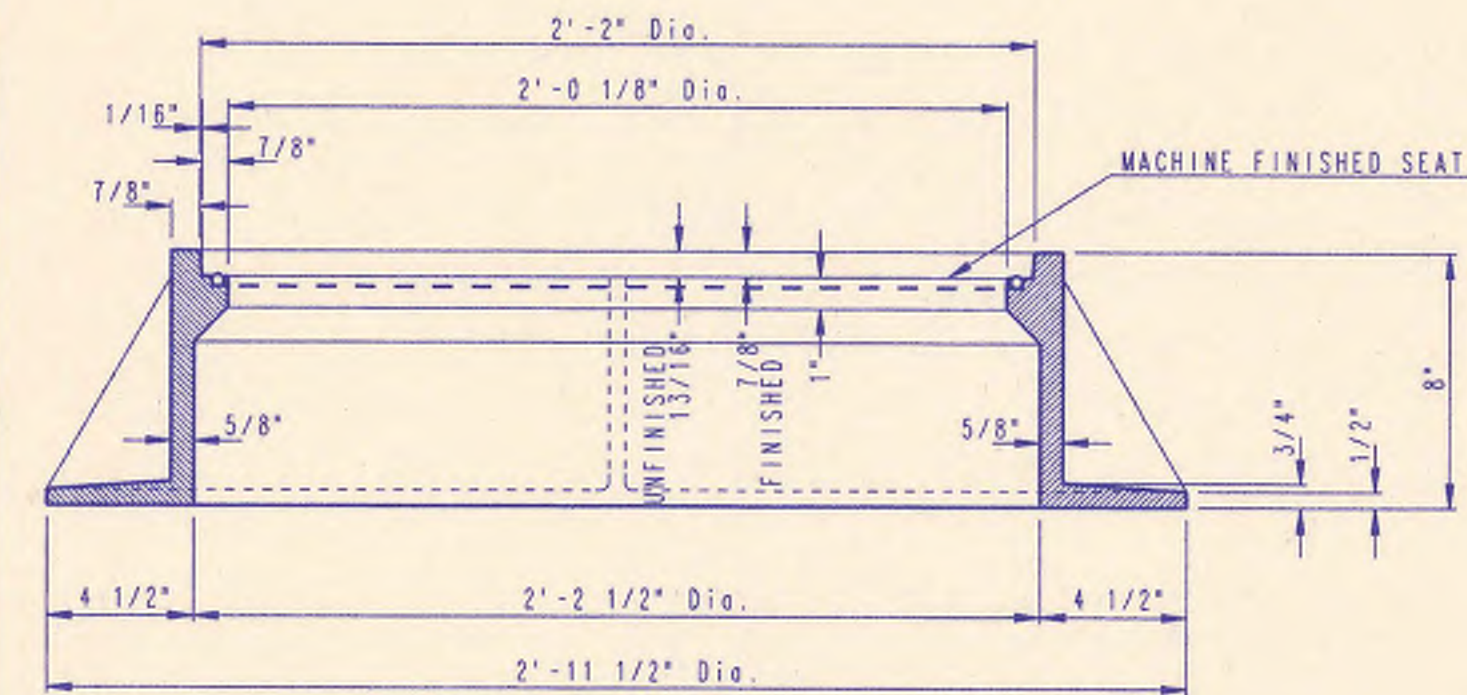
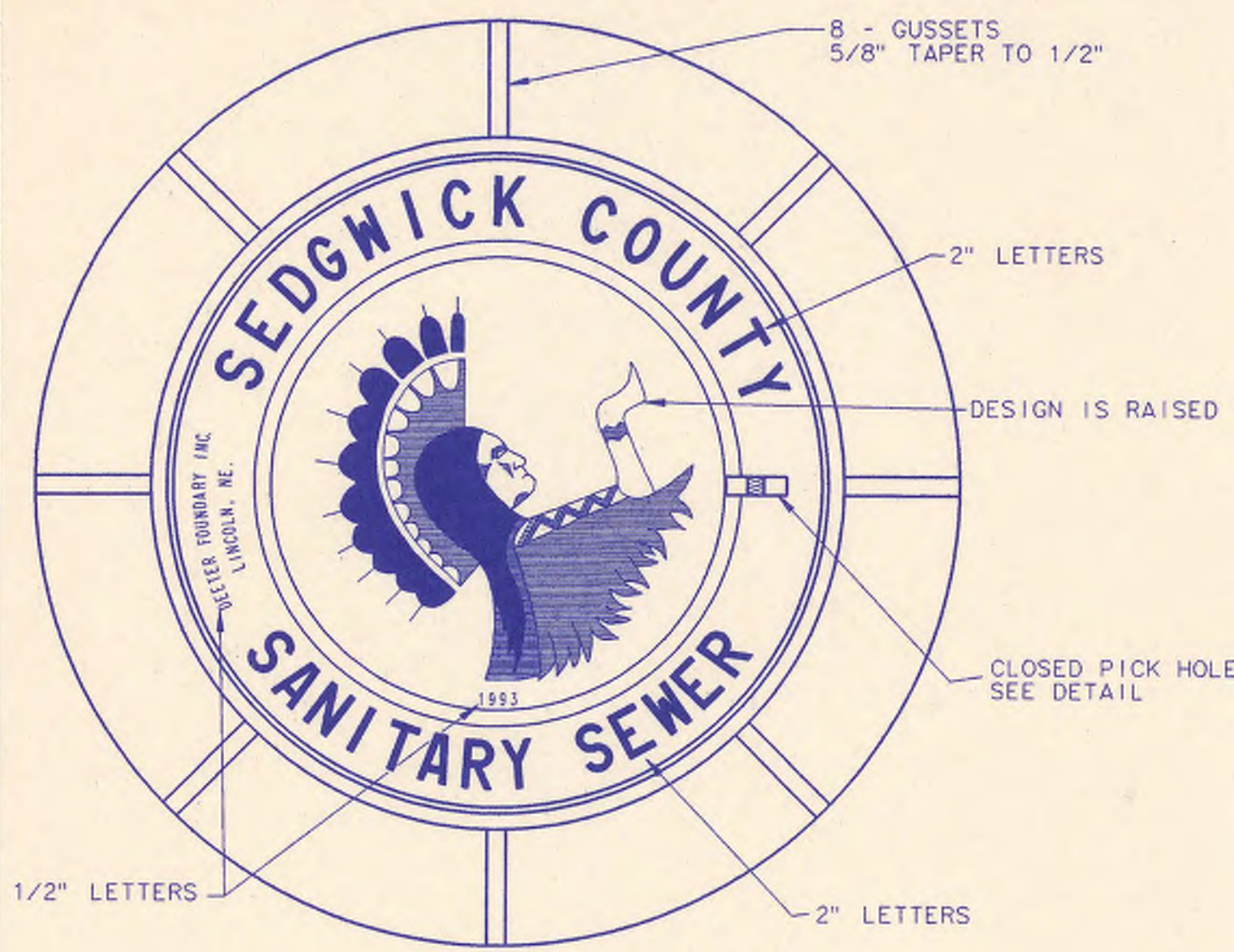


TYPICAL MANHOLE FLOOR SHAPING

- PRECAST MANHOLE NOTES**
1. IF, IN THE OPINION OF THE ENGINEER, THE MANHOLE SUBGRADE APPEARS UNSTABLE, THE CONTRACTOR WILL HAVE THE OPTION TO COMPACT SUBGRADE AS SHOWN OR INCREASE THE THICKNESS OF THE MANHOLE BASE AS DIRECTED BY THE ENGINEER.
 2. STEEL REINFORCING WILL BE REQUIRED IN ALL MANHOLE BASES.
 3. APPROVED FLEXIBLE WATERSTOP GASKETS WHICH MEET OR EXCEED THE TEST REQUIREMENTS OF ASTM C-923 SHALL BE INSTALLED TO JOIN THE SEWER TO THE MANHOLE WALL WHEN PLASTIC PIPE IS USED. SEWER PIPE EXTENDING FROM MANHOLES SHALL BE SUPPORTED WITH CONCRETE ENCASUREMENT A MINIMUM OF 3 FEET FROM THE MANHOLE WALL.
 4. THE MANHOLE FRAME SHALL BE SEATED ON AN APPROVED BUTYL-RUBBER SEALANT TO PROVIDE A WATER-TIGHT SEAL BETWEEN THE MANHOLE ADJUSTMENT RING AND THE MANHOLE FRAME.
 5. GASKETED PIPE CAPS SHALL BE PROVIDED BY THE PIPE SUPPLIER. GLUED OR CEMENTED CAPS WILL NOT BE ACCEPTED.
 6. ALL MANHOLE CONSTRUCTION SHALL BE WATER TIGHT.
 7. TOP OF MANHOLE FLOOR SLAB SHALL BE AT LEAST 3 INCHES BELOW THE FLOW LINE OF THE OUTLET PIPE TO INSURE SUFFICIENT MINIMUM THICKNESS OF SHAPED INVERT.
 8. ALL PRECAST CONCRETE MANHOLE SECTIONS SHALL CONFORM TO THE LATEST REVISION OF ASTM C478 AS MODIFIED BY THE SPECIFICATIONS.
 9. CONCRETE FOR MANHOLE BASES SHALL BE CLASS I AS DESCRIBED IN THE SPECIFICATIONS.
 10. PRECAST MANHOLES SHALL BE SET AT LEAST 4 INCHES INTO MANHOLE BASE.
 11. MANHOLES WITH PIPE SIZES 24" AND LARGER SHALL HAVE 5' INSIDE DIAMETER (MIN.).
 12. INSIDE DIAMETER OF FIVE-FOOT DIAMETER PRECAST MANHOLES SHALL REMAIN CONSTANT TO THE LOCATION OF THE REDUCING FLAT TOP WHICH CONNECTS THE FOUR-FOOT DIAMETER CONE SECTION TO THE FIVE-FOOT DIAMETER MANHOLE BARREL.

3	All Manholes self-sealing/MH Cover	RJ	1/25/93
2	Modified pipe connection note	RJ	6/01/92
1	Manhole Frame and Cover Note 1	RJ	7/16/91
No.	Revision	By	Date

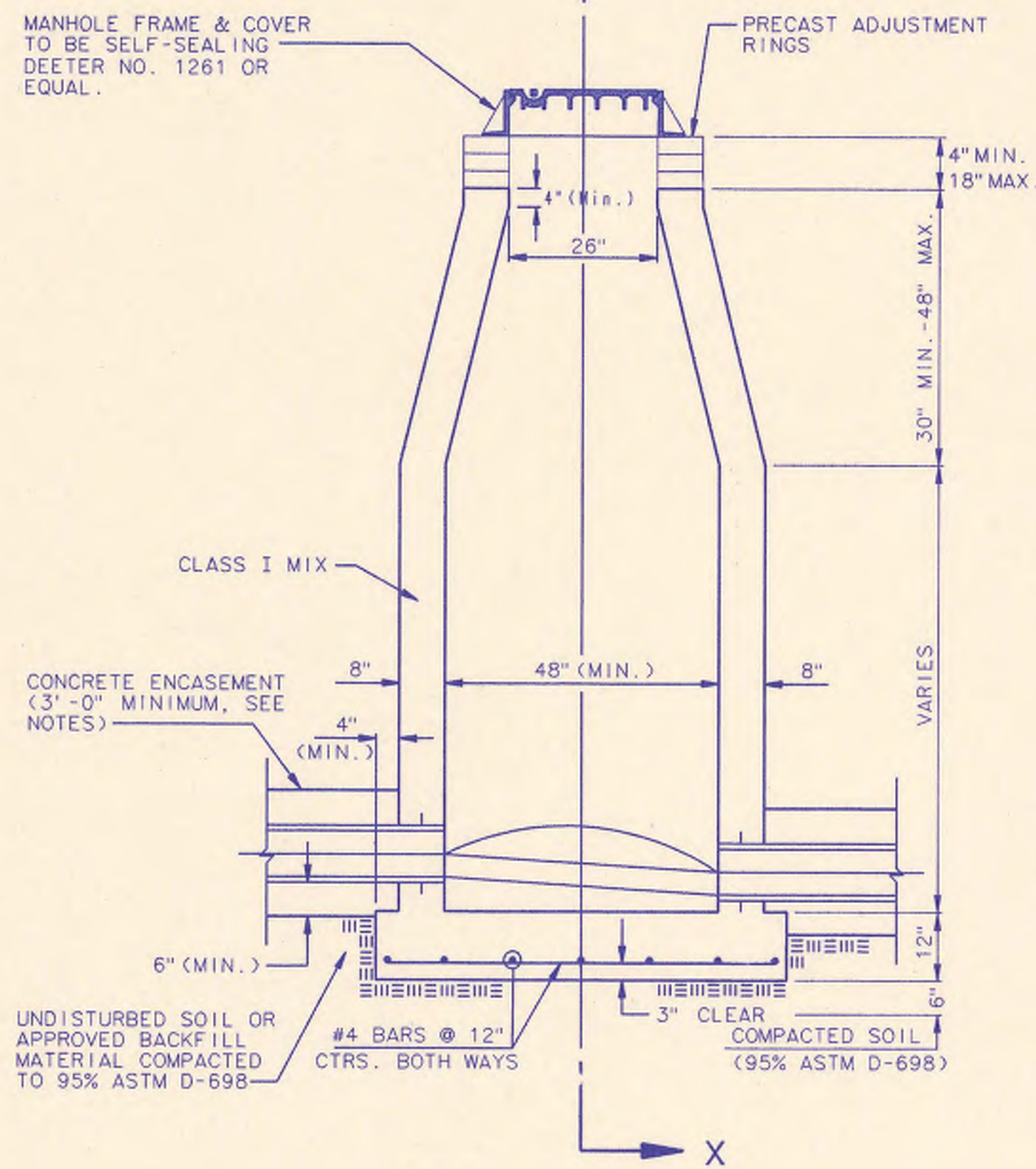
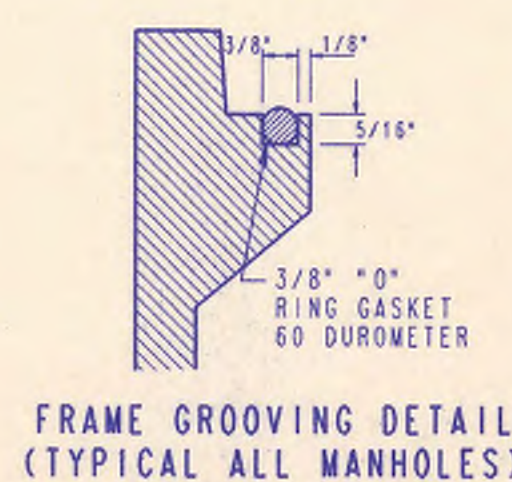
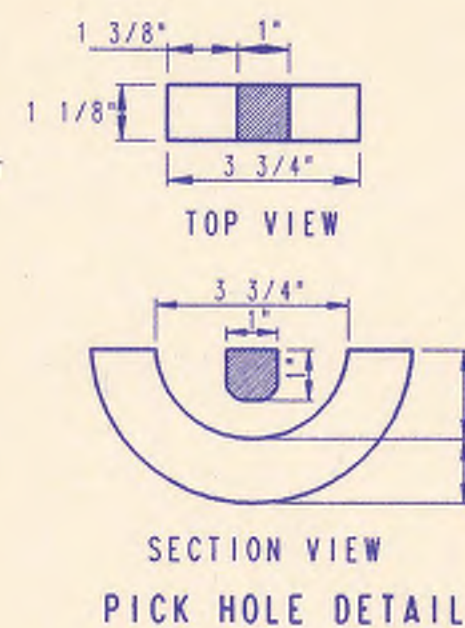
PRECAST MANHOLE DETAILS
ADOPTED AS STANDARD DESIGN SEPTEMBER, 1989
BY
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES
DAVID C. SPEARS, P.E. DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER



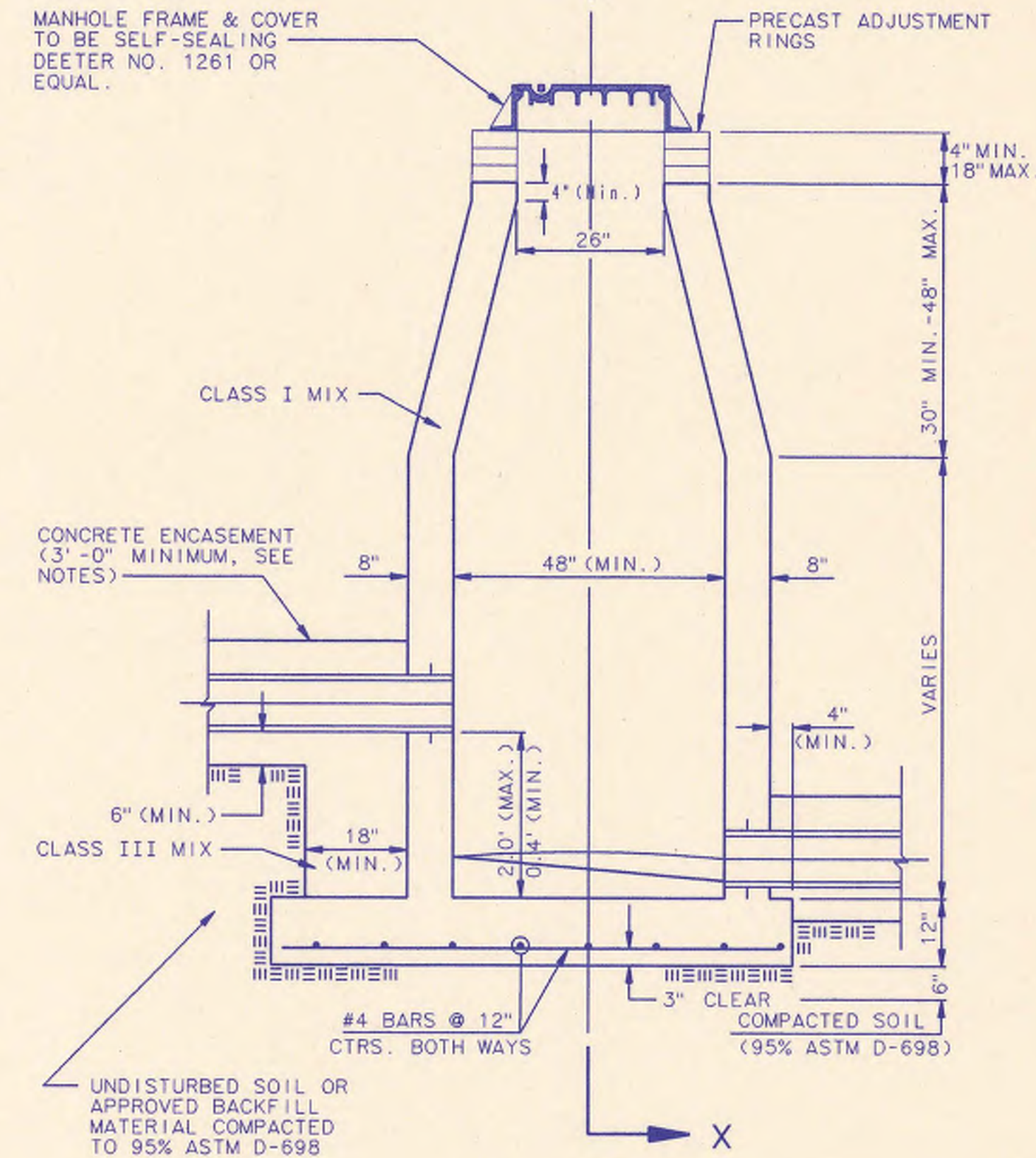
*OUTSIDE DIA. TOP OF COVER
**OUTSIDE DIA. BOTTOM OF COVER
MANHOLE FRAME AND COVER
(TOTAL WEIGHT = 430 LBS.)

MANHOLE FRAME AND COVER NOTES

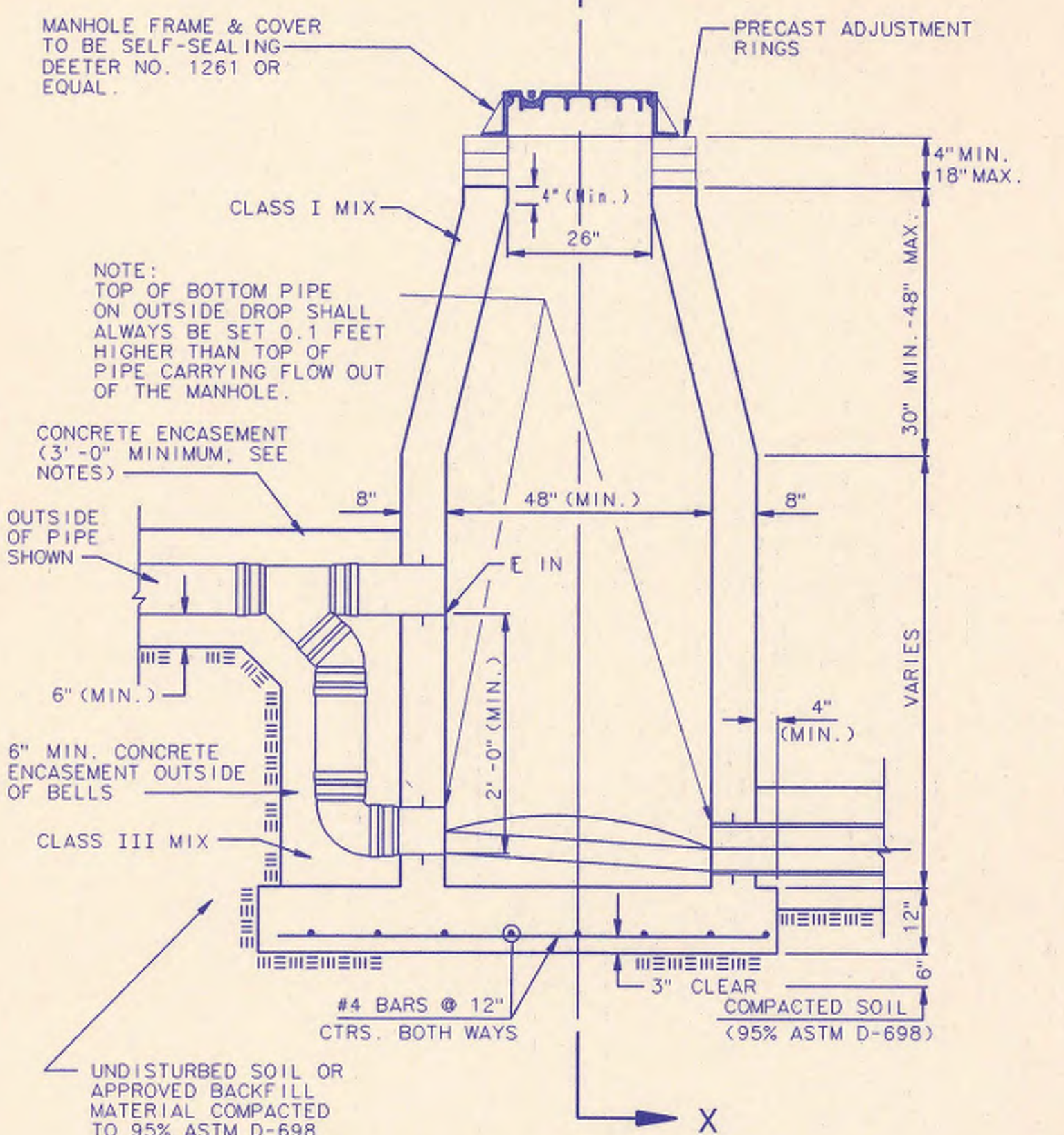
1. CAST IRON MANHOLE FRAME AND COVER SHALL CONFORM TO ASTM A-48, CLASS 35B, OR BETTER.
2. CASTINGS ARE TO BE MANUFACTURED TRUE TO PATTERN AND WITH SATISFACTORY FIT OF COMPONENT PARTS. CASTINGS SHALL BE FREE OF DEFECTS. DIMENSIONS AS DETAIL ON PLAN SHALL NOT DEVIATE BY $\pm 1/16"$ PER FOOT.
3. NO OTHER LETTERING OR MARKINGS OTHER THAN THOSE DETAILED ON PLAN WILL BE PERMITTED ON CASTINGS.
4. CASTINGS MUST BE DOMESTICALLY MANUFACTURED IN THE UNITED STATES OF AMERICA.
5. THE FRAMES AND COVERS SHALL BE FURNISHED WITH MACHINED HORIZONTAL BEARING SURFACES SO FITTING PARTS WILL NOT RATTLE OR ROCK UNDER TRAFFIC.
6. MANHOLE CASTINGS SHALL BE SELF-SEALING DEETER FOUNDRY INC. NO. 1261 OR APPROVED EQUAL, UNLESS OTHERWISE SPECIFIED IN THE SPECIAL CONDITIONS. (MINIMUM WT. 430 LBS.) ALL MANHOLE CASTINGS SHALL BE CONSIDERED SUBSIDIARY TO THE UNIT PRICES BID FOR THE VARIOUS MANHOLE TYPES.
7. GRIND ALL BURRS SMOOTH, CLEAN THOROUGHLY, THEN APPLY SHOP COAT OF ASPHALT BASE PAINT.
8. THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO MANUFACTURE. THE ENGINEER SHALL RETAIN THE RIGHT TO REJECT CASTINGS NOT CONFORMING TO THE SPECIFICATIONS OR THE APPROVED SHOP DRAWINGS.
9. THE MANHOLE FRAME SHALL BE FURNISHED WITH AN APPROVED "O" RING GASKET GROOVED INTO THE BEARING SURFACE OF THE MANHOLE FRAME (PER DETAIL). THE "O" RING GASKET SHALL NOT BE INSTALLED IN THE MANHOLE FRAME UNTIL AFTER FINAL INSPECTION AND ACCEPTANCE OF THE PROJECT BY THE ENGINEER.



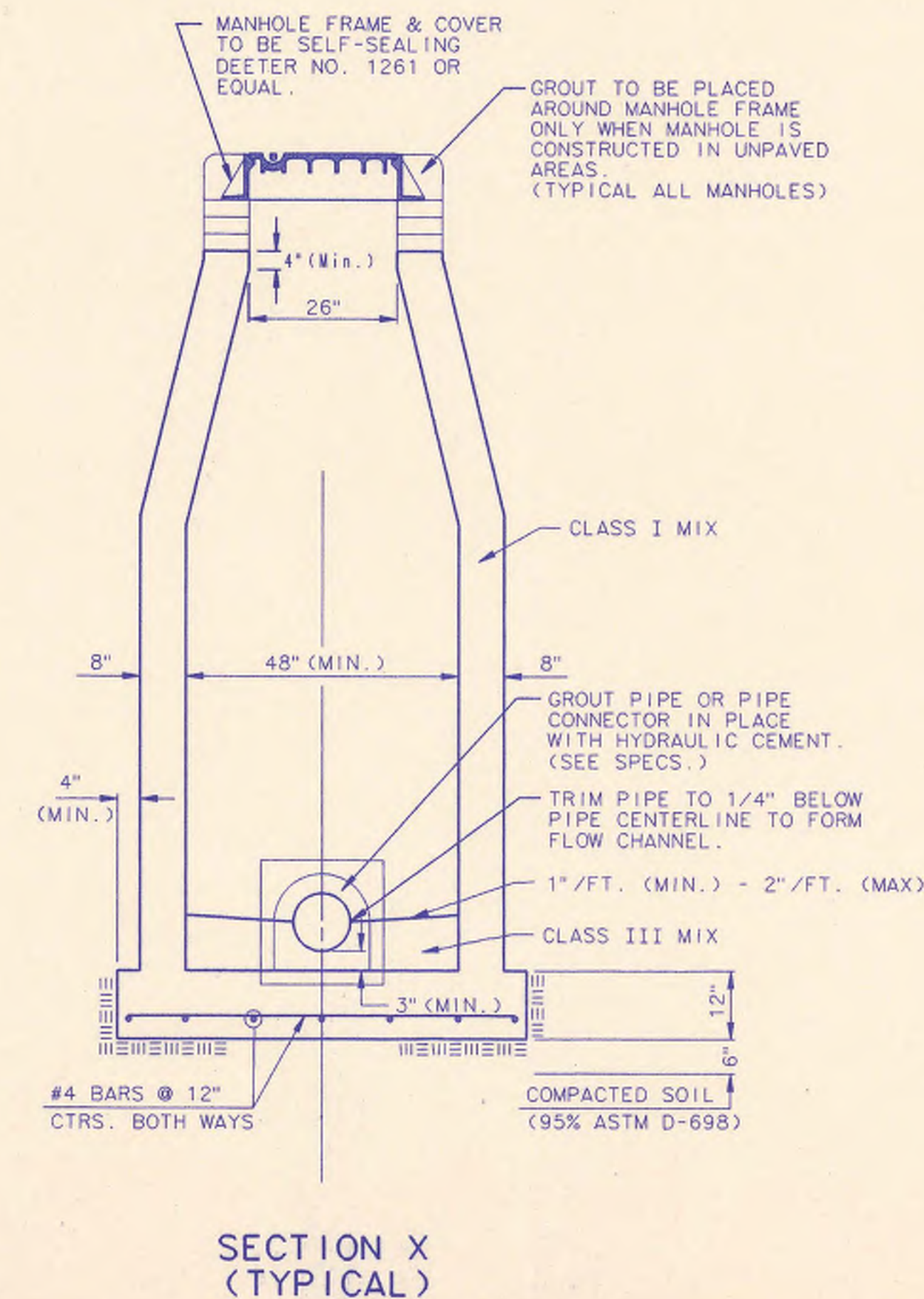
**CAST IN PLACE
STANDARD MANHOLE
TYPE "A"**



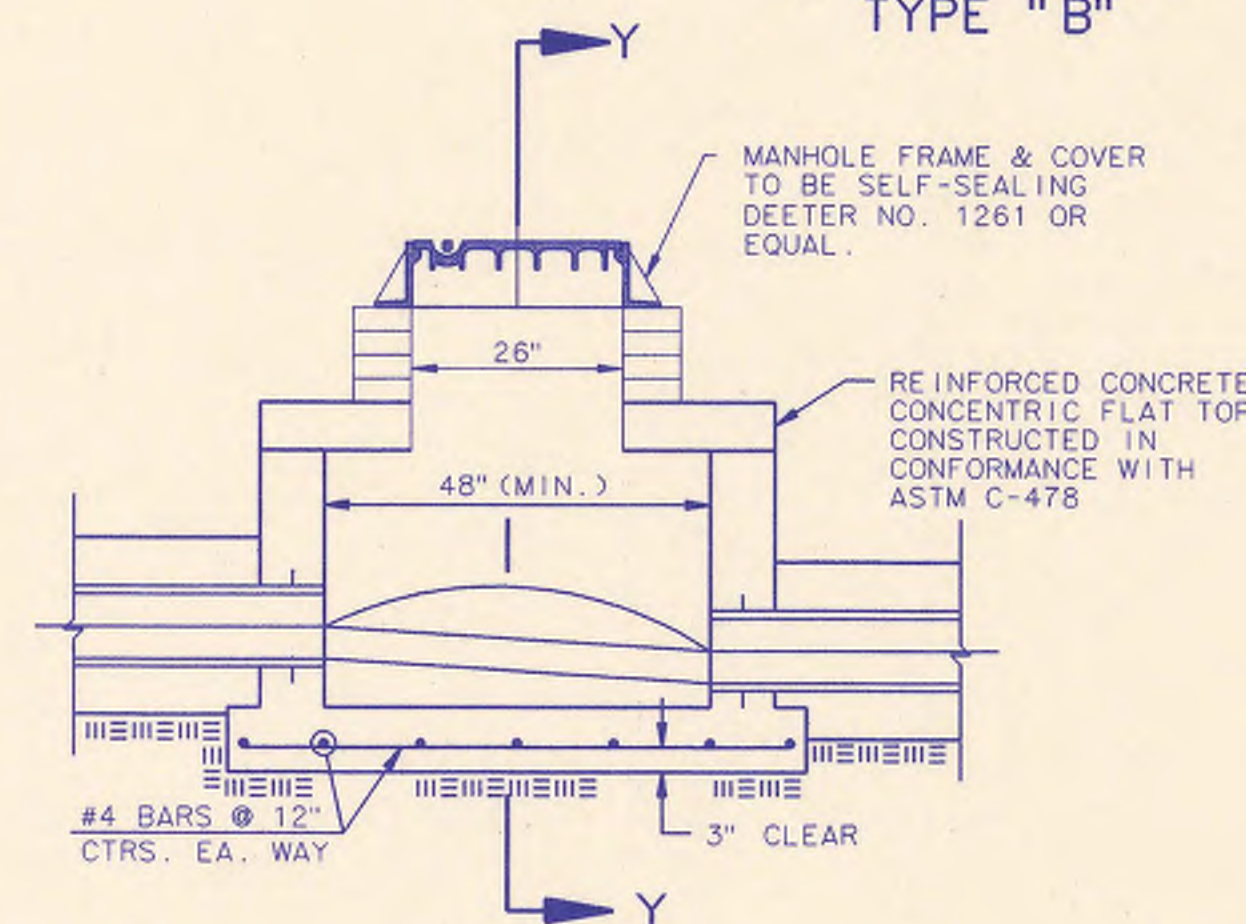
**CAST IN PLACE
DROP MANHOLE
TYPE "B"**



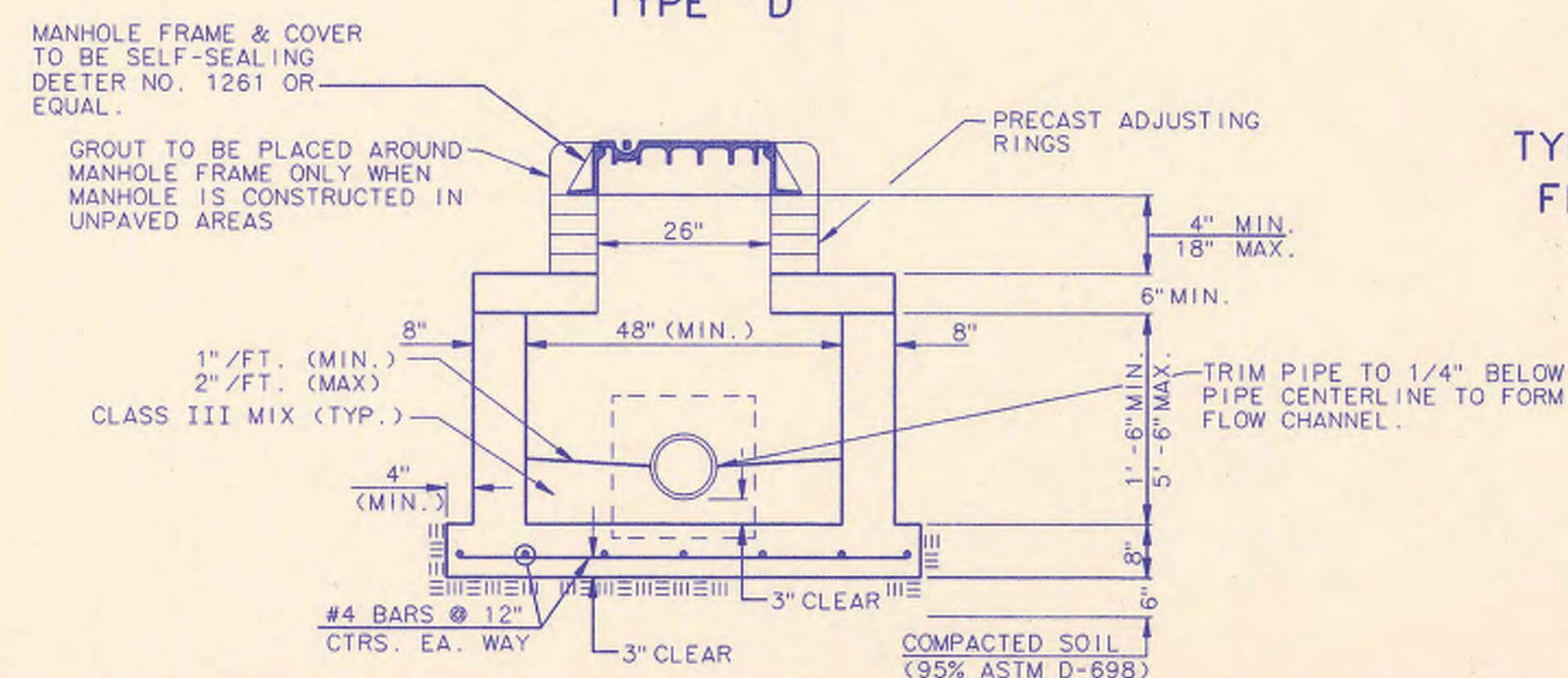
**CAST IN PLACE
OUTSIDE DROP MANHOLE
TYPE "C"**



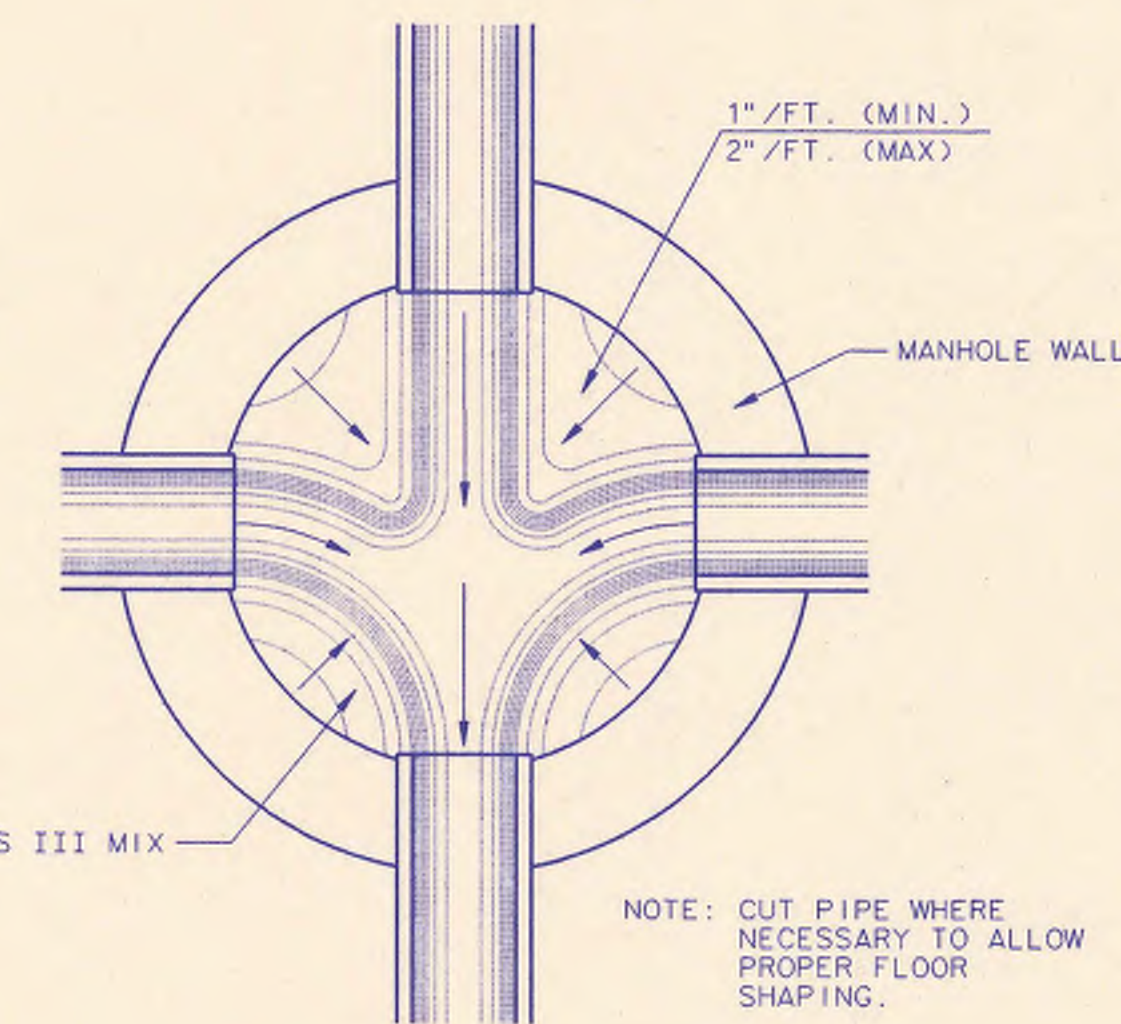
**SECTION X
(TYPICAL)**



**SHALLOW MANHOLE
TYPE "D"**



SECTION Y



**TYPICAL MANHOLE
FLOOR SHAPING**

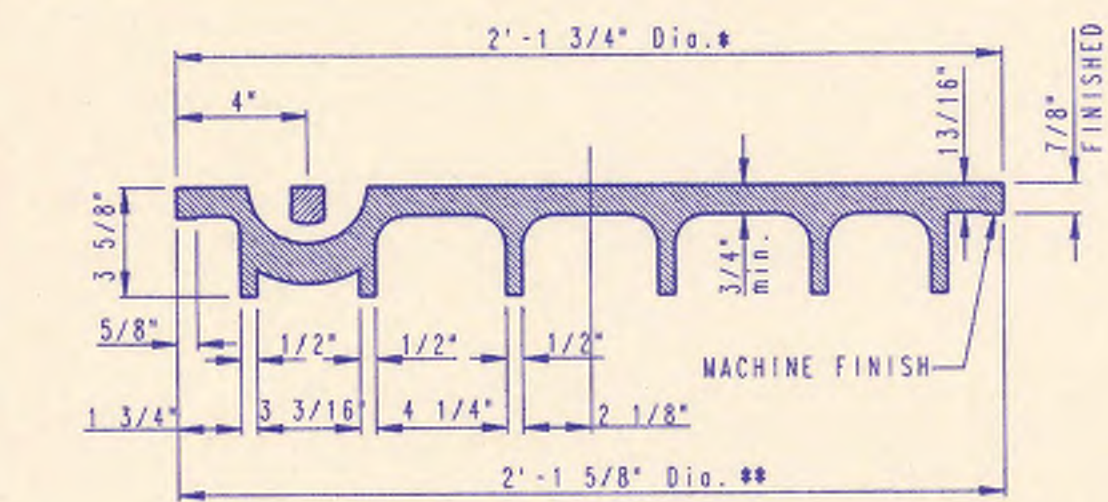
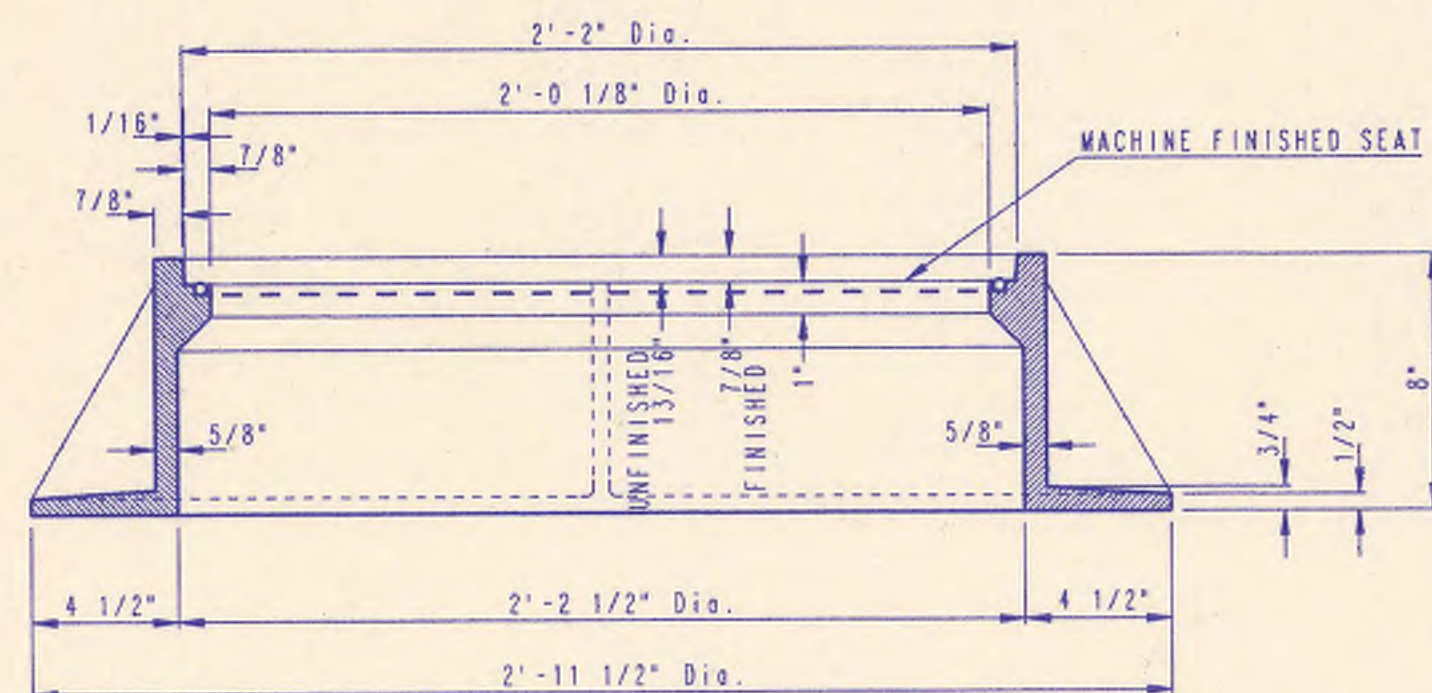
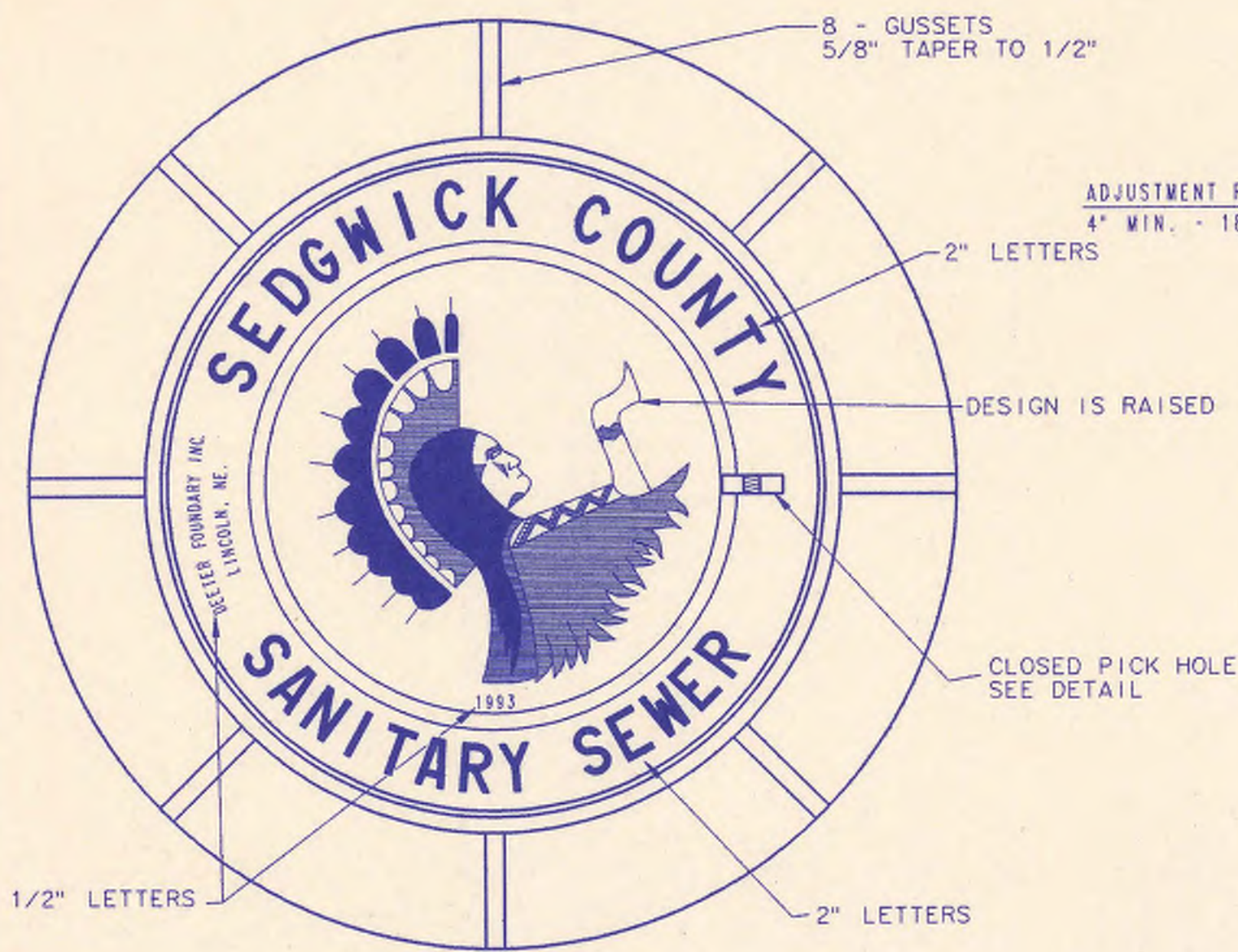
CAST-IN-PLACE MANHOLE NOTES

1. IF, IN THE OPINION OF THE ENGINEER, THE MANHOLE SUBGRADE APPEARS UNSTABLE, THE CONTRACTOR WILL HAVE THE OPTION TO COMPACT SUBGRADE AS SHOWN OR INCREASE THE THICKNESS OF THE MANHOLE BASE AS DIRECTED BY THE ENGINEER.
2. STEEL REINFORCING WILL BE REQUIRED IN ALL MANHOLE BASES.
3. WHEN OPENINGS ARE CUT IN MANHOLE WALLS FOR PIPE, THE PIPE OR PIPE CONNECTOR SHALL BE GROUTED IN PLACE WITH HYDRAULIC CEMENT. EXTERIOR OF COMPLETED CONNECTION SHALL BE SEALED WITH APPROVED COATINGS.
4. CAST-IN-PLACE CIRCULAR CONCRETE MANHOLES ARE TO BE CONSTRUCTED ONLY IN LOCATIONS WHERE IT IS OBVIOUS THAT ANY ADJUSTMENT OF THE MANHOLE TOP ELEVATIONS WHICH MAY BE NECESSARY WILL NOT REQUIRE MODIFICATION OF THE CONCRETE BARREL.
5. APPROVED FLEXIBLE WATERSTOP GASKETS WHICH MEET OR EXCEED THE TEST REQUIREMENTS OF ASTM C-923 SHALL BE INSTALLED TO JOIN THE SEWER TO THE MANHOLE WALL WHEN ABS COMPOSITE PIPE OR PVC PIPE IS USED. FOR OTHER TYPES OF PIPE THE SEWER EXTENDING FROM MANHOLES SHALL BE SUPPORTED WITH CONCRETE ENCASUREMENT A MINIMUM OF 3 FEET FROM THE MANHOLE WALL.
6. THE MANHOLE FRAME SHALL BE SEATED ON AN APPROVED BUTYL-RUBBER SEALANT TO PROVIDE A WATER-TIGHT SEAL BETWEEN THE MANHOLE ADJUSTMENT RING OR BRICK MASONRY COLLAR AND THE MANHOLE FRAME.
7. GASKETED PIPE CAPS SHALL BE PROVIDED BY THE PIPE SUPPLIER. GLUED OR CEMENTED CAPS WILL NOT BE ACCEPTED.
8. CONCRETE FOR MANHOLE BASES SHALL BE CLASS I AS DESCRIBED IN THE SPECIFICATIONS.
9. MANHOLES WITH PIPE SIZES LARGER THAN 15\"/>

No.	Revision	By	Date
3	All Manholes Self-sealing/ MH Cover	RJ	1/25/93
2	Modified Pipe connection note and MH Note 3	RJ	6/01/92
1	Manhole Frame and Cover Note 1	RJ	7/16/91

**CAST-IN-PLACE
MANHOLE DETAILS**

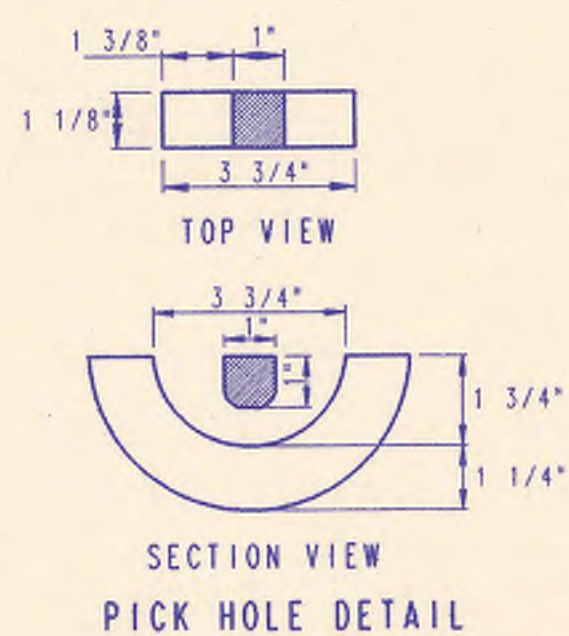
ADOPTED AS STANDARD DESIGN SEPTEMBER, 1989
BY
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES
DAVID C. SPEARS, P.E. DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER



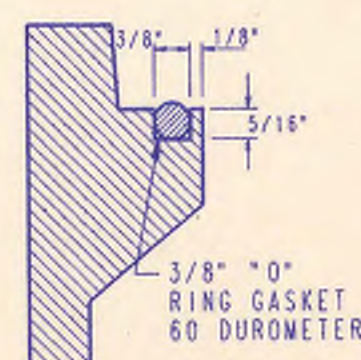
MANHOLE FRAME AND COVER
(TOTAL WEIGHT = 430 LBS.)

MANHOLE FRAME AND COVER NOTES

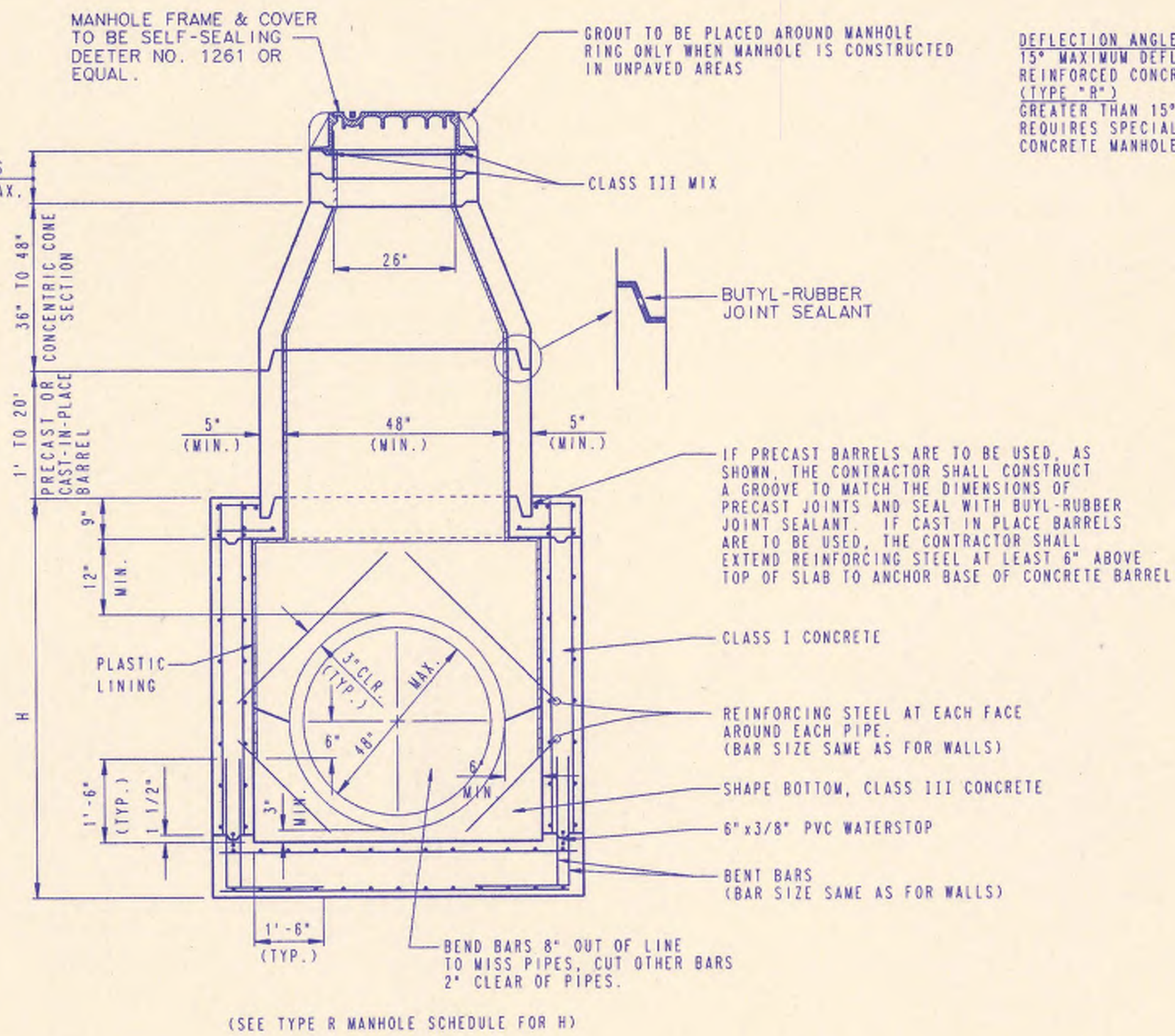
1. CAST IRON MANHOLE FRAME AND COVER SHALL CONFORM TO ASTM A-48, CLASS 35B, OR BETTER.
2. CASTINGS ARE TO BE MANUFACTURED TRUE TO PATTERN AND WITH SATISFACTORY FIT OF COMPONENT PARTS. CASTINGS SHALL BE FREE OF DEFECTS. DIMENSIONS AS DETAIL ON PLAN SHALL NOT DEVIATE BY $\pm 1/16"$ PER FOOT.
3. NO OTHER LETTERING OR MARKINGS OTHER THAN THOSE DETAILED ON PLAN WILL BE PERMITTED ON CASTINGS.
4. CASTINGS MUST BE DOMESTICALLY MANUFACTURED IN THE UNITED STATES OF AMERICA.
5. THE FRAMES AND COVERS SHALL BE FURNISHED WITH MACHINED HORIZONTAL BEARING SURFACES SO FITTING PARTS WILL NOT RATTLE OR ROCK UNDER TRAFFIC.
6. MANHOLE CASTINGS SHALL BE SELF-SEALING DEETER FOUNDRY INC. NO. 1261 OR APPROVED EQUAL. UNLESS OTHERWISE SPECIFIED IN THE SPECIAL CONDITIONS. (MINIMUM WT. ≥ 430 LBS.) ALL MANHOLE CASTINGS SHALL BE CONSIDERED SUBSIDIARY TO THE UNIT PRICES BID FOR THE VARIOUS MANHOLE TYPES.
7. GRIND ALL BURRS SMOOTH, CLEAN THOROUGHLY, THEN APPLY SHOP COAT OF ASPHALT BASE PAINT.
8. THE MANUFACTURER SHALL SUBMIT SHOP DRAWINGS TO THE ENGINEER FOR APPROVAL PRIOR TO MANUFACTURE. THE ENGINEER SHALL RETAIN THE RIGHT TO REJECT CASTINGS NOT CONFORMING TO THE SPECIFICATIONS OR THE APPROVED SHOP DRAWINGS.
9. THE MANHOLE FRAME SHALL BE FURNISHED WITH AN APPROVED "O" RING GASKET GROOVED INTO THE BEARING SURFACE OF THE MANHOLE FRAME (PER DETAIL). THE "O" RING GASKET SHALL NOT BE INSTALLED IN THE MANHOLE FRAME UNTIL AFTER FINAL INSPECTION AND ACCEPTANCE OF THE PROJECT BY THE ENGINEER.



**SECTION VIEW
PICK HOLE DETAIL**

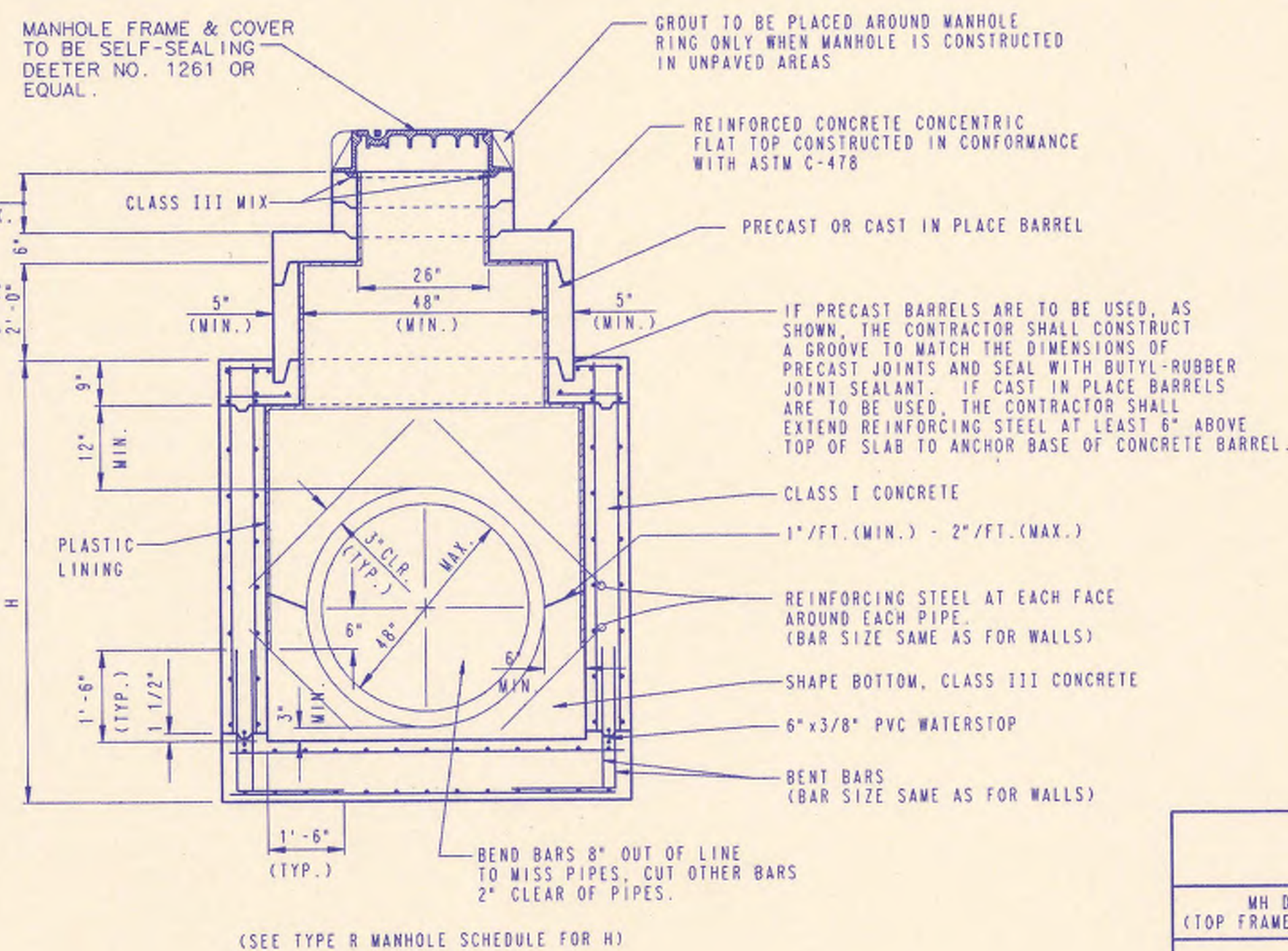


**FRAME GROOVING DETAIL
(TYPICAL ALL MANHOLES)**



**REINFORCED CONCRETE MANHOLE
(TOP MH TO $\frac{1}{2}$ OUT-9.0' MIN.)**

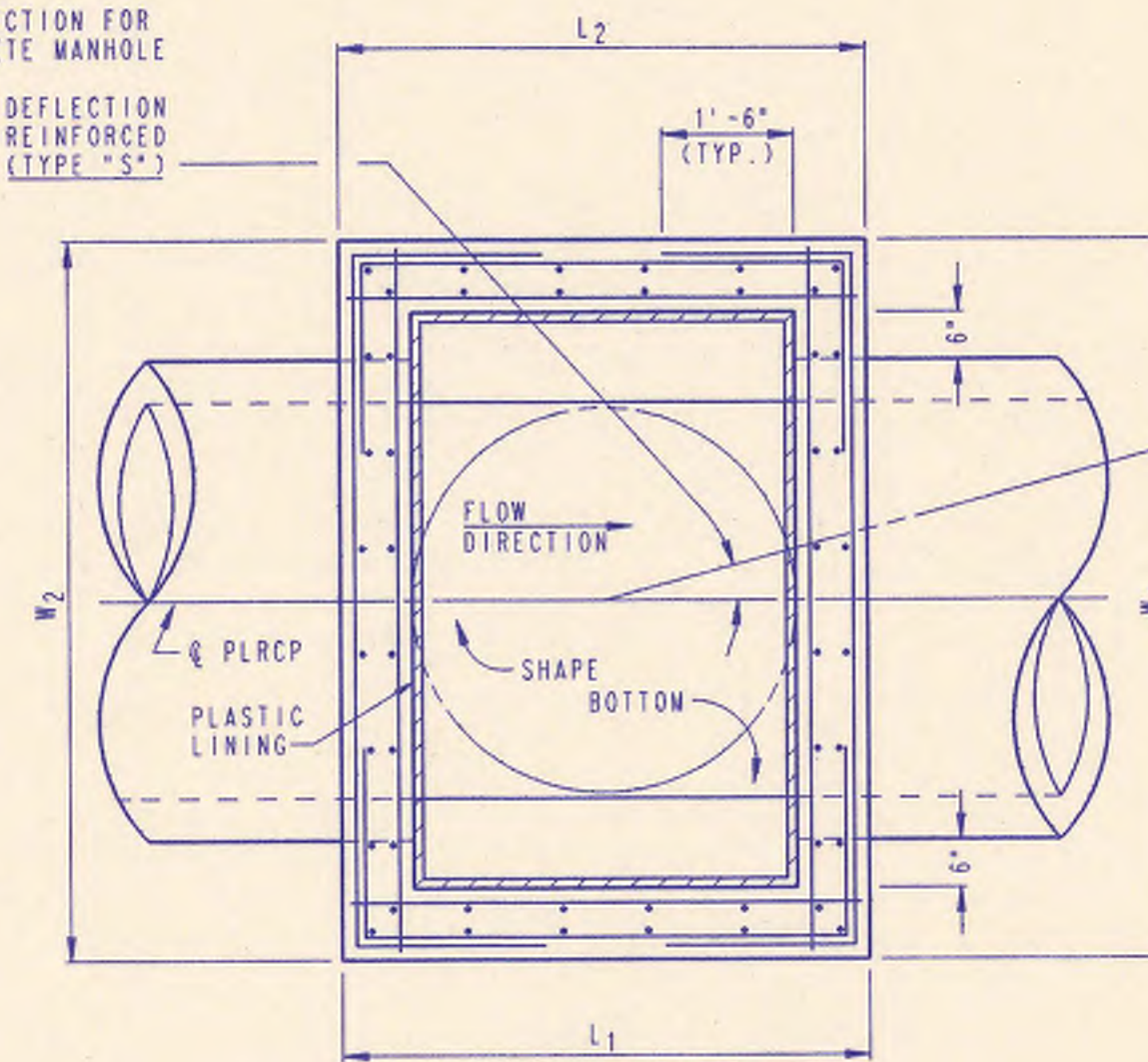
SEE "MANHOLE DIMENSIONS AND REINFORCING STEEL SCHEDULE" FOR WALL, FLOOR, AND TOP SLAB STEEL REINFORCEMENT.



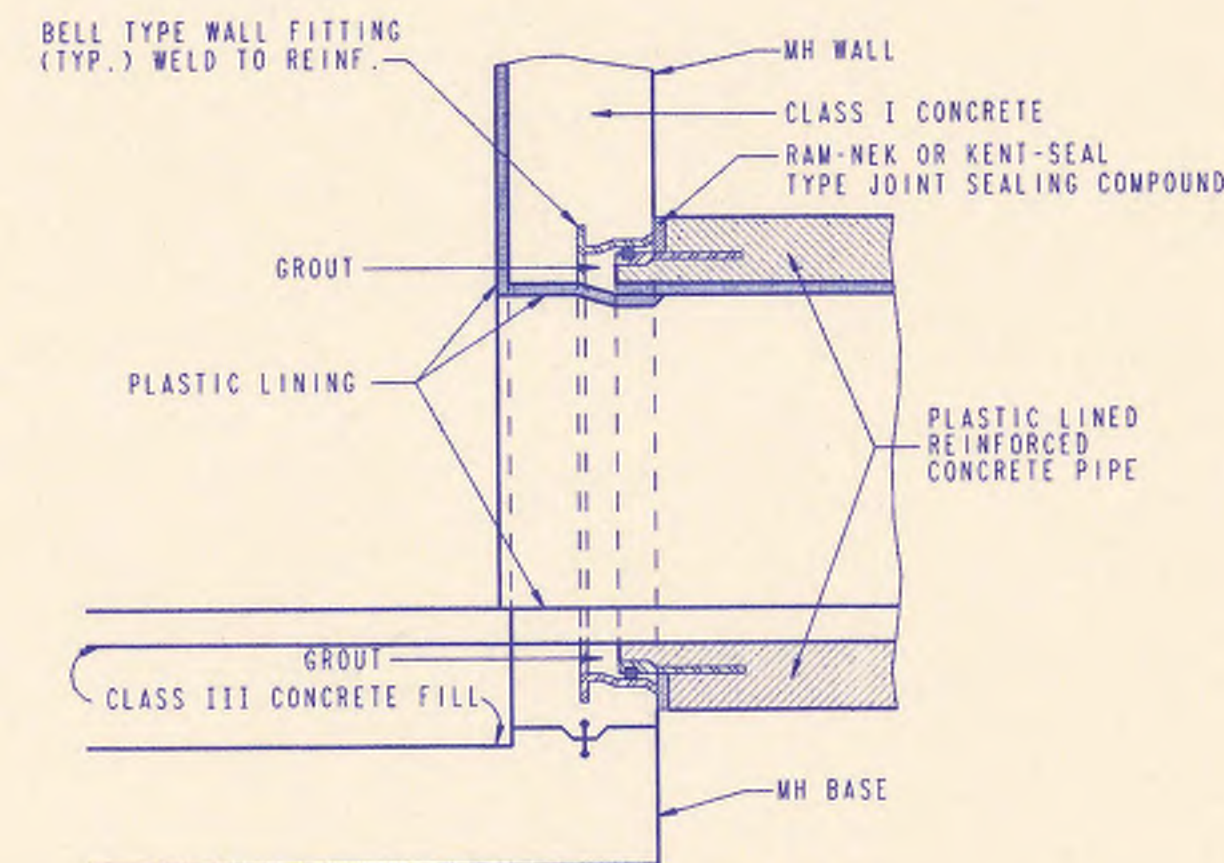
**REINFORCED CONCRETE MANHOLE
(TOP MH TO $\frac{1}{2}$ OUT-5.5' TO 10.8')**

SEE "MANHOLE DIMENSIONS AND REINFORCING STEEL SCHEDULE" FOR WALL, FLOOR, AND TOP SLAB STEEL REINFORCEMENT.

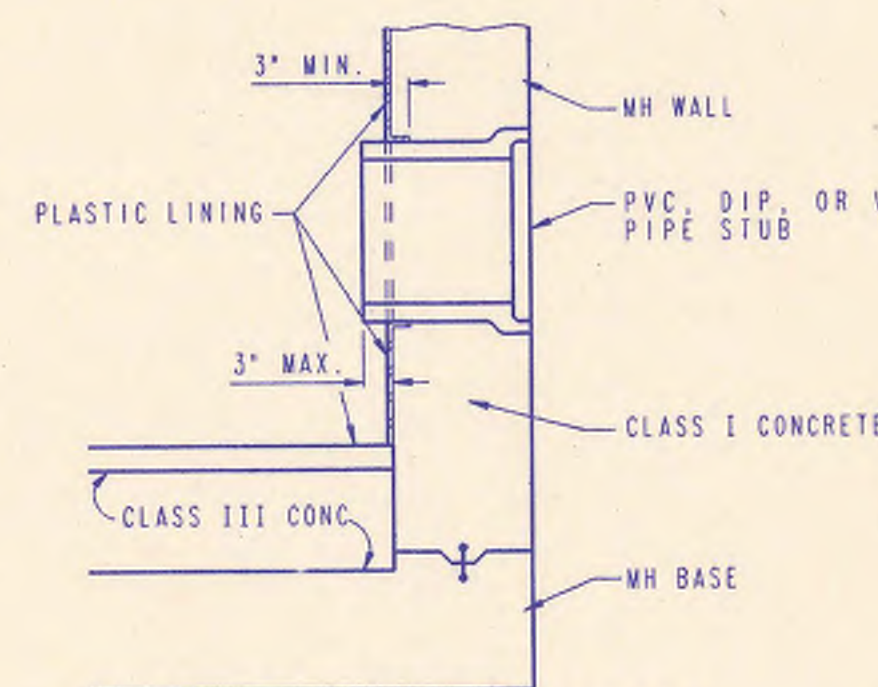
DEFLECTION ANGLE
15° MAXIMUM DEFLECTION FOR
REINFORCED CONCRETE MANHOLE
(TYPE "R")
GREATER THAN 15° DEFLECTION
REQUIRES SPECIAL REINFORCED
CONCRETE MANHOLE (TYPE "S")



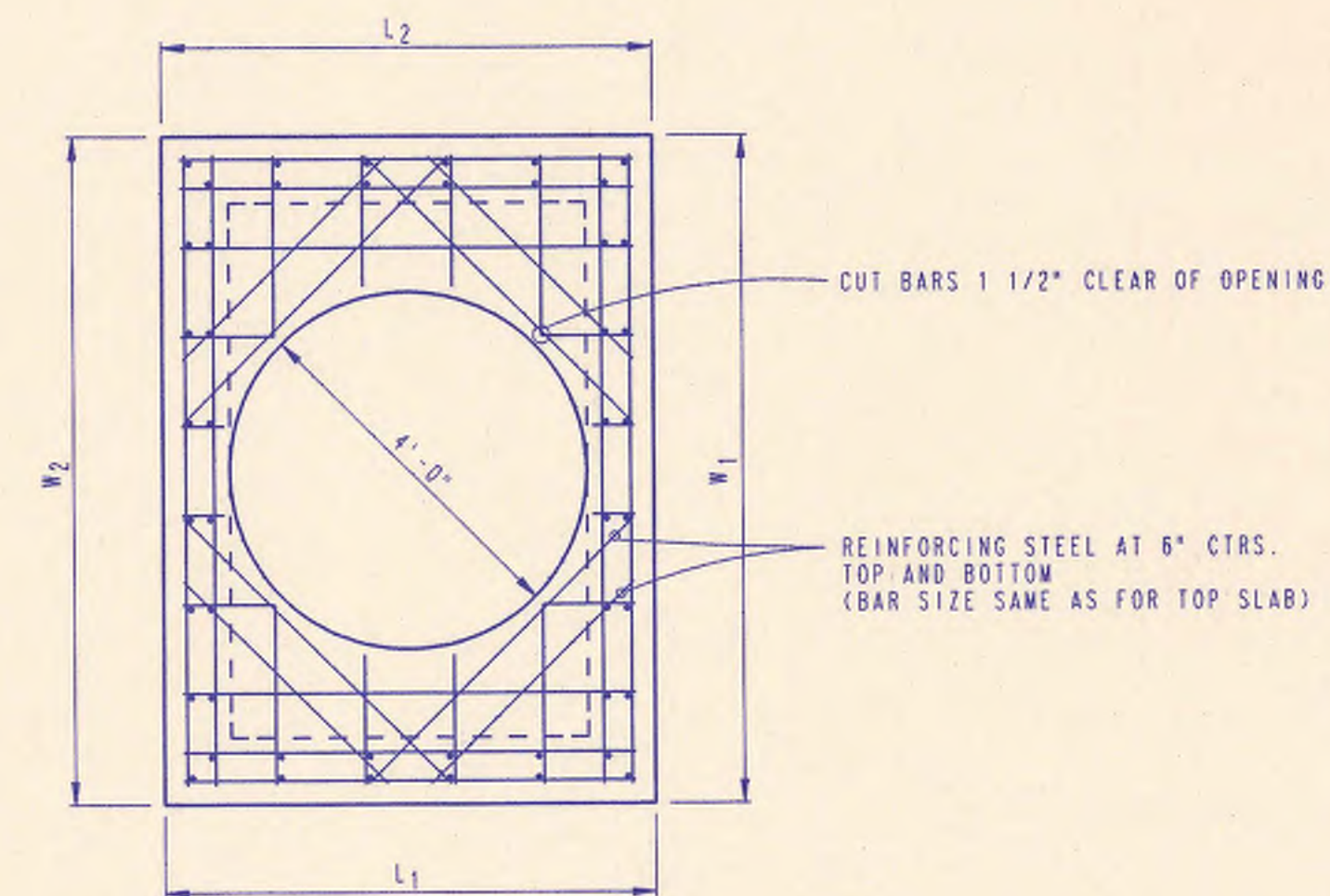
(SEE TYPE R MANHOLE SCHEDULE FOR W₁, W₂, L₁ AND L₂)
**REINFORCED CONCRETE MANHOLE
TYPICAL PLAN VIEW**



R.C.P. CONNECTION DETAIL



PIPE CONNECTION DETAIL



FOR DIMENSIONS, SEE TYPE "R"
MANHOLE SCHEDULE, THIS SHEET.

**REINFORCED CONCRETE MANHOLE
TYPICAL TOP SLAB REINFORCEMENT**

TYPE "R" MANHOLE SCHEDULE

MH NO.	PIPE SIZE	DIMENSIONS *				DEFLECTION ANGLE (d)
		H	W ₁	W ₂	L ₁	

* SIDE W₁ = DOWNSTREAM SIDE OF MANHOLE
SIDE W₂ = DIRECTLY OPPOSITE "W₁"
SIDE L₁ = ADJACENT TO, AND CLOCKWISE FROM "W₁"
SIDE L₂ = DIRECTLY OPPOSITE "L₁"
H = HEIGHT OF REINFORCED CONCRETE BOX

REINFORCED CONCRETE MANHOLE NOTES

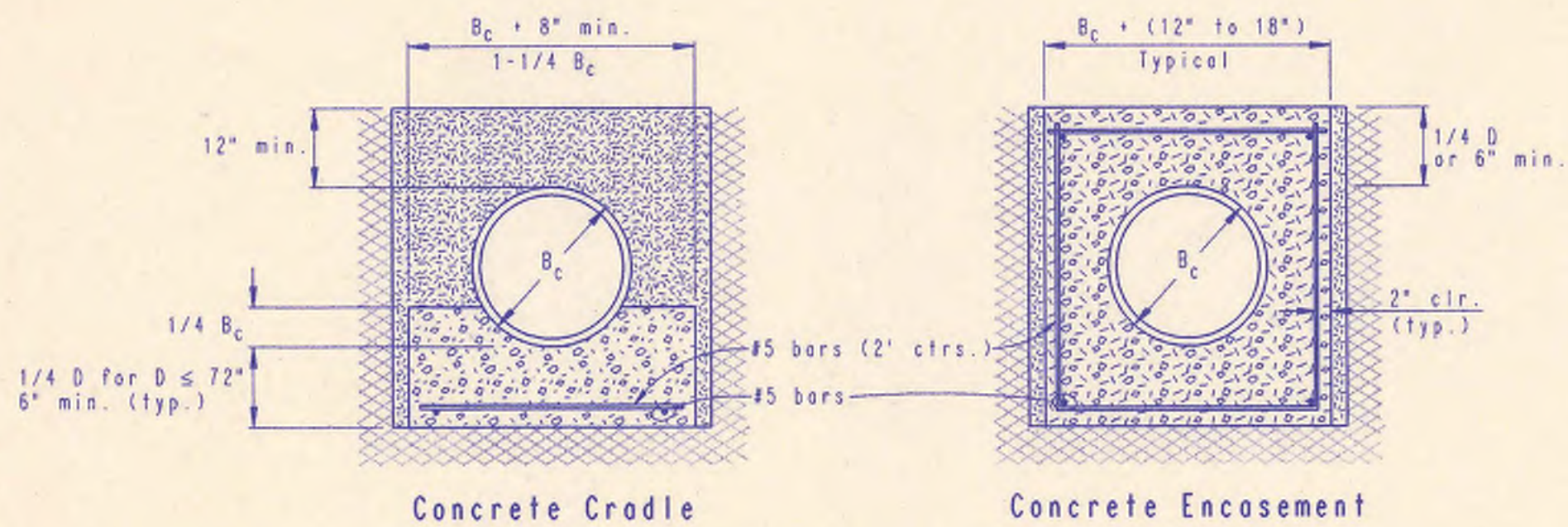
1. IF, IN THE OPINION OF THE ENGINEER, THE MANHOLE SUBGRADE APPEARS UNSTABLE, THE CONTRACTOR WILL HAVE THE OPTION TO COMPACT SUBGRADE AS SHOWN OR INCREASE THE THICKNESS OF THE MANHOLE BASE AS DIRECTED BY THE ENGINEER.
2. STEEL REINFORCING WILL BE REQUIRED IN ALL MANHOLE BASES.
3. WHEN OPENINGS ARE CUT IN MANHOLE WALLS FOR PIPE, THE PIPE OR PIPE CONNECTOR SHALL BE GROUTED IN PLACE WITH HYDRAULIC CEMENT. EXTERIOR OF COMPLETED CONNECTION SHALL BE SEALED WITH APPROVED COATINGS.
4. ALL MANHOLE CONSTRUCTION SHALL BE WATER TIGHT.
5. TOP OF MANHOLE FLOOR SLAB SHALL BE AT LEAST 3 INCHES BELOW THE FLOW LINE OF THE OUTLET PIPE TO INSURE SUFFICIENT MINIMUM THICKNESS OF SHAPED INVERT.
6. APPROVED FLEXIBLE WATERSTOP GASKETS WHICH MEET OR EXCEED THE TEST REQUIREMENTS OF ASTM C-923 SHALL BE INSTALLED TO JOIN THE SEWER TO THE MANHOLE WALL WHEN PLASTIC PIPE IS USED. SEWER PIPE EXTENDING FROM MANHOLES SHALL BE SUPPORTED WITH CONCRETE ENCASEMENT A MINIMUM OF 3 FEET FROM THE MANHOLE WALL.
7. GASKETED PIPE CAPS SHALL BE PROVIDED BY THE PIPE SUPPLIER. GROUTED OR CEMENTED CAPS WILL NOT BE ACCEPTED.
8. CONCRETE FOR MANHOLE WALLS AND BASES SHALL BE CLASS I AS DESCRIBED IN THE SPECIFICATIONS.
9. TYPE "R" MANHOLES SHALL BE REINFORCED CONCRETE MANHOLES WITH FOUR WALLS AS DETAILED ON THIS SHEET WITH WALL DIMENSIONS AS LISTED ON THE "TYPE R" MANHOLE SCHEDULE. THIS SHEET, TYPE "S" MANHOLES SHALL BE REINFORCED CONCRETE MANHOLES WITH FIVE OR MORE WALLS. REINFORCING FOR TYPE "S" MANHOLES SHALL BE AS DETAILED ON THIS SHEET.
10. WHERE TYPE "R" MANHOLES ARE REQUIRED, THE CONTRACTOR SHALL COMPLETE A TYPE "R" MANHOLE SCHEDULE AS SHOWN ABOVE FOR THE ENGINEERS APPROVAL PRIOR TO BEGINNING CONSTRUCTION. IF TYPE "S" MANHOLES ARE REQUIRED, THE CONTRACTOR SHALL SUBMIT SCALE PLAN VIEW MH DRAWINGS FOR THE ENGINEERS APPROVAL PRIOR TO BEGINNING CONSTRUCTION.

MANHOLE DIMENSIONS AND REINFORCING STEEL SCHEDULE FOR REINFORCED CONCRETE MANHOLES						
MH DEPTH (TOP FRAME TO $\frac{1}{2}$ OUT)	DESCRIPTION	THICKNESS	STEEL PLACEMENT		STEEL CLEARANCE	
5.5' TO 11.0'	TOP SLAB:	9"	#4 @ 12" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		1 1/2" EA. FC.	
	WALLS:	9"	#4 @ 12" MAX. CTRS. (BOTH WAYS), EACH FACE		1 1/2" EA. FC.	
11.0' TO 16.0'	FLOOR:	12"	#4 @ 8" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		2" EA. FC.	
	TOP SLAB:	9"	#5 @ 12" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		1 1/2" EA. FC.	
16.0' TO 22.0'	WALLS:	9"	#5 @ 12" MAX. CTRS. (BOTH WAYS), EACH FACE		1 1/2" EA. FC.	
	FLOOR:	12"	#5 @ 8" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		2" EA. FC.	
22.0' TO 30.0'	TOP SLAB:	9"	#5 @ 12" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		1 1/2" EA. FC.	
	WALLS:	12"	#5 @ 12" MAX. CTRS. (BOTH WAYS), EACH FACE		1 1/2" EA. FC.	
22.0' TO 30.0'	FLOOR:	12"	#5 @ 8" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		2" EA. FC.	
	TOP SLAB:	12"	#5 @ 12" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		1 1/2" EA. FC.	
	WALLS:	12"	#5 @ 12" MAX. CTRS. (BOTH WAYS), EACH FACE		1 1/2" EA. FC.	
	FLOOR:	12"	#6 @ 8" MAX. CTRS. (BOTH WAYS), TOP & BOTTOM		2" EA. FC.	

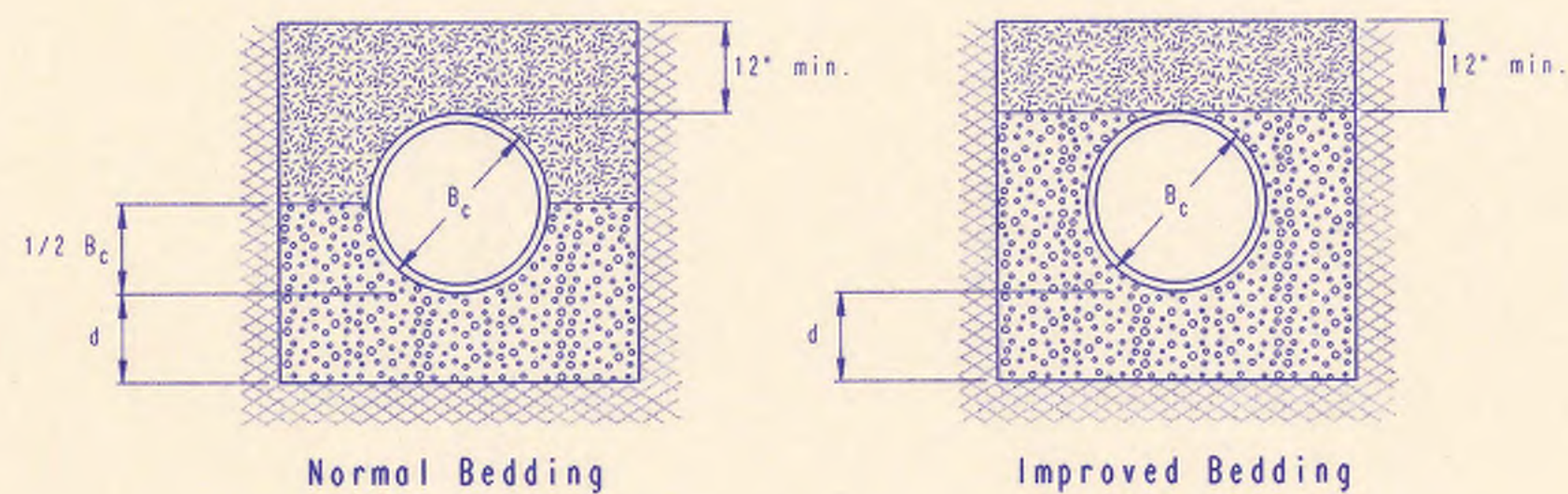
1	All Manholes Self-sealing/ MH Cover	RJ	1/25/93
No.	Revision	By	Date

**REINFORCED CONCRETE
MANHOLE DETAILS**

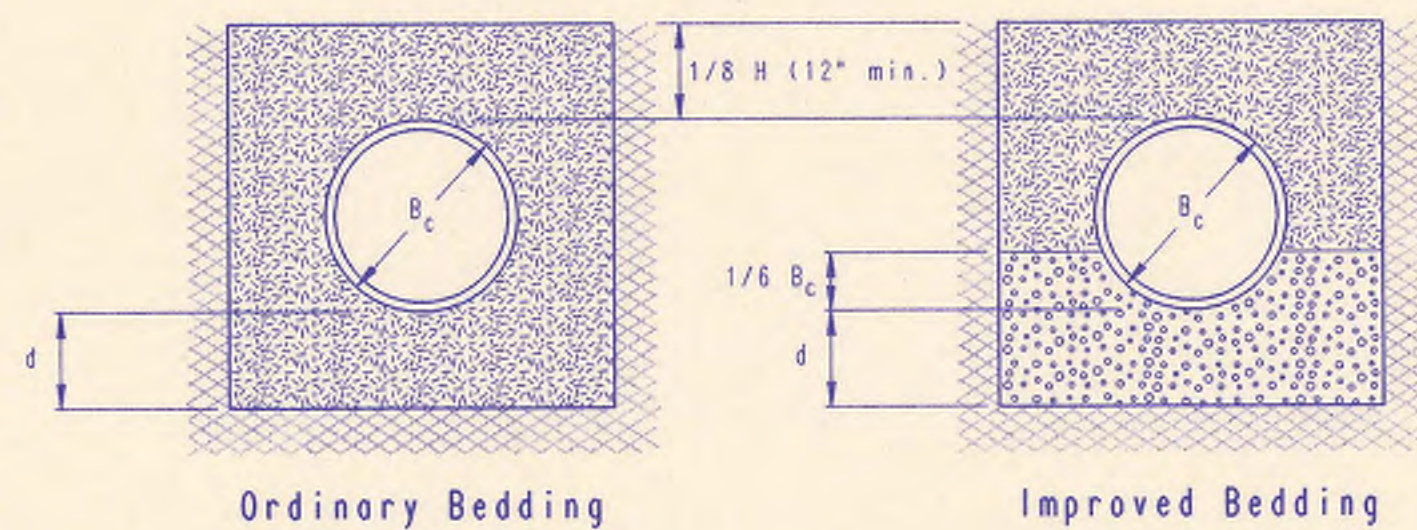
ADOPTED AS STANDARD DESIGN SEPTEMBER, 1989
BY
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES
DAVID C. SPEARS, P.E., DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER



CLASS A



CLASS B



CLASS C

PIPE ZONE BACKFILLING

- B_c = Outside Pipe Diameter
 H = Backfill from Top of Pipe to Existing Ground
 D = Inside Pipe Diameter
 d = Depth of Bedding Material Below Pipe
- Granular Bedding Material or Sand-Gravel Bedding
Compacted Embedment
Concrete (Class III)

Depth of Bedding Material Below Pipe		
D	d(min) Soil	d(min) Rock
27" & smaller	4"	6"
30" to 60"	5"	9"
66" & larger	6"	12"

Granular Bedding Material shall be an approved material consisting of durable crushed rock conforming with the requirements of the latest revision of ASTM C-33 Size No. 67 (3/4" to No. 4); to be placed in not more than 6" layers and compacted by slicing with a shovel or vibrating. Soundness, abrasion, and absorption limits shall be as required for coarse aggregates in Section 03010-Concrete Work in the specifications.

Sand-Gravel Bedding Material - sand-gravel mix meeting Type UD-1 of the 1990 Kansas Standard Specifications for State Road and Bridge Construction.

Compacted Embedment shall be an approved sand material free from debris, organic material, and stones with 100% passing the 3/4" sieve to be placed in uniform layers not more than 6" thick and compacted to 90 percent maximum density as determined by ASTM D1557. Granular Bedding Material may be substituted for all or part of Compacted Embedment Materials.

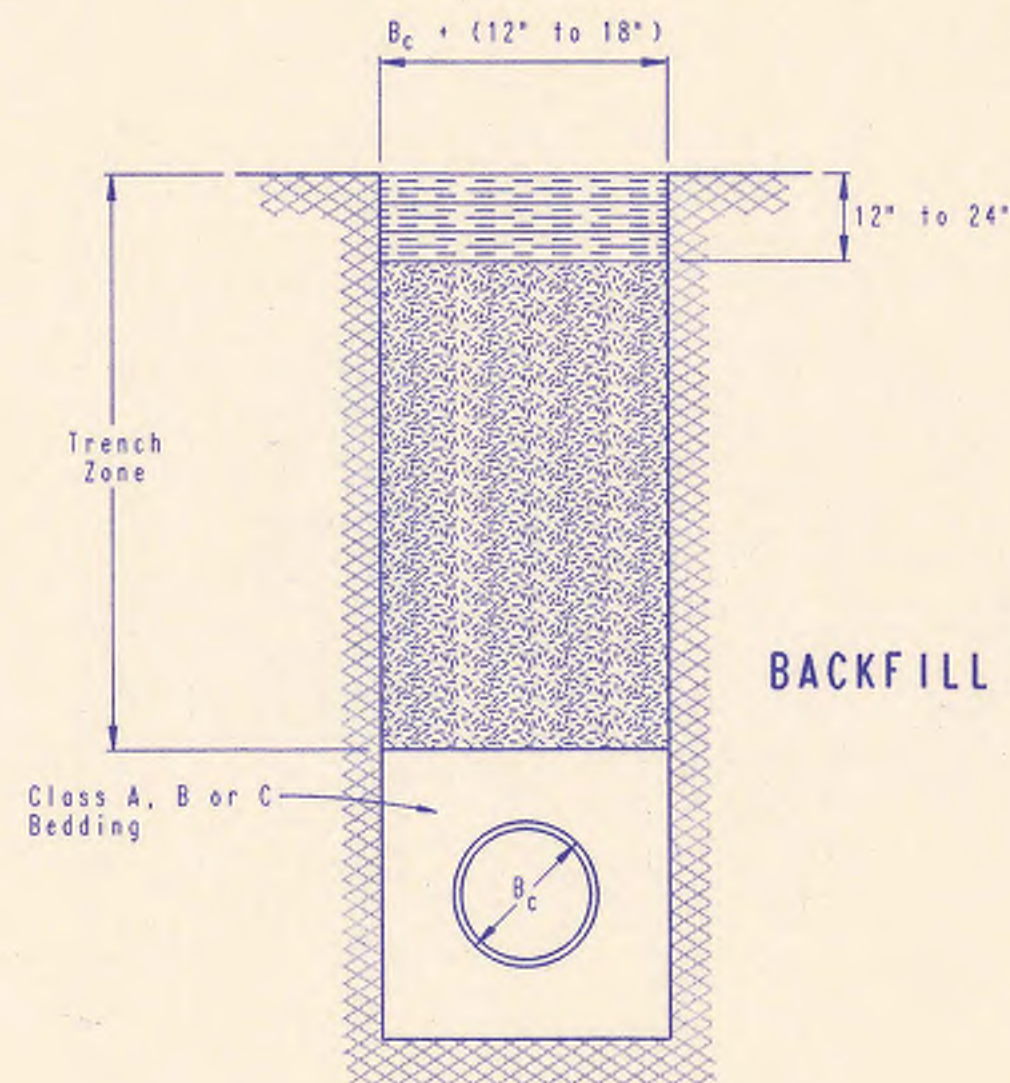
Class A "Concrete Cradle" and/or Class A "Concrete Encasement" is not required unless specified on the plans. However, where unexpected trench conditions exist or improper trenching is performed Class A Bedding may be required as determined by the Engineer.

Class B Bedding shall be used for all flexible pipe.

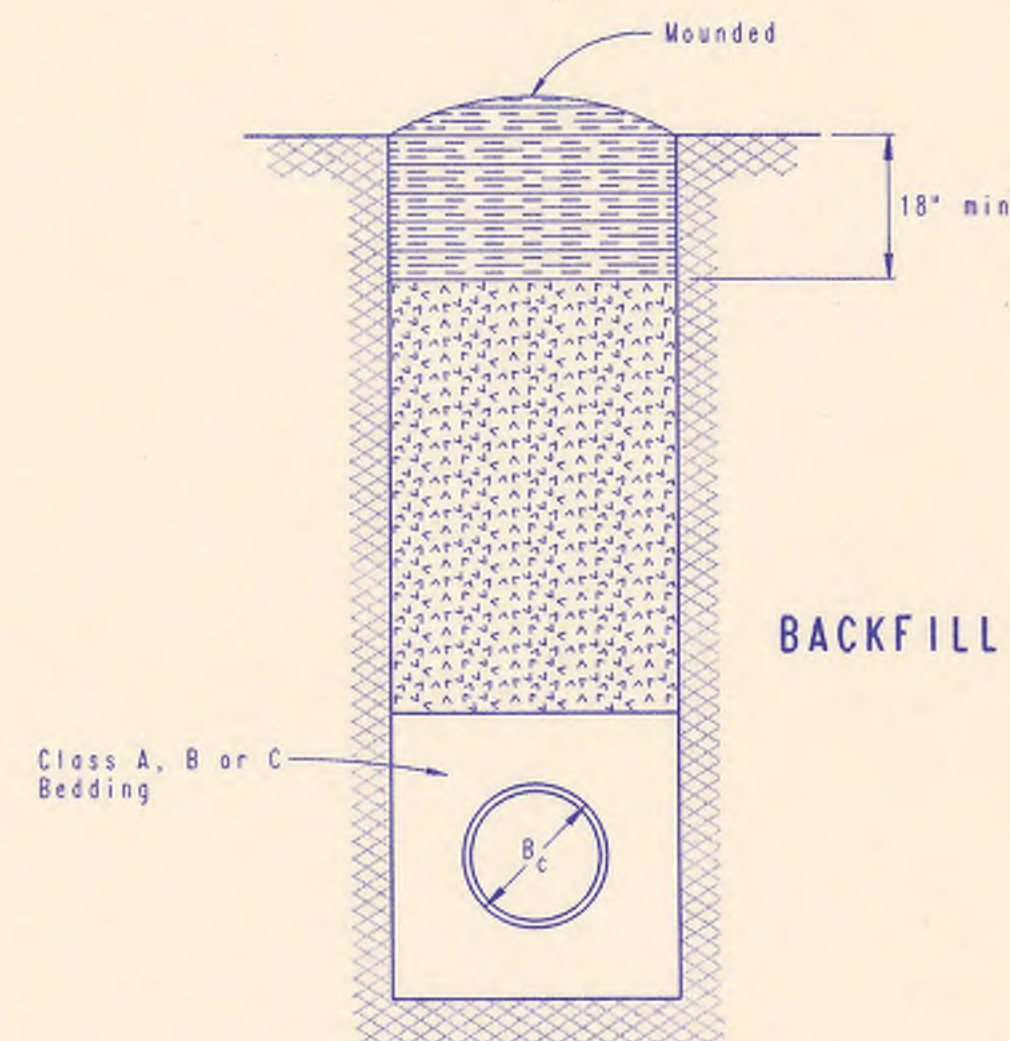
- Class B Normal Bedding shall be used for PVC Pipe unless wet conditions are encountered.
- Class B Improved Bedding shall be used for other flexible pipe, and for PVC pipe in wet conditions.

Class C Bedding shall be used for all rigid pipe.

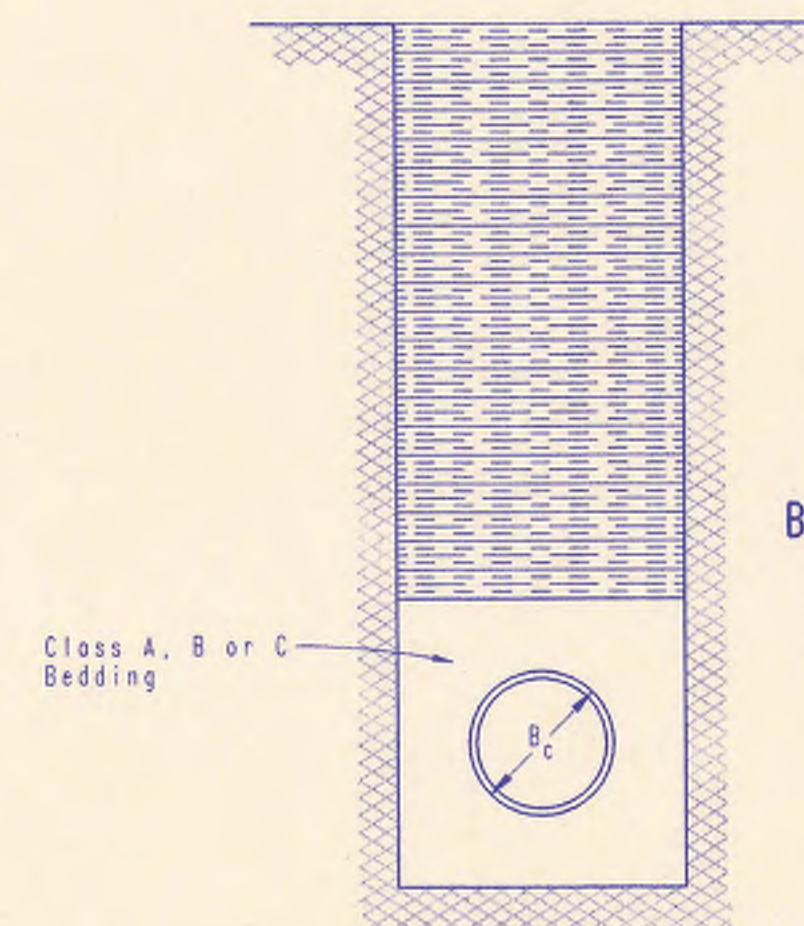
- Class C Ordinary Bedding shall be used for all rigid pipe unless wet conditions are encountered.
- Class C Improved Bedding shall be used for wet conditions existing in the trench, as directed by the Engineer, at no additional cost to the Owner. The dimensions shall be equal to that required for "rock" excavation (see specifications).



BACKFILL TYPE I



BACKFILL TYPE II



BACKFILL TYPE III

TRENCH ZONE BACKFILLING

- B_c = Outside Pipe Diameter
Compacted Granular Backfill
Uncompacted Earth Backfill
Compacted Earth Backfill

Compacted Granular Backfill material shall be an approved sand material free from debris, organic material and stones with 100% passing the 3/4" sieve and not more than 15% passing a No. 200 sieve; to be jetted and mechanically vibrated into place and compacted to 90% density as determined by ASTM D1557.

Uncompacted Earth Backfill material may be natural soil free from large clods or stones, brush, roots more than 2 inches in diameter, debris, and junk. Flooding with water shall be provided as directed by the Engineer.

Compacted Earth Backfill shall consist of material existing prior to trenching or selected material as directed by the Engineer, and shall be compacted to 90% density as determined by ASTM D1557.

Backfill: Backfill material and compaction requirements shall conform to either Type I, Type II or Type III as specified in the plans. One years maintenance will be required on all backfill.

Backfilling Through Rock: Backfilling through rock shall be performed as specified in the paragraph Backfill above, except that the Pipe Zone is increased to provide eighteen (18) inches of cover over the pipe. When approved by the Engineer the remainder of the backfill may be excavated rock provided the excavated rock has been broken up so that earth and rock will thoroughly mix and not result in voids around the larger pieces of rock. Any excess rock remaining after the trench has been backfilled shall be removed or wasted as directed by the Engineer.

Backfilling Under Pavement: Backfilling under existing or proposed pavement shall be performed as Backfill Type I to a level of two (2) feet from the bottom of the pavement. The remainder of the trench shall be backfilled with selected material, sufficiently damp to be properly compacted in layers not exceeding six (6) inches in depth, compaction shall be performed with mechanical tampers and continued until a relative density of 95 percent of modified density, in conformance with ASTM D1557 is obtained.

Backfilling Under Gravel Streets: Where the trench crosses or is in existing gravel surfaced streets, the backfill shall be compacted as provided in the paragraph "Backfilling Under Pavement".

No.	Revision	By	Date
BACKFILL DETAILS			
ADOPTED AS STANDARD DESIGN SEPTEMBER, 1989			
BY			
SEDGWICK COUNTY BUREAU OF PUBLIC SERVICES			
DAVID C. SPEARS, P.E. DIRECTOR, BUREAU OF PUBLIC SERVICES/COUNTY ENGINEER			

