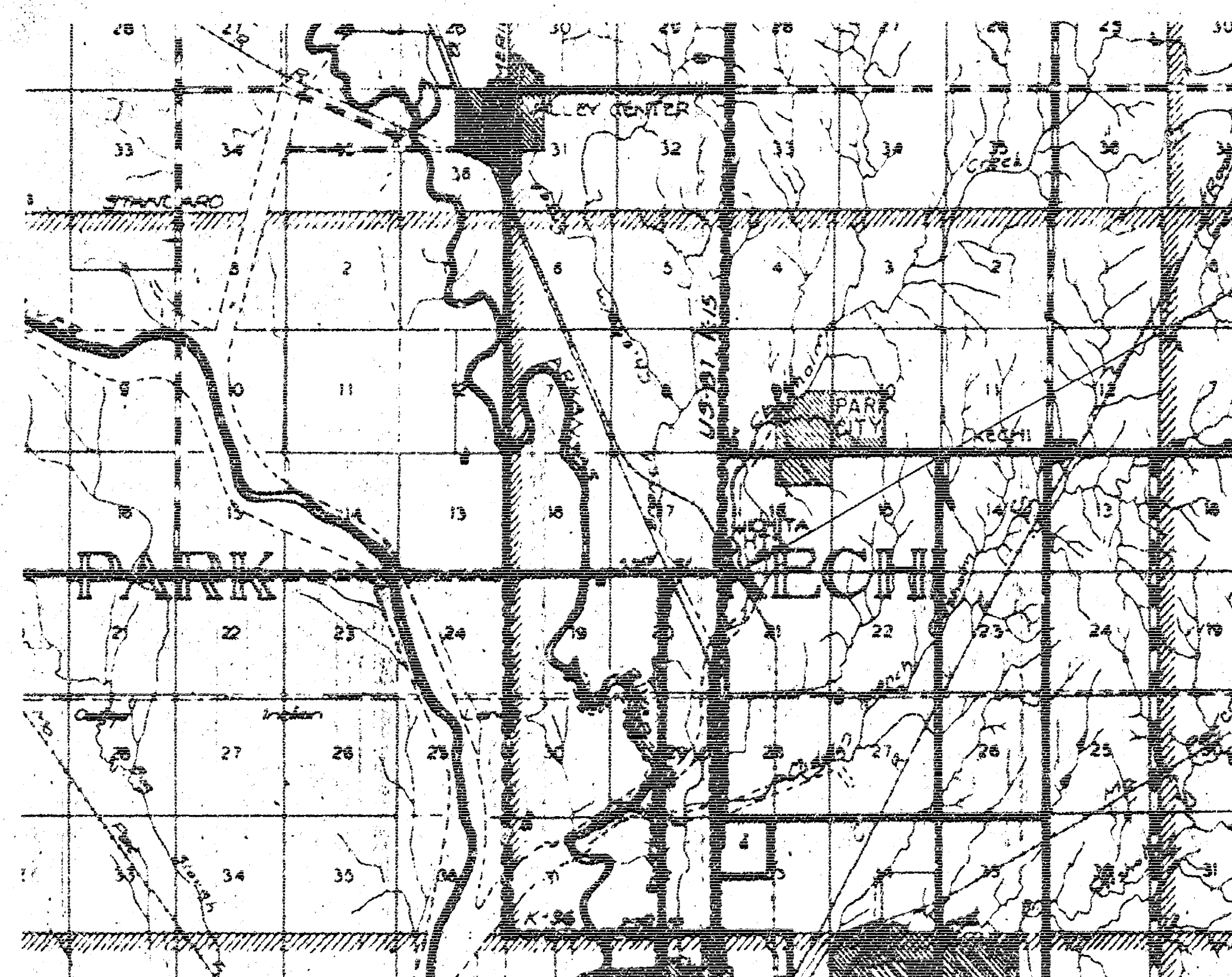


INDEX OF SHEETS	
SHEET NO. 1	TITLE SHEET
SHEET NO. 2	PLAN, PROFILE AND RIP-RAP PLAN
SHEET NO. 3	CONSTRUCTION LAYOUT
SHEET NO. 4	CONCRETE FLOOR DETAILS
SHEET NO. 5	STEEL LAYOUT & CONCRETE DETAILS
SHEET NO. 6	ABUTMENT, PIER, AND AUXILIARY DETAILS
SHEET NO. 7	BAR DIAGRAMS & REINFORCING STEEL QUANTITIES
SHEET NO. 8	STANDARD FILE DETAILS
SHEET NO. 9	CROSS SECTIONS



BRIDGE-GRADING & RIP-RAP

STATE	PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
KANSAS				



CONSTRUCT BRIDGE NO. 606-27-983 (2°12'-20" LT. SKEW)
CONTINUOUS STEEL-BEAM, WITH
CONCRETE DECK - 48'-0" RDWY.
WITH ALUMINUM HANDRAIL.
SPANS: 30'-4 @ 42'-6" - 30'

TRAFFIC COUNT - 600 CARS PER DAY

INCLOSURE WITH LETTER DATED 19 Dec 1957
FROM 120 Park Ave

REC'D DEC 19 1957

CONVENTIONAL SIGNS

COUNTY LINE	-----
SECTION LINE	-----
WIRE FENCE	-----
HEDGE ROW	-----
RAILROADS	-----
SURVEY LINE	-----
RIGHT OF WAY	-----
TELEPHONE POLE	-----
POWER POLE	-----
TRAVELED WAY	-----

NET LENGTH OF PROJECT	750	FT.	.142	MILES
NET LENGTH OF BRIDGES	232'-8"	FT.	.044	MILES
NET LENGTH OF ROAD	517.33	FT.	.098	MILES
EXCEPTIONS	NONE	FT.	---	MILES
ADDITIONS	NONE	FT.	---	MILES
GROSS LENGTH OF PROJECT	750	FT.	.142	MILES

PLANS PREPARED BY

SEDGWICK CO. ENGR. DEPT.

DATE NOVEMBER 1957

APPROVED

DATE

CHAIRMAN
BOARD OF COUNTY COMMISSIONERS

APPROVED

COUNTY ENGINEER

DATE NOVEMBER 1957

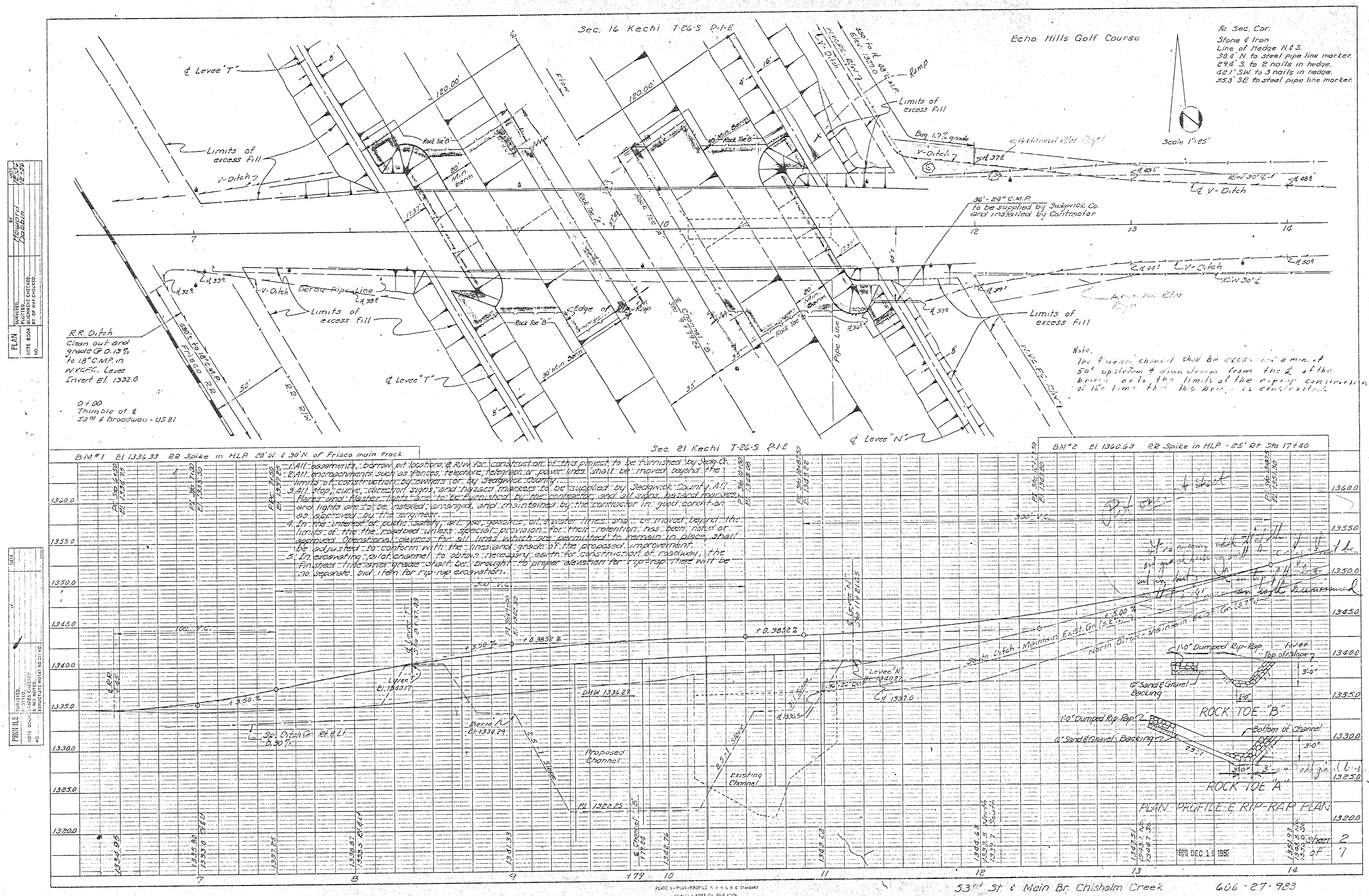
APPROVED

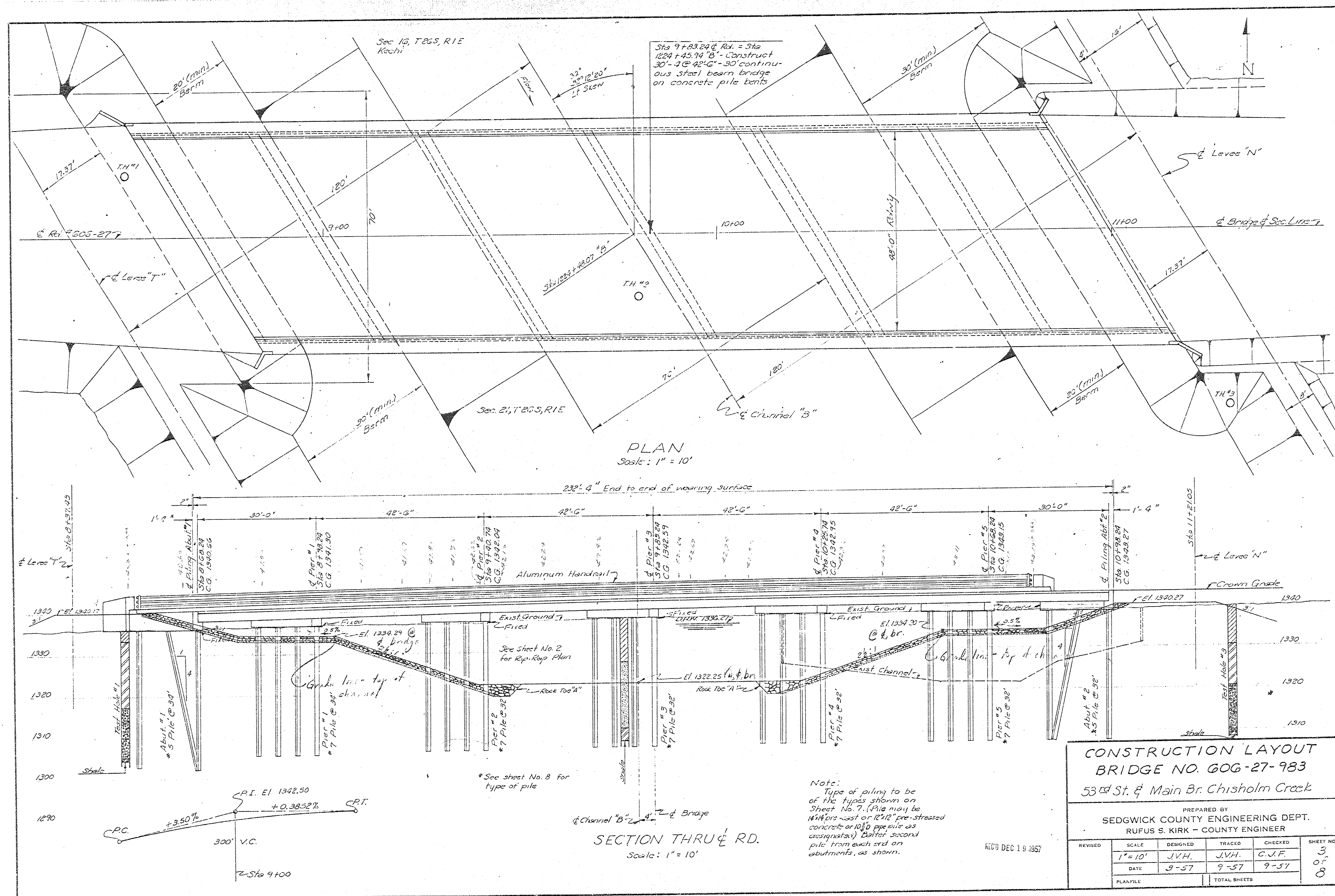
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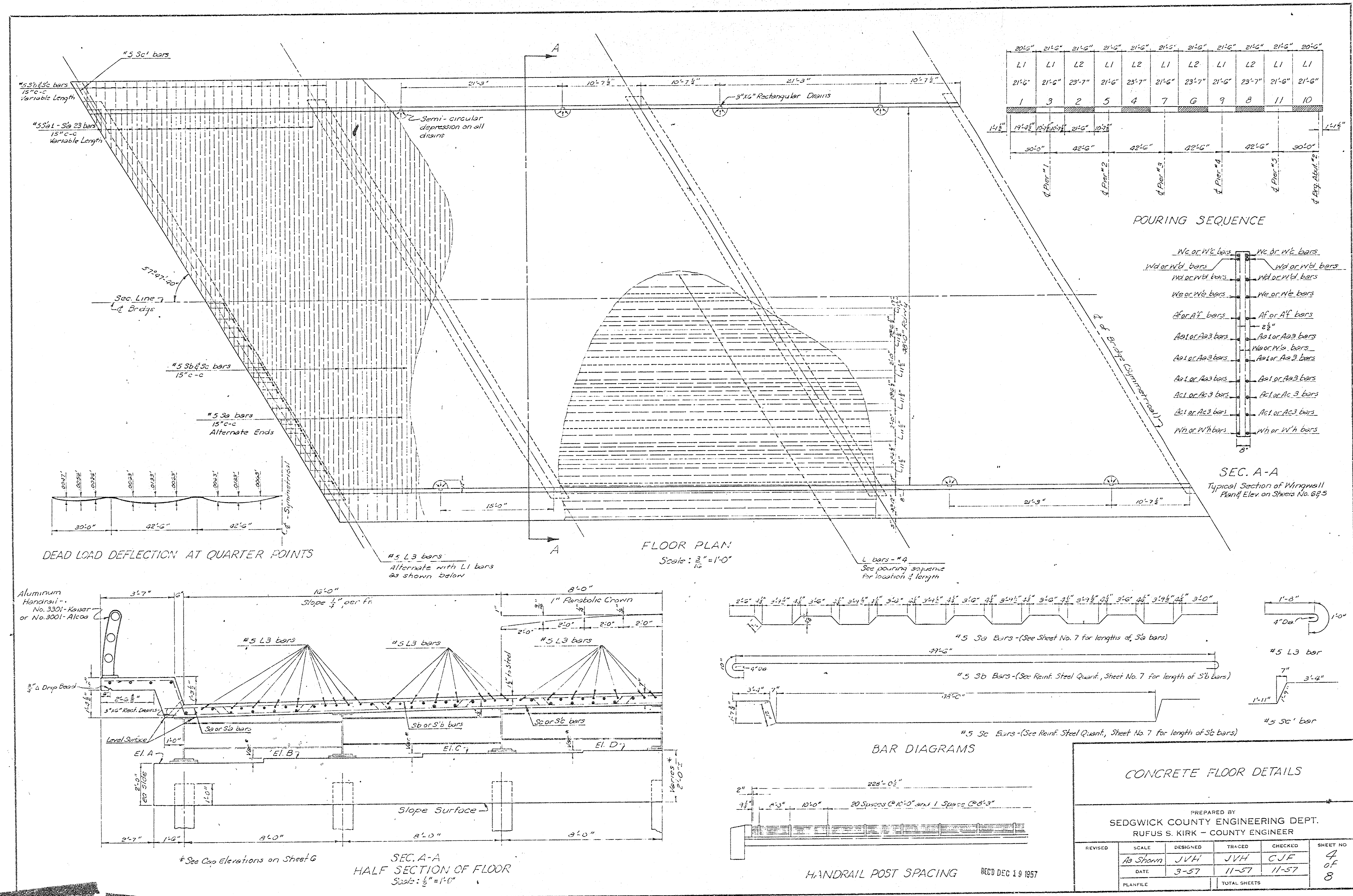
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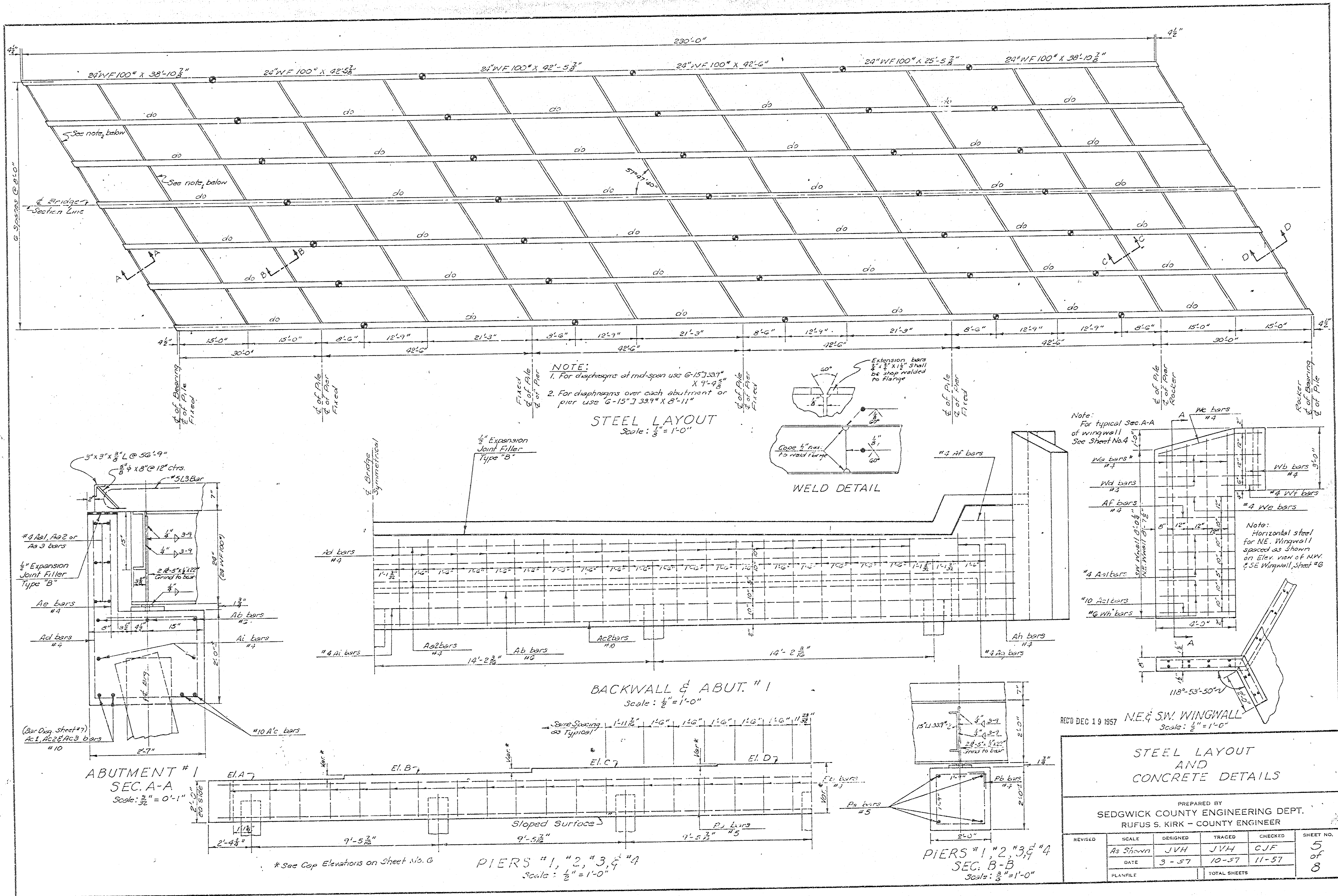
Per John A. Officer

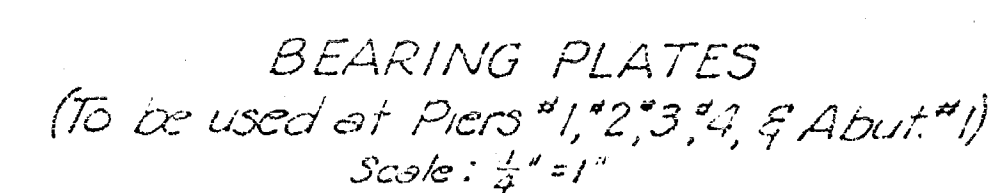
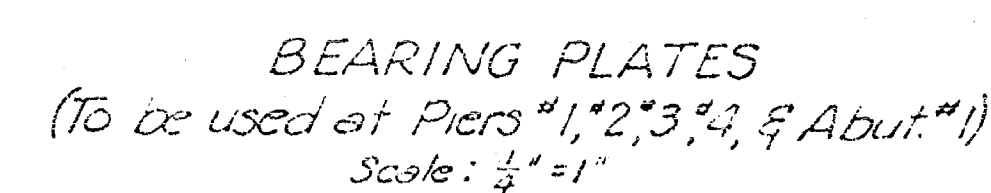
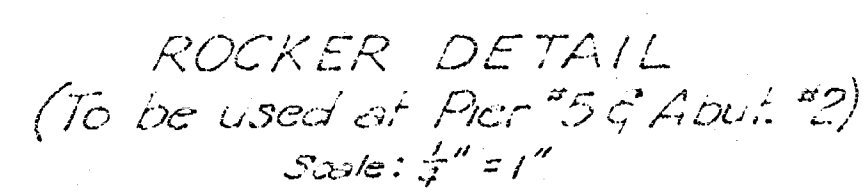
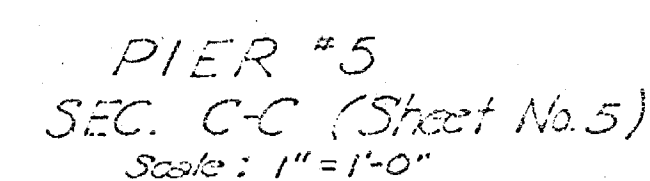
COUNTY CROSSINGS
53 RD. MAIN BRANCH





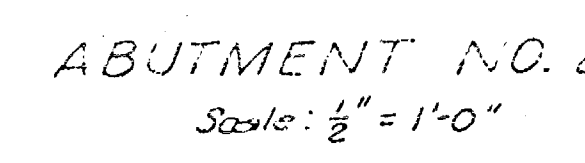
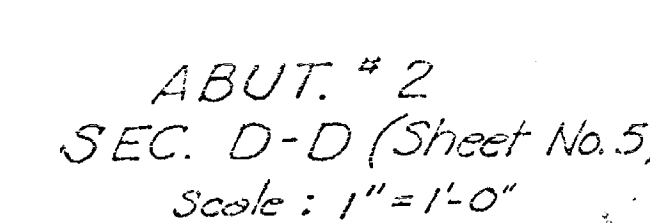






Note:
Horizontal stress for NW
Wingwall spaced as shown
on Elev. view
of NE & SW
Wingwall, Sh. # 5

Note:
For typical Section A-A
of wingwall see Sheet No. 4



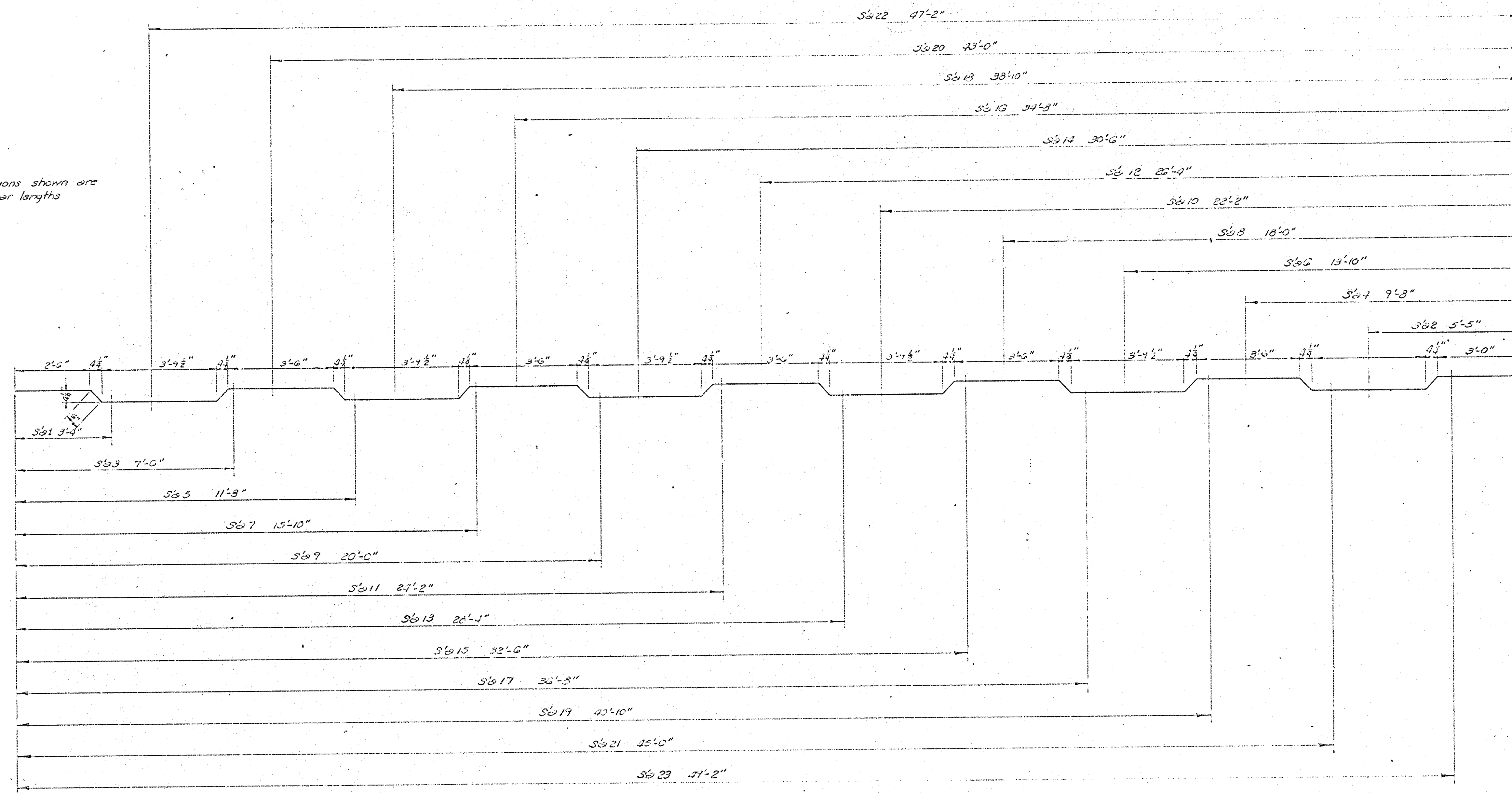
SUMMARY OF QUANTITIES		
ITEM	QUANTITY	UNIT
Test Piling	72	Lin Ft.
Concrete, Class A(AE)	3897	Cu. Yd.
Piling	1,398	Lin. Ft.
Structural Steel	188,060	Lbs.
Reinforcing Steel	44,940	Lbs.
Bending Bars	4,595	Lbs.
Aluminum Hardware	456	Lin. Ft.
Expansion, Common	3,370	Cu. Yd.
12" Heavy Rip-Rap	1040	Cu. Yd.
6" Sand & Gravel Bedding	340	Cu. Yd.



PREPARED BY
SEDGWICK COUNTY ENGINEERING DEPT.
RUEHLS S. KIRK - COUNTY ENGINEER

REVISED	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	<i>As Shown</i>	JVH	JVH	CJF	6
	DATE	3-57	11-57	11-57	of 8
PLANFILE			TOTAL SHEETS		

Notes:
Dimensions shown are
actual bar lengths

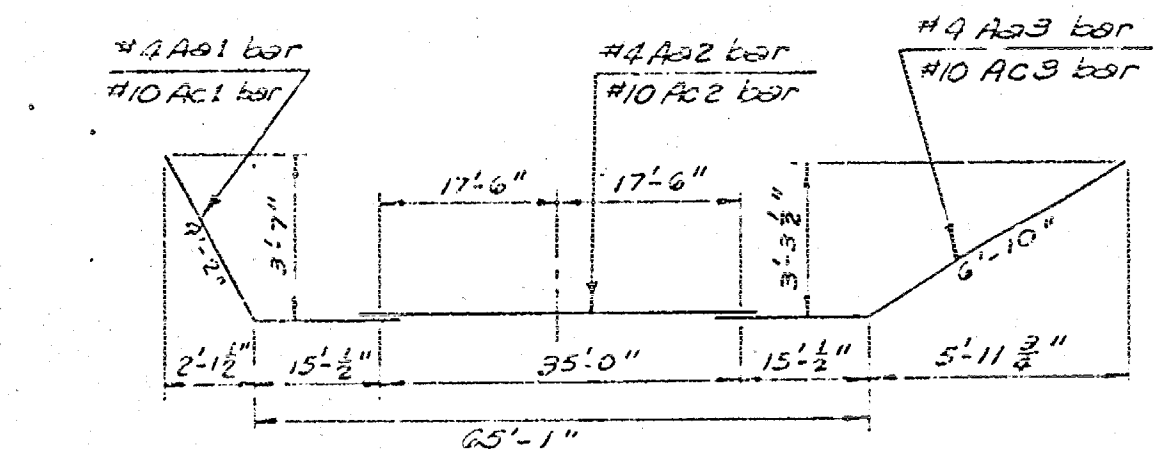


S601 through S623 Bar Diagrams (2 Sets Right)
Scale: 1/8" = 1'-0"

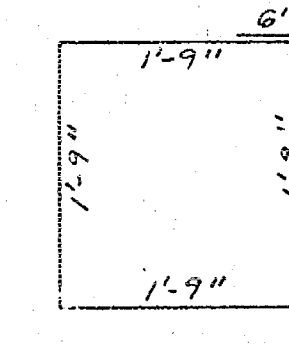
REINFORCING STEEL QUANTITIES

Bar Design	Location	Size	No.	Length	Weight	Remarks	Bar Design	Location	Size	No.	Length	Weight	Remarks
Aa1	Blk wall & Wingwalls	#4	12	19'-10"		Bar diag on Sh. #7	L1	Slab	#4	448	21'-6"		Straight Bar
Aa2	" " "	#4	12	36'-0"		"	L2	"	#4	256	23'-7"		"
Aa3	" " "	#4	12	22'-6"		"	L3	"	#4	92	2'-6"		Bar diag Sh. #7
Ab	Abutment	#6	6	60'-0"	541	Straight bar	Pa	Cap (Piers)	#3	20	60'-0"		Straight Bar
Ac1	Abut. & Wingwalls	#10	4	20'-4"		Bar diag on Sh. #7	Pb	"	#4	190	8'-0"		Bar diag Sh. #7
Ac2	" " "	#10	4	37'-11"		"	Sa	Slab	#5	135	51'-9"		Bar diag Sh. #7
Ac3	" " "	#10	4	23'-10"		"	Sa1	"	#5	26	7'-0"		Bar diag Sh. #7
Ad	Abutment	#10	4	60'-0"	1033	Straight Bar	Sb	"	#5	134	51'-2"		Bar diag Sh. #7
Ae	Blk wall & Abut.	#4	38	11'-0"		Bar diag on Sh. #7	Sb1	"	#5	48	5'-2"		Bar diag Sh. #7
Af	" " "	#4	38	11'-7"		"	Sb2	"	#5	134	5'-2"		Bar diag Sh. #7
Ag	" " "	#4	36	3'-0"		Straight Bar	Sb3	"	#5	50	3'-0"		Bar diag Sh. #7
Ah	" " "	#4	36	3'-6"		"	Sb4	"	#5	1	7'-0"		Bar diag Sh. #7
Ai	Blk wall & NE & SW	#4	4	6'-11"		Bar diag Sh. #7	Wa	SW Wingwall	#4	8	9'-0"		Bar diag Sh. #7
Aj	Blk wall & NW & SE	#4	4	9'-7"	26	"	Wa1	SE Wingwall	#4	14	16'-8"		Bar diag Sh. #7
Al	Backwall	#4	8	4'-6"		Straight Bar	Wa2	NE Wingwall	#4	16	24'-9"		Straight Bars
Am	"	#4	8	5'-6"		"	Wa3	NE & SW Wingwalls	#4	4	6'-0"		Bar diag Sh. #7
An	Abutment	#4	4	60'-0"	160	"	Wa4	NW & SE Wingwalls	#4	4	8'-6"		"
											Total	44,940	

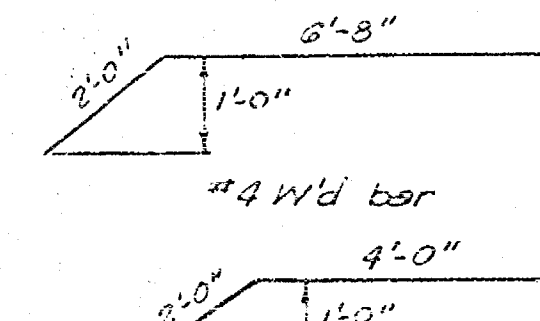
NOTE:
Dimensions of bar lengths are indicated to
the nearest inch.



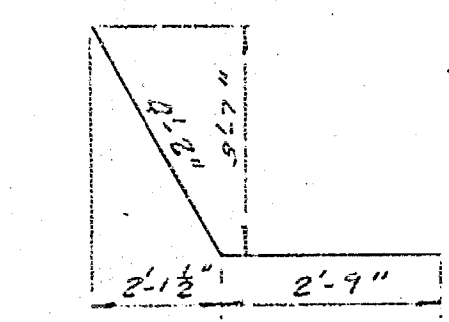
Aa1, Aa2, Aa3, Ac1, Ac2, & Ac3 bars



#4 Pb bars

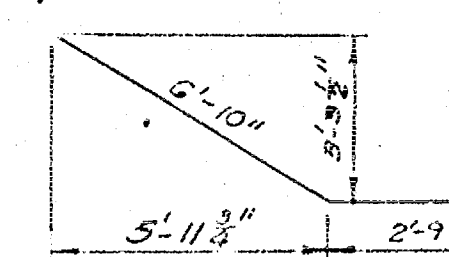


#4 Wd bar

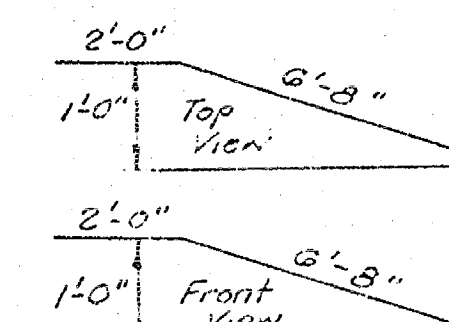


#4 Af bars

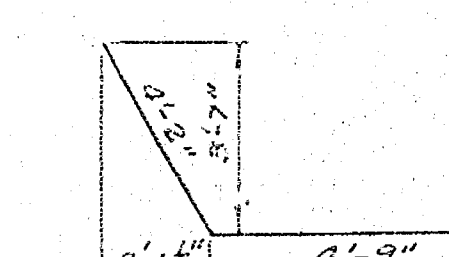
#4 Af & #4 Aa1 bars



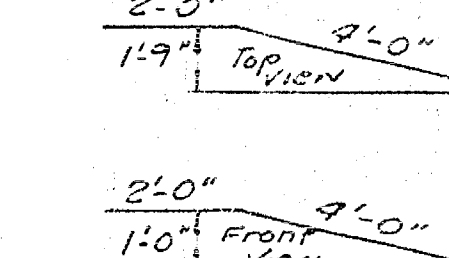
#4 Af bars



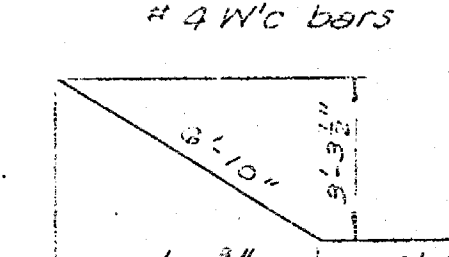
#4 Wc bars



#6 Wb bars



#4 Wc bars



#6 Wb bars

BAR DIAGRAMS
No Scale

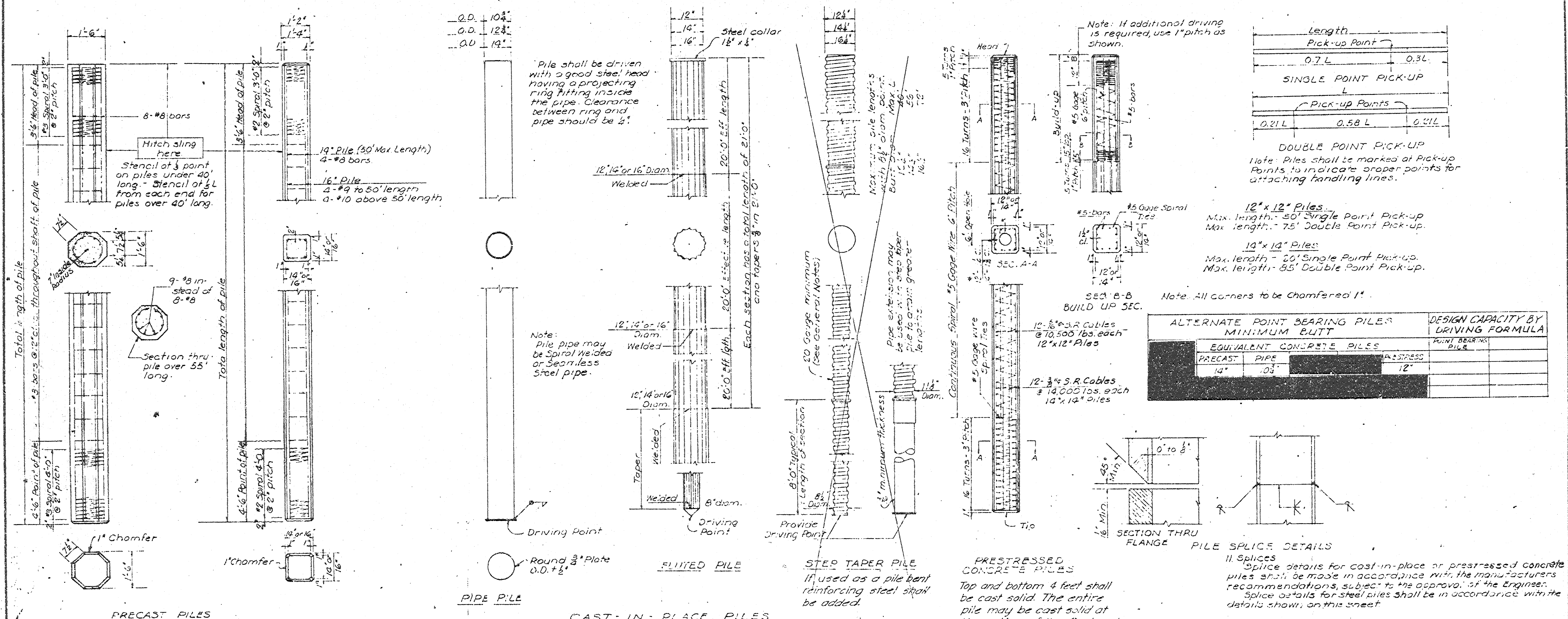
RECD OF 10 1957

BAR DIAGRAMS AND REINFORCING STEEL QUANTITIES

PREPARED BY
SEDGWICK COUNTY ENGINEERING DEPT.
RUFUS S. KIRK - COUNTY ENGINEER

REVISED	SCALE	DESIGNED	TRACED	CHECKED	SHEET NO.
	As Shown	J.V.H.	J.V.H.	C.V.F.	7 of 8
	DATE	3-57	11-57	11-57	
	PLANFILE		TOTAL SHEETS		

PUR. ROAD DIV. NO.	STATE	PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
5	KANSAS		195		



General Notes

1. Specifications
Standard Specifications for State Road and Bridge Construction as currently used by State Highway Commission of Kansas.
2. Choice of Piles
As indicated in the plans, piles will be considered as point bearing piles or as friction piles. Where point bearing piles are specified, the contractor may elect to use either the steel pile specified on the facing plans or the equivalent precast concrete, cast-in-place concrete or prestressed concrete pile shown in the table on this sheet. Where friction piles are specified the contractor may elect to use either the size and type concrete pile specified on the facing plans or the equivalent precast concrete, cast-in-place concrete or prestressed concrete pile shown in the table on this sheet. Steel piles are not included as an alternate where friction piles are specified. Other types of concrete piles not shown here are subject to the approval of the Engineer.
3. Concrete
All concrete for precast and cast-in-place shall be Class 'A' $f_c = 3000$ p.s.i. Concrete for prestressed piles shall be Class AAA $f_c = 4000$ p.s.i.
4. Reinforcement
Reinforcing bars shall be new billet steel of intermediate grade without exception. Hoops and spirals may be either plain or deformed bars.

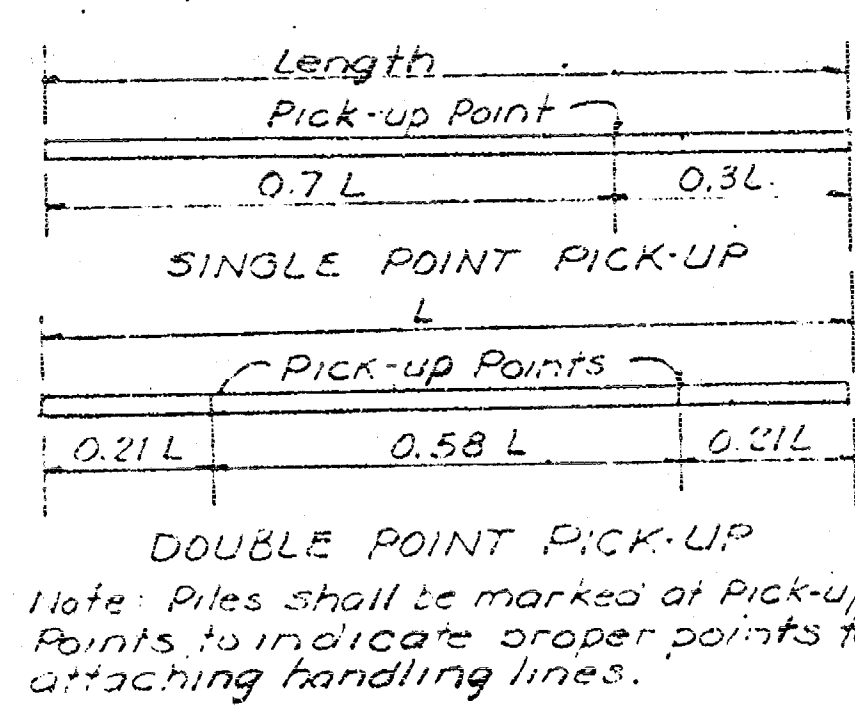
5. Precast Piles
Precast piles shall conform to the requirements of Section 59.310 of the specifications.
6. Cast-in-place Shells
A. Pile shells shall have a minimum thickness as follows:
1. Piles driven without mandrel - 8 gage except fluted pile use 9 gage minimum.
2. Piles driven with mandrel. Shell shall be of sufficient strength and thickness to withstand driving without injury and to resist harmful distortion under buckling due to soil pressure after being driven and the mandrel removed.
B. Pile shells shall meet the following material requirements:
1. Fluted steel shells and steel collar - SAE 1010 classification for cast shell in steel.
2. Spiral welded steel - ASTM Designation, A252, Grade 2 Electric Fusion-Welded Spiral Seam Steel Pipe.
3. Seamless steel pipe - ASTM A252, welded and seamless steel pipe.
4. Corrugated steel shell - SAE 1000.
C. The contractor shall maintain on the job at all times prior to and during the filling of the shells, a light suitable for their inspection.

7. Steel Piles
Steel pile material shall meet the requirements of ASTM A753.
8. Pile Points
All cast-in-place piles shall be equipped with a steel driving point of 1" minimum thickness. Driving points shall be mill welded to the pile shell. Driving points shall be either hot pressed steel meeting the requirements of SAE 1020 for forged steel, or cast steel meeting the requirements of ASTM A248, Grade 2, or structural steel meeting the requirements of ASTM A36, Grade 2.
Steel piles shall have a square cut end only. No driving point is required.
9. Welding
All field welding shall meet the requirements of Section 51.34 of the specifications.
10. Paint
Shall comply with the Kansas Standard Specifications (1955 Edition).

REC'D DEC 19 1957

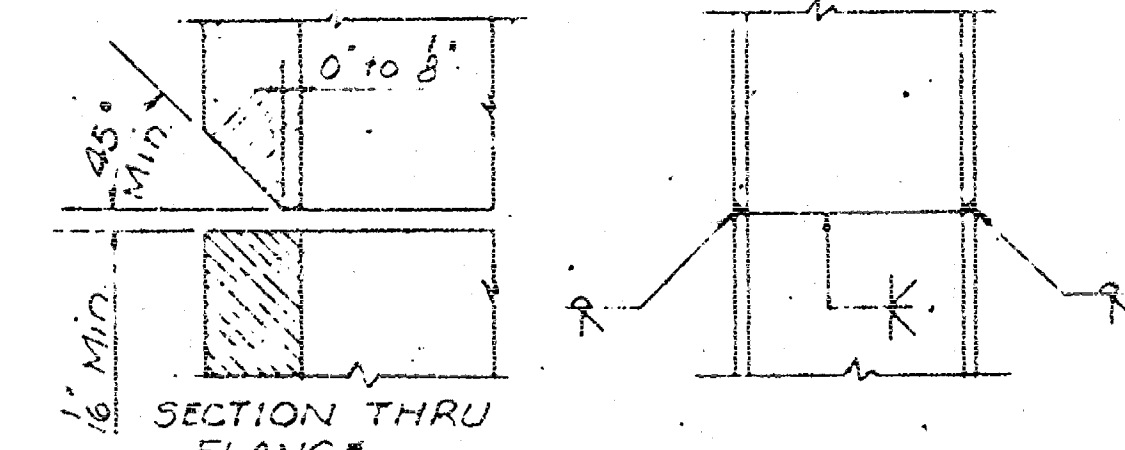
11. Splices
Splice details for cast-in-place or prestressed concrete piles shall be made in accordance with the manufacturers recommendations, subject to the approval of the Engineer. Splice details for steel piles shall be in accordance with the details shown on this sheet.
12. Driving Formula
Piles shall be driven to the minimum bearing value as specified on the Construction Layout as determined by the driving formulas stipulated in the specifications.
13. Mill Test Reports
Notarized mill test reports, in triplicate, shall be furnished by the contractor for all steel pile and cast-in-place pile shells.
14. Payment
Payment for all piles will be made as set forth in the specifications.
15. Test Piles
Test piles shall be driven where called for on the Bridge plans. All test piles shall be located so that they will become part of the Bridge pile system.

ALTERNATE POINT BEARING PILES		DESIGN CAPACITY BY DRIVING FORMULA	
MINIMUM BUTT		POINT BEARING	
EQUIVALENT CONCRETE PILES		PILE	
PRECAST PILE	MINIMUM BUTT	MINIMUM BUTT	MINIMUM BUTT
14"	12"	14"	12"



12" x 12" Piles
Max. length - 50' Single Point Pick-up
Max. length - 75' Double Point Pick-up.
14" x 14" Piles
Max. length - 20' Single Point Pick-up
Max. length - 35' Double Point Pick-up.

Note: All corners to be Chamfered 1".



SECTION THRU PILE SPLICE DETAILS