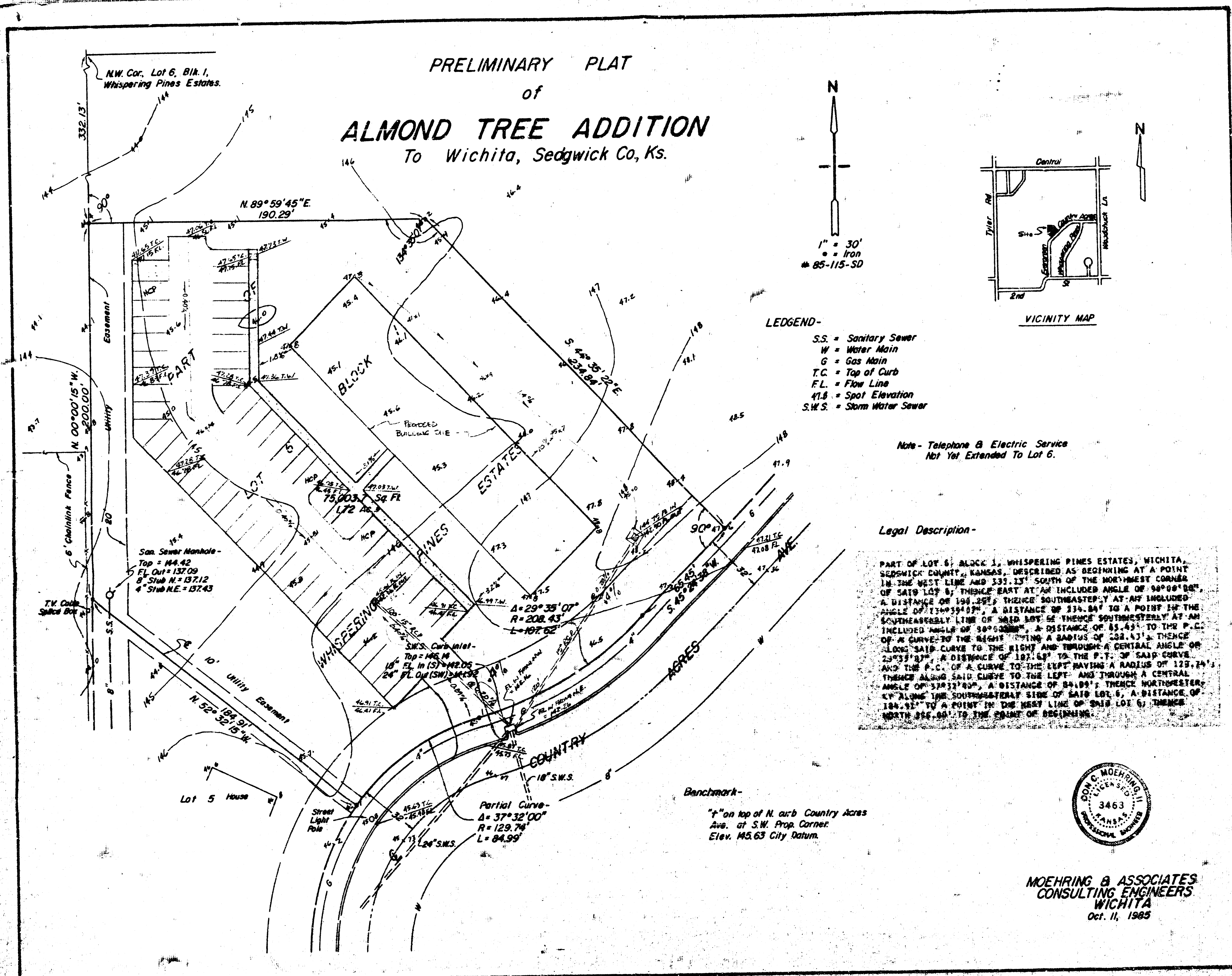




10-27-85

ALMOND TREE ADDITION  
DRAINAGE CALCULATIONS:

① EXISTING SOIL TYPE - MILAN LOAM (See attached sheet for soil characteristics)  
Hydrologic Soil Group "B"

② Existing Runoff Calculations - Consider the Rational Formula method for determining discharge rates; Coefficient of runoff "C", from the Ad Hoc Drainage Committee for Urban Lawn Group "C" = 0.20  
Site Area = 1.72 Ac.  
From F.A.A. method for time of concentration,  
 $T_{10} = 1.8(1.1 - C)L_0^{1/2} / S^{1/2}$ ; Where  $L_0$  = Length of overland flow  
 $S$  = Average slope of overland flow in percent.  
 $= 43.4 - 44.0 / 40 = 1.29\%$   
 $\therefore T_{10} = 1.8(1.1 - 0.20)240^{1/2} / 1.29^{1/2}$   
 $= 27 \text{ min.}$   
 $i_2$  = Rainfall intensity per 2 year frequency  
 $= 3.13 \text{ in/hr}$   
 $Q = C \times i_2 \times A = 0.20 \times 3.13 \times 1.72$   
 $= 1.1 \text{ cfs}$

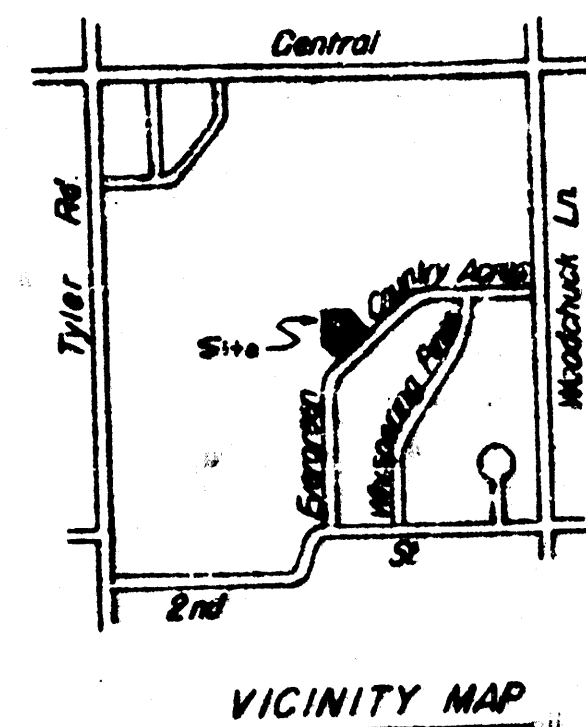
③ Developed Runoff Calculations - Applying the same method as above for developed conditions, a weighted "C" value may be found;  
Description      Area      "C"      Area x "C"  
Urban Lawn      1.07      0.20      0.214  
Impervious Surfaces      0.65      0.85      0.552  
 $\Sigma = 1.72$        $\Sigma = 0.766$   
 $\therefore \text{Weighted "C"} = 0.766 / 1.72 = 0.45$   
 $L_0 = 240 \text{ ft.}, L_1 = 50 \text{ ft.}, S_1 = 0.80\%; V_1 = 2.0 \text{ fps}$   
 $T_{10} = 1.8(1.1 - 0.45)250^{1/2} / 0.80^{1/2} = 20 \text{ min.}$   
 $T_2 = 50 / 2 \times 60 = 0.4$   
 $\therefore \text{UCE } T_c = 21 \text{ min.}$



ALMOND TREE ADDITION  
(Drainage)

PRELIMINARY PLAT  
of  
**ALMOND TREE ADDITION**  
To Wichita, Sedgwick Co, Ks.

1" = 30'  
• = Iron  
# 85-115-SD



LEGEND-

- S.S. = Sanitary Sewer
- W = Water Main
- G = Gas Main
- T.C. = Top of Curb
- F.L. = Flow Line
- 41.8 = Spot Elevation
- S.W.S. = Storm Water Sewer

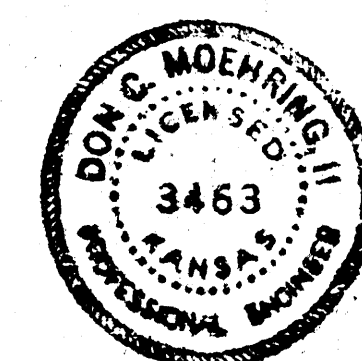
Note - Telephone & Electric Service  
Not Yet Extended To Lot 6.

Legal Description-

PART OF LOT 6, BLOCK 1, WHISPERING PINES ESTATES, WICHITA, SEDGWICK COUNTY, KANSAS, DESCRIBED AS BEGINNING AT A POINT IN THE WEST LINE AND 332.13' SOUTH OF THE NORTHWEST CORNER OF SAID LOT 5; THENCE EAST AT AN INCLUDED ANGLE OF 98°00'00", A DISTANCE OF 198.78'; THENCE SOUTHEASTERLY AT AN INCLUDED ANGLE OF 134°39'07" A DISTANCE OF 234.84' TO A POINT IN THE SOUTHEASTLY LINE OF SAID LOT 6; THENCE SOUTHWESTERLY AT AN INCLUDED ANGLE OF 106°01'00", A DISTANCE OF 65.45' TO THE P.C. OF A CURVE TO THE RIGHT HAVING A RADIUS OF 342.43'; THENCE ALONG SAID CURVE TO THE RIGHT AND THROUGH A CENTRAL ANGLE OF 28°35'37", A DISTANCE OF 182.63' TO THE P.T. OF SAID CURVE; AND THE P.C. OF A CURVE TO THE LEFT HAVING A RADIUS OF 125.24'; THENCE ALONG SAID CURVE TO THE LEFT AND THROUGH A CENTRAL ANGLE OF 174°21'00", A DISTANCE OF 24.99'; THENCE NORTHWESTERLY ALONG THE SOUTHWESTERLY SIDE OF SAID LOT 6, A DISTANCE OF 184.35' TO A POINT IN THE WEST LINE OF SAID LOT 6; THENCE NORTH 84°00' TO THE POINT OF BEGINNING.

Benchmark-

"B" on top of N. on Country Acres  
Ave. at S.W. Prop. Corner.  
Elev. 145.63 City Datum.



MOEHRING & ASSOCIATES  
CONSULTING ENGINEERS  
WICHITA  
Oct. 11, 1983

$$i_c = 3.56 \text{ in/hr}$$

$$Q_2 = 3.56 \times 0.45 \times 1.72 = 2.8 \text{ cfs}$$

Try a 15" R.C.P. @ 0.40% gradient.  
Applying Manning's Formula to evaluate the hydraulic capacity;

$$Q = 1.486 \times A \times R^{2/3} \times S^{1/2}$$

where; n = Manning's roughness coefficient, for concrete pipe use (n = 0.012).

A = Cross-Sectional area of flow, for 15" pipe (A = 1.227 ft<sup>2</sup>).

R = Hydraulic radius, or flow area per wetted perimeter (R = 0.312 ft).

S = Slope of pipe (S = 0.004 ft/ft).

$$Q = 1.486 \times 1.227 \times (0.312)^{2/3} \times (0.004)^{1/2}$$

$$Q_{max} = 4.4 \text{ cfs} > 2.8 \text{ cfs} \quad \text{OK}$$

Try a 15" R.C.P. @ 0.20% gradient

$$Q = 1.486 \times 1.227 \times (0.312)^{2/3} \times (0.002)^{1/2}$$

$$Q_{max} = 3.1 \text{ cfs} > 2.8 \text{ cfs} \quad \text{OK}$$

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