



MEMO

TO: Don S. Johnson, P.E.	PROJECT NO. 36-072-3-149
Design Engineer	PROJECT: Georgetown
City of Wichita	Retention Pond
ATTN: Wichita, KS 67202	DATE: March 19, 1985
COPIES TO: Mr. E. Morris	FROM: Dick Linn
	REFERENCE: Project Final Inspection

PLEASE ADVISE IMMEDIATELY OF ANY MISCONCEPTIONS OR OMISSIONS YOU BELIEVE TO BE CONTAINED HEREIN

Attached is the Certificate of Completion for the above-referenced project. The project has been reshaped, regraded, seeded and mulched. Minor maintenance of rip-rap areas will be performed later this year after construction activity on the adjacent properties has been completed. The bank slopes will be checked at that time to correct any erosion problems or areas which may require additional seeding.

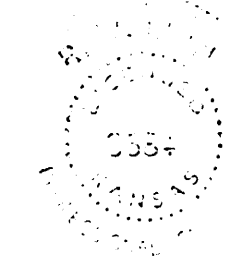
The required volume of detention is 3.7 acre feet which is provided at elevation 137.35 (0.64' below the top of the curb at the low point adjacent to the pond). Also attached is a mylar of the plan for regrading and seeding.

Re: Lot 1, Block One, Georgetown II Addition,
Wichita, Sedgewick County, Kansas

I certify to Columbia Savings Association, P.A., and to Mid Kansas Federal Savings and Loan Association of Wichita that a drainage facility consisting of a storm water detention system and related outfall has been substantially completed in accordance with the requirements of the City of Wichita, Sedgewick County, Kansas; that the facility is sufficient to substantially fulfill the requirements contained in that certain Ordinance dated August 22, 1981, by the City of Wichita, Sedgewick County, Kansas, as amended, recorded on Film 439 at page 140, and that certain Restrictive Covenant dated October 27, 1981, by Georgetown Partnership, recorded on Film 627 at page 161, and with the community Unit Plan referred to therein and that the construction completed to date with respect to the drainage facility has been inspected and approved by all governmental authorities having jurisdiction as being in conformity with the foregoing requirements.

Dated this 19th day of March, 1985.

Dick Linn



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Design Engineer
City of Wichita
ATTN: Wichita, KS 67202
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The work performed under this contract has been inspected by authorized representatives of the owner, contractor, and engineer and the project is hereby declared to be completed on the above date.

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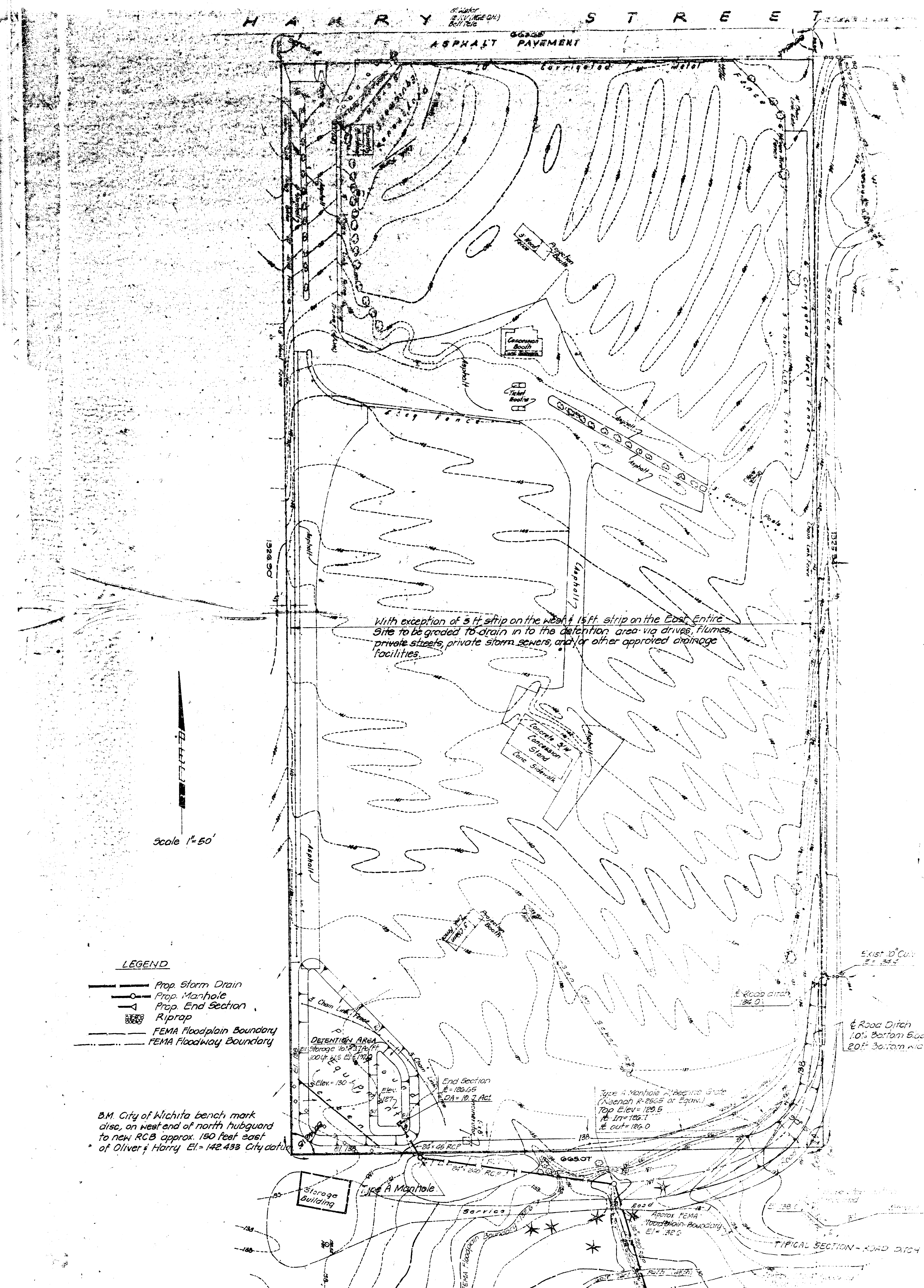
5. In the event said association, its successors or assigns, or the undersigned, shall fail at any time to maintain the drainage facilities consisting of the detention pond and related outfall, the City of Wichita may cause a written notice of delinquency upon the association, acting through the City of Wichita, to be filed with the City of Wichita. Said notice shall grant 30 days within which the association may fulfill the obligation. If said obligation is not fulfilled within the time specified, the City of Wichita, in order to preserve the taxable value of the properties within the addition and to prevent said facilities from becoming a nuisance, may enter upon said common area and perform the obligations listed in the Notice of Delinquency. All costs incurred by the City of Wichita in carrying out the obligations of the undersigned or the association may be assessed against the undersigned or the association, their successors or assigns, upon receipt of said notice of delinquency, failure to pay the obligation, or said notice of delinquency, apply for a hearing before the period to be provided in said notice to appeal said obligations, and the Board of City Commissioners to appeal said obligations, and further proceedings under said notice shall be suspended pending the outcome of any proceedings with respect to such appeal.

This Covenant executed as of the date first above written.

GEORGETOWN PARTNERSHIP,
By: Kansas General Properties, Inc., a partner
By: William E. Morris
By: Dick Linn
By: Dick Linn

State of Kansas
County of Sedgewick

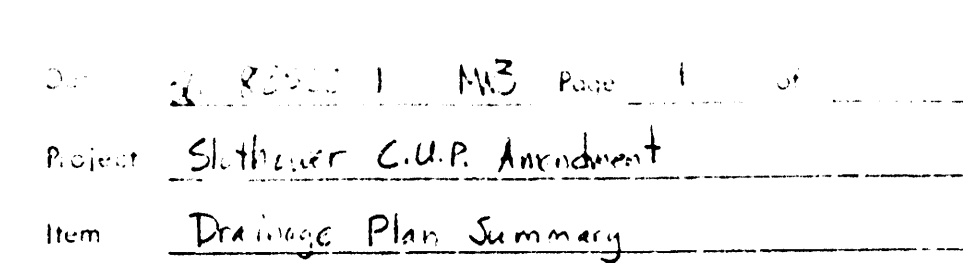
Be it remembered that on this 19th day of March, 1985, before me, a Notary Public in and for the State of Kansas, came William E. Morris, William E. Morris, Partner, and Dick Linn, Partner, all of the Partners of Georgetown Partnership, a Kansas Partnership, to me personally known to be the same persons who executed the foregoing instrument of writing and duly acknowledged the execution of same for and on behalf of and as the voluntary act and deed of said partnership. In testimony whereof, I have hereunto set my hand and seal of office.



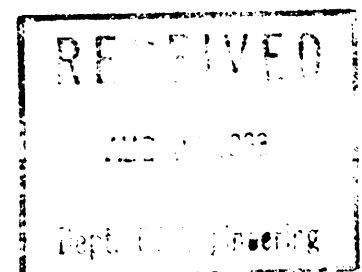
- Prop. Storm Drain
- Prop. Manhole
- Prop. End Section
- Riprap
- FEMA Floodplain Boundary
- FEMA Floodway Boundary

B.M. City of Wichita bench mark
also, on west end of north hubguard
to new RCB approx. 180 feet east
of Oliver & Harry El. = 142.433 City datum

No.	Revision	By	Date



30° CMP: $K = 223$ (m.s. 0.034) $Q = K\sqrt{S} = 34 \text{ cfs}$



Time of Conc. (Tc) = 60.69 minutes
Intensity = 3.90 inches/h
C factor = 0.21
Peak Dis. (cfs)^M 16.37

Project No: 36-0002-1 Date: 7/5/63 Sht. No. 5 of 11

Project: Stetham Creek Improvement by: AKD

TR-55 LAG-TC METHOD PEAK DISCHARGE COMPUTATION SHEET

Location: _____ Watershed Condition AMC

Remarks: Design Conditions: Temp Imp = 66 °C Temp. C = 86

P INPUT

1. 7.8 in (24-hour) _____ Table 2-1 or Figure 3-1
Rainfall _____ yr. Freq. _____

2. 86 _____ **T_c FACTOR**
Runoff Curve No. 5 Constant 1.67

3. 1500 ft² Figure 3-3
Hydraulic Length _____ 9. 0.29 hr L
Basic Lag _____

4. 1.2 % Y Figure 3-4 0.63 F_{adj}
Watershed Slope _____ Hydr. Length Adj. _____

5. 75 %
Hydr. Length Modified _____ 11. 0.68 F_{imp}
Imp. Area Adj. _____

6. 66 % Figure 3-5 0.21 hr T_c
Impervious Area _____ 12. 0.21 hr T_c
Basic Lag _____

7. 14.7 ac. $\frac{1}{40} = 0.025$ sq mi. 0.63 sq mi. T_c
Drainage Area (ba) _____ 13. 6.14 in F_{imp}
Runoff Volume _____

8. 0 %
Ponds, Swamps _____ 14. 72.9 cfs/min T_c
Basic Lag _____

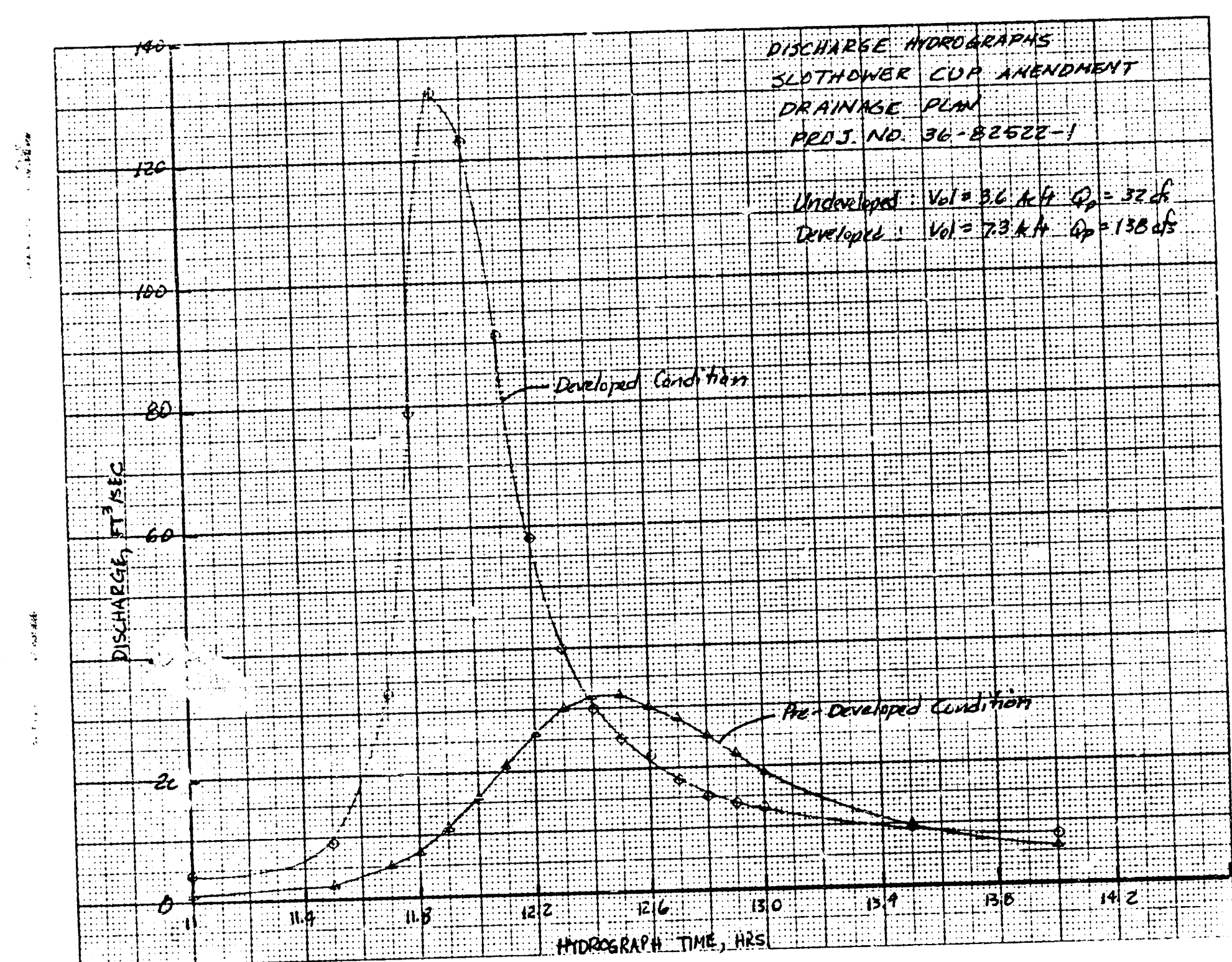
9. 6.63 sq mi. T_c
Drainage Area _____ 15. 6.63 sq mi. T_c
Drainage Area _____

10. 0 %
Ponds, Swamps _____ 16. 0 %
Ponds, Swamps _____

ADJUSTED PEAK DISCHARGE 17. 136 cfs

$K = 6.47 \times 1.7 = 11.0$
 640

		Discharge, cfs																						
Subarea		T ₁ hr	T ₂ hr	K mi ² -in	11.0 hrs.	11.5 hrs.	11.7 hrs.	11.8 hrs.	11.9 hrs.	12.0 hrs.	12.1 hrs.	12.2 hrs.	12.3 hrs.	12.4 hrs.	12.5 hrs.	12.6 hrs.	12.7 hrs.	12.8 hrs.	12.9 hrs.	13.0 hrs.	13.5 hrs.	14.0 hrs.	16.0 hrs.	20.0 hrs.
UNDEVELOPED	IC	0.000	0.019	1	2	5	7	11	16	21	26	30	32	32	30	28	25	22	19	10	6	3	2	
DEVELOPED	IC	0.000	0.000	4	9	33	79	131	123	91	58	40	30	23	22	18	15	14	13	9	8	5	3	





Date 7/5/83 MJB Page 10 of
 Project Slethner CUP Amendment
 Item Outfall Control Structure - Node A

HW Elev = 137' for 16" yr
 TW Elev 3' from 4" pipe = 130.5 =
 1" head @ Node B gate

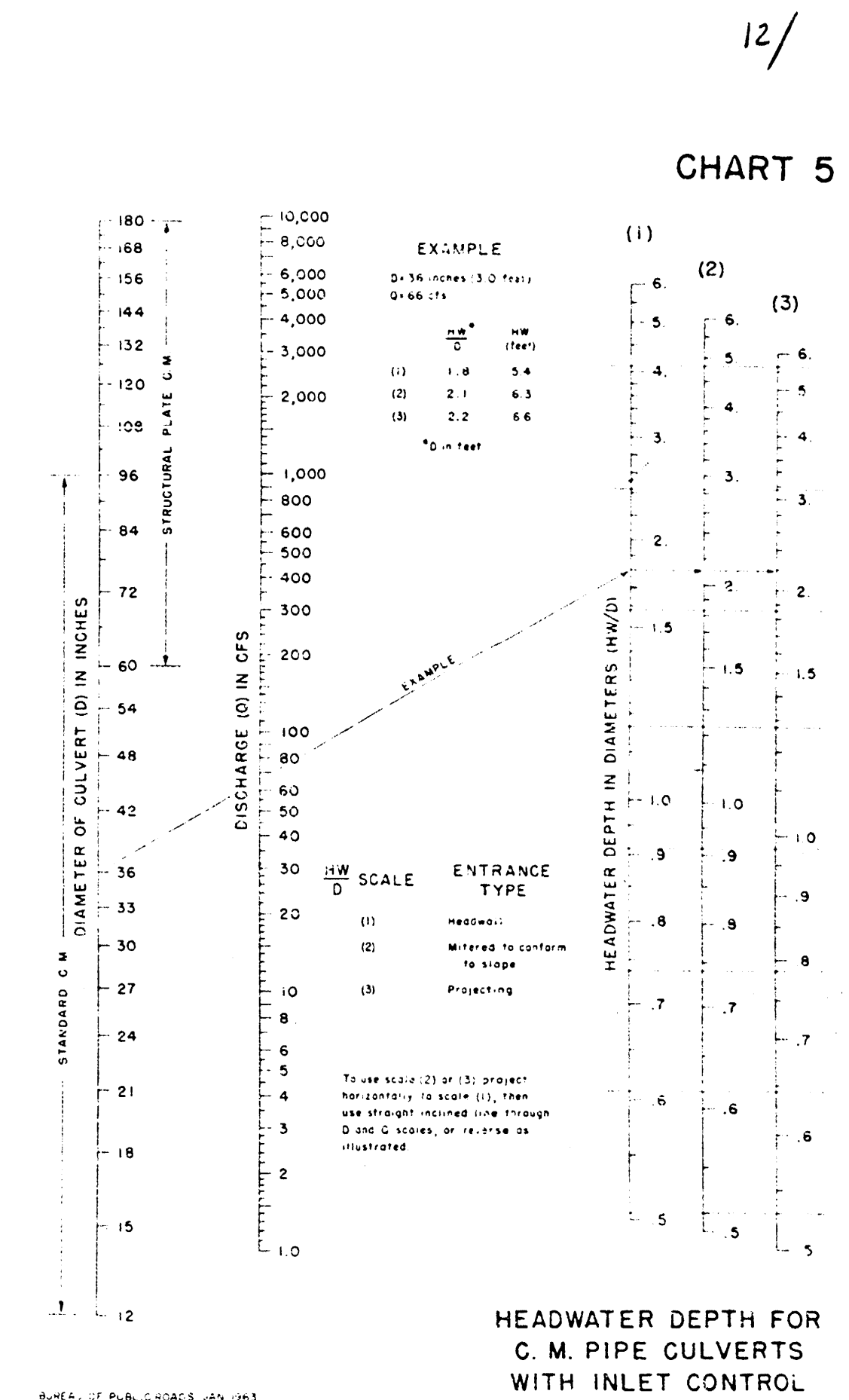
$L = 275'$
 $J = 0.0226$ F1/FT
 $Q = 32 \frac{1}{2}$ MMX

Try 24" RCP
 $Q = K \sqrt{H}$
 $K = \frac{Q}{\sqrt{H}}$
 $\frac{1.49 AR^{2.63}}{n} = \frac{Q}{\sqrt{H}}$
 $\frac{nQ}{1.49 \sqrt{H}} = AR^{2.63}$
 $\frac{(0.0226)(32)}{1.49 \sqrt{27.5}} = AR^{2.63} = 3.36$
 use 30" CMP

Try RCP
 $Q = K \sqrt{H}$
 $K = \frac{Q}{\sqrt{H}}$
 $\frac{32}{\sqrt{27.5}} = 6.08$
 use 24" RCP.

Check inlet control
 $z \approx$ CMP
 $\frac{H_1}{H_2} = 1.3$
 $HW = 1.3(2.5 + 12.5) = 13' <$

$24" RCP$
 $\frac{H_1}{D} = 2.15$
 $HW = 2.15(2 + 12.8) = 13.3$
 Use 24" RCP or 30" CMP



Peak Dis. (cfs) = 36.26

DIRECTORS

- M. J. B. HAYES
- H. G. M. VAN DER WERF
- P. J. C. VAN DER WERF
- J. A. M. VAN DER WERF
- F. D. M. VAN DER WERF
- J. A. M. VAN DER WERF
- M. J. B. HAYES
- J. A. M. VAN DER WERF
- J. A. M. VAN DER WERF
- J. A. M. VAN DER WERF

July 7, 1983

36-44

Dick Linn, P.E.
Manager, Land Development



MEMO

TO: Steve Lackey
Design Chief Engineer
City Hall - 2nd Floor
435 N. Main
Wichita, Kansas 67202

PROJECT: Slothower Addition

DATE: July 16, 1980

FROM: Kristen Harr

REFERENCE: Drainage Plan

COPIES TO: AITN: Paul Johnston, Louise Olivarez, Mike Lindehak, Dick Linn

PLEASE ADVISE IMMEDIATELY OF ANY MISCONCEPTIONS OR OMISSIONS YOU BELIEVE TO BE CONTAINED HEREIN.

Enclosed is the Drainage Plan for Slothower Addition. The drainage plan is based on the calculations as submitted with the drainage concept. He anticipates filing the Final Plan on July 23, 1980, and would appreciate any comments prior to the above date. Should any additional information be required in your review of the plan, please contact Dick Linn or myself.

July 1, 1980

Steve Lackey, Design Chief Engineer
Engineering Division, Department of Public Works
7th Floor - City Hall
435 N. Main
Wichita, Kansas 67202

RE: Meadowlark Addition - Drainage Concept
PFD File No. 30-79151-1081

Dear Mr. Lackey:

The Meadowlark C.U.P. has been approved with the following general Provision #4:

Drainage: At the time of platting, the applicant shall submit a drainage plan for the entire development and guarantee drainage improvements as may be required. The difference between developed 100 year storm water runoff and undeveloped 100 year storm water runoff shall be retained on the undeveloped 100 year storm water runoff shall be retained on the undeveloped 100 year storm water runoff.

The "undeveloped" runoff has been calculated based on the area soil type and as a pasture in good condition. This would be identical to the adjacent golf course condition. The runoff flow rate is 31 cfs.

The proposed multi-family residential development is assumed to be 50% impervious and 50% open area. The flow rate is calculated to be 58 cfs for the design storm (100 year, 6 hour).

The detention volume required is 2.8 acre - feet. Attached are copies of the calculations. Also attached are calculations for the outlet pipe.

The attached Drainage Concept provides for a detention pond with a minimum capacity of 2.8 acre feet, and 1' freeboard. The entire site is to be graded to drain into the pond (except a 10' strip adjacent to the west and east property lines).

The Drainage Plan will be submitted for review and approval prior to submission of the Final Plat.

Very Truly Yours,

Richard M. Linn, P.E.
Manager, Land Development

cc: Mike Lindehak
Tom Allen
Louise Olivarez



Date: 7/1/80 MMB Page 1 of 1
Project: Slothower C.U.P. Amendment
Item: Drainage Plan Summary

1) Hydrology by SCS TR-55 method. Hydrographs by tabular method (Chapter 5, TR-55)

2) Pre-developed conditions assumed to be pasture in good condition. Pre-developed $Q_p = 32$ cfs with runoff volume = 3.6 ac-ft.

3) Developed condition assumptions:
Slopes 1.5 to 3.1 Acres Commercial w/ 25% Impervious Area
Slopes 3.2 to 10.4 Acres Apt./Multi-Family w/ 20% Impervious Area
Composite $CN = 68$
 $Q_p = 135$ cfs with runoff volume = 7.3 ac-ft.

4) Detention Facility

Major Storm: $Flr Elev = 126.65$ Major Storm: $Flr Elev = 126.65$
 $Top Elev = 130$ $Top Elev = 130$
 $5.11 Slope$ $5.11 Slope$
 $Vel = 2.1$ $Vel = 2.1$

Total Avail. Storage = 3.7 ac-ft

5) Outlet Canal Structure

$H.W. = 126.65$ ft/ft (Assume 100-yr peak on 24 hr. basis at reach)

24" RCP: $K = 225$ ($Q = 0.013$) $Q = K \sqrt{H} = 35$ cfs

30" CMP: $K = 122$ ($Q = 0.004$) $Q = K \sqrt{H} = 24$ cfs

Project No: 36-82522-1 Date: 7/1/80 Sht No: 2 of 2
Project: Slothower C.U.P. Amendment E: MMB dtd 7/1/80

TR-55 LAG-Tc METHOD PEAK DISCHARGE COMPUTATION SHEET

Location: Meadows Addition - Drainage Concept

Remarks: Pre-developed - Pasture Good Condition

INPUT (24-hour) Table 2-1 or Figure 10
Rainfall: 7.5 in (24-hr. freq.)
CN: 68
Runoff Curve No.: 61
Hydraulic Length: 1500 ft
Watershed Slope: 1.5%
Hydr. Length Modified: 1500 ft
Impervious Area: 0%
Drainage Area (DA): 15.7 ac
Ponds, Swamps: 0

Tc FACTOR
Constant: 1.67
Basic Lag: 9 hr
Hydr. Length Adj.: 1.00
F_{imp}: 1.00
Tc: 9 hr
Figure 5-2
Basic Peak Disch.: 135 cfs

PEAK FACTOR
 $Q = \frac{(P-0.25)^2}{P+0.85}$
 $S = \frac{1000}{CN} - 10$
 $L = \frac{1.48 S^{0.7}}{1900 \sqrt{Y}}$
 $T_c = 1.48 L F_{imp}$
Runoff Volume: 7.3 ac-ft

ADJUSTED PEAK DISCHARGE: 135 cfs

Computation of Peak Discharge

Date: 7/1/80

1) Subarea: 1.0 inches
Recurrence Interval (hrs.): 100
Runoff Curve Number: 61
Watershed Slope: 1.5%
Area of Basin: 15.7 acres

2) Detention Facility
Detention Volume (ac-ft): 2.8
Detention Time of Conc. (hrs.): 1.0
Peak Disch. (cfs): 135

Peak Discharge by Technical Release No. 55 (1975)

Peak Disch. (cfs): 135
Capacity: 2.8 ac-ft
Detention Time: 1.0 hr

Peak Discharge by Modified Rational Formula (Rosinelli)

Time of Conc. (hrs.): 1.0
Intensity: 3.0 inches/hr.
Q Factor: 0.21
Peak Disch. (cfs): 135



Date: 7/1/80 MMB Page 4 of 4
Project: Slothower C.U.P. Amendment
Item: Drainage Plan Summary

Form 100

DA = 15.7 ac

CN = 68

Runoff Curve No. = 61

Hydraulic Length = 1500 ft

Watershed Slope = 1.5%

Hydr. Length Modified = 1500 ft

Impervious Area = 0%

Drainage Area (DA) = 15.7 ac

Ponds, Swamps = 0

ADJUSTED PEAK DISCHARGE = 135 cfs

Project No: 36-82522-1 Date: 7/1/80 Sht No: 5 of 5
Project: Slothower C.U.P. Amendment E: MMB dtd 7/1/80

TR-55 LAG-Tc METHOD PEAK DISCHARGE COMPUTATION SHEET

Location: Meadows Addition - Drainage Concept

Remarks: Pre-developed - Pasture Good Condition

INPUT (24-hour) Table 2-1 or Figure 10
Rainfall: 7.5 in (24-hr. freq.)
CN: 68
Runoff Curve No.: 61
Hydraulic Length: 1500 ft
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Hydr. Length Modified: 1500 ft
Impervious Area: 0%
Drainage Area (DA): 15.7 ac
Ponds, Swamps: 0

Tc FACTOR
Constant: 1.67
Basic Lag: 9 hr
Hydr. Length Adj.: 1.00
F_{imp}: 1.00
Tc: 9 hr
Figure 5-2
Basic Peak Disch.: 135 cfs

PEAK FACTOR
 $Q = \frac{(P-0.25)^2}{P+0.85}$
 $S = \frac{1000}{CN} - 10$
 $L = \frac{1.48 S^{0.7}}{1900 \sqrt{Y}}$
 $T_c = 1.48 L F_{imp}$
Runoff Volume: 7.3 ac-ft

ADJUSTED PEAK DISCHARGE: 135 cfs

Computation of Peak Discharge

Date: 7/1/80

1) Subarea: 1.0 inches
Recurrence Interval (hrs.): 100
Runoff Curve Number: 61
Watershed Slope: 1.5%
Area of Basin: 15.7 acres

2) Detention Facility
Detention Volume (ac-ft): 2.8
Detention Time of Conc. (hrs.): 1.0
Peak Disch. (cfs): 135

Peak Discharge by Technical Release No. 55 (1975)

Peak Disch. (cfs): 135
Capacity: 2.8 ac-ft
Detention Time: 1.0 hr

Peak Discharge by Modified Rational Formula (Rosinelli)

Time of Conc. (hrs.): 1.0
Intensity: 3.0 inches/hr.
Q Factor: 0.21
Peak Disch. (cfs): 135

Location: Meadows Addition - Drainage Concept

Remarks: Pre-developed - Pasture Good Condition

Developed: 60% Imp.

HYDROGRAPH COMPUTATION BY TR-55 METHOD

Discharge, cfs

Subarea	T _c	K	11.0	11.5	11.7	11.8	11.9	12.0	12.1	12.2	12.3	12.4	12.5	12.6	12.7	12.8	12.9	13.0	13.5	14.0	16.0	20.0
UNDEVELOPED	hrs.		hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.	hrs.
0.0	0.0	0.0	1	2	5	7	11	16	21	26	30	32	32	30	28	25	22	19	10	6	3	2
DEVELOPED	0.25	0.25	4	9	33	79	131	123	91	51	40	30	25	22	18	15	14	13	9	8	5	3



Date: 7/1/80 MMB Page 5 of 5
Project: Slothower C.U.P. Amendment
Item: Drainage Plan Summary

Pre-developed Conditions:

$Q_p = 32$ cfs
 $Vol = 3.6$ ac-ft

Developed Conditions:

$Q_p = 135$ cfs
 $Vol = 7.3$ ac-ft

Storage Required = 3.7 ac-ft

Detention Time = 1.0 hr

"Time of Storage"

$Flr Elev = 126.65$ ft (At 1' F.B.)

Top Elev = 130 ft

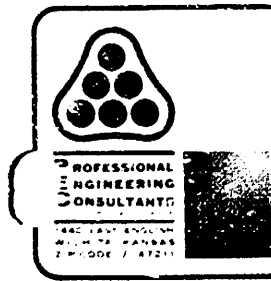
Bottom Elev = 126.65 ft

Mid Elev = 128.325 ft

$Vol = \frac{1}{2} (T + B) \times H$

$= \frac{1}{2} (0.25 + 0.41) \times 1.0$

$= 0.33$ ac-ft



Date: 7/1/80 MMB Page 10 of 10
Project: Slothower C.U.P. Amendment
Item: Outlet Canal Structure - Check 4

HW Elev = 137 ft

TW Elev = 136.5 ft

1' from 136.5 ft to 137 ft

2' from 137 ft to 139 ft

3' from 139 ft to 142 ft

4' from 142 ft to 146 ft

5' from 146 ft to 151 ft

6' from 151 ft to 157 ft

7' from 157 ft to 164 ft

8' from 164 ft to 172 ft

9' from 172 ft to 181 ft

10' from 181 ft to 191 ft

11' from 191 ft to 202 ft

12' from 202 ft to 214 ft

13' from 214 ft to 227 ft

14' from 227 ft to 241 ft

15' from 241 ft to 256 ft

16' from 256 ft to 272 ft

17' from 272 ft to 289 ft

18' from 289 ft to 307 ft

19' from 307 ft to 326 ft

20' from 326 ft to 346 ft

Check inlet control

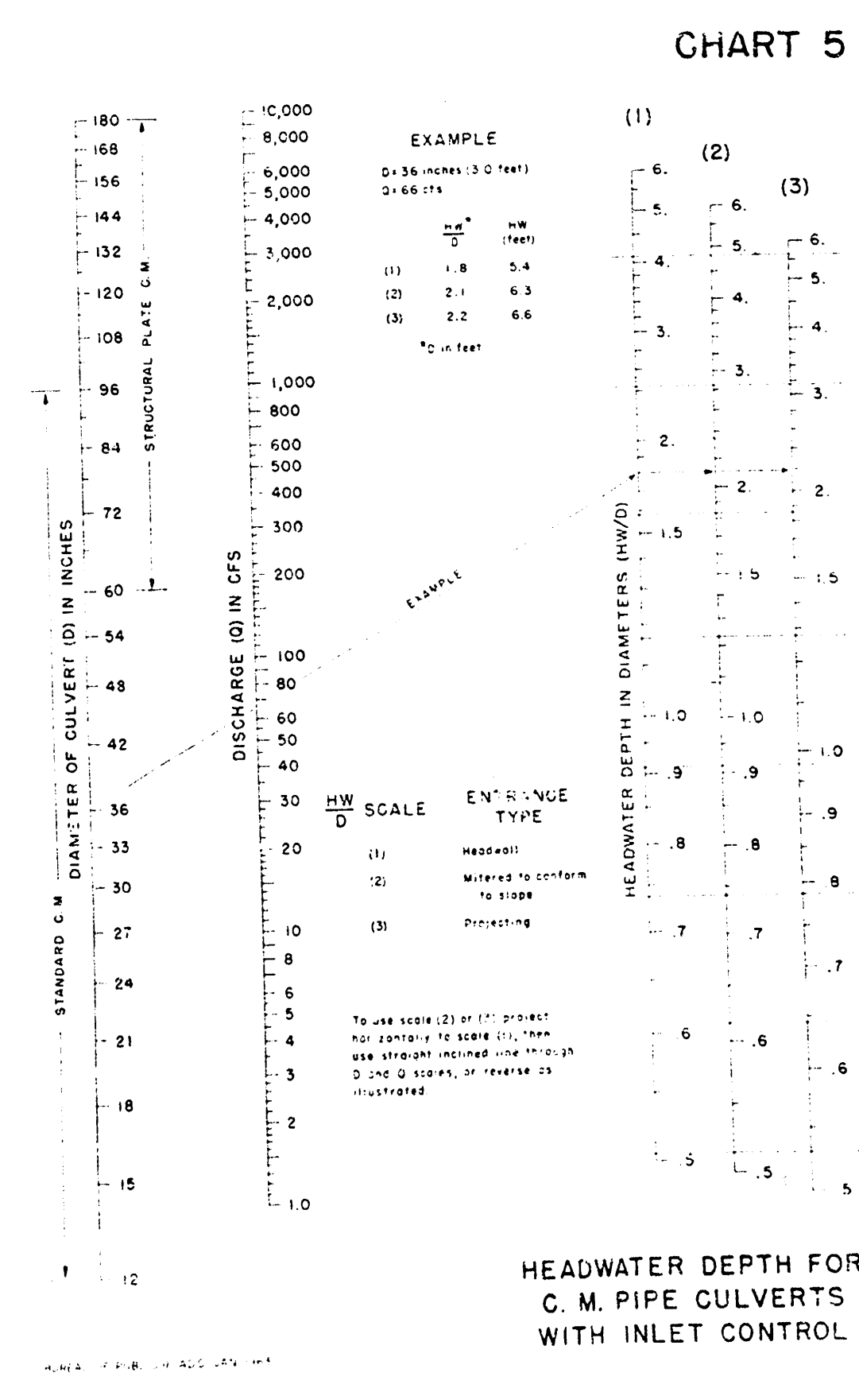
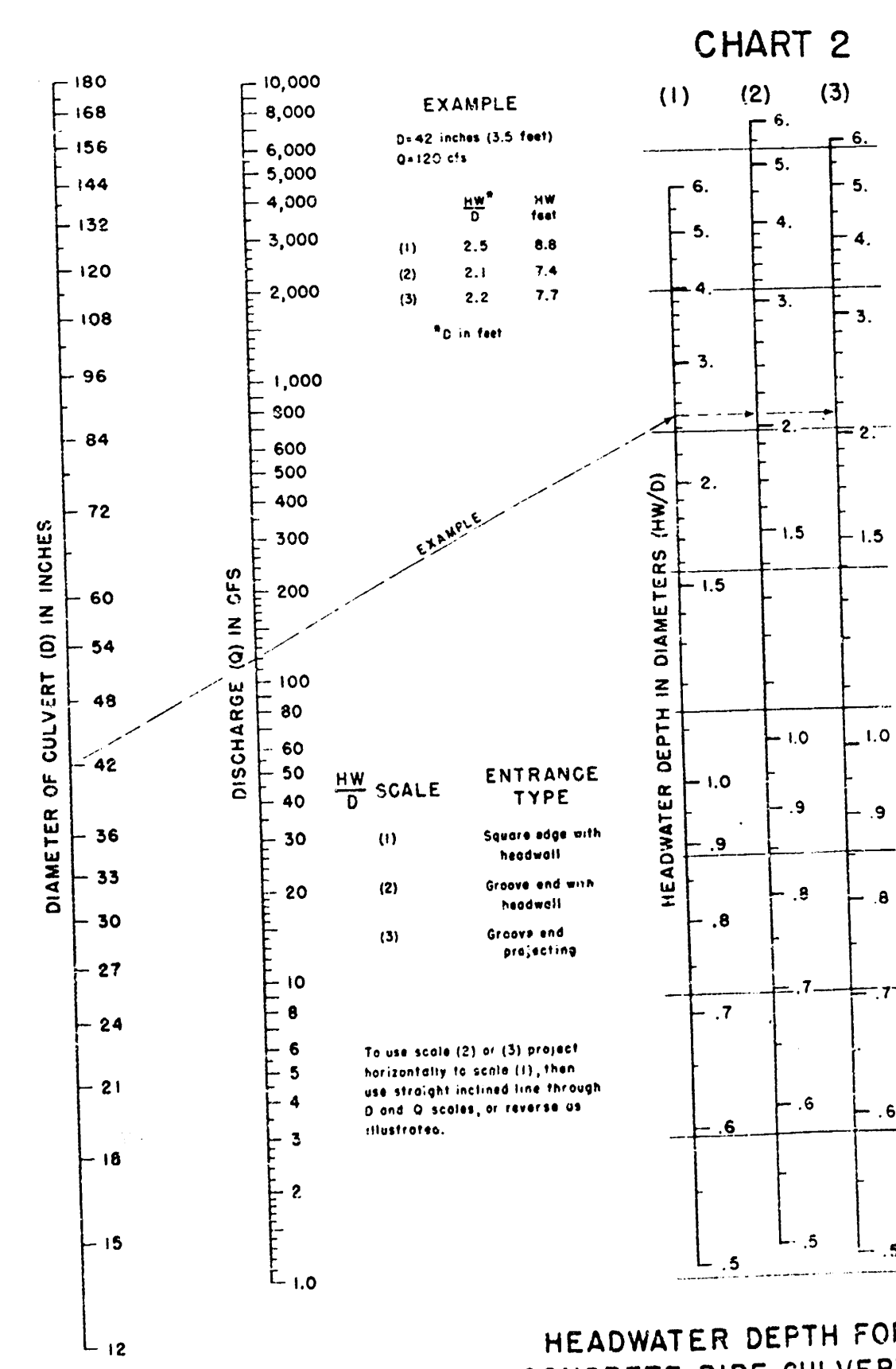
2' CMP

 $H = 1.3$ $HW = 1.25 + 1.3 = 2.55 < 2.7$

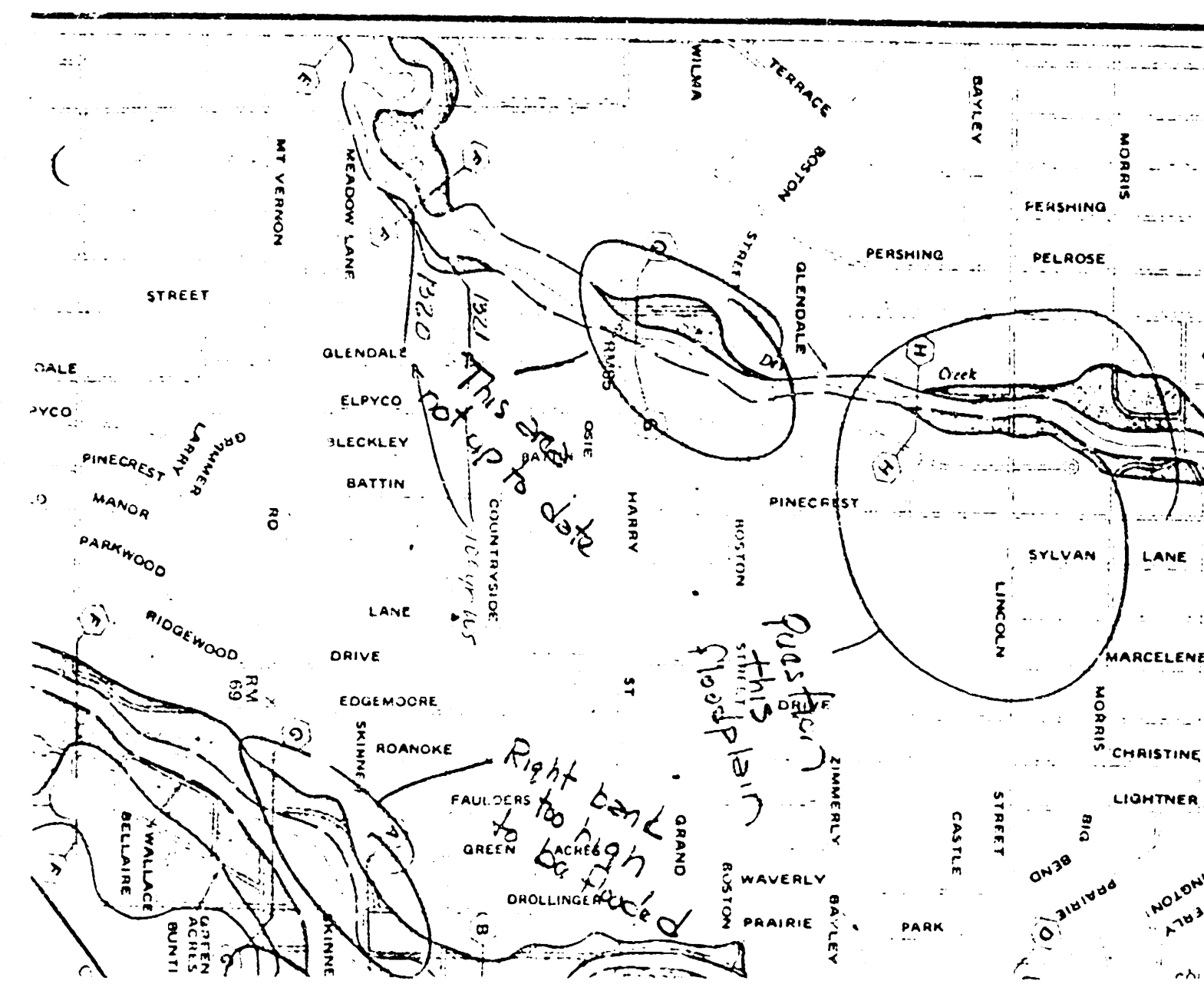
24" RCP

 $H = 2.15$ $HW = 2.15 + 1.3 = 3.45 < 3.7$

Use 24" RCP or 30" CMP



GEORGETOWN:
(Drainage)



1340-1181.4 = 1326 City of London

11. 1000'



Date 7/12/1983 Page of
Project Sh. 1000 R&B Area, 4000'
Item Ditch design

DA: 30% short for 20% long
10% short for 5% long

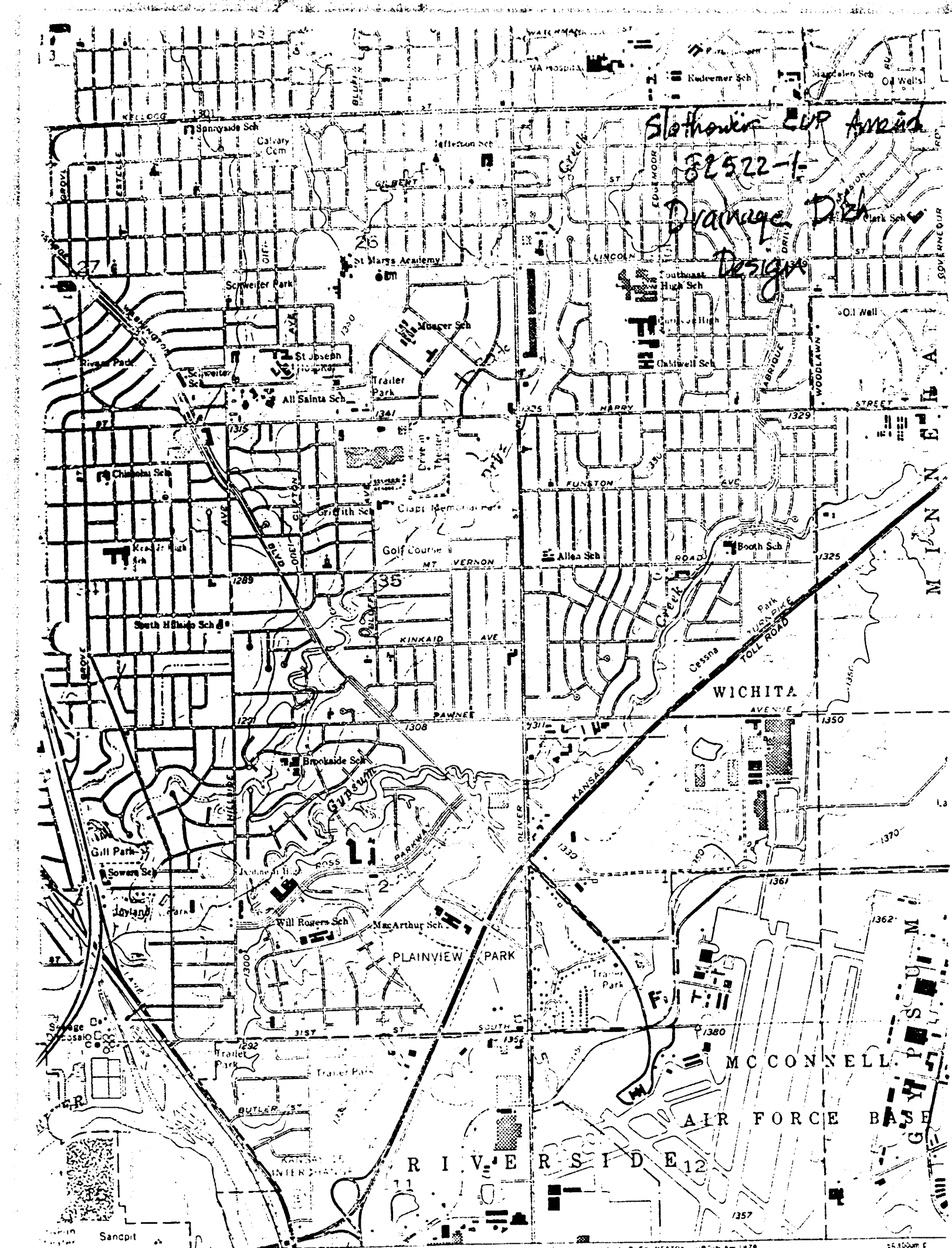
CN: $(50 \times 45 \times 10 + 20 \times 15 \times 6)$
 $+ (50 \times 10 \times 10 + 12 \times 10 \times 36) / 3$
= 37

$\% \text{ } \log_p = 23\%$

$L = 2400'$
 $q = 12 \text{ k}$
 $S_1 = 0.2$
 $S_2 = 0.2$
 $S_3 = 0.2$
 $S_4 = 0.2$

From Jan 3 1902 to
2 1902 1902

Q. 10-11
ca 2-4 pc
y² 2.6'
S. 10.0. 0.2



Slothower EUP Amad
82522-1-
Drainage Dist
Design

First objective: to determine the relationship between the

Rainfall 1.1 inches
Reference Interval (hrs.) 25
Runoff Curve Number 73
Hvd. Len. 2400 feet
Slope 1.2 %
% of RLM 50 %
% Imp. 33 %
Area of basin 52 acres

```

Computed Data
  Basic Loe Factor (hrs.)
  Hdr. Length Adj.
  Lmf. Area Adj.
  Runoff Volume (in.)
  Computed Time or Cord. (hrs)

```

Peak Discharge in Technical Release No. 55 (1975)

Peak Day, 1991	15,000
Combin. -	473,000
Total, 1991	488,000

Peak Discharge by Modified Rational Formula (Rational
Discharge for 25 year rain.

Time of Cond. (T ₁) =	33.66 minutes
Intensity =	3.91 inches/hr.
C factor =	0.37
Peak Dis. (d ₁) =	57.70

```
Radius=1.47 inches
Reference Interval=1000
Current Curve Interval=1000
min. Len. 1000 0.00
Slope 1.0 %
% of RIM 90 %
% max. 90 %
% of R. 1000 1000
```

Completed Data	
Basic Cell Factor (hrs.)	0.00
Edg. Growth Adj.	0.07
Int. Cell Adj.	0.00
Boundary Growth Adj.	1.00
Unadjusted Cell Growth (hrs.)	1.07

Ann. Disasters & Emergency Medicine 2004; 2: 1-10

[illegible]

Inst. Discharge of Debt & Personal Finance
 1000 1st Ave. S. Suite 100
 Minneapolis, MN 55404
 Tel: 612-338-1100
 Fax: 612-338-1101
 Email: info@dischargeofdebt.com

Time of Conv. (hr) =	10.0
Intensity =	2.0
C Factor =	1.1
Peak Max. Value =	10.2

Continuation of Army Discharge
Date: 07-03-1998

Journal: *Classroom Assessment Techniques - Language Literacy Section*

Rainfall: 7.8 inches
Recurrence Interval (yrs.): 100
Rucker Curve Number: 70
Hd. Lin. 200% flow
Slope: 1.0 %
% of RCM 50 %
% imp. 30 %
Area of catch 22 acres

Computed Data	
Basic Loss Factor (hrs.)	0.64
Hour Length Adj.	0.87
Imp. Prod. Adj.	0.78
Subtotal (hrs.)	4.68
Duration Time of Comp. (hrs.)	0.56

Peak Discharge by Technical Release No. 53 (1978)

Price per share	100.00
Dividend	10.00
Yield	10.00%

Post Discharge by Modified Rational Formula (Forcillier
Discharge for 100 year freq.)

Time of Cond. (hr) =	83.66 minutes
Intensity =	8.22 inches/hr.
C factor =	0.42
Peak Dis. (ft) =	84.19

$\text{NORM_DIST}(\text{AVERAGE}(\text{B2:D2}), \text{STDEV}(\text{B2:D2}), \text{TRUE})$

[illegible]

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$$= \frac{1}{2} \left(\frac{1}{2} + \frac{1}{2} \right) = \frac{1}{2}$$

STORY FREQUENCY = 100-YEAR

***** POLYMER AREA *****										***** ANALYSIS *****	
MODE	TH	AREA	ELPAC	LENGTH	TIME	IN	OUT	TO	FROM	UNIT	
MODE	TH	AREA	ELPAC	LENGTH	TIME	IN	OUT	TO	FROM	UNIT	
1	2.020	16.71	0.00	0.2	16.70	8.97	34.20	16.04	8.67	1.0	
2	3.020	0.00	0.00	0.2	0.00	0.00	0.00	16.04	8.68	0.0	

9. POWER ADDITION OUTPUT. LTR# PW# 7/6/83

* * * - H Y D R A U L I C

MODE	HIGH-LEVEL (SW/PT)	FRONT (PT)	REAR (PT)	TRANSITION (PT)	NAPOLE (PT)	DR	CTI (T)
1	0.00000	0.0000	0.0000	0.0000	0.0000		000
2	0.00000	0.0000	0.0000	0.0000	0.0000		000
3	0.00000	0.0000	0.0000	0.0000	0.0000		000

PAUL: * *

RAINFALL (IN)	STATION	DATE	TOTAL (FT)	WIND- ELEVATION (FT)	WIND- ELEVATION (FT)	D.F.
0.0500	000	0-0500	0.0500	000-0500	000-0500	0.0500
0.0500	036	0-036	0.0500	036-0500	036-0500	0.0500
0.0500	026	0-026	0.0500	026-0500	026-0500	0.0500