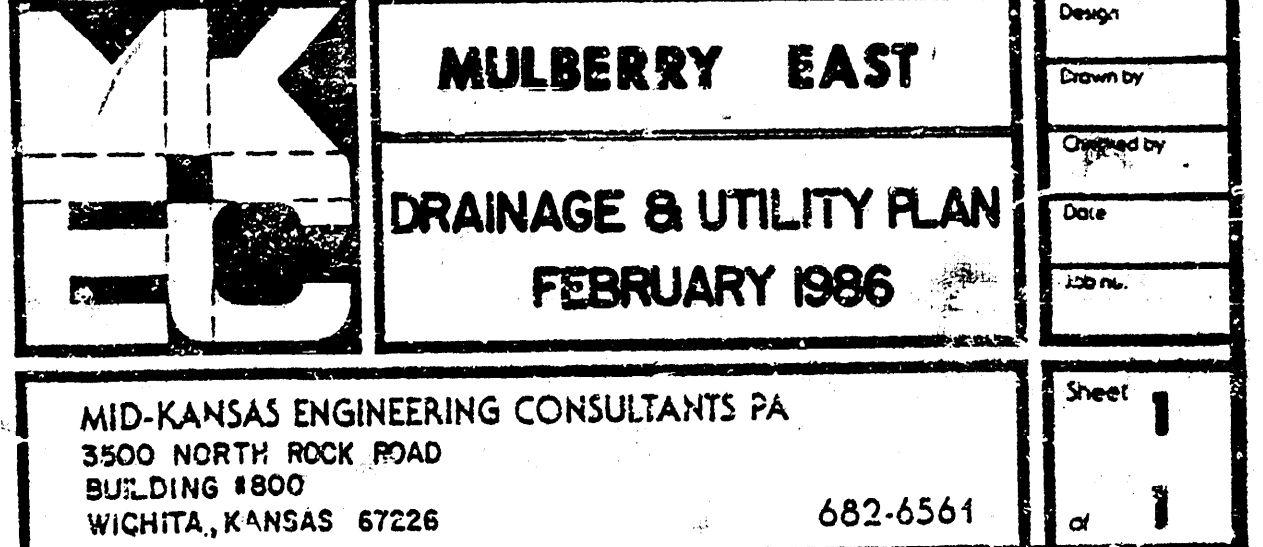


MULBERRY EAST

[illegible][illegible]

East to the pond, the system could handle 9070.35 cfs.
 $\therefore Q = 92.2 - 72.85 = 19.35 \text{ ft}^3/\text{s}$
 HITESANE Run 3 w/ a 2" RCP from the inlet @ E corner NE corner at Hitesane across the gate down to the ponds. The system could handle $Q = 92.25 \text{ cfs} \approx 80.5 \text{ cfs}$.
 The preferable alternative would be HITESANE Run 3 with the 2" RCP from the inlet @ S corner or IR & IS to the pond.
 Per U.S.D., for w/d = 36 FPD, a 12" dia. pipe
 20' x 36" dia. pipe
 20' x 36" dia. pipe
 20' x 36" dia. pipe

JUNCTION NO	ELEVATION	DEMAND	PRESSURE (PSI)	HYDRAULIC SURGE
1	209.9	0.0	5.5	212.4
2	207.5	0.0	1	215.6
3	205.1	0.0	1	219.6
4	204.8	0.0	1	209.9
5	206.1	0.0	2.2	205.8
6	203.3	0.0	2	205.2
7	202.0	0.0	3	208.1
8	201.0	0.0	0	205.8
9	206.4	0.0	1	205.0
10	199.5	0.0	0.7	205.2
11	197.8	0.0	0.9	199.9
12	196.5	0.0	0.7	195.1

THE NET SYSTEM DEMAND = 0
 SUMMARY OF INFLUENCES ON RDT_RESULTS =
 PIPE NO. FLOW LOSS
 1 15.71 1.35
 2 11.35 0.7
 3 57 0
 4 1.02 0
 5 -8.520001 0
 6 15.99 0
 7 -36.16 0

$20.3 - 36.16 = 44.16 \text{ cfs} = 2 \text{ not plotted}$