



MEMO

TO: City of Wichita, Engineering Dept. PROJECT NO. 36-83713-1120
PROJECT: Briarwood Phase II (4th Addition)
ATTN: Chris Breitenstein, P. E. DATE: April 4, 1984

Transmitted herewith are two (2) prints of the Drainage Plan for the proposed Briarwood 4th Addition, along with supporting calculations.

The plan was submitted last Friday, March 30, 1984, for hearing by the Subdivision Committee on April 12, 1984.

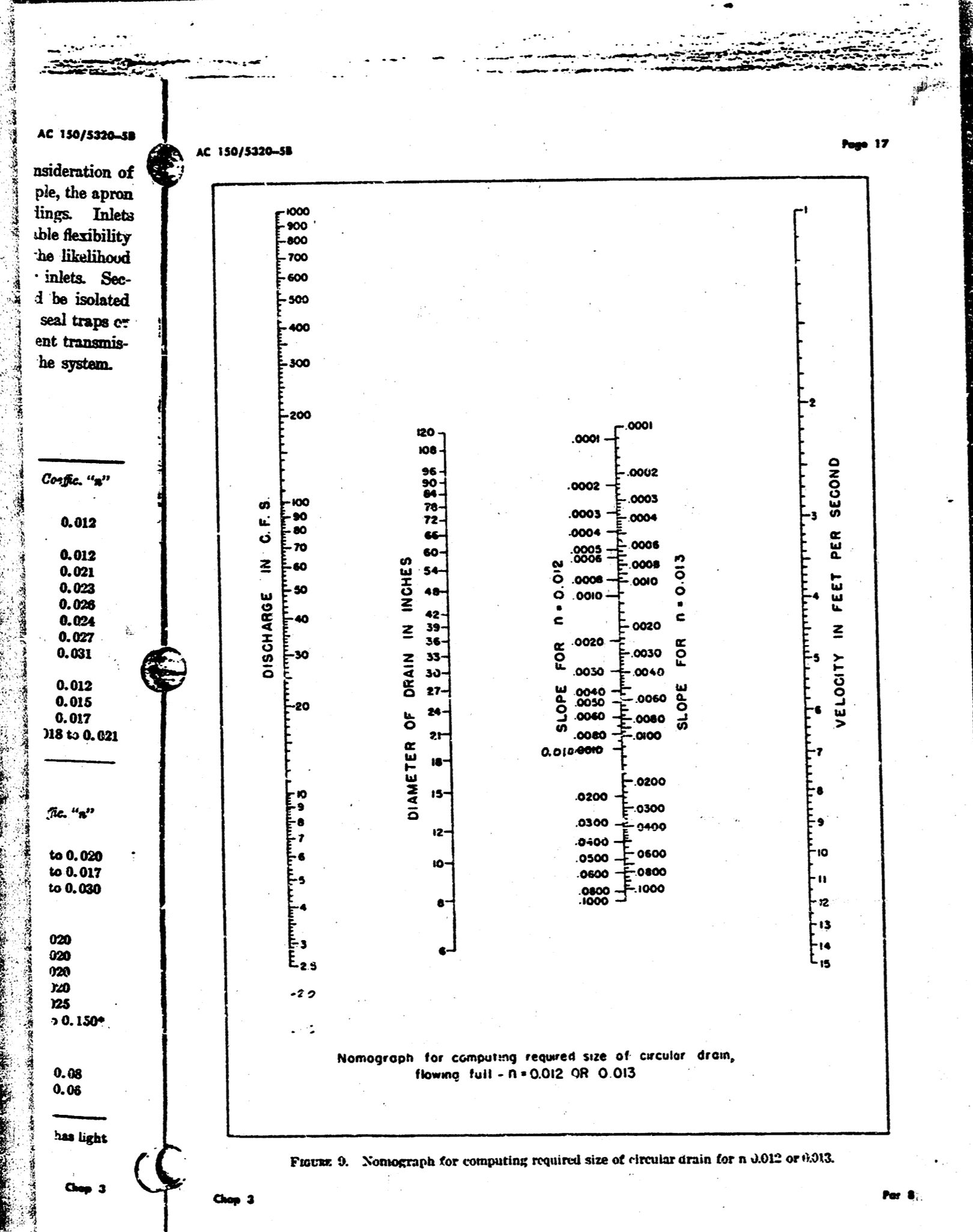
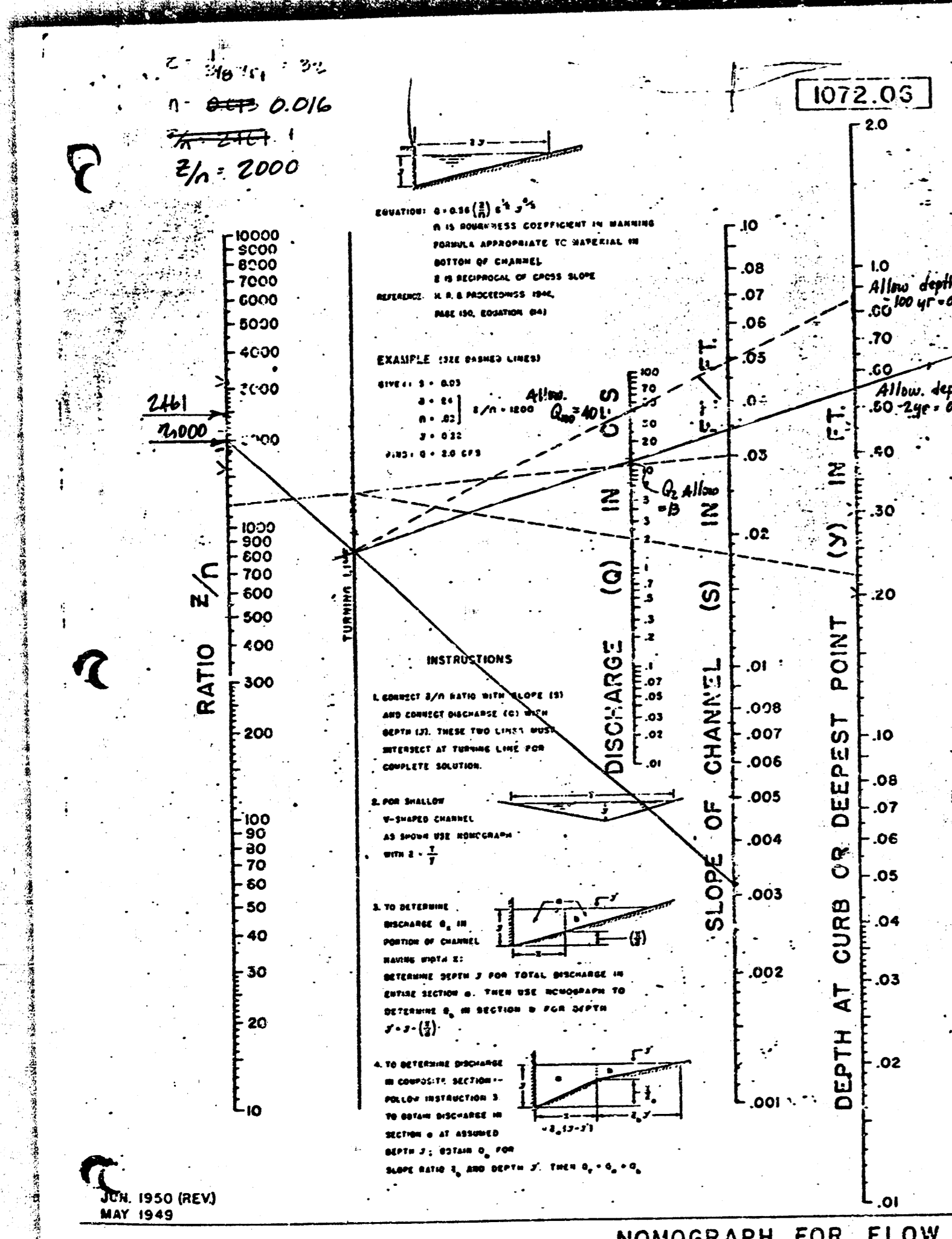
Briarwood Phase II (4th Add.) PEC File 36-83713-1120

SYSTEM 100

- 1. Due to minimal difference in 2 yr & 100 yr pipe sizes, use 100 yr storm
2. Street flow allowed to be 5' above curb depth for 2 yr storm. See attached graph
3. Street flow allowed to be 0.5' above curb depth for 100 yr storm (depth allowed = 0.55')

HYDROLOGY DATA SHEET PAGE 1 OF 2
PROJECT: BRIARWOOD PHASE II (4th Add.) PROJECT NO. 36-83713-1120
ITEM: STORM SEWER DESIGN - SYSTEM 100 DATE: APRIL 3, 1984
RETURN PERIOD: 2 COMPUTATIONS BY: CSS REVISIONS BY:

HYDROLOGY DATA SHEET PAGE 2 OF 2
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RAINFALL INTENSITY TABLE for SEDGWICK COUNTY, KANSAS. Table with columns for duration in minutes and return periods of 1-yr, 2-yr, 5-yr, 10-yr, 25-yr, 50-yr, 100-yr.

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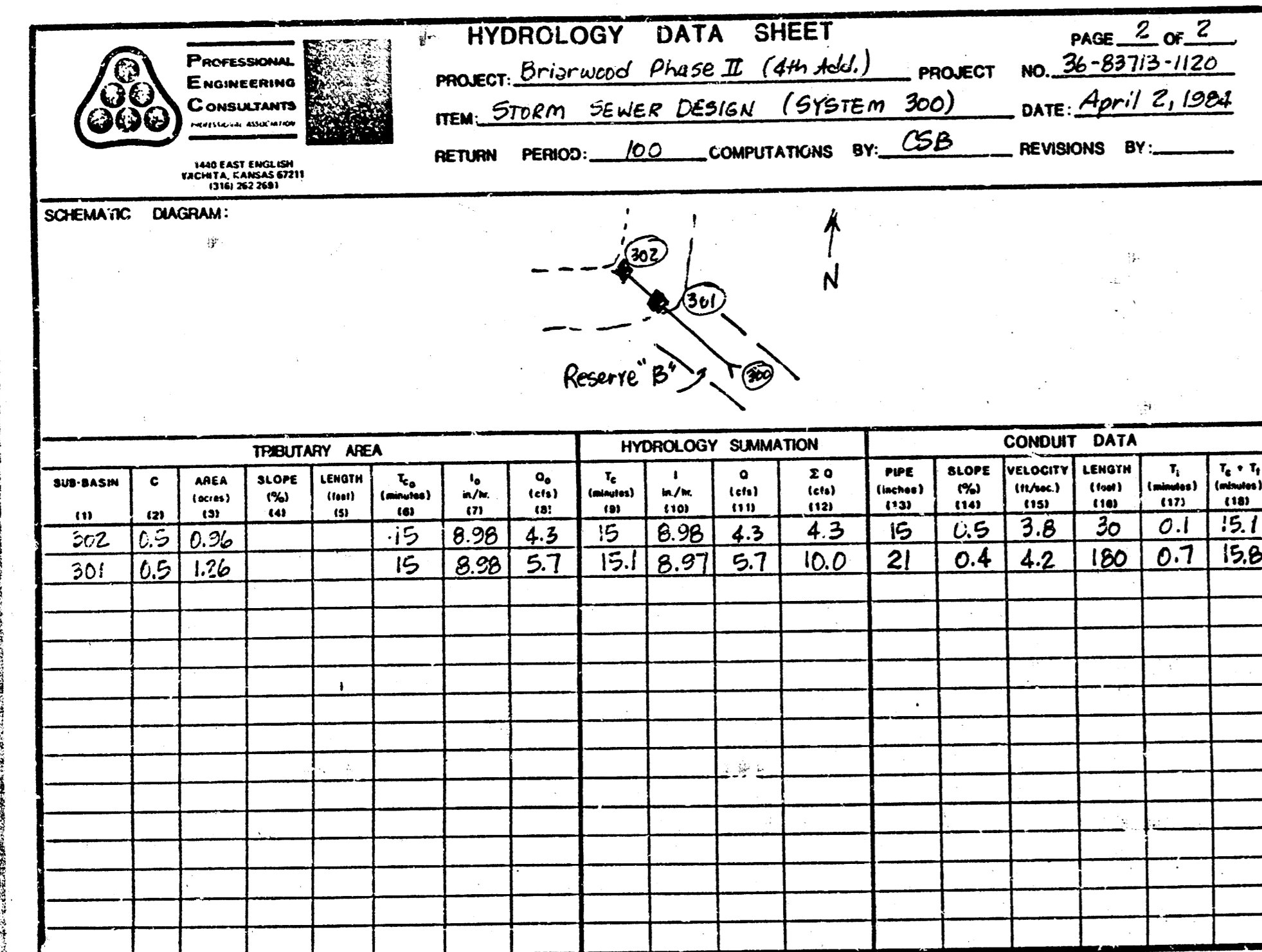
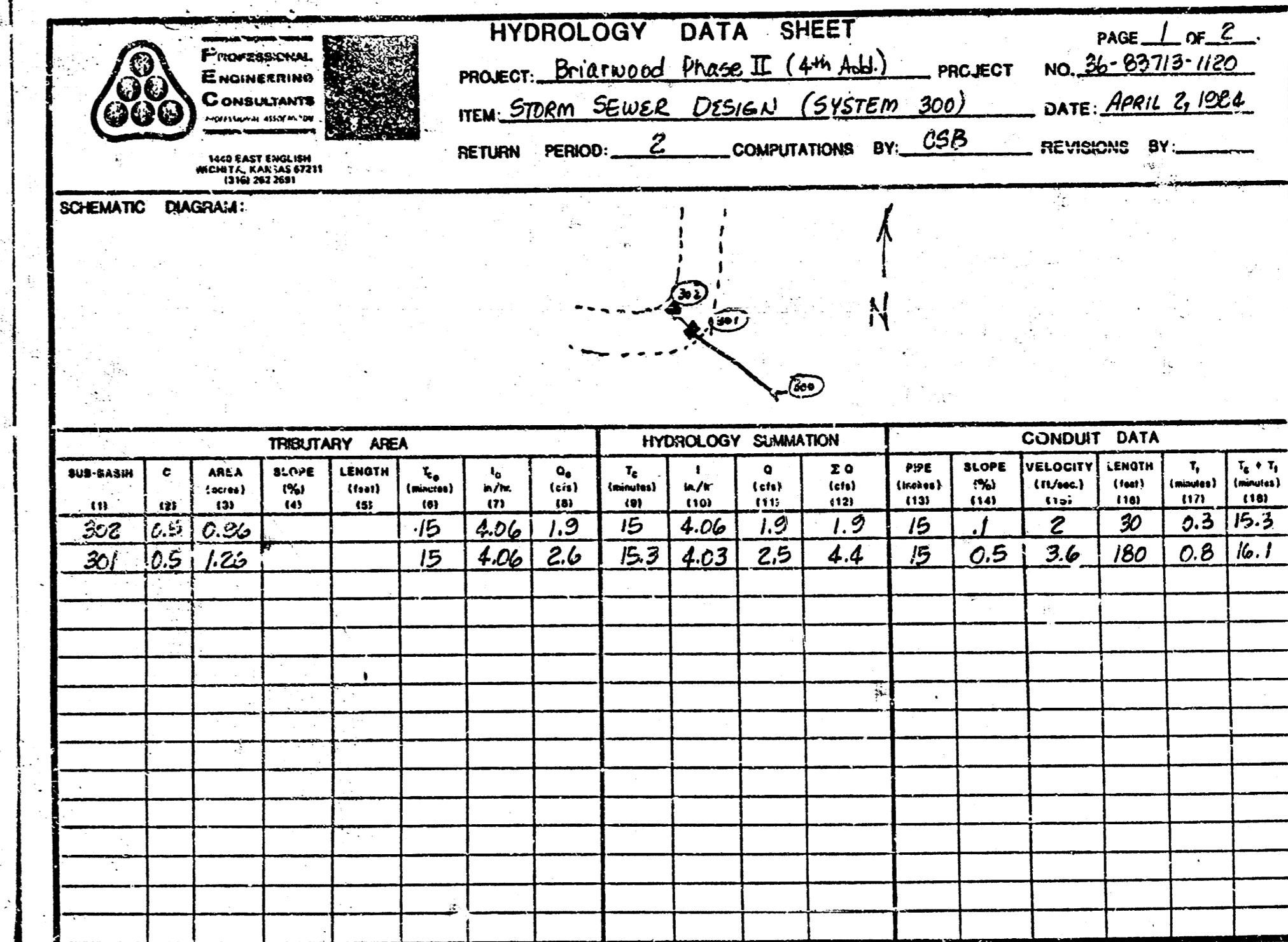
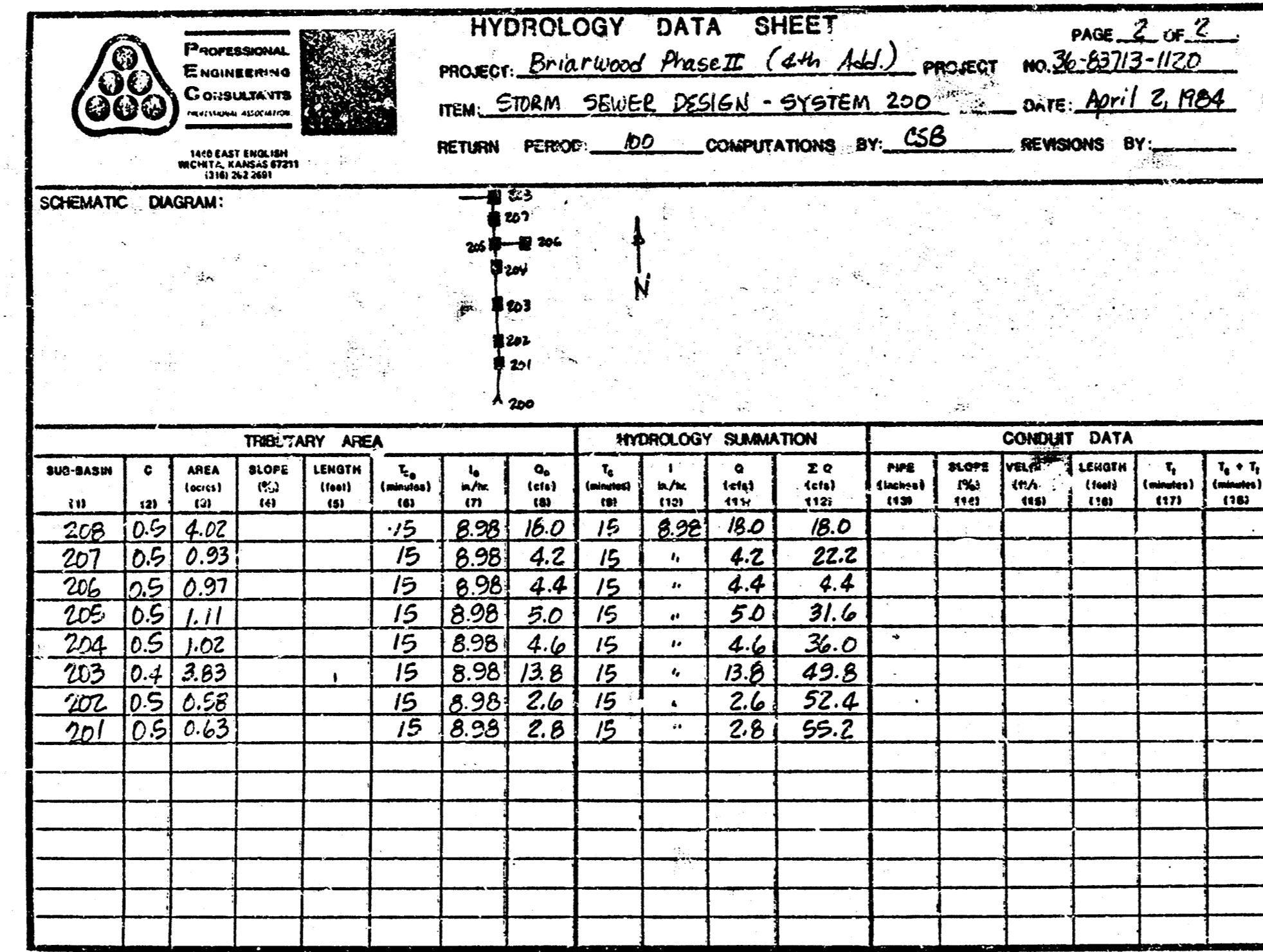
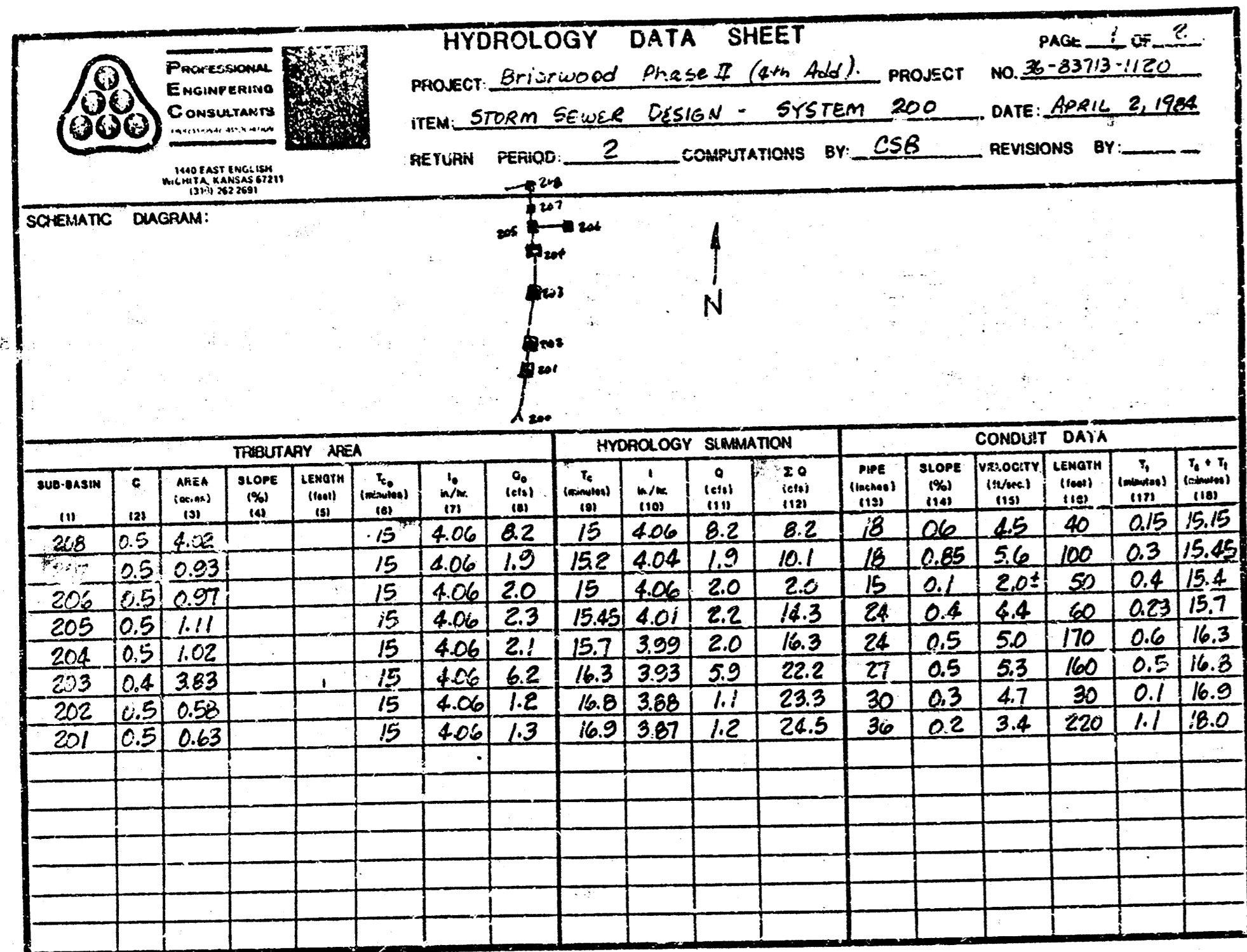
Briarwood Phase II (4th Addition) SYSTEM 200
1. Design this system for 2 year storm in pipe. Difference (Q100 - Q2) overlaid in swales.
2. Street flow for 2 yr storm (Some criteria as System 100)
3. Street flow for 100 yr storm (Some criteria as System 100)

Briarwood Phase II (4th Addition) SWALE DESIGN (Continued)
FOR SWALE DESIGN: USE MANNING'S EQ.
a) NODE 204 - NODE 203 Q = 19.7 cfs
b) NODE 203 - NODE 202 Q = 27.6 cfs
c) NODE 201 - NODE 200 Q = 30.7 cfs

SWALE DESIGN (Continued)
Using above swale section @ slope of 0.4%, Water Depth Between Node 204 & 203 = 1.3'
Node 203 & 202 = 1.5'
Node 201 & 200 = 1.5'

6) Check pipe s/w size out of Node 208.
Use Weir Equation Q = CLH^3/2 where Q = 6.2 cfs, L = unknown, C = 3, H = 1.0'
Use Orifice Equation Q = CA sqrt(2gh) where Q = 6.2, C = 0.6, A = unknown, g = 32.2 ft/sec^2, h = 1.0'

7) Check inlet size of Node 203 (Drop inlet)
Use Weir Equation Q = CLH^3/2 where Q = 6.2 cfs, L = unknown, C = 3, H = 1.0'
Use Orifice Equation Q = CA sqrt(2gh) where Q = 6.2, C = 0.6, A = unknown, g = 32.2 ft/sec^2, h = 1.0'



Briarwood Phase II (4th Addition)

SYSTEM 300

- Design this system to carry the 100-yr Q. The increase in cost for bigger pipe is offset by omitting ditch lining or other type of erosion control at swale outfall.
- Street flow for 2-year storm (same criteria as System 100)
 $Q_{max-street} = 13 cfs$
 Street flows to both Nodes 302 & 301 are less than 13. Therefore street flows for 2-yr. storm OK.
- Street flow for 100-year storm (same criteria as System 100)
 $Q_{max-street} = 40 cfs$
 Street flows to both Nodes 302 & 301 are less than 40 cfs. Therefore street flows for 100-year storm OK.
- Inlet capacity
 Both inlets in sump condition
 Q_2 allowable = 2 cfs / foot (see System 100) x 5' = 10 cfs.
 Both inlet Q_2 for 100 year storm < 10 cfs. Therefore, use 1 C-O-W inlet at each node.

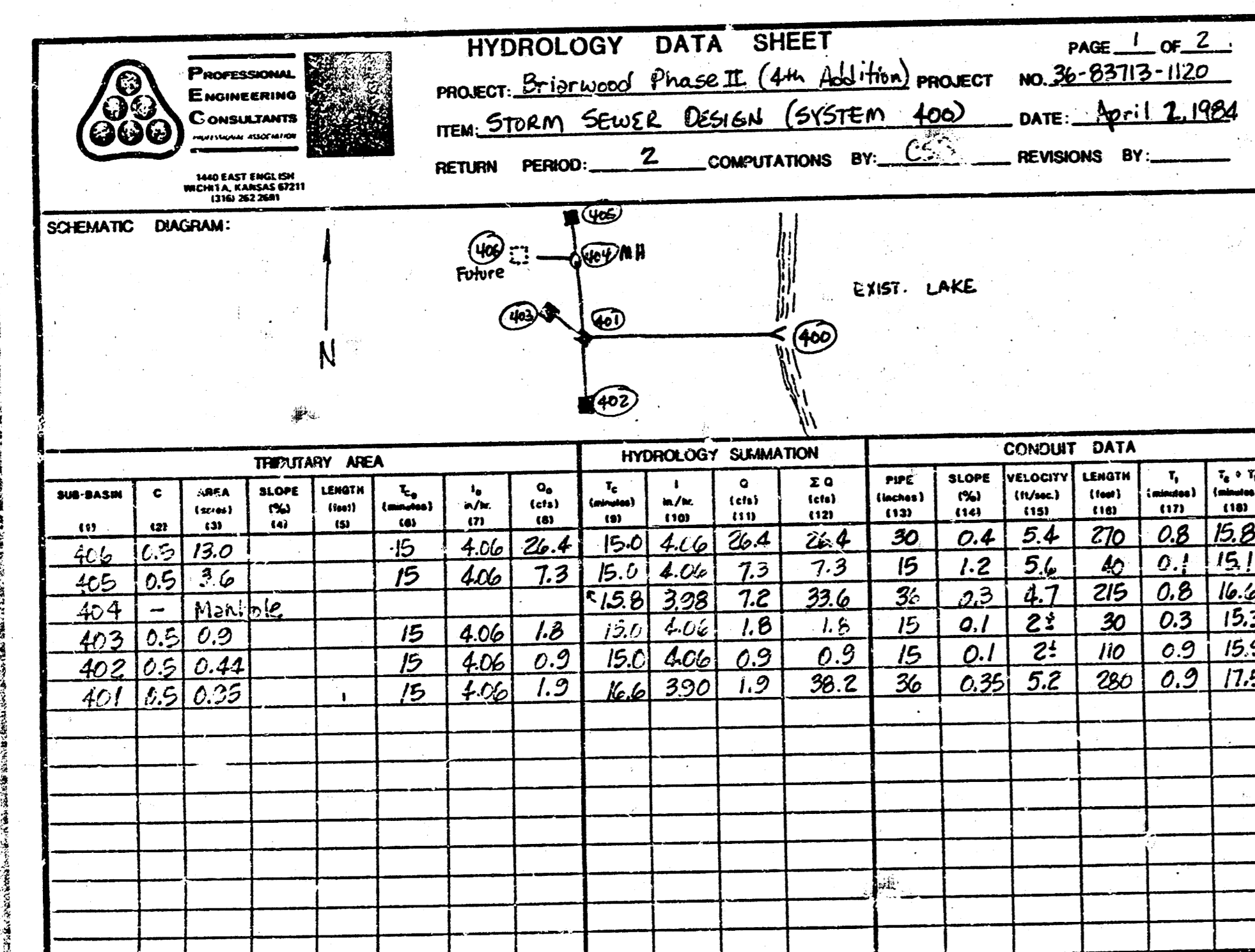
SYSTEM 400

- Design this system with Q_2 in pipe. Difference ($Q_{100} - Q_2$) in swale.
- Street flow for 2-yr. storm (same criteria as System 100)
 $Q_{max-street} = 13 cfs$
 Street flows to all nodes in Briarwood 4th are less than 13. Therefore street flow for 2-year storm OK.
 NOTE: Upon plotting of north portion of parcel, street flows will be checked for Node 406.
- Street flow for 100 yr. storm.
 Check each segment:
 Node 406 to 404: $58.4 - 21.4 = 37.0$
 Node 405 to 406: $16.7 - 7.3 = 9.4$
 Node 404 to 401: $14.6 - 8.6 = 6.0$
 Node 403 to 401: $4.0 - 1.8 = 2.2$
 Node 402 to 401: $2.0 - 0.9 = 1.1$
 Node 401 to 400: $84.9 - 35.2 = 49.7$

3) (continued)

Use Manning's Eq'n to determine a more exact amount of flow on the above street section

- Determine 'composite' n
 $59 LF @ 0.030 = 0.99$
 $29 LF @ 0.016 = 0.46$
 $n = 1.45 / 62 = 0.023$
- Determine Cross-Sectional Area
 $1/2 \times 16.5' \times 0.3' = 2.48$
 $0.4 \times 29' = 11.60$
 $1/2 \times 14.5' \times 0.45 = 3.26$
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 $1/2 \times 14.5' \times 0.3' = 2.48$
 23.08
- Assume wetted perimeter = 62'
 $Hazenlet Slope = \frac{1.486}{0.023} \times 23.08 \times (0.3722)^{1/3} \times (0.0032)^{-1/2}$
 $= \frac{1.486}{0.023} \times 23.08 \times 0.51748 \times 0.05666$
 $= 43.6 cfs$



5) (continued)

d	A	P	R	$R^{2/3}$	$AR^{2/3}$
1.5	13.50	18.25	0.7379	0.8179	11.04
1.6	15.36	19.46	0.78711	0.8539	13.12
1.7	17.34	20.65	0.83643	0.8892	15.41

Water depth = 1.7' Velocity = $\frac{Q}{A} = \frac{46.7}{17.34} = 2.7$

3) (continued)

Street flow (100 yr.) is right at the maximum of Node 401. OK

- Inlet capacity.
 All inlets in sump condition.
 Allowable $Q = 2 cfs / ft$ (see System 100) x 5' (COW min) = 10 cfs
 All inlets in Briarwood 4th OK
 Future Nodes 406-407-408-409 will be checked when north portion plotted.
- Swale Design.
 Use Manning's Eq'n.
 $Q = \frac{1.486}{n} A R^{2/3} S^{1/2}$ where $Q = 46.7 cfs$
 $n = 0.035$
 $S = 0.5\%$
 $AR^{2/3} = \frac{Q \cdot n}{1.486 \cdot S^{1/2}} = \frac{46.7 \times 0.035}{1.486 \times 0.02071}$
 $= 15.65$

