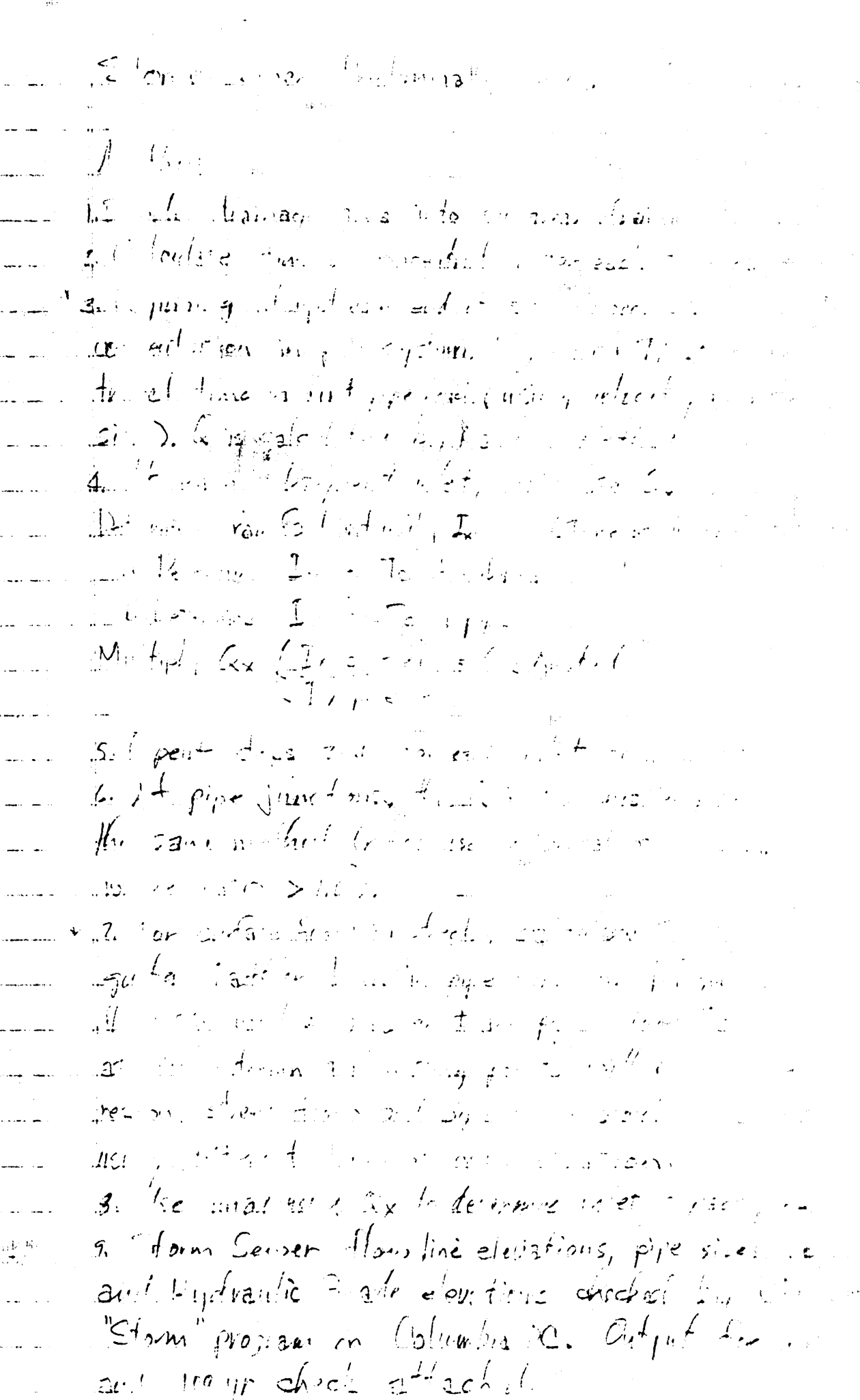




16. From Seven-Weekendary Dossier - The Garden
12-11/15 re Initial Entry Form / Checklist -
See map below for change names data.

$F_{10} = 1.2$ $R_{10} = 1.02$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$
 $L_1 = 0.15 (1.02) (7.4) = 0.91$ $C_{10} = 0.91 / 0.6 = 1.51$
 Pipe 2 on product
 $F_{10} = 1.2$ $D_{10} = 1.1$ $R_{10} = 5$
 $T_1 = 14.1$ $D_1 = 15.0$ $C_1 = 1.3$ $I_1 = 4.6$ $T_1 = 15.0$
 $C_1 = 0.6 (1.3) (15.0) = 13.7$
 $L_1 = 0.15 (1.2) (13.7) = 2.47$ $C_{10} = 2.47 / 1.3 = 1.90$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.9) (15.0) = 16.7$
 $L_1 = 0.15 (1.2) (16.7) = 3.00$
 $C_{10} = 3.00 / 1.9 = 1.58$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.58) (15.0) = 14.2$
 $L_1 = 0.15 (1.2) (14.2) = 3.21$
 $C_{10} = 3.21 / 1.58 = 2.03$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (2.03) (15.0) = 18.3$
 $L_1 = 0.15 (1.2) (18.3) = 3.30$
 $C_{10} = 3.30 / 2.03 = 1.63$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.63) (15.0) = 14.7$
 $L_1 = 0.15 (1.2) (14.7) = 3.25$
 $C_{10} = 3.25 / 1.63 = 2.00$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (2.00) (15.0) = 18.0$
 $L_1 = 0.15 (1.2) (18.0) = 3.24$
 $C_{10} = 3.24 / 2.00 = 1.62$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.62) (15.0) = 14.6$
 $L_1 = 0.15 (1.2) (14.6) = 3.22$
 $C_{10} = 3.22 / 1.62 = 1.99$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.99) (15.0) = 17.9$
 $L_1 = 0.15 (1.2) (17.9) = 3.21$
 $C_{10} = 3.21 / 1.99 = 1.61$
 $T_1 = 14.1$ $D_1 = 5.0$ $C_1 = 0.6$ $I_1 = 7.4$ $T_1 = 15.0$
 $C_1 = 0.6 (1.61) (15.0) = 14.5$
 $L_1 = 0.15 (1.2) (14.5) = 3.20$
 $C_{10} = 3.20 / 1.61 = 1.99$

Faintly visible text on the left side of the page:
 The 2nd ...
 ...
 ...
 ...

SEPTEMBER 18, 198

BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316252-7271 • 315 ELLIS • WICHITA, KANSAS 67211

1" = 150'

[illegible]

24.6 combined as Δ on printout

Final: (35) Dir: 1.9, 11.9, 9.0, 11.0

1. for $Q_1 = 15.0$ $Q_2 = 4.03$ $Q_3 = 3.76$ $I_1 = 7.83$ $I_2 = 2.77$

2. $Q_1 = 0.63(3.0) = 1.89$ cfs

3. $Q_2 = 0.5(2.0) = 1.0$ cfs @ 2.77 min

4. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

5. $Q_1 = 0.63(3.0) = 1.89$ cfs

6. $Q_2 = 0.5(2.0) = 1.0$ cfs @ 2.77 min

7. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

8. $Q_2 = 0.63(2.0) = 1.26$ cfs

9. $Q_1 = 0.63(3.0) = 1.89$ cfs

10. $Q_2 = 0.5(2.0) = 1.0$ cfs

11. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

12. $Q_1 = 0.63(3.0) = 1.89$ cfs

13. $Q_2 = 0.5(2.0) = 1.0$ cfs

14. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

15. $Q_2 = 0.63(2.0) = 1.26$ cfs

16. $Q_1 = 0.63(3.0) = 1.89$ cfs

17. $Q_2 = 0.5(2.0) = 1.0$ cfs

18. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

19. $Q_1 = 0.63(3.0) = 1.89$ cfs

20. $Q_2 = 0.5(2.0) = 1.0$ cfs

21. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

22. $Q_2 = 0.63(2.0) = 1.26$ cfs

23. $Q_1 = 0.63(3.0) = 1.89$ cfs

24. $Q_2 = 0.5(2.0) = 1.0$ cfs

25. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

26. $Q_1 = 0.63(3.0) = 1.89$ cfs

27. $Q_2 = 0.5(2.0) = 1.0$ cfs

28. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

29. $Q_2 = 0.63(2.0) = 1.26$ cfs

30. $Q_1 = 0.63(3.0) = 1.89$ cfs

31. $Q_2 = 0.5(2.0) = 1.0$ cfs

32. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

33. $Q_1 = 0.63(3.0) = 1.89$ cfs

34. $Q_2 = 0.5(2.0) = 1.0$ cfs

35. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

36. $Q_2 = 0.63(2.0) = 1.26$ cfs

37. $Q_1 = 0.63(3.0) = 1.89$ cfs

38. $Q_2 = 0.5(2.0) = 1.0$ cfs

39. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

40. $Q_1 = 0.63(3.0) = 1.89$ cfs

41. $Q_2 = 0.5(2.0) = 1.0$ cfs

42. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

43. $Q_2 = 0.63(2.0) = 1.26$ cfs

44. $Q_1 = 0.63(3.0) = 1.89$ cfs

45. $Q_2 = 0.5(2.0) = 1.0$ cfs

46. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

47. $Q_1 = 0.63(3.0) = 1.89$ cfs

48. $Q_2 = 0.5(2.0) = 1.0$ cfs

49. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

50. $Q_2 = 0.63(2.0) = 1.26$ cfs

51. $Q_1 = 0.63(3.0) = 1.89$ cfs

52. $Q_2 = 0.5(2.0) = 1.0$ cfs

53. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

54. $Q_1 = 0.63(3.0) = 1.89$ cfs

55. $Q_2 = 0.5(2.0) = 1.0$ cfs

56. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

57. $Q_2 = 0.63(2.0) = 1.26$ cfs

58. $Q_1 = 0.63(3.0) = 1.89$ cfs

59. $Q_2 = 0.5(2.0) = 1.0$ cfs

60. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

61. $Q_1 = 0.63(3.0) = 1.89$ cfs

62. $Q_2 = 0.5(2.0) = 1.0$ cfs

63. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

64. $Q_2 = 0.63(2.0) = 1.26$ cfs

65. $Q_1 = 0.63(3.0) = 1.89$ cfs

66. $Q_2 = 0.5(2.0) = 1.0$ cfs

67. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

68. $Q_1 = 0.63(3.0) = 1.89$ cfs

69. $Q_2 = 0.5(2.0) = 1.0$ cfs

70. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

71. $Q_2 = 0.63(2.0) = 1.26$ cfs

72. $Q_1 = 0.63(3.0) = 1.89$ cfs

73. $Q_2 = 0.5(2.0) = 1.0$ cfs

74. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

75. $Q_1 = 0.63(3.0) = 1.89$ cfs

76. $Q_2 = 0.5(2.0) = 1.0$ cfs

77. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

78. $Q_2 = 0.63(2.0) = 1.26$ cfs

79. $Q_1 = 0.63(3.0) = 1.89$ cfs

80. $Q_2 = 0.5(2.0) = 1.0$ cfs

81. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

82. $Q_1 = 0.63(3.0) = 1.89$ cfs

83. $Q_2 = 0.5(2.0) = 1.0$ cfs

84. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.76$

85. $Q_2 = 0.63(2.0) = 1.26$ cfs

86. $Q_1 = 0.63(3.0) = 1.89$ cfs

87. $Q_2 = 0.5(2.0) = 1.0$ cfs

88. for $Q_1 = 15.0$ $I_1 = 7.83$ $Q_2 = 0.63$ $I_2 = 3.83$

89. $Q_1 = 0.63(3.0) = 1.89$ cfs

C_p values for Q_{max} output (window 3.0.01) (6)

100% yr. Design Storm (Qs = sheet flow)

12" to 15" diameter sewer flowlines enter from the bottom of the manhole.

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

Front (12) to 101 (Qs from the manhole to the 102 manhole)

of the sewer was installed in 1907, its capacity is low and is allowed to flow east in 101st.

101st (12) to 102 manhole (101st to 102nd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

102nd (12) to 103 (102nd to 103rd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

103rd (12) to 104 (103rd to 104th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

104th (12) to 105 (104th to 105th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

105th (12) to 106 (105th to 106th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

106th (12) to 107 (106th to 107th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

107th (12) to 108 (107th to 108th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

108th (12) to 109 (108th to 109th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

109th (12) to 110 (109th to 110th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

110th (12) to 111 (110th to 111th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

111th (12) to 112 (111th to 112th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

112th (12) to 113 (112th to 113th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

113th (12) to 114 (113th to 114th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

114th (12) to 115 (114th to 115th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

115th (12) to 116 (115th to 116th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

116th (12) to 117 (116th to 117th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

117th (12) to 118 (117th to 118th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

118th (12) to 119 (118th to 119th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

119th (12) to 120 (119th to 120th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

120th (12) to 121 (120th to 121st)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

121st (12) to 122 (121st to 122nd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

122nd (12) to 123 (122nd to 123rd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

123rd (12) to 124 (123rd to 124th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

124th (12) to 125 (124th to 125th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

125th (12) to 126 (125th to 126th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

126th (12) to 127 (126th to 127th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

127th (12) to 128 (127th to 128th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

128th (12) to 129 (128th to 129th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

129th (12) to 130 (129th to 130th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

130th (12) to 131 (130th to 131st)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

131st (12) to 132 (131st to 132nd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

132nd (12) to 133 (132nd to 133rd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

133rd (12) to 134 (133rd to 134th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

134th (12) to 135 (134th to 135th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

135th (12) to 136 (135th to 136th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

136th (12) to 137 (136th to 137th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

137th (12) to 138 (137th to 138th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

138th (12) to 139 (138th to 139th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

139th (12) to 140 (139th to 140th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

140th (12) to 141 (140th to 141st)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

141st (12) to 142 (141st to 142nd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

142nd (12) to 143 (142nd to 143rd)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

143rd (12) to 144 (143rd to 144th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

144th (12) to 145 (144th to 145th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

145th (12) to 146 (145th to 146th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

146th (12) to 147 (146th to 147th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

147th (12) to 148 (147th to 148th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

148th (12) to 149 (148th to 149th)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

149th (12) to 150 (149th to 150th)

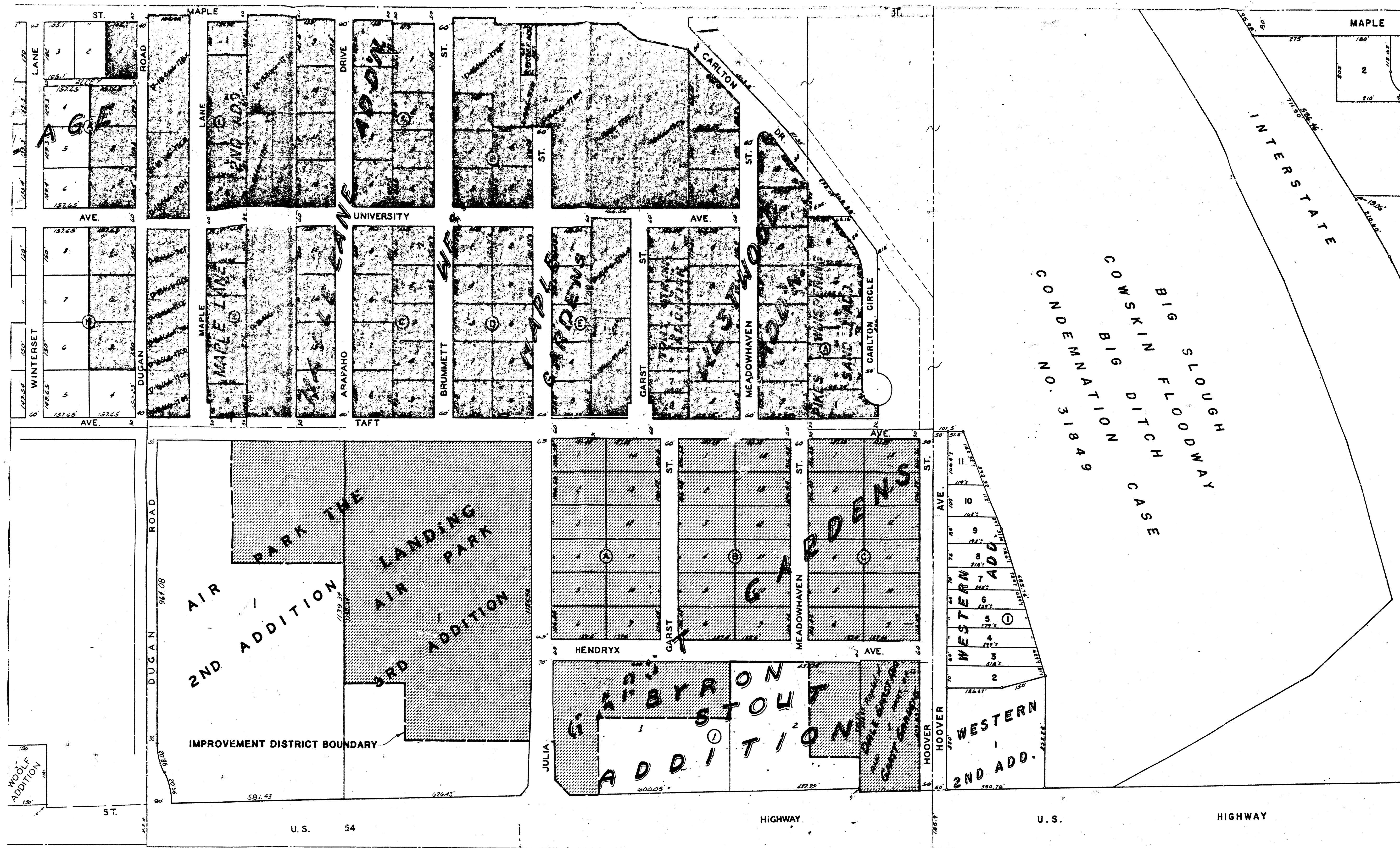
Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

150th (12) to 151 (150th to 151st)

Rainfall (100%) = 3.93 cfs @ $T_c = 3.64$ min.

151st (12) to 152 (151st to 152nd

Punkt (10a) D 112 113 114 115
 T: 2104 = 50,0% 100 = 500 Qm = 1000 Ims = 500
 Kms = 0,500 (2104) (500) = 50,00 cfs
 K₂ = 50,41 ($\frac{500}{112}$) = 40,41 cfs 112 = 50,41
 D₁ für 100 = 20,7 100 = 400 Qm = 200 Ims = 500
 Kms = 0,500 (500) (20,7) = 10,35 cfs
 K₂ = 10,41 ($\frac{500}{112}$) = 11,41 cfs
 Total Q_{sum} = 77,41 + 10,41 + 10,41 = 98,22 cfs (10 + 10)
 L₁ = 4,1 cfs Q₂ = 17,22 + 4,1 = 21,32 cfs 17,22 cfs 4,1 cfs
 Punkt D 108
 T₁ = 24,7 100 = 40,0% 100 = 500 Qm = 500
 Kms = 0,500 (24,7) (500) = 12,35 cfs
 K₂ = 12,35 ($\frac{500}{112}$) = 11,02 cfs 112 = 12,35
 Total Q_{sum} = 17,22 + 11,02 = 28,24 cfs (10 + 10)
 L₁ = 5,18 cfs Q₂ = 17,22 + 5,18 = 22,40 cfs 17,22 cfs 5,18 cfs
 Punkt (20) D 116 117 118 119
 T₁ = 288 100 = 36,0% 100 = 500 Qm = 500
 Kms = 0,500 (288) (500) = 28,80 cfs
 K₂ = 28,80 ($\frac{500}{112}$) = 25,71 cfs 112 = 28,80
 Total Q_{sum} = 17,22 + 25,71 = 42,93 cfs
 L₁ = 10,21 cfs Q₂ = 17,22 + 10,21 = 27,43 cfs 17,22 cfs 10,21 cfs
 Total Q_{sum} = 42,93 + 10,21 = 53,14 cfs



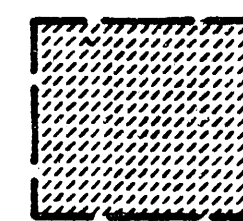
NOTES:
1. FOR SYSTEM LAYOUT, SEE DRAINAGE PLAN DATED 9-18-87.
2. FOR LOT/PARCEL AREAS, SEE ATTACHED AREA LIST.



SCALE: 1" = 150'
APPROXIMATE



Property to be assessed at
Residential Use Valuation



Property to be assessed at
Commercial Use Valuation

PROPOSED IMPROVEMENT DISTRICT FOR DRAINAGE IMPROVEMENTS IN THE AREA FROM KELLOGG TO MAPLE AND FROM DUGAN TO THE BIG DITCH

Rev. Oct. 2, 1987
SEPT. 18, 1987

BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316225-7271 • 315 ELLIS • WICHITA, KANSAS 67214

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Point (A) D111 (B) D112
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

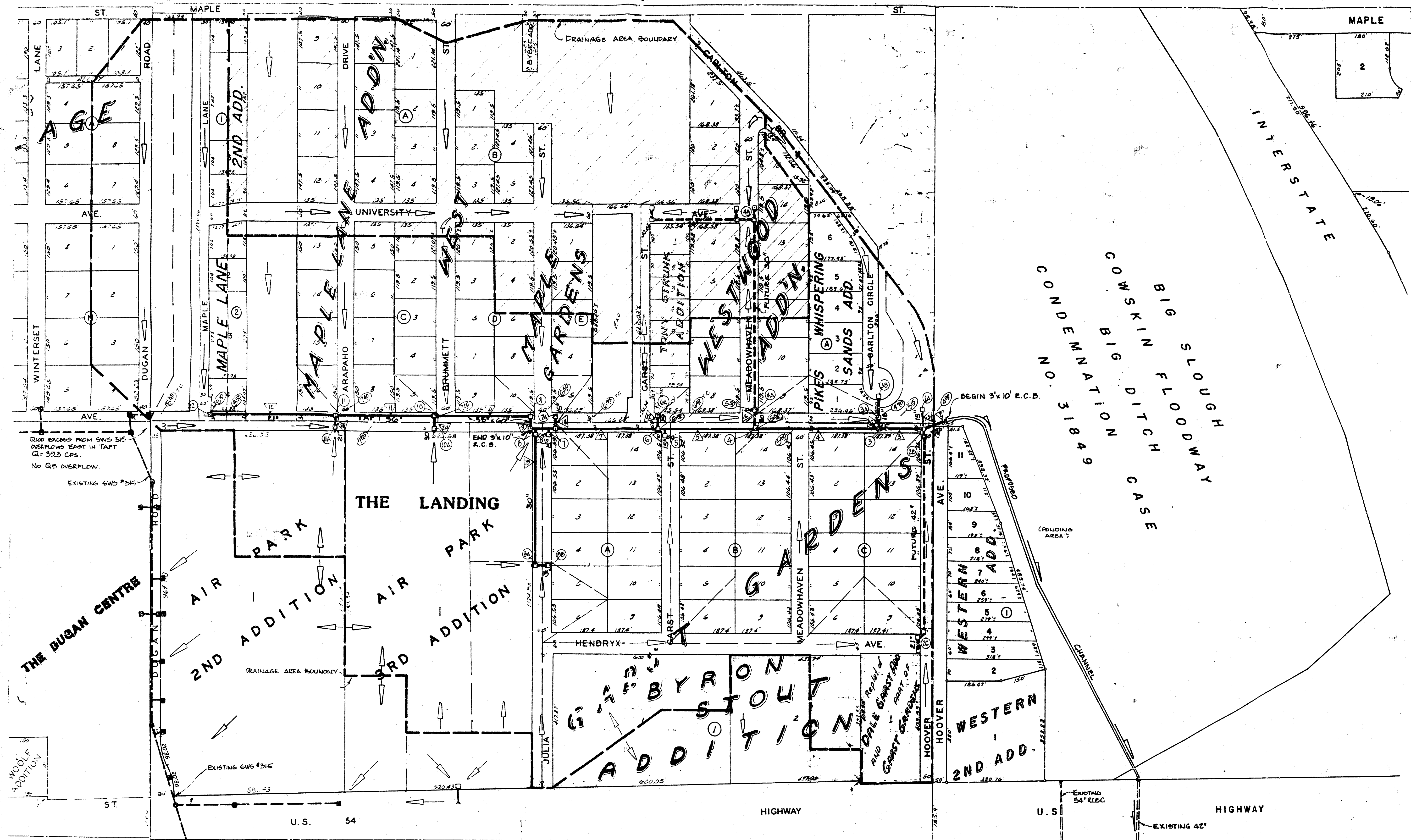
Summary of storm water runoff
1. For D111 = 175.00' (175.00' x 1.00' = 175.00')
2. For D112 = 175.00' (175.00' x 1.00' = 175.00')
3. For D113 = 175.00' (175.00' x 1.00' = 175.00')
4. For D114 = 175.00' (175.00' x 1.00' = 175.00')
5. For D115 = 175.00' (175.00' x 1.00' = 175.00')
6. For D116 = 175.00' (175.00' x 1.00' = 175.00')
7. For D117 = 175.00' (175.00' x 1.00' = 175.00')
8. For D118 = 175.00' (175.00' x 1.00' = 175.00')
9. For D119 = 175.00' (175.00' x 1.00' = 175.00')
10. For D120 = 175.00' (175.00' x 1.00' = 175.00')

Project: THE LANDING - PRELIMINARY STORM SEWER DESIGN
Date: 1/1/1980
Time: 11:45:2

INPUT DATA LISTING

CD	L2	MAX Q	ADJ Q	LENGTH	FL 1	FL 2	CTL/TW	D	U	S	KJ	KE	KH	LC	L1	L3	L4	A1	A3	A4	J	N
2	2	158.0	158.0	100.00	111.10	111.14	.00	36	120	3	.10	.10	.00	1	3	13	14	30	60	20	5.00	.013
2	3	126.0	126.0	155.00	111.14	111.21	.00	36	120	3	.10	.10	.00	0	4	0	15	0	0	90	5.00	.013
2	4	107.0	107.0	420.00	111.21	111.38	.00	36	120	3	.10	.10	.00	0	5	0	16	0	0	90	5.00	.013
2	5	85.4	85.4	300.00	111.38	111.50	.00	36	120	3	.10	.10	.00	0	6	0	0	0	0	90	5.00	.013
2	6	85.4	85.4	30.00	111.50	111.52	.00	36	120	3	.10	.10	.00	0	7	0	17	0	0	90	5.00	.013
2	7	72.9	72.9	350.00	111.52	111.66	.00	36	120	3	.10	.10	.00	0	8	0	18	0	0	90	5.00	.013
2	8	65.3	65.3	90.00	111.66	111.70	.00	36	120	3	.10	.10	.00	0	9	19	0	90	90	0	5.00	.013
2	9	50.8	50.8	50.00	111.70	111.80	.00	48	0	3	.10	.10	.00	0	10	0	0	90	0	0	5.00	.013
2	10	46.1	46.1	330.00	111.80	112.44	.00	48	0	3	.10	.10	.00	0	11	20	0	0	0	0	5.00	.013
2	11	19.3	19.3	330.00	112.44	114.12	.00	36	0	3	.10	.10	.00	0	12	21	0	0	0	0	5.00	.013
2	12	6.9	6.9	420.00	114.07	115.71	.00	21	0	1	.10	.10	.00	0	0	0	0	0	0	0	5.00	.013
2	13	31.6	31.6	50.00	111.54	111.64	.00	36	0	1	.10	.10	.00	3	0	0	0	0	0	0	5.00	.013
2	14	1.3	1.0	50.00	110.89	110.99	.00	15	0	1	.10	.10	.00	0	0	0	0	0	0	0	5.00	.013
2	15	6.9	5.1	50.00	111.96	112.04	.00	18	0	1	.10	.10	.00	4	0	0	0	0	0	0	5.00	.013
2	16	19.8	13.7	50.00	111.88	111.98	.00	24	0	1	.10	.10	.00	0	0	0	0	0	0	0	5.00	.013
2	17	2.6	2.1	50.00	112.27	112.37	.00	15	0	1	.10	.10	.00	7	0	0	0	0	0	0	5.00	.013
2	18	2.7	2.3	50.00	111.41	111.51	.00	15	0	1	.10	.10	.00	8	0	0	0	0	0	0	5.00	.013
2	19	9.7	8.1	270.00	111.95	112.45	.00	24	0	1	.10	.10	.00	9	0	0	0	0	0	0	5.00	.013
2	20	26.9	22.2	50.00	113.46	113.56	.00	30	0	1	.10	.10	.00	11	0	0	0	0	0	0	5.00	.013
2	21	12.3	10.4	50.00	114.87	114.97	.00	21	0	1	.10	.10	.00	12	0	0	0	0	0	0	5.00	.013

Line	Q	D	U	Dn	Dc	Flow	St-full	V1	2	FL 1	FL 2	HG 1	HG 2	D1	D2	TJ	TW
No	(cfs)	(in)	(in)	(ft)	(ft)	Type	(ft/ft)	(ft/s)	(ft/s)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)	(ft)
1						Hydraulic grade line control =	114.15										
2	158.0	36	120	3.00	1.98	Full	.00175	5.3	1.3	111.10	111.14	114.15	114.33	3.05	3.19	.00	.00
3	126.0	36	120	3.00	1.70	Full	.00112	4.2	1.2	111.14	111.21	114.58	114.75	2.44	3.54	.00	.00
4	107.0	36	120	3.00	1.52	Full	.00080	3.6	1.6	111.21	111.38	114.88	115.22	3.67	3.86	.00	.00
5	85.4	36	120	2.61	1.31	Full	.00051	2.8	1.8	111.38	111.50	115.34	115.49	3.96	3.99	.00	.00
6	85.4	36	120	2.18	1.31	Full	.00051	2.8	1.8	111.50	111.52	115.52	115.53	4.02	4.01	.00	.00
7	72.9	36	120	2.33	1.18	Full	.00037	2.4	1.4	111.52	111.66	115.60	115.73	4.08	4.07	.00	.00
8	65.3	36	120	2.09	1.10	Full	.00030	2.2	1.2	111.66	111.70	115.76	115.79	4.10	4.09	.00	.00
9	50.8	48	0	2.69	2.13	Full	.00125	4.0	1.0	111.70	111.80	116.00	116.06	4.30	4.26	.00	.00
10	46.1	48	0	2.51	2.03	Full	.00103	3.7	1.7	111.80	112.44	116.57	116.91	4.77	4.45	.00	.00
11	19.3	36	0	1.76	1.40	Full	.00084	2.7	1.7	112.44	112.45	117.05	117.33	3.59	3.21	.00	.00
12	6.9	21	0	1.39	.97	Full	.00190	2.9	1.9	114.87	115.71	117.34	118.14	2.47	2.04	118.26	.00
3						Hydraulic grade line control =	114.45										
13	31.6	36	0	3.00	1.02	Part	.00224	4.5	1.5	111.54	111.64	114.45	114.57	2.91	2.93	114.41	.00
3						Hydraulic grade line control =	114.45										
14	1.3	15	0	.59	.45	Full	.00042	1.1	1.1	110.89	110.99	111.45	114.47	3.56	3.48	114.49	.00
4						Hydraulic grade line control =	114.82										
15	6.9	18	0	1.50	1.01	Full	.00429	3.9	1.9	111.96	112.04	114.82	115.33	2.86	2.90	115.29	.00



NOTE:
SEE ACCOMPANYING PRELIMINARY DESIGN CALCULATIONS
FOR Q VALUES. SEE COMPUTER PRINTOUT FOR PIPE FLOW LINE
AND HYDRAULIC GRADE ELEVATIONS.

△ PIPE LINE NUMBER ON PRINTOUT
⊙ INLET NUMBER ON CALCULATIONS
⊙ TOP OF CURB ELEVATION (ADD 110.00)

DRAINAGE PLAN FOR THE LANDING

October 2, 1987
SEPTEMBER 18, 1987
BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316262-271 • 315 ELLIS • WICHITA, KANSAS 67211

16 19.8 24 0 2.00 1.10 Full .00766 6.3 6 111.88 111.98 116.31 116.69 4.43 4.71 117.37 .00

Project: THE LANDING - PRELIMINARY STORM SEWER DESIGN Date: 1/1/1980 Time: 0:41:28

STORM DRAIN ANALYSIS RESULTS

Line No	Q (cfs)	D (in)	V (ft/s)	Dn (ft)	Dc (ft)	Flow Type	St-full	V1 (ft/s)	V2 (ft/s)	FL1 (ft)	FL2 (ft)	HG1 (ft)	HG2 (ft)	D1 (ft)	D2 (ft)	TU (s)	TU (s)
7	2.6	15	0	.93	.64	Full	.00161	2.1	2.1	112.27	112.37	116.83	116.91	4.56	4.54	116.99	.00
8	2.7	15	0	.95	.65	Full	.00170	2.2	2.2	111.41	111.51	117.18	117.26	5.77	5.75	117.34	.00
9	9.7	24	0	1.57	1.11	Full	.00184	3.1	3.1	111.95	112.49	117.24	117.74	5.29	5.25	117.92	.00
11	26.9	30	0	2.50	1.77	Full	.00430	5.5	5.5	113.46	113.56	117.78	118.00	4.32	4.44	118.51	.00
12	12.3	21	0	1.75	1.31	Full	.00603	5.1	5.1	114.87	114.97	118.14	118.44	3.27	3.47	118.88	.00

LIST OF ABBREVIATIONS

V1, FL1, D1 and HG1 refer to downstream end
V2, FL2, D2 and HG2 refer to upstream end
X - Distance in feet from downstream end to point where HG intersects soffit in seal condition
X(N) - Distance in feet from downstream end to point where water surface reaches normal depth by either drawdown or backwater
X(J) - Distance in feet from downstream end to point where hydraulic jump occurs in line
F(J) - The computed force at the hydraulic jump

Project: THE LANDING - PRELIMINARY STORM SEWER DESIGN Date: 1/1/1980 Time: 0:41:28

INPUT DATA LISTING

CD	L2	MAX Q	ADJ Q	LENGTH	FL1	FL2	CTL/TU	D	V	S	N	KE	KM	LC	L1	L3	L4	A1	A3	A4	TU	TU
8	1	218.0	188.0	100.00	111.10	111.14	.00	36	120	3	.10	.10	.00	1	3	13	14	30	60	20	5.00	.019
2	3	178.0	156.0	195.00	111.14	111.21	.00	36	120	3	.10	.10	.00	0	4	15	0	0	0	90	5.00	.019
2	4	147.0	137.0	420.00	111.21	111.38	.00	36	120	3	.10	.10	.00	0	5	16	0	0	0	90	5.00	.019
2	5	114.0	116.0	300.00	111.38	111.50	.00	36	120	3	.10	.10	.00	0	6	0	0	0	0	0	5.00	.019
2	6	116.0	116.0	30.00	111.50	111.52	.00	36	120	3	.10	.10	.00	0	7	0	17	0	0	90	5.00	.019
2	7	103.0	103.0	350.00	111.52	111.66	.00	36	120	3	.10	.10	.00	0	8	0	18	0	0	90	5.00	.019
2	8	95.0	95.0	90.00	111.66	111.70	.00	36	120	3	.10	.10	.00	0	9	19	0	90	90	0	5.00	.019
2	9	50.8	50.8	50.00	111.70	111.80	.00	48	0	3	.10	.10	.00	0	10	0	0	0	0	0	5.00	.019
2	10	46.1	46.1	330.00	111.80	112.46	.00	48	0	3	.10	.10	.00	0	11	20	0	0	0	90	5.00	.019
2	11	19.3	19.3	330.00	112.46	114.12	.00	36	0	3	.10	.10	.00	0	12	21	0	0	90	0	5.00	.019
2	12	6.9	6.9	420.00	114.12	115.71	.00	21	0	1	.10	.10	.00	0	0	0	0	0	0	0	5.00	.019
2	13	31.6	31.6	50.00	115.71	116.64	.00	36	0	1	.10	.10	.00	0	3	0	0	0	0	0	5.00	.019
2	14	1.3	1.0	50.00	116.64	116.99	.00	15	0	1	.10	.10	.00	3	0	0	0	0	0	0	5.00	.019
2	15	6.9	5.1	50.00	116.99	117.06	.00	18	0	1	.10	.10	.00	4	0	0	0	0	0	0	5.00	.019
2	16	19.8	13.7	50.00	117.06	111.98	.00	24	0	1	.10	.10	.00	5	0	0	0	0	0	0	5.00	.019
2	17	2.6	2.1	50.00	111.98	112.37	.00	15	0	1	.10	.10	.00	7	0	0	0	0	0	0	5.00	.019
2	18	2.7	2.3	50.00	112.37	111.51	.00	15	0	1	.10	.10	.00	8	0	0	0	0	0	0	5.00	.019
2	19	9.7	8.1	270.00	111.51	112.49	.00	24	0	1	.10	.10	.00	9	0	0	0	0	0	0	5.00	.019
2	20	26.9	22.2	50.00	112.49	113.56	.00	30	0	1	.10	.10	.00	11	0	0	0	0	0	0	5.00	.019
2	21	12.3	10.4	90.00	113.56	114.97	.00	21	0	1	.10	.10	.00	12	0	0	0	0	0	0	5.00	.019

Project: THE LANDING - PRELIMINARY STORM SEWER DESIGN Date: 1/1/1980 Time: 0:41:28

STORM DRAIN ANALYSIS RESULTS

WILLIAM L. KOBEL, L.S.
JOHN E. LYNCH, L.S.
B. BRENT WOOTEN, P.E.
BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316262-271 • 315 ELLIS • WICHITA, KANSAS 67211

Revised September 22, 1987

Preliminary Storm Sewer Estimate - Alternate A
(The Landing and adjacent areas - includes system in Taft, Hoover, Meadowlark and Garst)

Estimated Cost of Construction \$547,920
+ 25% 136,980
Project Cost \$684,900

Assessment Areas
The Landing = 15.28 Ac.
Other Commercial = 23.93 Ac.
Residential (see Drainage Plan) = 56.12 Ac.
Total B.D. Area = 95.33 Ac.

Limit assessment for The Landing to \$100,000
\$100,000 ÷ 665,603 s.f. = \$0.15/s.f.
Other commercial @ \$0.15/s.f. x 1,042,198 s.f. = 156,329
Residential @ \$0.375/s.f. x 2,444,663 s.f. = 916,572
\$348,000

Total assessed share of cost -348,000
City share of Project Cost \$336,895
(=94.2%)

Typical residential lot (17,000 s.f.)
assessment @ \$0.375/s.f. = \$637.50

WILLIAM L. KOBEL, L.S.
JOHN E. LYNCH, L.S.
B. BRENT WOOTEN, P.E.
BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316262-271 • 315 ELLIS • WICHITA, KANSAS 67211

September 22, 1987

Preliminary Storm Sewer Estimate - Alternate B
(The Landing and adjacent areas - includes system in Taft, Hoover, Meadowlark and Garst)

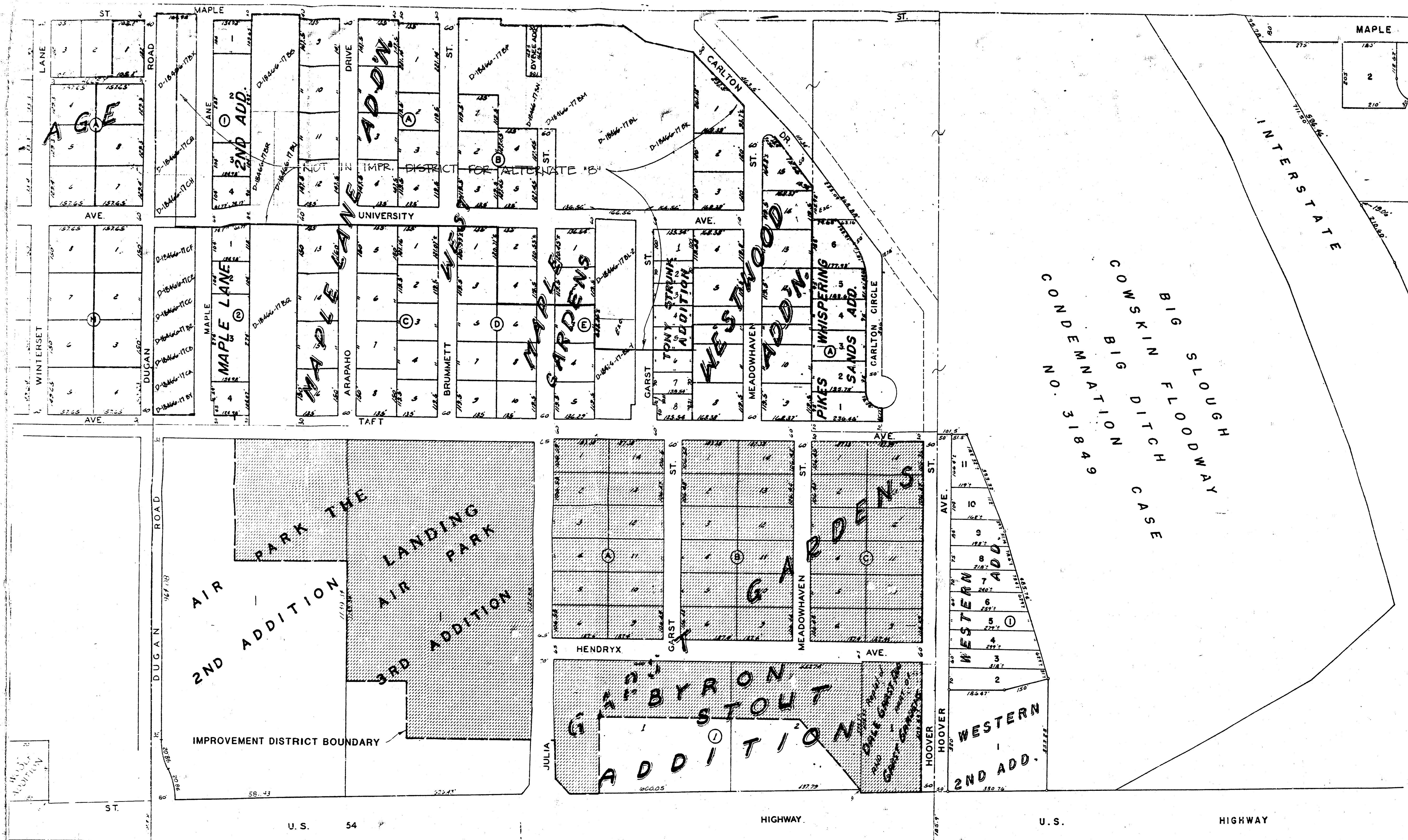
Estimated Cost of Construction \$499,378
+ 25% 124,844
Project Cost \$624,222

Assessment Areas
The Landing = 15.28 Ac.
Other Commercial = 23.93 Ac.
Residential (see Drainage Plan) = 24.59 Ac.
Total B.D. Area = 63.80 Ac.

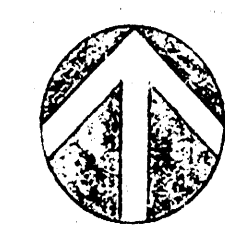
Limit assessment for The Landing to \$100,000
\$100,000 ÷ 665,603 s.f. = \$0.15/s.f.
Other commercial @ \$0.15/s.f. x 1,042,198 s.f. = 156,329
Residential @ \$0.375/s.f. x 1,71,297 s.f. = 64,244
\$220,573

Total assessed share of cost -220,573
City share of Project Cost \$320,443
(=51.3%)

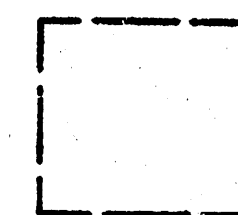
Typical residential lot (17,000 s.f.)
assessment @ \$0.375/s.f. = \$637.50



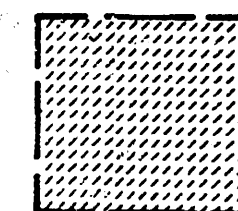
NOTES:
1. FOR SYSTEM LAYOUT, SEE DRAINAGE PLAN DATED 9-18-87.
2. FOR LOT PARCEL AREAS, SEE ATTACHED AREA LIST.



SCALE: 1" = 150'
APPROXIMATE



Property to be assessed at
Residential Use Valuation



Property to be assessed at
Commercial Use Valuation

PROPOSED IMPROVEMENT DISTRICT FOR DRAINAGE IMPROVEMENTS

IN THE AREA FROM KELLOGG TO MAPLE
AND FROM DUGAN TO THE BIG DITCH

ALTERNATE "B"

SEPT. 18, 1987

B. BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316/262-7271 • 315 ELLIS • WICHITA, KANSAS 67211

The Landing Storm Sower - complete system

Tract	Area	\$/s.f.	Share
Bybee Addition			
Lot 1	8,141	0.0375	\$ 305.29
*Byron Stout Addition			
The west 153 feet of lot 1, except Street R/W	61,382	0.15	9,207.30
North 180 feet of Lot 1, except west 153 feet	80,460	"	12,069.00
Part of Lot 2	100,954	"	15,143.10
242,796			
*Garst Gardens			
Lots 1-6, 9-14, Block A (12)	219,950	0.15	2,994.00
" " " " Block B "	219,951	"	2,994.00
" " " " Block C "	219,940	"	2,991.00
718,722			
Maple Lane 2nd Addition			
Lot 1, Block 1	14,038	0.0375	526.43
Lot 2, "	38,064	"	1,427.40
Lot 3, "	14,038	"	526.43
Lot 4, "	13,093	"	493.09
Lot 1, Block 2	14,983	"	561.86
Lot 2, "	14,038	"	526.43
Lot 3, "	36,985	"	1,386.94
Lot 4, "	13,588	"	509.55
158,827			
Pike's Whispering Sands Addition			
Lot 1, Block A	20,500	0.0375	768.75
Lot 2, "	17,612	"	660.45
Lot 3, "	17,344	"	657.90
Lot 4, "	17,477	"	655.39
Lot 5, "	17,340	"	650.25
Lot 6, "	22,260	"	834.75
112,733			
*Replat of Dale Garst & Part of Garst Gardens			
Lot 1, except part replatted as Byron Stout Addition	81,190	0.15	12,178.50
*The Landing			
Lot 1, except the south 182 feet	665,603	0.15	99,840.45
*Terry Strunk Addition			
Lot 1	13,554	0.0375	508.28
Lots 2-7 (6)	89,487	"	355.76
Lot 8	16,644	"	381.15
80,640			

* Commercial Use Assumed

Tract	Area	\$/s.f.	Share
West Maple Gardens			
Lot 1, Block A	29,854	0.0375	\$1,119.51
Lots 2-4, " (3)	816,133	"	604.99
Lots 1-3, Block B (3)	816,133	"	604.99
Lots 4-5, " (2)	817,206	"	645.21
Lot 1, Block C	16,351	"	613.16
Lots 2-5, " (4)	816,133	"	604.99
Lot 1, Block D	16,115	"	611.81
Lot 2, "	16,285	"	610.69
Lots 3-10, " (8)	816,133	"	604.99
Lot 1, Block E	16,446	"	616.73
Lot 2, "	16,317	"	611.89
Lot 3, "	16,307	"	611.51
Lot 4, "	16,297	"	611.14
Lot 5, "	16,287	"	610.76
885,265			
Westerlea Village			
Lot 1, Block A	19,128	0.0375	717.30
Lot 7, "	20,340	"	762.75
Lots 8-9, " (2)	820,384	"	764.40
Lots 1-3, Block N (3)	823,648	"	886.80
153,180			
Westwood Addition			
Lot 1	29,877	0.0375	1,120.29
Lots 2-3 (2)	820,206	"	757.73
Lot 4	20,153	"	755.74
Lots 5-14 (10)	820,121	"	754.54
Lot 15	16,212	"	607.95
307,864			
Unplatted Tracts (by Key Number)			
D-184-66-17 BK	86,085	0.0375	3,228.19
BL-1	94,866	"	3,557.48
BL-2	51,830	"	1,943.63
BH	52,281	"	1,960.54
BH	96,902	"	3,633.79
BN	8,141	"	305.29
BP	61,901	"	2,321.29
BQ	98,988	"	3,712.05
BR	24,747	"	928.01
BS	49,494	"	1,856.03
BU	24,747	"	928.01
BV	99,943	"	1,872.86
BY	10,124	"	379.65
BZ	"	"	"
CA	"	"	"
CB	"	"	"
CC	"	"	"
CD	"	"	"
CE	"	"	"
CF	20,217	"	759.20
CG	18,897	"	708.64
CH	"	"	"
818,709			
Total B.D.	4,152,464	= 95.33 Ac	

BAUGHMAN COMPANY, P.A.
SURVEYING & ENGINEERING
316/262-7271 • 315 ELLIS • WICHITA, KANSAS 67211

September 21, 1987

Preliminary Storm Sewer Estimate
(The Landing and adjacent areas)

Estimated Cost of Construction \$547,920
+ 25% 136,380
Project Cost \$684,300

Assessment Areas
The Landing = 15.28 Ac.
Other Commercial = 19.74 Ac.
Residential = 56.12 Ac.
Total B.D. Area = 91.14 Ac.

Unit assessment for The Landing to
\$100,000 ÷ 665,003 s.f. = \$0.15/s.f.
other commercial ÷ 30,15/s.f. x 869,784 s.f. = 129,118
Residential ÷ 3,037/s.f. x 2,444,463 s.f. = 91,675
Total assessed share of cost = 320,793

City share of Project Cost \$364,100
(=53.2%)

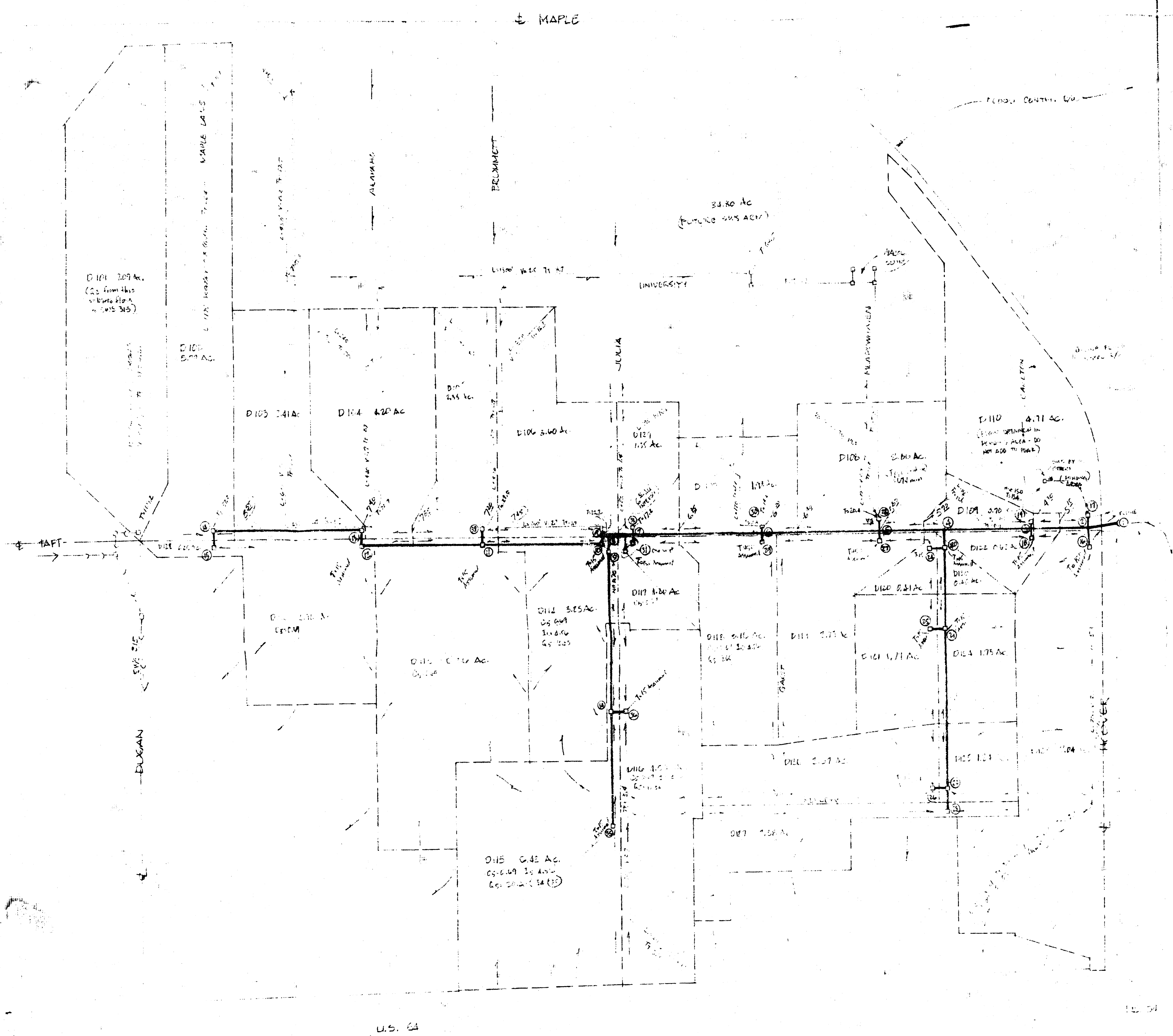
Average residential lot (17,000 s.f.)
assessment ÷ 50,037/s.f. = \$637.50

Tract	Area	\$/s.f.	Share
The Landing Storm Sower			
* Byron Stout Addition			
Lot 1, except part replatted as Dale Garst & Part of Garst Gardens	81,190	0.15	12,178.50
185,000			
Maple Lane Addition			
Lots 1-4, 6-9-12 (6)	619,910	0.0375	744.74
Lots 5-8, 10-13-16 (8)	620,250	"	751.38
324,160			
Maple Lane 2nd Addition			
Lot 1, Blk 1	14,038	0.0375	526.43
Lot 2, Blk 1	38,064	"	1,427.40
Lot 3, Blk 1	14,038	"	526.43
Lot 4, "	13,093	"	490.99
Lot 1, Blk 2	14,983	"	561.86
Lot 2, "	14,038	"	526.43
Lot 3, "	36,985	"	1,386.94
Lot 4, "	13,588	"	509.55
158,827			
Garst Gardens			
Lots 1-6, 9-14, Block A (12)	219,950	0.15	2,994.00
Lots 1-6, 9-14, Block B (12)	219,951	"	2,994.00
Lots 1-6, 9-14, Block C (12)	219,940	"	2,991.00
718,722			

Tract	Area	\$/s.f.	Share
*Replat of Dale Garst & Part of Garst Gardens			
Lot 1, except part replatted as Byron Stout Addition	81,190	0.15	12,178.50
Bybee Addition			
Lot 1	8,141	0.0375	305.29
The Landing			
Lot 1, except the south 182 feet	665,603	0.15	99,840.45
Terry Strunk Addition			
Lot 1	13,554	0.0375	508.28
Lots 2-7 (6)	89,487	"	355.76
Lot 8	16,644	"	381.15
80,640			
West Maple Gardens			
Lot 1, Block A	29,854	0.0375	1,119.51
Lots 2-4, " (3)	816,133	"	604.99
Lots 1-3, Block B (3)	816,133	"	604.99
Lots 4-5, " (2)	817,206	"	645.21
Lot 1, Block C	16,351	"	613.16
Lots 2-5, " (4)	816,133	"	604.99
Lot 1, Block D	16,115	"	611.81
Lot 2, "	16,285	"	610.69
Lots 3-10, " (8)	816,133	"	604.99
Lot 1, Block E	16,446	"	616.73
Lot 2, "	16,317	"	611.89
Lot 3, "	16,307	"	611.51
Lot 4, "	16,297	"	611.14
Lot 5, "	16,287	"	610.76
885,265			
Westerlea Village			
Lot 1, Block A	19,128	0.0375	717.30
Lot 7, "	20,340	"	762.75
Lots 8-9, " (2)	820,384	"	764.40
Lots 1-3, Block N (3)	823,648	"	886.80
153,180			
Westwood Addition			
Lot 1	29,877	0.0375	1,120.29
Lots 2-3 (2)	820,206	"	757.73
Lot 4	20,153	"	755.74
Lots 5-14 (10)	820,121	"	754.54
Lot 15	16,212	"	607.95
307,864			
Unplatted Tracts (by Key Number)			
D-184-66-17 BK	86,085	0.0375	3,228.19
BL-1	94,866	"	3,557.48
BL-2	51,830	"	1,943.63
BH	52,281	"	1,960.54
BH	96,902	"	3,633.79
BN	8,141	"	305.29
BP	61,901	"	2,321.29
BQ	98,988	"	3,712.05
BR	24,747	"	928.01
BS	49,494	"	1,856.03
BU	24,747	"	928.01
BV	99,943	"	1,872.86
BY	10,124	"	379.65
BZ	"	"	"
CA	"	"	"
CB	"	"	"
CC	"	"	"
CD	"	"	"
CE	"	"	"
CF	20,217	"	759.20
CG	18,897	"	708.64
CH	"	"	"
818,709			

Tract	Area	\$/s.f.	Share
West Maple Gardens (cont)			
Lot 1, Blk B	16,133	0.0375	604.99
Lot 5, Blk E	16,285	"	610.76
885,265			
Westerlea Village			
Lot 1, Blk A	19,128	0.0375	717.30
Lot 7, "	20,340	"	762.75
Lots 8-9, " (2)	820,384	"	764.40
Lots 1-3, Block N (3)	823,648	"	886.80
153,180			
Westwood Addition			
Lot 1	29,877	0.0375	1,120.29
Lots 2-3 (2)	820,206	"	757.73
Lot 4	20,153	"	755.74
Lots 5-14 (10)	820,121	"	754.54
Lot 15	16,212	"	607.95
307,864			
Unplatted Tracts (by Key Number)			
D-184-66-17 BK	86,085	0.0375	3,228.19
BL-1	94,866	"	3,557.48
BL-2	51,830	"	1,943.63
BH	52,281	"	1,960.54
BH	96,902	"	3,633.79
BN	8,141	"	305.29
BP	61,901	"	2,321.29
BQ	98,988	"	3,712.05
BR	24,747	"	928.01
BS	49,494	"	1,856.03
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BY	10,124	"	379.65
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CE	"	"	"
CF	20,217	"	759.20
CG	18,897	"	708.64
CH	"	"	"
818,709			

Tract	Area	\$/s.f.	Share
Unplatted Tracts			
D-184-66-17 BK	86,085	0.0375	3,228.19
BL-1	94,866	"	3,557.48
BL-2	51,830	"	1,943.63
BH	52,281	"	1,960.54
BH	96,902	"	3,633.79
BN	8,141	"	305.29
BP	61,901	"	2,321.29
BQ	98,988	"	3,712.05
BR	24,747	"	928.01
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BY	10,124	"	379.65
BZ	"	"	"
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CB	"	"	"
CC	"	"	"
CD	"	"	"
CE	"	"	"
CF	20,217	"	759.20
CG	18,897	"	708.64
CH	"	"	"
818,709			
Total B.D.	4,152,464	= 95.33 Ac	



All areas within Taft assumed to be 10% (C=1.49)
 This part of the Landing from the adjacent area and property of the Landing is assumed to be 10% (C=1.49). All other commercial areas assumed to be 10% (C=1.49) and residential areas assumed to be 10% (C=1.49).

WILLIAM L. KORNBLAU, L.S.
 JOHN E. LUNDHOLM, L.S.
 BAUGHMAN COMPANY, P.A.
 SURVEYING & ENGINEERING
 316/262-7271 • 315 ELLIS • WICHITA, KANSAS 67211

September 22, 1987

Preliminary Storm Sewer Estimate - Alternate B
 (The Landing and adjacent areas - excluding that part of storm sewer north of Taft).

Estimated Cost of Construction + 25% \$498,358
 Project Cost \$622,947

Assessment Areas
 The Landing = 15.20 Ac.
 Other Commercial = 23.93 Ac.
 Residential (from Drainage Plan) = 24.55 Ac.
 Total B.D. Area = 63.68 Ac.

Limit assessment for The Landing to \$100,000 (+ 665,603 s.f. = \$0.15/s.f.)
 Other commercial @ \$0.15/s.f. x 1,042,198 s.f. = 156,330
 Residential @ \$0.0375/s.f. x 1,071,297 s.f. = 40,174
 Total assessed share of cost \$296,504

City share of Project Cost \$326,443
 (\$52,443)

Typical residential lot (17,000 s.f.)
 assessment @ \$0.0375/s.f. = \$637.50

The Landing Storm Sewer - complete system

Tract	Area	S/s.f.	Share
<u>West Bee Addition</u>			
Lot 1	8,141	0.0375	\$ 305.25
<u>*Byron Stout Addition</u>			
The west 153 feet of Lot 1, except Street R/W	61,382	0.15	9,207.33
North 180 feet of Lot 1, except west 153 feet	80,460	"	12,069.00
Part of Lot 2	100,954	"	15,143.10
	242,796		
<u>*Garst Gardens</u>			
Lots 1-6, 9-14, Block A (12)	819,960	0.15	2,994.00
" " " Block B	819,951	"	2,992.66
" " " Block C "	819,940	"	2,991.00
	718,212		
<u>Maple Lane 2nd Addition</u>			
Lot 1, Block 1	14,038	0.0375	526.46
Lot 2, "	38,064	"	1,427.44
Lot 3, "	14,038	"	526.46
Lot 4, "	13,093	"	490.95
Lot 5, Block 2	14,983	"	561.83
Lot 2 "	14,038	"	526.46
Lot 3 "	36,985	"	1,386.00
Lot 4 "	13,589	"	509.95
	158,827		
<u>Pike's Whispering Sands Addition</u>			
Lot 1, Block A	20,400	0.0375	768.00
Lot 2, "	17,612	"	660.00
Lot 3, "	17,544	"	657.56
Lot 4, "	17,477	"	655.55
Lot 5, "	17,340	"	650.00
Lot 6, "	22,260	"	834.00
	112,733		
<u>*Replat of Dale Garst & Part of Garst Gardens</u>			
Lot 1, except part replatted as Byron Stout Addition	81,190	0.15	12,178.50
<u>*The Landing</u>			
Lot 1, except the south 182 feet	665,603	0.15	99,840.00