

Wichita, KS • 316.684.9600

PHASE 9 STADIUM PRESS BOX AND FIELD NETTING IMPROVEMENTS
SCHUELS STRYKER SPORTS
WICHITA, KANSAS



02023
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PRESSBOX FLOOR AND ROOF FRAMING PLANS

PROJECT NO.		
DATE	APR 2023	
SCALE	AS NOTED	
DESIGNED	DRAWN	CHECKED
DM	MB	DM
NO.	ISSUED FOR CONSTRUCTION	04-07-23
NO.	REVISION	DATE
SHEET NO.		
SS1.1		

GENERAL STRUCTURAL NOTES

General Contractor shall review and stamp all the shop drawings before submitting for review. Field verify (FV) all existing dimensions, elevations, and conditions. Notify the Architect for direction if the actual existing conditions differ from the conditions shown or implied on the drawings.

Verify all dimensions and elevations with the Architectural Drawings.

See the Architectural Drawings for the exact dimensions for openings in the walls, roof, and floor systems.

Verify all mechanical and electrical opening sizes and locations with the mechanical and electrical contractors.

No pipes, sleeves, or etc. shall pass through the beams or columns unless indicated on the plan.

The contractor shall design, provide, and maintain temporary bracing, shoring, guying, etc. and other methods as required to prevent any excessive loading and to stabilize the structural elements during construction. These methods shall remain in place until all members and final connections have been completed.

The general, mechanical, and electrical contractors shall be responsible for the design of all embeds, inserts, anchors, and supplemental framing systems required for the support of the architectural, mechanical, and electrical systems which are not detailed on the structural drawings.

Do not hang or attach any architectural, mechanical, or electrical elements or systems from the metal roof deck or any joist bridging unless specifically approved otherwise.

The building structural system is designed per the International Building Code – 2018 Edition.

The owner and/or contractor shall perform all material testing and inspection requirements for compliance with the governing building codes, the project specifications, the local building department, and the following Structural Special Inspection Notes.

Structural steel and metal deck erection shall comply with OSHA Standard 29 CFR Part 1926, Subpart R, and all other governing regulations. Structural steel suppliers and fabricators shall incorporate the requirements of this standard into the materials fabricated and supplied on this project.

DESIGN LOADS

Building structure is designed for the following loads and criteria:

Risk Category:
Dead: Weight of materials and construction plus weight of fixed service equipment
Assumed roof dead load: 15 psf
Assumed floor dead load: 75 psf

Live:
Roof live load: 20 psf (non-reducible)
Floor Live Load: 100 psf
Pressbox:
Exit facilities: 100 psf

Snow:
Ground snow load: Pg = 15 psf
Snow load importance factor: Is = 1.1
Exposure factor: Ce = 1.0
Thermal factor: Ct = 1.2
Flat roof snow load: Pn = 14 psf
Low-slope roof minimum snow load: Pm = 10 psf

Rain: Rain intensity: I = 3.9 in/hr

Wind: Basic wind speed (3-second gust): V = 120 MPH Ultimate
Wind = 93 MPH Nominal
Ke = 0.85
Ground elevation factor: C
Wind exposure category: C
Internal pressure coefficient: ±0.18

Seismic: Seismic importance factor: Ie = 1.25
Site class: D
Mapped spectral response accelerations: Sa = 0.091 S1 = 0.055
Spectral response coefficients: Sds = 0.097 Sd1 = 0.088
Seismic Design Category: B
Analysis procedure: Equivalent lateral force
Basic seismic-force resisting system: Ordinary reinforced masonry shear walls
Response modification factor: R = 2.0
Seismic response coefficient: Cs = 0.061
Design base shear: V = 0.061W

STRUCTURAL SPECIAL INSPECTIONS

One or more qualified independent testing and inspecting agencies shall perform the material testing and inspection requirements as outlined in the project specifications and the section.

Testing and inspection reports shall be furnished to the Building Official, the Architect, and the Structural Engineer.

Reports shall indicate that the materials tested and the work inspected are or are not in conformance with the Contract Documents. Discrepancies shall be brought to the attention of the Contractor for correction. If the discrepancies are not corrected, the discrepancies shall be reported to the Building Official, the Architect, and the Structural Engineer.

The testing and inspecting agencies shall submit a final report for each type of work stating that any discrepancies noted in the testing and inspection have been corrected and that the structural work was, to the best of their knowledge, performed in conformance with the Contract Documents.

The testing and inspection program does not relieve the Contractor of any responsibility for constructing the project in accordance with the Contract Documents and for controlling the quality of construction.

The Contractor shall be responsible for the scheduling and the timely notification of the testing and inspection agencies of the need for material testing or inspections.

Work which requires testing or inspection shall be ready for testing or inspection at the time of the testing and inspecting agency's visit. No work shall be performed which would conceal items to be tested or inspected until the work has been reviewed and accepted.

The following types of work require special inspection (IBC references refer to the International Building Code edition referenced above):

- Inspection of fabricators shall comply with IBC Section 1704.2.5.
- Testing and inspection of steel construction shall comply with IBC Section 1705.2, IBC Table 1705.2.3, American Institute of Steel Construction (AISC) Specification for Structural Steel Buildings 360, and Steel Deck Institute (SDI) Standard for Quality Control and Quality Assurance for Installation of Steel Deck.
 - Submit material test reports, manufacturer's certifications, product data sheets, welding procedure specifications, welding personnel performance qualification records, fabricator/erector quality control manual, fabricator/erector inspector qualifications as specified. Contractor shall maintain same for review by Architect/Engineer as indicated in AISC 360 Chapter IV.
 - Submit AWS performance qualification records for personnel performing shop fabrication or field erection welding.
 - Perform visual inspection of the fabricated or erected steel and steel joist framing to verify compliance with the construction drawings, including member location, bracing, stiffeners, bridging, and connection types.
 - Perform visual inspection of all shop fabrication and field erection welds.
 - Perform ultrasonic inspection of all partial or complete joint penetration welds during the shop fabrication and field erection.
 - Perform continuous inspection of all fillet welds greater than 5/16" during the shop fabrication and field erection.
 - Perform visual inspection of all snug-tightened (Type ST) bolted connections.
 - Observe the pre-installation verification testing for prestressed bolted connections defined in the Research Council on Structural Connections (RCSC) Specification for Structural Joints Using High Strength Bolts, Section 7.
 - Perform visual inspection of all bolted connections using tension control bolts or slip-critical (Type SC) bolted connections and prestressed (Type P) bolted connections.
 - Perform visual inspection of the placement of anchor rods and embed plates in concrete and concrete masonry. Verify diameter, grade, type, length, and embedment of anchors prior to placing concrete or grout.
 - Perform visual inspection and bend testing of headed stud shear connectors in compliance with AWS D1.1, Section 7.
 - Perform visual inspection of the metal floor and roof deck welding and fastener installation.
- Testing and inspection of concrete construction shall comply with IBC Section 1705.3 and IBC Table 1705.3.
 - Perform sampling and testing of cast-in-place concrete as specified.
 - Perform periodic observation of formwork and reinforcing for size, cover, spacing, positioning, lap lengths and locations.
 - Perform periodic inspection of concrete curing and protection procedures, including compliance with the hot and cold weather requirements defined in the specifications.
 - Contractor shall maintain records of all batch reports and delivery tickets on each load of concrete delivered to the project site for periodic review by the Architect/Engineer.
- Testing and inspection of masonry construction shall comply with the Level 2 quality assurance requirements of TMS 602 Section 3.1 and TMS 602 Section 1.6 and TMS 602 Section 3 and 4.
 - Periodically verify the proportions of site prepared mortar and grout.
 - Periodically verify the masonry construction complies with the site tolerances defined in TMS 602 Section 3.4F.
 - Perform periodic inspection of the mortar joint construction.
 - Perform periodic inspection of the reinforcing steel grade, type, size, placement and positioning and the block core cleaning and preparation.
 - Perform continuous inspection of the grout placement for proper consolidation, reconsolidation, and placement of the grout lift heights.
 - Periodically verify the type, size, and location of anchors and embeds for anchorage of masonry to other construction.
 - Periodically observe the preparation of the mortar specimens per ASTM C780 and grout specimens per ASTM C1019 for testing and as specified.
- Testing and inspection of post-installed anchors and post-installed reinforcing bars shall comply with IBC Section 1705.1.1 and IBC Table 1705.3.
 - Perform an initial installation inspection for each type and size of post-installed anchor and reinforcing bar. Any change in the personnel performing the installation shall require an initial installation inspection.
 - Perform periodic inspections to verify that the installations continue to be properly performed.
 - Installation inspections shall verify anchor/reinforcing bar type, diameter, embedment depth, spacing, adhesive type and expiration date, hole dimensions, base material, hole cleaning procedures, tightening/installation torque, maximum impact wrench torque rating, and adherence to the manufacturer's printed installation instructions.
 - Perform visual observation of all completed post-installed anchor and post-installed reinforcing bar installations.
 - Perform continuous installation inspections for all post-installed anchors and reinforcing bars that are installed in horizontally or upwardly inclined orientations to resist sustained tension loads.

CAST-IN-PLACE CONCRETE

Concrete shall have the following properties and minimum compressive strengths at 28-days.

Exterior Slabs on Metal Deck: 4500 psi

Aggregate for normal weight concrete shall meet ASTM C33. Aggregates shall be proportioned such that mix design shall contain a minimum of 50% coarse aggregates by gradation requirements set forth in ASTM C33. Coarse aggregate shall meet No. 57 grading requirements.

Exterior exposed concrete shall have from 4 to 7% entrained air.

Concrete shall be in strict conformance with the current "ACI Manual of Concrete Practice".

No aluminum shall be placed in the concrete.

Do not install saw-cut joints in elevated slabs on metal deck.

No electrical conduit shall be cast in a structural concrete system or concrete slabs on metal deck without approval from the Architect/Engineer.

REINFORCING STEEL

Welded wire reinforcement (WWR) shall meet ASTM A1064. Lap splice all welded wire reinforcement the cross wire spacing plus 2 inches. Furnish all welded wire reinforcement in flat sheets.

Reinforcing steel shall meet ASTM A615 - 60,000, unless noted otherwise.

Weldable reinforcing bars shall meet ASTM A706 - 60,000. Use E80xx electrodes for the welding of all A706 reinforcing bars, or as required to comply with AWS D1.4.

Reinforcing steel shall have adequate coverage as indicated in ACI 318 for the given exposure.

Reinforcing shall be continuous and lapped a minimum of 24 inches or 36 bar diameters whichever is greater, unless otherwise noted.

Reinforcing shall be detailed according to the ACI Detailing Manual and shall be prepared under the supervision of a professional engineer licensed to practice in the State of Kansas.

Provide corner lap bars to match in size and spacing of all wall, trench footing, and grade beam horizontal bars. Corner bars are not required in the wall footings, unless specifically indicated.

Fan main reinforcing around openings in the structural members. Do not field cut bars unless the Architect's approval is obtained.

Provide 2-#5, 4'-0" longer than opening dimension, on all sides of the openings in the slabs and walls.

Provide 3-inch slab bolster with continuous bottom plate at 4'-0" maximum centers for positioning all grade beam bottom bars and all footing bottom bars.

Provide bar supports for all bars in slabs or mat footing cast on grade at a maximum of 4'-0" in each direction.

Provide bar supports for all bars in elevated slabs on deck at a maximum of 4'-0" in each direction.

Mark each bundle of the reinforcing with weatherproof tags.

Welding of all reinforcing bars shall conform to AWS D1.4, "Structural Welding Code - Reinforcing Steel".

CONCRETE MASONRY

Concrete masonry units (CMU) shall comply with ASTM C90, have a minimum compressive strength of 2000 psi on net area at 28-days, and be made of lightweight concrete aggregate unless noted otherwise.

Mortar for use in concrete masonry shall be Type S and comply with ASTM C 270 proportion or proprietary specification.

Concrete masonry shall have a net area compressive strength (f'm) of 2000 psi minimum.

Grout shall comply with ASTM C 476 with a minimum 28-day compressive strength of 2500 psi. Grout solid all reinforced vertical block cores, bond beams, cores with anchors, and cores below grade.

Provide vertical CMU reinforcement as indicated on the plan and sections. Bars for typical lift shall be shop cut for 4'-0" lifts plus a minimum 48 bar diameters lap. Field cut bars for top lift and non-typical lengths.

Vertical reinforcement bars in CMU walls shall be placed and held in the center of the block cores, unless noted otherwise.

Provide vertical masonry reinforcing galvanized bar positioners at 46 inches o.c. at each vertical reinforcing bar. Provide bar positioners to match the wall thickness, bar size, and bar position as required.

Provide dowels from the foundation to match in size and spacing of all vertical CMU reinforcement.

Provide standard hook at the end of all vertical masonry reinforcing into top bond beam at roof bearing elevation.

Provide at least one vertical rebar at each end, side of control joints, jamps, corner, and intersection of all CMU walls. Size of rebar to match the size of typical vertical reinforcing. If the wall does not contain any vertical CMU reinforcing, provide 1-#4 vertical at the described locations.

Provide 2-#4's continuous for all bond beams unless otherwise indicated on the plan. Furnish in shop lengths and field cut.

See the plans (including architectural), sections and notes for the locations.

Provide one corner bar to match each horizontal bond beam.

Provide an 8-inch deep bond beam with top Elevation 108-8 at all exterior CMU walls, unless detailed otherwise.

Provide horizontal joint reinforcing in all concrete masonry unit walls at 16 inches o.c., unless noted otherwise.

Provide masonry control joints at a maximum spacing of 24'-0" o.c., unless noted or shown otherwise.

Coordinate all control joint locations with the Architect/Engineer. Control joints shall not occur below or directly adjacent to the joist, beam or lintel bearing points. Locate a minimum of 24 inches from joints of wall openings.

Fill all beam bearing pockets in masonry walls solid with grout. Provide temporary forms on the inside or exposed face of the wall flush with the face of the wall to retain grout placement.

Provide 1-#4 vertical at the described locations.

Provide standard size holes for all bolts and anchors in steel framing members unless noted otherwise (1/16" larger hole than diameter of bolt or anchor).

Where oversized holes are required or desired in steel framing members to accommodate the drill bit size on post-installed anchors, provide a 3/16" thick plate washer at each post-installed anchor location with a standard hole or 1/16" larger hole than the anchor diameter in the center of the plate washer. After the anchors and the steel framing members have been installed, add the plate washer on each anchor prior to installing the nut and tightening the anchor. After the anchor has been properly tightened, weld the plate washer to the steel framing member with a 3/16" fillet weld along each vertical edge of the plate washer.

All beams bearing on masonry directly supporting the roof framing systems shall be positively anchored to the bearing walls with anchor rods (or an equivalent method) to resist uplift forces.

Provide an angle frame to support the metal deck at all openings greater than 9" x 9".

Provide an angle frame below the perimeter curb of all mechanical roof top equipment and around the deck openings below the mechanical equipment. Where the equipment perimeter curb is parallel to the roof joists, provide L4x4x1/4 between the joists at a maximum spacing of 6'-0" o.c. and provide L4x4x1/4 between the angles and below the curbs. The contractor shall coordinate all mechanical unit sizes and locations. The perimeter curb shall be directly connected to the angle framing to resist all imposed loading on the roof top equipment. Where applicable, connections through the metal roof deck to the perimeter angle beneath the curb shall use shims for solid bearing at the metal deck flutes. The contractor shall be responsible for coordination with the mechanical contractor for the connection of the roof top equipment curbs to the angle framing.

Welding shall conform to AWS D1.1, "Structural Welding Code - Steel". All welds shall be AWS prequalified welded joints.

No unauthorized welds will be accepted.

E70xx electrodes shall be used for all welding, unless noted otherwise.

Use E80xx electrodes for the welding of all ASTM A706-60,000 weldable reinforcing bars or as required to comply with AWS D1.4.

Steel lintels shall be provided over all the openings in the masonry walls, unless otherwise detailed.

See Lintel Schedule for lintel requirements indicated on the drawings.

Provide lintels as indicated below for openings which are not indicated in the Lintel Schedule:

Span:	0'-0" to 2'-0"	8" deep bond beam w/ 2-#4
	2'-1" to 3'-0"	3-1/2x3-1/2x1/4 angle
	3'-1" to 4'-0"	4x3-1/2x1/8 angle
	4'-0" to 6'-0"	5x3-1/2x3/8 angle
	6'-1" to 8'-0"	6x3-1/2x3/8 angle
	6'-1" up	W8x24 with 1/4" continuous plate

Note: Furnish one angle for each 4-inch width of wall. Lintels shall have a minimum of 8 inches of bearing on concrete masonry at each end.

Not all masonry openings that require lintels are shown on the structural drawings. Refer to the architectural and mechanical drawings for the size and location of additional openings in the masonry walls.

Galvanize all steel lintels in exterior masonry walls.

Provide solid grouted masonry units below bearing of all lintels, beams, or etc. Grout block cores with 2500 psi grout.

Field completed welding and bolted connections shall be reviewed and accepted by the field inspection and testing agency prior to the installation of subsequent work.

Oversized structural steel shall conform to ASTM A123 for members and ASTM A153 for connection hardware.

Hot-dip galvanize steel framing members as specified where specifically noted on the drawings. Provide venting relief holes as required but locate on the bottom side of or similar non-visible locations where the members are exposed on the exterior of the building. Show or note the location of venting holes on the shop drawing submittal.

Non-metallic shrinkage-resistant grout shall meet ASTM C 1107.

STRUCTURAL STEEL

Structural steel shall meet the latest AISC "Specification for Structural Steel Buildings."

The typical steel framing connection details shown on the drawings represent the general type of connection detail expected to be implemented in the connection design, unless specifically approved otherwise.

The steel fabricator and detailer shall be responsible for the final detailing of all steel framing connections which are not explicitly detailed on the contract documents based on the defined beams or bracing and reactions or member forces. The submitted shop drawings shall clearly show and note all shop and field bolting and welding requirements and the final connection details.

All member loads, reactions, and moments defined on the drawings are ASD service-load level, unless noted otherwise.

Steel framing members shall only be spliced at locations shown on the design drawings or as shown on and approved on the shop drawings.

Structural steel shop drawings shall be prepared under the supervision of a professional engineer licensed to practice in the State of Kansas.

Steel plates and shapes shall meet ASTM A36 except wide flange sections shall meet ASTM A992, Fy = 50 ksi.

Structural steel tubing shall meet ASTM A500, Grade C, Fy = 50 ksi and structural piping shall meet ASTM A53, Grade B, Fy = 35 ksi.

Beam and column connections shall be made with ASTM F3125, Grade A325 (Type 1) bolts and accessories.

Connections shall be designed as snug-tightened (Type ST) bolted connections, unless noted otherwise.

Headed studs and shear connectors shall meet ASTM A108 and A29, Grade 1015-1020, and AWS D1.1, Type B.

Unheaded anchor rods shall be ASTM F1554, Grade 36 or ASTM F1554, Grade 55 (Supplement S1).

Threaded steel rods shall meet ASTM A307, Grade B, ASTM F1554, Grade 36, or an approved equal or greater strength threaded rod. Threaded rods cast in concrete or post-installed in concrete or masonry shall be thoroughly cleaned of all surface oils.

Provide 3/8" plate washers above all oversized holes (hole diameters greater than 1/16" larger than anchor diameter) in the column base plates. Provide standard hole size in plate washers.

Anchor rods set in concrete shall be furnished with double nuts and shall be set with a template.

Standard size holes for all bolts and anchors in steel framing members unless noted otherwise (1/16" larger hole than diameter of bolt or anchor).

Where oversized holes are required or desired in steel framing members to accommodate the drill bit size on post-installed anchors, provide a 3/16" thick plate washer at each post-installed anchor location with a standard hole or 1/16" larger hole than the anchor diameter in the center of the plate washer. After the anchors and the steel framing members have been installed, add the plate washer on each anchor prior to installing the nut and tightening the anchor. After the anchor has been properly tightened, weld the plate washer to the steel framing member with a 3/16" fillet weld along each vertical edge of the plate washer.

All beams bearing on masonry directly supporting the roof framing systems shall be positively anchored to the bearing walls with anchor rods (or an equivalent method) to resist uplift forces.

Provide an angle frame to support the metal deck at all openings greater than 9" x 9".

Provide an angle frame below the perimeter curb of all mechanical roof top equipment and around the deck openings below the mechanical equipment. Where the equipment perimeter curb is parallel to the roof joists, provide L4x4x1/4 between the joists at a maximum spacing of 6'-0" o.c. and provide L4x4x1/4 between the angles and below the curbs. The contractor shall coordinate all mechanical unit sizes and locations. The perimeter curb shall be directly connected to the angle framing to resist all imposed loading on the roof top equipment. Where applicable, connections through the metal roof deck to the perimeter angle beneath the curb shall use shims for solid bearing at the metal deck flutes. The contractor shall be responsible for coordination with the mechanical contractor for the connection of the roof top equipment curbs to the angle framing.

Welding shall conform to AWS D1.1, "Structural Welding Code - Steel". All welds shall be AWS prequalified welded joints.

No unauthorized welds will be accepted.

E70xx electrodes shall be used for all welding, unless noted otherwise.

Use E80xx electrodes for the welding of all ASTM A706-60,000 weldable reinforcing bars or as required to comply with AWS D1.4.

Steel lintels shall be provided over all the openings in the masonry walls, unless otherwise detailed.

See Lintel Schedule for lintel requirements indicated on the drawings.

Provide lintels as indicated below for openings which are not indicated in the Lintel Schedule:

Span:	0'-0" to 2'-0"	8" deep bond beam w/ 2-#4
	2'-1" to 3'-0"	3-1/2x3-1/2x1/4 angle
	3'-1" to 4'-0"	4x3-1/2x1/8 angle
	4'-0" to 6'-0"	5x3-1/2x3/8 angle
	6'-1" to 8'-0"	6x3-1/2x3/8 angle
	6'-1" up	W8x24 with 1/4" continuous plate

Note: Furnish one angle for each 4-inch width of wall. Lintels shall have a minimum of 8 inches of bearing on concrete masonry at each end.

Not all masonry openings that require lintels are shown on the structural drawings. Refer to the architectural and mechanical drawings for the size and location of additional openings in the masonry walls.

Galvanize all steel lintels in exterior masonry walls.

Provide solid grouted masonry units below bearing of all lintels, beams, or etc. Grout block cores with 2500 psi grout.

Field completed welding and bolted connections shall be reviewed and accepted by the field inspection and testing agency prior to the installation of subsequent work.

Oversized structural steel shall conform to ASTM A123 for members and ASTM A153 for connection hardware.

Hot-dip galvanize steel framing members as specified where specifically noted on the drawings. Provide venting relief holes as required but locate on the bottom side of or similar non-visible locations where the members are exposed on the exterior of the building. Show or note the location of venting holes on the shop drawing submittal.

Non-metallic shrinkage-resistant grout shall meet ASTM C 1107.

Non-composite metal form deck

Metal deck shall be 1-1/2" minimum, 22 gage, form deck, with the following section properties:

to 0.173 in ²	in 0.177 in ²	Sn 0.177 in ²
to 0.173 in ²	in 0.177 in ²	Sn 0.177 in ²

Deck shall be galvanized and shall conform to ASTM A924 and/or A653, Fy = 50 ksi, with a minimum G80 coating.

Deck shall be connected to supports with size 12 screw fasteners at a maximum fastener spacing of 12 inches.

Provide size 10 screw side lap fastener between supports at 16 inches o.c.

COLD-FORMED STRUCTURAL FRAMING

Cold-formed structural framing members shall be in accordance with ASTM C955 and shall have engineering properties calculated in conformance with the AISI "Specifications for the Design of Cold-Formed Steel Structural Members".

Cold-formed structural framing members shall be installed to conform with ASTM C-1007.

Cold-formed structural framing members and accessories shall have a minimum protective coating equal to G60 galvanized finish. Properly clean the welded or damaged area and apply zinc-rich paint to all areas where the galvanized finish is damaged.

All steel shall conform to one of the following ASTM Standards: ASTM A653, A875, A792, or A463.

All 33 and 43 mil products shall be formed from steel with a minimum yield of 33,000 psi.

All 54, 58 and 97 mil products shall be formed from steel with a minimum yield of 50,000 psi.

Stud and "C" joist sections shall have with nominal 1-5/8" flanges and a minimum 1/2" return, unless noted otherwise.

"Z" purlin sections shall have with nominal 2-1/2" flanges and a minimum 1" return, unless noted otherwise.

Members shall be of depth and mil thickness as indicated on the plan and sections.

Track sections shall meet or exceed the mil thickness of the stud members with nominal 1-1/4" flanges, unless noted otherwise.

Framing components shall be cut square for attachment to perpendicular members.

Studs or joists used in lintels and horizontal or sloped framing members shall be un-punched, unless noted otherwise.

Load bearing studs shall have a minimum of 10-inch unpunched length at the top and bottom of each stud unless recommended otherwise by the stud manufacturer.

All studs in load bearing walls must be fully seated into the top and bottom track.

Provide web stiffeners, connection angles, and miscellaneous hardware required to complete all the connections.

Provide wall stud bridging spaced at 4'-0" maximum on centers in the exterior and load bearing walls, for full height of walls, including height of parapet.

Fasten the bridging member to each stud with clip angle and screw fasteners. Install 2 screw fasteners between clip angle and stud and between clip angle and bridging member.

Provide deflection track at the top of all non-load bearing stud walls where the top of wall abuts the bottom of the structure. Fastening of framing components shall be with self-tapping screws or welding of sufficient size to insure the strength of the connection.

Welds shall be performed by operators qualified in accordance with Section 6.0 of the American Welding Society's "Structural Welding Code - Sheet Metal" (AWS D1.3).

Attach studs to track with a minimum of one screw per stud flange, unless otherwise noted.

Anchor base track to floor/ceiling with a 0.157" powder actuated fasteners at 16 inches o.c., embed 1-1/2" into the concrete.

POST-INSTALLED ANCHORS

Post-installed anchors and post-installed reinforcing bars shall be installed per the manufacturer's printed installation instructions. Holes shall be drilled with the required bit type and size to provide the minimum embedment length specified in the Structural Drawings. Holes shall be cleaned prior to installing the anchor or reinforcing bar with the brush and compressed air method or with the manufacturer's proprietary drill bit and dust extraction system.

The installation of all post-installed anchors and post-installed reinforcing bars shall be performed by personnel trained and certified by the American Concrete Institute Concrete Reinforcing Steel Institute or trained by the post-installed anchor and/or adhesive manufacturer for the type being used.

Post-installed anchor types shall be as follows:

Expansion anchors installed into concrete shall be Hilti Kwik Bolt T22, Simpson Strong-Tie Strong-Bolt 2, or DeWalt Power-Stud+ SD2 anchors or an approved equal.

Expansion anchors installed into solid grouted masonry shall be Hilti Kwik Bolt T22, Simpson Strong-Tie Strong Bolt 2, or DeWalt Power-Stud+ SD1 anchors or an approved equal.

Adhesive anchors or reinforcing bars installed into concrete shall use Hilti HIT-HY 200 Adhesive Anchoring System or an approved equal, Hilti HIT-RE 500 V3, Simpson Strong-Tie SET-3G, DeWalt AC208+, and DeWalt Pure 110+ or an approved equal adhesive anchoring systems for adhesive anchors installed into concrete.

Adhesive anchors or reinforcing bars installed into solid grouted masonry, hollow block masonry, or brick masonry shall use Hilti HIT-HY 270 Adhesive Anchoring System or an approved equal. Adhesive anchors installed into hollow block masonry shall be of depth and mil thickness as indicated on the plan and sections.

Screw anchors installed into concrete shall be Hilti KH-EZ, Simpson Strong-Tie Titan HD, or DeWalt Screw Bolt+ anchors or an approved equal.

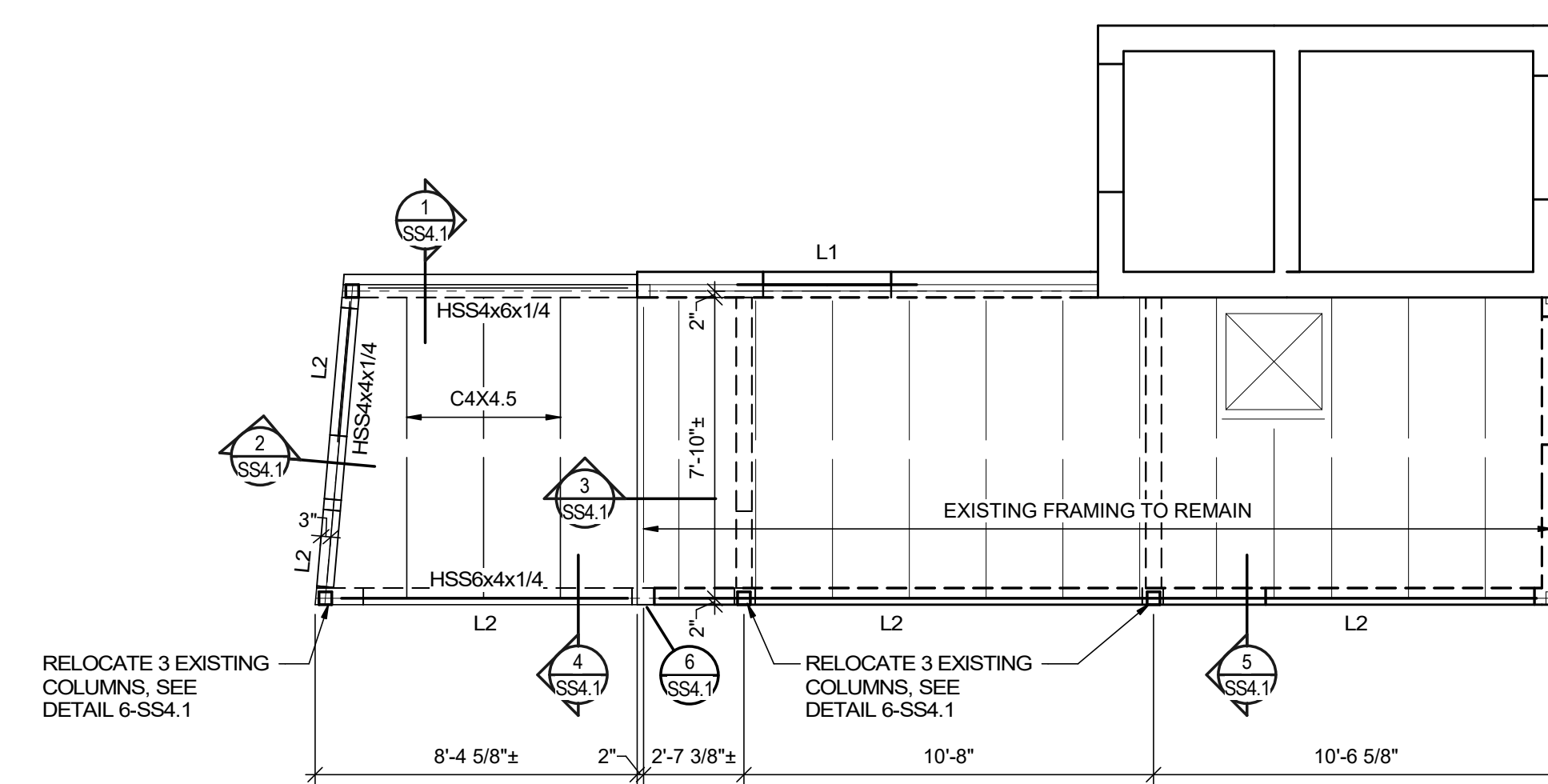
Screw anchors installed into solid grouted masonry shall be Hilti KH-EZ, Simpson Strong-Tie Titan HD, or DeWalt Screw Bolt+ anchors or an approved equal.

Screw anchors installed into brick masonry shall be Hilti KH-EZ, Simpson Strong-Tie Titan HD, or DeWalt Screw Bolt+ anchors or an approved equal.

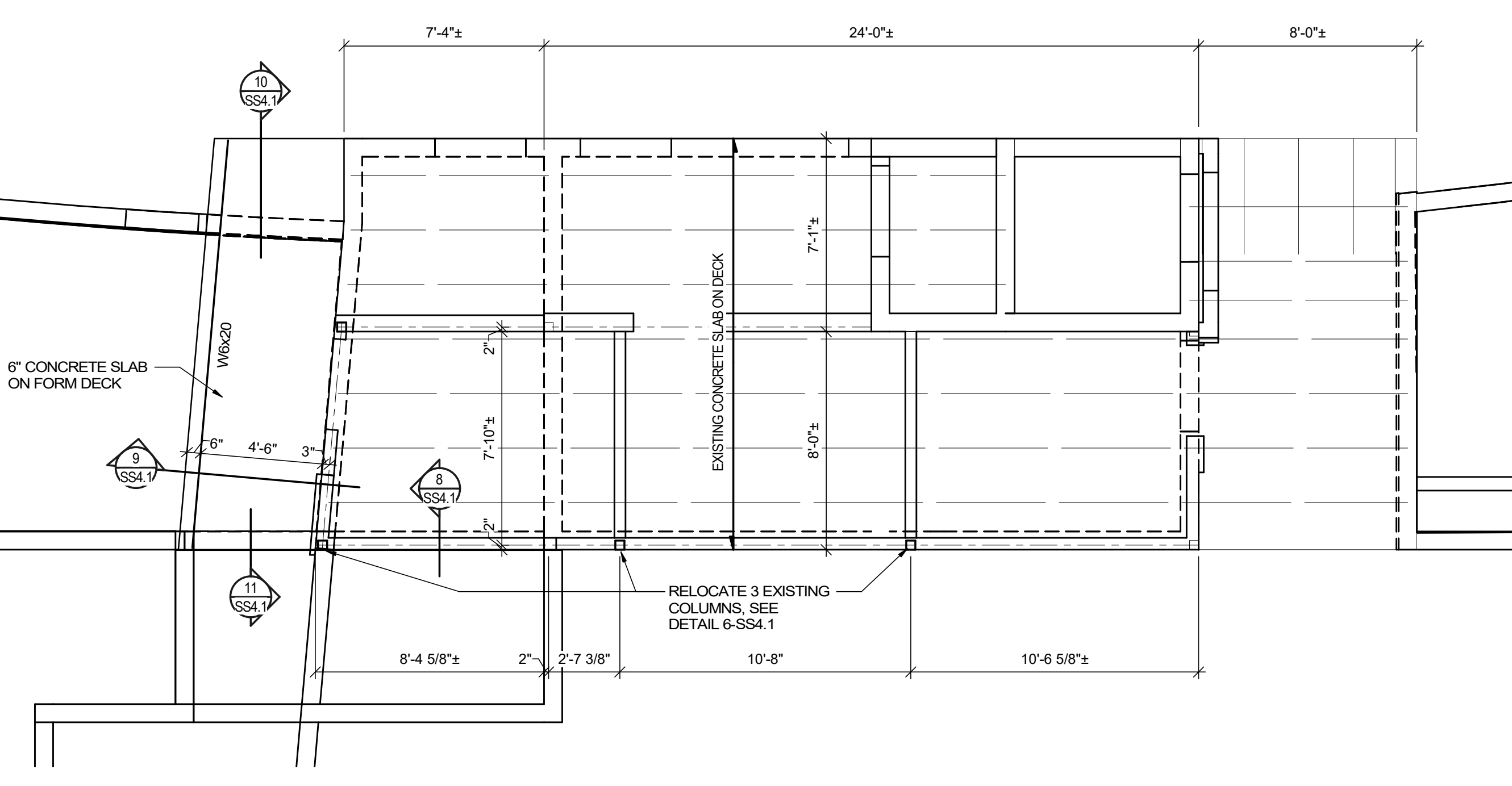
A piston plug injection procedure approved by the adhesive manufacturer shall be used for the injection of adhesive into all holes greater than 10 inches in depth.

Post-installed expansion anchors must be tightened to the anchor manufacturer's recommended installation torque.

The installation of all post-installed anchors and post-installed reinforcing bars shall be reviewed and accepted by the field testing and inspection agency.



ROOF FRAMING PLAN - PRESSBOX
1/4" = 1'-0"



FLOOR FRAMING PLAN - PRESSBOX
1/4" = 1'-0"

PLAN MARKS

L# LINTEL MARK, SEE SCHEDULE, SHEET SS4.1

PLAN NOTES

- SEE GENERAL STRUCTURAL NOTES, SHEET SS1.1, FOR ADDITIONAL NOTES AND INFORMATION.
- PLAN DIMENSIONS ARE TO FACE OF STUD OR MASONRY WALL, UNLESS NOTED OTHERWISE. SEE ARCHITECTURAL FOR DIMENSIONAL LOCATION OF MASONRY WALLS WHERE NOT DIMENSIONED ON STRUCTURAL DRAWINGS.
- SEE SHEET SS4.1 FOR LINTEL SCHEDULE. SEE ARCHITECTURAL DRAWINGS FOR SIZES AND LOCATIONS OF DOOR AND WINDOW OPENINGS. SEE MECHANICAL/ELECTRICAL PLUMBING DRAWINGS FOR MECHANICAL AND ELECTRICAL EQUIPMENT LOCATIONS.
- SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SIZE AND LOCATION OF ROOF OPENINGS. VERIFY THE SIZE AND LOCATION OF ALL SUPPORTED MECHANICAL UNITS WITH THE MECHANICAL DRAWINGS AND THE MECHANICAL CONTRACTOR.

PRIOR TO CONSTRUCTION OR FABRICATION, CONTRACTOR SHALL FIELD VERIFY ALL EXISTING DIMENSIONS, ELEVATIONS, AND CONDITIONS. NOTIFY ARCHITECT FOR DIRECTION IF THE ACTUAL CONDITIONS DIFFER FROM THE CONDITIONS SHOWN OR IMPLIED ON THE DRAWINGS.

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